



**FOUNDATION INVESTIGATION REPORT  
REHABILITATION OF HIGHWAY 21  
FROM BAYFIELD TO GODERICH, ONTARIO**

**CULVERT REPLACEMENT AT STATION 21+055**

**MINISTRY OF TRANSPORTATION ONTARIO - WEST REGION  
PURCHASE ORDER NUMBER 3009-E-0022  
GWP 834-93-00**

**MTO GEOCRES NO. 40P12-29**

Submitted to:

**Ministry of Transportation Ontario - West Region**  
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## LIMITATION OF REPORT

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## 1.0 INTRODUCTION

AMEC Environment and Infrastructure, a Division of AMEC Americas Limited (“AMEC”), was retained by the Ministry of Transportation Ontario - West Region (“MTO”) to provide Detail Design Services for the Rehabilitation of Highway 21, Township of Goderich, Ontario. The project highway is about 20 km long stretching northerly from about 1.85 km south of Bayfield River Bridge (Bayfield) to about 0.17 km north of Huckins Street (Goderich). To provide the required geotechnical information for the Detail Design Services, AMEC conducted a foundation investigation at the locations of eight (8) existing culverts identified for rehabilitation / replacement by the AMEC Design Team. A site plan showing the culvert locations /stations is presented on Drawing No. 1.

The foundation investigation for the culverts comprised advancing a total of 21 boreholes (BH G1 to BH G21) as listed in Table 1.1. Culvert details, as provided by the Design Team, including the stations, type, dimensions and boreholes drilled are summarized in Table 1.1.

**Table 1.1- Culvert Details\***

Station	Existing Culvert		Boreholes Drilled	Proposed Work	Remark
	Type	Dimension			
10+200 to 10+300	Concrete-open rigid frame	1.80 x 1.20 x 34.3 m	BH G1 to BH G6	Replacement or extension	Two culverts at Jowett's Grove Road Intersection with Hwy 21
	Concrete-open	0.91 x 0.91 x 40.0 m			
10+550	CSP	0.61 m diameter and 24.4 m length	BH G7 and BH G8	Replacement	
10+705	CSP	0.61 m diameter and 24.0 m length	BH G9 and BH G10	Replacement	
11+187	CSP	0.46 m diameter and 24.7 m length	BH G11 and BH G12	Replacement	
11+873	CSP	0.61 m diameter and 21.7 m length	BH G13 and BH G14	Replacement	
12+810	CSP	0.61 m diameter and 19.3 m length	BH G15 and BH G16	Replacement	
21+055	Concrete-open rigid frame	1.82 m x 5.49 m x 23.4 m	BH G17, BH G18 and BH G19	Replacement	
22+826	CSP	0.76 m diameter and 21.4 m length	BH G20 and BH G21	Replacement	

\* From Culvert Summary Table provided by AMEC Design Team

The purpose of the foundation investigation was to obtain information on the subsurface conditions at the culvert sites by means of boreholes, in-situ tests and laboratory tests on



selected samples. Based on AMEC's interpretation of the data obtained in the investigation, recommendations are provided on the geotechnical aspects of replacement of the culverts.

As per Terms of Reference (TOR) in the Request for Quotation (Purchase Order Number: 3009-E-0022, dated 10 March 2011), separate reports have been prepared for each culvert site. This report presents the results of foundation investigation for the culvert at Station 21+055.

The design discussion and recommendations for Culvert at Station 21+055, including factual results of the soil conditions encountered in the boreholes and laboratory tests, are presented in a separate report titled "Foundation Investigation and Design Report".

## **2.0 SITE AND PROJECT DESCRIPTION**

The culvert site (at Station 21+055) is located at the existing watercourse (Naftel's Creek) crossing Highway 21, approximately 600 m north of Kitchigami Road, between Bayfield and Goderich, Ontario (Drawing No. 1).

At this location, Highway 21 is a two-lane asphaltic-concrete paved road with gravel shoulders on both sides, and runs on top of an embankment built up above the surrounding grade. The surrounding area is primarily rural in nature, with active agricultural operations and farm houses / vacant lands / wood lot.

Based on the Culvert Summary Table provided, the existing culvert at this location is a concrete-open rigid frame structure (1.82 m x 5.49 m x 24.4 m). Based on drawing provided by MTO (ETR Plate), the fill cover over the culvert is about 3.0 m. AMEC Design Team recommended for the replacement of the existing culvert.

Based on information provided to AMEC, the replacement culvert could be concrete open footing, rectangular or arch, which would be supported by strip footings. No further design information is available at this time of writing this report. It is likely the invert of the replacement culvert would be established at the same level as the existing culvert invert.

Site photographs showing the culvert are presented in Appendix C (Photographs 1 to 3).

## **3.0 GEOLOGY**

Based on Map 2556 (Southern Sheet): 'Quaternary Geology of Ontario' prepared by Ministry of Northern Development and Mines of Ontario (1991), the site is located in an area of transition where the overburden comprise (i) St. Joseph Till (Huron - Georgian Bay lobe) consisting of silt to silty clay matrix, clay content increases southward, clast poor, and (ii) Glaciolacustrine deposits consisting of sand, gravelly sand and gravel; nearshore and beach deposits; and (iii) Glaciolacustrine deposits consisting of silt and clay, minor sand, basin and quiet water deposits.

## **4.0 INVESTIGATION PROCEDURES**

### **4.1 Field Investigation**

In accordance with the Terms of Reference for this investigation, a total of four (4) boreholes (BH G17, BH G17A, BH G18 and BH G19) were advanced at the site. Boreholes BH G17 was drilled adjacent to the culvert inlet, while Borehole BH G18 and BH G19 were drilled on the east and west shoulder respectively. Borehole BH G17A was advanced about 1.5 m from the location of Borehole BH G17 by augering (without sampling) to install a monitoring well for hydrogeological study. The hydrogeological findings are presented in a separate report. The borehole locations are presented on Drawing No. 2.

The fieldwork was performed on 17 May and 18 May 2011 after acquiring all necessary permits for road occupancy, and clearing underground utilities. The ground surfaces at the borehole locations were surveyed with reference to the nearest geodetic benchmark (GBM 0011989U065, Sta. 19+755, El 206.086).

The boreholes were advanced using solid-stem continuous-flight augers, with a track-mounted power-auger drilling rig under the full-time supervision of experienced geotechnical personnel from AMEC. Soil samples were generally taken at 0.76 m intervals for the initial 3 m of the borehole, and 1.5 m thereafter, while performing the Standard Penetration Test (SPT) in accordance with ASTM D1586. This consisted of freely dropping a 63.5 kg (140 lbs.) hammer for a vertical distance of 0.76 m (30 inches) to drive a 51 mm (2 inches) diameter O.D. split-barrel (split spoon) sampler into the ground. The number of blows of the hammer required to drive the sampler into the relatively undisturbed ground by a vertical distance of 0.30 m (12 inches) was recorded as SPT 'N' value of the soil which indicated the consistency of cohesive soils or the compactness of non-cohesive soils.

The groundwater conditions were observed in the boreholes during sampling and upon completion of drilling. The groundwater depth measurements are presented on the Record of Boreholes. Two monitoring wells were installed, one each in Borehole BH G17 and Borehole BH G17A for the long term monitoring of groundwater level by the project hydrogeological team (the hydrogeological report is submitted separately). The groundwater levels were measured within the monitoring wells on 14 June and 22 June 2011. The results of groundwater measurements are shown on the Record of Boreholes and summarized in Table 5.3 (Section 5.0).

Upon completion of drilling, the boreholes were backfilled with bentonite in accordance with the general requirements of Ministry of the Environment Regulation 903.

The soil samples were transported to AMEC's Advanced Soil Laboratory in Scarborough (Toronto) for further examination and laboratory soil testing. The program of laboratory testing included, where applicable, the grain size analysis, Liquid and Plastic Limits, in-situ water content determination, and soil corrosivity analysis.



The results of the in-situ and laboratory tests are presented in the corresponding Record of Boreholes (Appendix A) and Laboratory Test Results (Appendix B).

## **4.2 Laboratory Tests**

All soil samples were subjected to visual identification shown on the Record of Boreholes in Appendix A.

In accordance with the Terms of Reference for this investigation, the following tests were conducted:

- In-situ water content determination (34);
- Grain size distribution analysis (6);
- Atterberg Limit tests (6); and
- Soil Corrosivity (1).

The results of laboratory tests are included on the Record of Boreholes in Appendix A. The grain size distribution curves and Plasticity Chart are shown in Appendix B.

## **5.0 SUB-SURFACE CONDITIONS**

Based on the investigation results, the soil profile consisted predominantly of ground surface cover (topsoil or sand and gravel fill) underlain by fill soils (sandy silt / sand) overlying native deposits (silt and clayey silt) which extended to the termination depths of the boreholes (elevations 191.5 m to 192.5 m  $\pm$ ).

The stratigraphic units and groundwater conditions at the borehole locations are discussed in the following sections. Detailed information is provided in the Record of Boreholes (Appendix A). Stratigraphical cross sections showing the existing culvert is provided in Drawing No. 3.

Soil and groundwater conditions may vary between and beyond the borehole locations.

### **5.1 Topsoil**

Topsoil was encountered at the existing grade in Borehole BH G17 drilled adjacent to the culvert inlet area. The measured thickness of topsoil was about 400 mm. The topsoil consisted primarily of organic matter with some rootlets and soils.

The thickness of topsoil could vary beyond the borehole location.

## **5.2 Fill Soils**

### **Sand and Gravel Fill**

Boreholes BH G18 and BH G19 drilled through the shoulder (east and west) areas of Highway 21 encountered sand and gravel fill at the existing grade. The measured thickness of sand and gravel fill was about 500 mm in Borehole BH G18, and 600 mm in Borehole BH G19.

Two SPT N-values measured within the sand and gravel fill were 30 blows and 36 blows per 0.3 m. The water contents determined within the sand and gravel fill were 5 % and 6 %.

### **Sandy Silt Fill**

Below the topsoil in Borehole BH G17, sandy silt fill was encountered. The sandy silt fill extended to a depth of about 1.4 m below the existing grade.

The sandy silt fill was brown in colour and contained trace gravel, clay and organic matter.

A single SPT N-value measured within the sandy silt fill was 21 blows per 0.3 m. The water content determined within the sandy silt fill was 37 %.

### **Sand Fill**

Underneath the sand and gravel fill, sand fill was encountered in Boreholes BH G18 and BH G19 up to depths of about 6.3 m (Elevation 201.6) and 6.2 m (Elevation 201.5) below the existing grade, respectively.

The sand fill was brown in colour and contained trace to some silt and gravel. Trace organic matter was found in Borehole BH G19.

The SPT N-values measured within the sand fill ranged from 2 blows to 42 blows per 0.3 m. The water contents determined for the sand fill ranged from 8.0 % to 15.0 %.

## **5.3 Silt**

Native silt deposit was encountered below the sandy silt fill in Borehole BH G17; and underneath the sand fill in Boreholes BH G18 and BH G19. The silt extended to depths ranging from 11.3 to 13.0 m below the existing grade.

The silt was grey in color, and contained some to 'with' sand, trace clay, sand and cobbles / boulders. The SPT 'N' values of the silt were all greater than 50 blows per 0.3 m indicating a very dense compactness condition and possibly trace cobbles/boulders. The measured moisture contents in the silt ranged from 12 % to 16 %.

Grain size analyses were performed on two (2) samples of the silt, and the results are presented in Table 5.1. The silt was non-plastic.

**Table 5.1 - Results of Grain Size Analysis**

Borehole No.	Sample No.	Depth (Elevation) (m)	Percent Distribution (%)			
			Gravel	Sand	Silt	Clay
BH G18	SS 6	7.6 - 7.7 (200.3 – 200.2)	3	22	68	7
BH G19	SS 8	7.6 - 7.7 (200.1 – 200.0)	2	18	75	5

The grain size distribution curves are presented in Figure No. B 1 in Appendix B.

#### 5.4 Clayey Silt

Clayey silt was encountered below the sandy silt in all boreholes. The clayey silt extended to the termination depths of the boreholes at about 12.3 m to 15.4 m below the existing grade (Elevation 191.5 m to 192.4 m).

The clayey silt was grey in color, and contained trace sand. The SPT 'N' values of the clayey silt were all greater than 50 blows per 0.3 m indicating a hard consistency condition and possibly trace cobbles/boulders. The measured moisture contents in the clayey silt ranged from 13 % to 15 %.

Grain size analyses and Atterberg Limit tests were completed on 3 samples of the clayey silt, and the results are presented in Table 5.2.

**Table 5.2 - Grain Size Distribution Analysis and Atterberg Limit Test Results**

Borehole No.	Sample No.	Depth (Elevation) (m)	Grain Size Distribution				Atterberg Limit			USCS Modified Group Symbol
			Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit	Plastic Limit	Plasticity Index	
BH G17	SS 11	12.2 - 12.3 (191.6 - 191.5)	0	1	83	16	21	15	6	CL - ML
BH G18	SS 10	13.7-13.8 (194.1 - 194.0)	0	1	85	14	24	16	8	CL
BH G19	SS 12	13.7-13.8 (194.0 - 193.9)	0	1	76	23	24	15	9	CL

The grain size distribution curves are presented in Figure No. B 2, and the plasticity chart is presented in Figure No. B 3, in Appendix B.



## 5.5 Groundwater Conditions

Groundwater conditions in the open boreholes were observed during and on completion of drilling. Groundwater was measured at about 7.6 m below the existing grade (elevation 196.3 m  $\pm$ ) in Borehole BH G17, about 9.9 m (elevation 198.0 m  $\pm$ ) in Borehole BH G18 and about 8.1 m (elevation 199.7 m  $\pm$ ) in Borehole BH G19.

The groundwater levels were also measured in the monitoring wells installed in Boreholes BH G17 and BH G17A on 14 and 22 June 2011. The results of groundwater measurements are shown on the Record of Boreholes and summarized in Table 5.3.

**Table 5.3 - Results of Groundwater Measurements**

Borehole	Measured Groundwater Level			Remarks
	Date	Depth(m)	Elevation(m)	
BH G17	17 May 2011	7.6 m $\pm$	196.3 m $\pm$	Completion of drilling
	14 June 2011	1.8 m $\pm$	202.1 m $\pm$	In monitoring well
	22 June 2011	2.0 m $\pm$	201.9 m $\pm$	In monitoring well
	16 August 2011	2.3 m $\pm$	201.7 m $\pm$	In monitoring well
	17 May 2012	2.1 m $\pm$	201.8 m $\pm$	In monitoring well
BH G17A	14 June 2011	1.3 m $\pm$	202.6 m $\pm$	In monitoring well
	22 June 2011	1.3 m $\pm$	202.6 m $\pm$	In monitoring well
	16 August 2011	1.5 m $\pm$	202.4 m $\pm$	In monitoring well
	17 May 2012	1.4 m $\pm$	202.5 m $\pm$	In monitoring well
BH G18	18 May 2011	9.9 m $\pm$	198.0 m $\pm$	Completion of drilling
BH G19	17 May 2011	8.1 m $\pm$	199.7 m $\pm$	Completion of drilling

It should be pointed out that the groundwater at the site would fluctuate seasonally and can be expected to be somewhat higher during the spring months and in response to major weather events.

## 5.6 Soil Corrosivity Testing

One soil sample (BH G17 - SS3) was analysed by Maxxam Analytics Laboratory in Mississauga to determine the soil corrosivity potential with respect to concrete and steel. The Certificate of Analysis is included in Appendix B. A summary of the test results are presented in Table 5.4.

**Table 5.4 - Results of Corrosivity Testing**

Soil Sample No.	pH	Electrical Conductivity Umho/cm	Resistivity (ohms-cm)	Chloride (µg/g)	Sulphate (µg/g)
BH G17 - SS3	7.9	317	3200	49	120

The test results have shown that the sulphate content of the soil is 120 ppm (µg/g). As per Table 3 "Additional Requirements for concrete subjected to sulphate attack", Clause 4.1.1.6 of CSA Standards Specification A23.1-09, any soil which has sulphate content below 0.1% (i.e., 1,000 ppm or µg/g) is not considered aggressive with respect to concrete. As such, in accordance with Table 6 of CSA A23.1-09, Type GU (general use) cement can be used for concrete.

## 6.0 CLOSURE

The sub-soil information contained in this report should be used solely for the purpose of foundation assessment of the Culvert at Station 21+055 on Highway 21, between Bayfield and Goderich, Ontario.

The Limitations of Report is an integral part of this report.

This report was prepared by Mohammad Mollah, M.Eng., P.Eng., and was reviewed by Dr. Prapote Boonsinsuk, Ph.D., P.Eng.

Sincerely,

**AMEC Environment & Infrastructure,  
 a Division of AMEC Americas Limited**



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*Shami Malla*  
 Shami Malla, P. Eng.  
 Project Manager

**AMEC Environment & Infrastructure,  
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**LIMITATIONS OF REPORT**

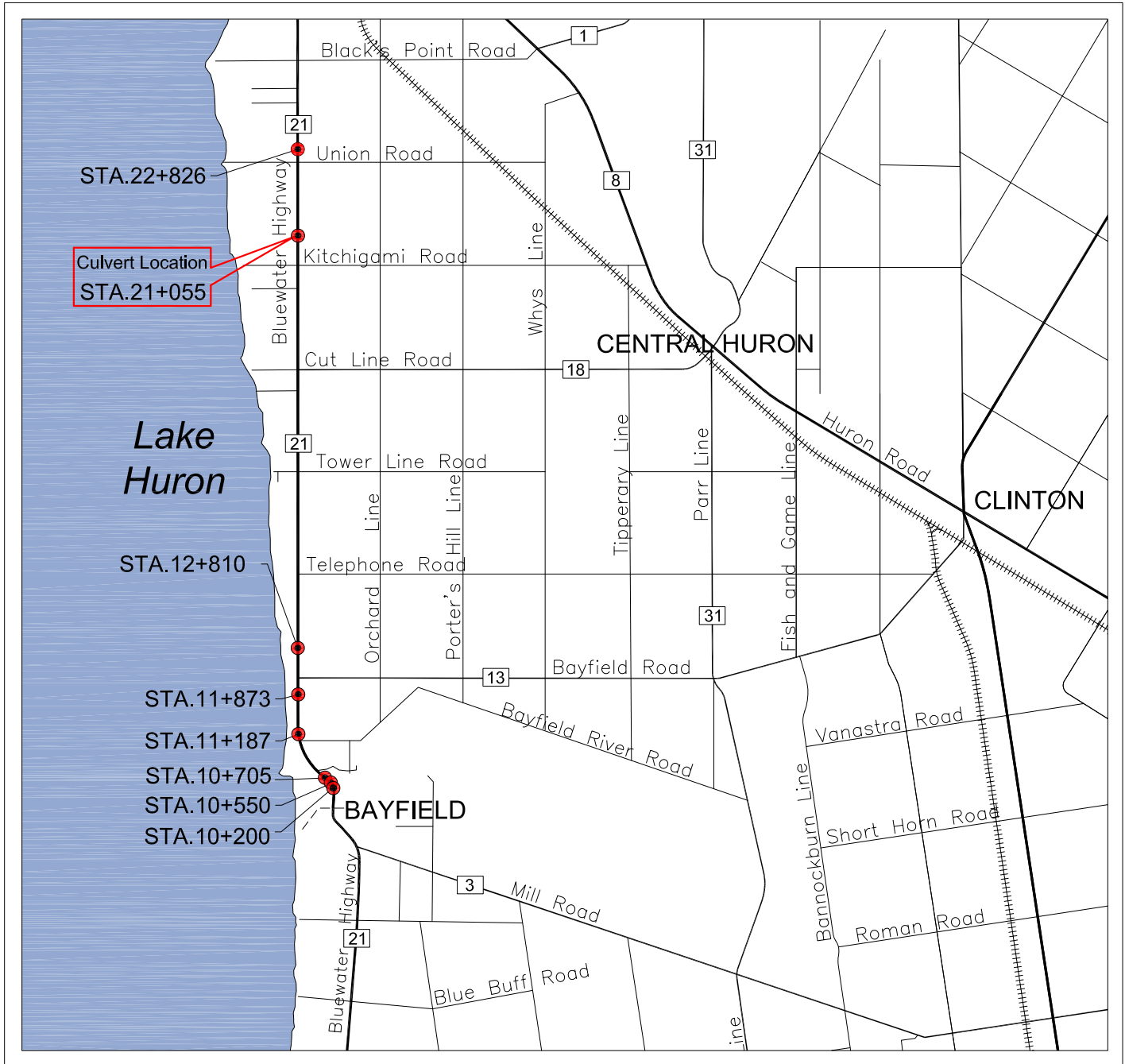
The information contained herein in no way reflects on the environmental aspects of the project, unless otherwise stated. Subsurface and groundwater conditions between and beyond the testholes may differ from those encountered at the testhole locations, and conditions may become apparent during construction, which could not be detected or anticipated at the time of the site investigation.

The number of testholes may not be sufficient to determine all the factors that may affect construction methods and costs. The contractors bidding on this project or undertaking the construction should, therefore, make their own interpretation of the factual information presented and draw their own conclusions as to how the subsurface conditions may affect their work. This work has been undertaken in accordance with normally accepted geotechnical engineering practices.

This report was prepared specifically for the culvert at Station 21+055 in Highway 21, Ontario, as described in the report. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. AMEC Environment & Infrastructure, a Division of AMEC Americas Limited, accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

## **DRAWINGS**

<b>DRAWING NO. 1</b>	<b>CULVERT LOCATION PLAN</b>
<b>DRAWING NO. 2</b>	<b>BOREHOLE LOCATION PLAN</b>
<b>DRAWING NO. 3</b>	<b>STRATIGRAPHIC CROSS SECTIONS</b>



#### SCALE

1500m 0 1500 3000 4500 6000m

#### LEGEND



CULVERT LOCATION

**AMEC Environment & Infrastructure,  
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CLIENT LOGO



CLIENT

**MINISTRY OF  
TRANSPORTATION ONTARIO  
WEST REGION**

TITLE  
**CULVERT LOCATION PLAN**

PROJECT  
**REHABILITATION OF HIGHWAY 21 - FROM BAYFIELD TO GODERICH, ONTARIO**  
PURCHASE ORDER NUMBER: 3009-E-0022, WP 834-93-00, GEOCREs No.: 40P12-29

DWN BY:  
KW

CHK'D BY:  
PB

PROJECTION:  
-

DATUM:  
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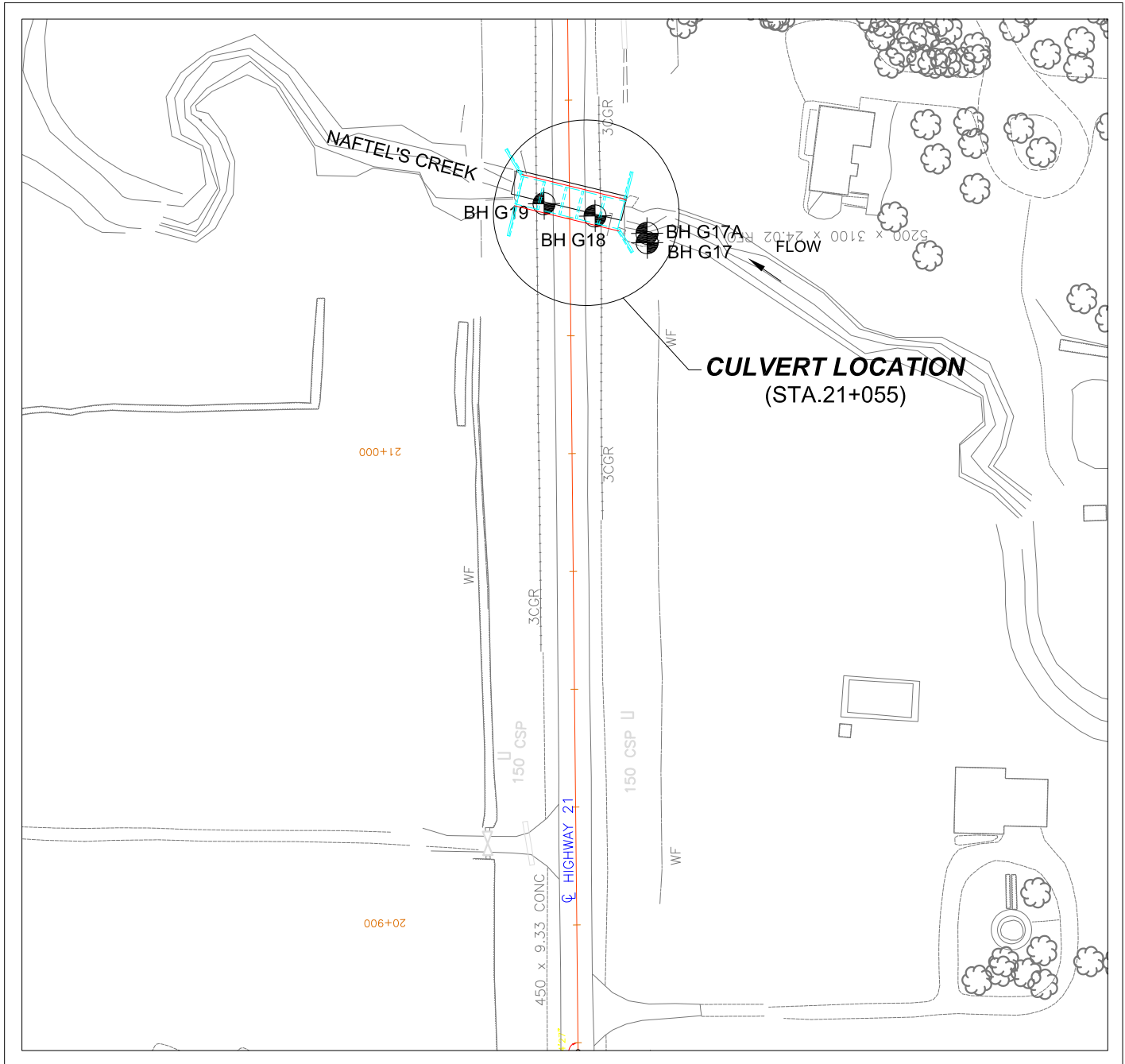
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DATE:  
JANUARY 2013

PROJECT NO:  
TP110076

DRAWING No.

1



SCALE



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CLIENT LOGO



CLIENT

MINISTRY OF  
TRANSPORTATION ONTARIO  
WEST REGION

TITLE  
BOREHOLE LOCATION PLAN

PROJECT  
REHABILITATION OF HIGHWAY 21 - FROM BAYFIELD TO GODERICH  
PURCHASE ORDER NUMBER: 3009-E-0022, WP 834-93-00, GEOCREs No.: 40P12-29

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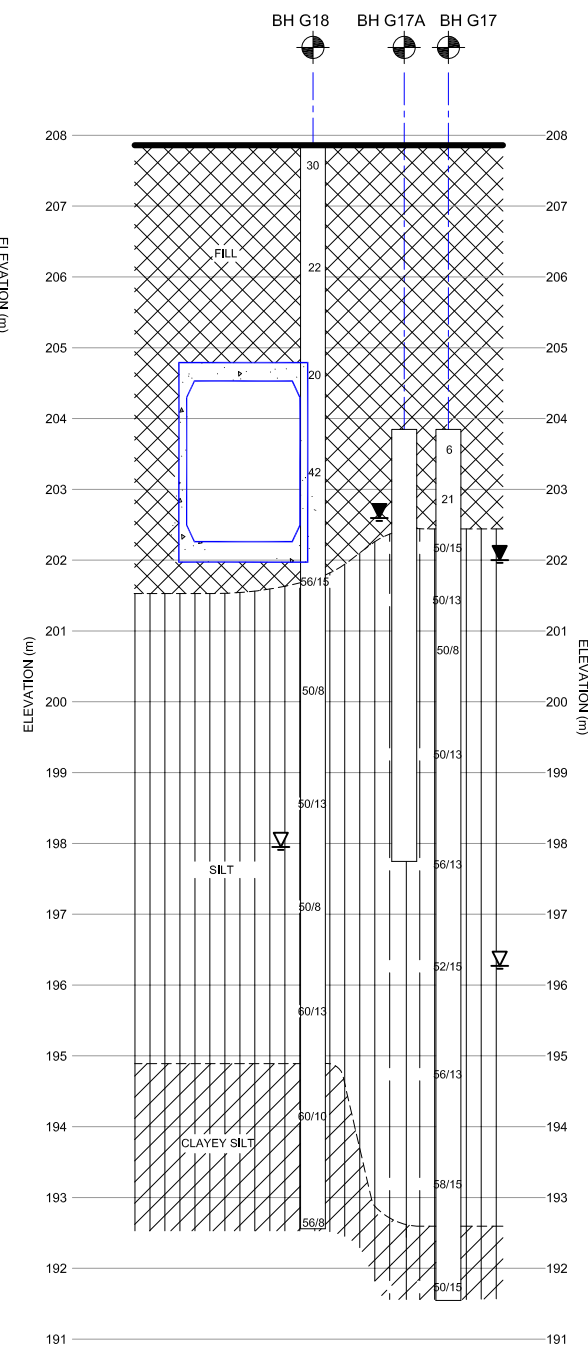
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JANUARY 2013

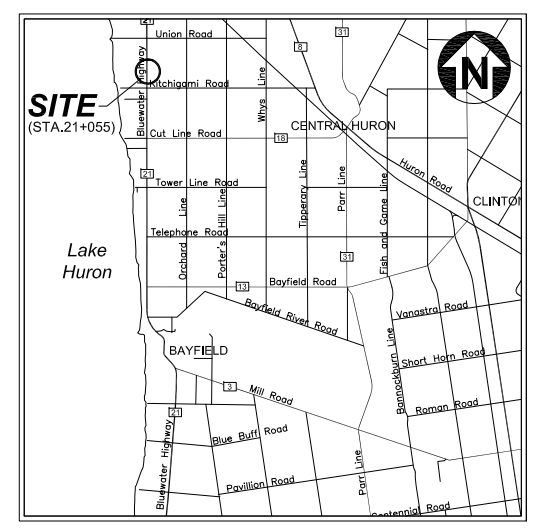
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TP110076




DRAWING No.  
2



**METRIC**  
DIMENSIONS ARE IN METRES  
AND/OR MILLIMETRES  
UNLESS OTHERWISE SHOWN

PURCHASE ORDER NUMBER: <b>3009-E-0022</b>	
G.W.P. No. <b>834-93-00</b>	
REHABILITATION OF HWY 21 FROM BAYFIELD TO GODERICH GEOCRES No.40P12-29 <b>CULVERT AT STA 21+055</b> <b>STRATIGRAPHIC CROSS SECTION</b>	
 <p><b>AMEC Environment &amp; Infrastructure,</b> a Division of AMEC Americas Limited</p>	





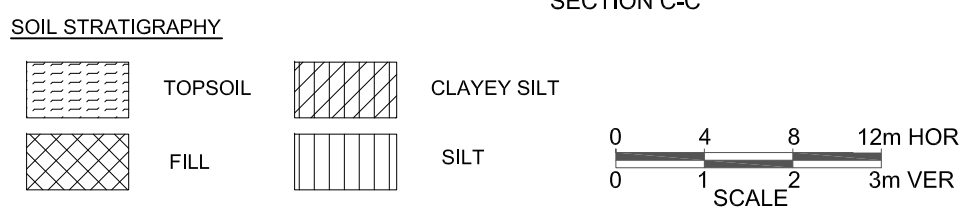
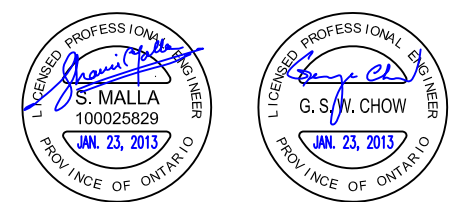
LEGEND			
	BOREHOLE LOCATION		
	GROUNDWATER LEVEL AT TIME OF INVESTIGATION		
	GROUNDWATER LEVEL IN MONITORING WELL		
EOP	EDGE OF PAVEMENT		
SHR	SHOULDER ROUND		

DESCRIPTION	UTM COORDINATES		ELEVATION (m)
	NORTHING	EASTING	
BH G17	4835074	443127	203.85
BH G18	4835073	443116	207.86
BH G19	4835080	443105	207.75

NOTES:

1. The boundaries between soil strata have been established only at borehole locations. Between boreholes, the boundaries are assumed from geological evidence and may be subject to considerable error.
2. This drawing is for subsurface information only. Surface details and features are for conceptual illustration.
3. Borehole without   was dry.



REVISIONS					
DESIGN PB	CHK PB	CODE CHBDC-06	CL 625-ONT	DATE JAN. 2013	
DRAWN KW	CHK HS	SITE 21+055		DWG 3	

**APPENDIX A**  
**RECORD OF BOREHOLES**



## EXPLANATION OF BOREHOLE LOG

This form describes some of the information provided on the borehole logs, which is based primarily on examination of the recovered samples, and the results of the field and laboratory tests. Additional description of the soil/rock encountered is given in the accompanying geotechnical report.

### GENERAL INFORMATION

Project details, borehole number, location coordinates and type of drilling equipment used are given at the top of the borehole log.

### SOIL LITHOLOGY

#### ***Elevation and Depth***

This column gives the elevation and depth of inferred geologic layers. The elevation is referred to the datum shown in the Description column.

#### ***Lithology Plot***

This column presents a graphic depiction of the soil and rock stratigraphy encountered within the borehole.

#### ***Description***

This column gives a description of the soil strata, based on visual and tactile examination of the samples augmented with field and laboratory test results. Each stratum is described according to the *MTC Soil Classification Manual*.

The compactness condition of cohesionless soils (SPT) and the consistency of cohesive soils (undrained shear strength) are defined as follows (*Ref. MTC Soil Classification Manual*):

Compactness of	
<u>Cohesionless Soils</u>	<u>SPT N-Value*</u>
Very loose	0 to 5
Loose	5 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	> 50

Consistency of	
<u>Cohesive Soils</u>	<u>Undrained Shear Strength</u>
	<u>kPa</u>
Very soft	0 to 12
Soft	12 to 25
Firm	25 to 50
Stiff	50 to 100
Very stiff	100 to 200
Hard	Over 200

\* For penetration of less than 0.3 m, N-values are indicated as the number of blows for the penetration achieved (e.g. 50/25: 50 blows for 25 centimeter penetration).

### Soil Sampling

Sample types are abbreviated as follows:

SS	Split Spoon	TW	Thin Wall Open (Pushed)	RC	Rock Core	GS	Grab Sample
AS	Auger Sample	TP	Thin Wall Piston (Pushed)	WS	Washed Sample	AR	Air Return Sample

Additional information provided in this section includes sample numbering, sample recovery and numerical testing results.

### Field and Laboratory Testing

Results of field testing (e.g., SPT, pocket penetrometer, and vane testing) and laboratory testing (e.g., natural moisture content, and limits) executed on the recovered samples are plotted in this section.

### Instrumentation Installation

Instrumentation installations (monitoring wells, piezometers, inclinometers, etc.) are plotted in this section. Water levels, if measured during fieldwork, are also plotted. These water levels may or may not be representative of the static groundwater level depending on the nature of soil stratum where the piezometer tips are located, the time elapsed from installation to reading and other applicable factors.

### Comments

This column is used to describe non-standard situations or notes of interest.

# MTC SOIL CLASSIFICATION

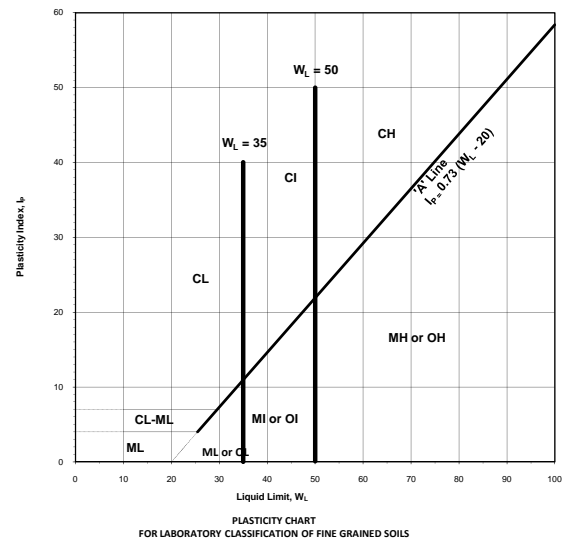
## Based on MTC Soil Classification Manual



MAJOR DIVISION					GROUP SYMBOL	TYPICAL DESCRIPTION	INFORMATION REQUIRED FOR DESCRIBING SOILS	LABORATORY CLASSIFICATION CRITERIA										
COARSE GRAINED SOILS (MORE THAN HALF BY WEIGHT LARGER THAN 75µm)	GRAVELS MORE THAN HALF THE COARSE FRACTION LARGER THAN 4.75mm	CLEAN GRAVELS (LITTLE OR NO FINES)	WIDE RANGE IN GRAIN SIZE & SUBSTANTIAL AMOUNTS OF ALL INTERMEDIATE PARTICULAR SIZE	GW	WELL GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	GIVE TYPE, NAME, IF NECESSARY, INDICATE APPROX % OF SAND & GRAVEL ; MAX SIZE; ANGULARITY, SURFACE CONDITION, & HARDNESS OF THE COARSE GRAINS, LOCAL OR GEOLOGICAL NAME & OTHER PERTINENT DESCRIPTIVE INFORMATION, & SYMBOL IN PARENTHESIS.	$C_u = \frac{D_{60}}{D_{10}}$ GREATER THAN 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ BETWEEN 1 AND 3											
			PREDOMINANTLY ONE SIZE OF A RANGE OF SIZES WITH STONE INTERMEDIATE SIZES MISSING	GP	POORLY GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES													
		GRAVEL WITH FINES (APPLICABLE AMOUNT OF FINES)	NON PLASTIC FINES (FOR IDENTIFICATION PROCEDURES SEE ML BELOW)	GM	SILTY GRAVELS, POORLY GRADED GRAVEL-SAND- SILT MIXTURES													
			PLASTIC FINES (FOR IDENTIFICATION PROCEDURES SEE CL BELOW)	GC	CLAYEY GRAVELS, POORLY GRADED GRAVEL-SAND-CLAY MIXTURES													
	SANDS MORE THAN HALF THE COARSE FRACTION SMALLER THAN 4.75mm	CLEAN SANDS (LITTLE OR NO FINES)	WIDE RANGE IN GRAIN SIZE & SUBSTANTIAL AMOUNT OF ALL INTERMEDIATE PARTICLE SIZES	SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	FOR UNDISTURBED SOILS ADD INFORMATION ON STRATIFICATION, DEGREE OF COMPACTNESS, CEMENTATION, MOISTURE CONDITION & DRAINAGE CHARACTERISTICS	NOT MEETING ALL GRADATION REQUIREMENTS FOR GW											
			PREDOMINANTLY ONE SIZE OR A RANGE OF SIZES WITH SOME INTERMEDIATE SIZE MISSING	SP	POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES													
		SANDS WITH FINES (APPLICABLE AMOUNT OF FINES)	NON PLASTIC FINES (FOR IDENTIFICATION PROCEDURES SEE ML BELOW)	SM	SILTY SANDS, POORLY GRADED SAND-SILT MIXTURES					ATTERBERG LIMITS BELOW A-LINE OR $I_p$ LESS THAN 4	ABOVE A-LINE WITH $I_p$ BETWEEN 4 AND 7 ARE BORDERLINE CASES REQUIRING USE OF DUAL SYMBOLS							
			PLASTIC FINES (FOR IDENTIFICATION PROCEDURES SEE CL BELOW)	SC	CLAYEY SANDS, POORLY GRADED SAND-CLAY MIXTURES													
	FINE-GRAINED SOILS (MORE THAN HALF BY WEIGHT SMALLER THAN 75µm)	IDENTIFICATION PROCEDURE ON FRACTION SMALLER THAN 425µm					USE GRAIN SIZE CURVE IN IDENTIFYING THE FACTORS AS GIVEN UNDER FIELD IDENTIFICATION	DETERMINE PERCENTAGE OF GRAVEL & SAND FROM GRAIN SIZE CURVE, DEPENDING ON PERCENTAGE OF FINES (FRACTION SMALLER THAN 75 µm) COARSE GRAINED SOILS ARE CLASSIFIED AS FOLLOWS:  LESS THAN 5% GW, GP, SW, SP MORE THAN 12% GM, GC, SM, SC 5% TO 12% BORDER LINE CASES REQUIRE USE OF DUAL SYMBOL	$C_u = \frac{D_{60}}{D_{10}}$ GREATER THAN 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ BETWEEN 1 AND 3	NOT MEETING ALL GRADATION FOR SW	ATTERBERG LIMITS ABOVE A- LINE WITH $I_p$ GREATER THAN 7	ABOVE A-LINE WITH $I_p$ BETWEEN 4 AND 7 ARE BORDERLINE CASES REQUIRING USE OF DUAL SYMBOLS						
		LIQUID LIMIT LESS THAN 35	DRY STRENGTH (CRUSHING CHARACTERISTICS)	DILATANCY (REACTION TO SHAKING)	TOUGHNESS (CONSISTENCY NEAR PLASTIC LIMIT)													
NONE			QUICK	NONE	ML	INORGANIC SILTS & SANDY SILTS OR SLIGHTLY PLASTICITY, ROCK FLOUR												
MEDIUM TO HIGH			NONE TO VERY SLOW	MEDIUM	CL	SILTY CLAYS (INORGANIC), GRAVELLY CLAYS, SANDY CLAYS, LEAN CLAYS												
SLIGHT TO MEDIUM			SLOW	SLIGHT	OL	ORGANIC SILT OF LOW PLASTICITY, ORGANIC SANDY SILTS												
LIQUID LIMIT BETWEEN 35 AND 50		NONE TO SLIGHT	SLOW TO QUICK	SLIGHT	MI	INORGANIC COMPRESSIBLE FINE SANDY SILT WITH CLAY OF MEDIUM PLASTICITY, CLAYEY SILTS												
		HIGH	NONE	MEDIUM TO HIGH	CI	SILTY CLAYS (INORGANIC) OF MEDIUM PLASTICITY												
		SLIGHT TO MEDIUM	VERY SLOW	SLIGHT	OI	ORGANIC SILTY CLAYS OF MEDIUM PLASTICITY												
		SLIGHT TO MEDIUM	SLOW TO NONE	MEDIUM	MH	INORGANIC SILTS, HIGHLY COMPRESSIBLE MICACEOUS OR DIATOMEACACOUS FINE SANDY SILTS, ELASTIC SILTS												
LIQUID LIMIT GREATER THAN 50		HIGH TO VERY HIGH	NONE	HIGH	CH	CLAYS (INORGANIC) OF HIGH PLASTICITY, FAT CLAYS												
		MEDIUM TO HIGH	NONE TO VERY SLOW	SLIGHT TO MEDIUM	OH	ORGANIC CLAYS OF HIGH PLASTICITY												
					</													

USE GRAIN SIZE CURVE IN IDENTIFYING THE FACTORS AS GIVEN UNDER FIELD IDENTIFICATION

FRACTION	U.S STANDARD SIEVE SIZE		DEFINING RANGES OF PERCENTAGE BY WEIGHT OF MINOR COMPONENTS		
GRAVEL	COARSE	PASSING	RETAINED	PERCENT	DESCRIPTOR
		75 mm	26.5 mm	40-50 30-40 20-30 10-20 1-10	AND Y/EY WITH SOME TRACE
	FINE	26.5 mm	4.75 mm		
SAND	COARSE	4.75 mm	2.00 mm		
	MEDIUM	2.00 mm	425 µm		
	FINE	425 µm	75 µm		
FINES (SILT OR CLAY BASED ON PLASTICITY)		75 µm			
OVERSIZED MATERIAL					
ROUNDED OR SUBROUNDED: COBBLES 75 mm TO 200 mm BOULDERS > 200 mm				NOT ROUNDED: ROCK FRAGMENTS > 75 mm ROCKS > 0.76 CUBIC METRE IN VOLUME	



**BOUNDARY CLASSIFICATION:** BOUNDARY CLASSIFICATION: SOILS POSSESSING CHARACTERISTICS OF TWO GROUPS ARE DESIGNATED BY COMBINATIONS OF GROUP SYMBOLS FOR EXAMPLE GW-GC WELL GRADED GRAVEL-SAND MIXTURE WITH CLAY BINDER



AMEC Earth & Environmental,  
a Division of AMEC American

[www.amec.com](http://www.amec.com)

**MTC SOIL CLASSIFICATION MANUAL  
ENGINEERING PROPERTIES OF SOIL**



TYPICAL NAMES OF SOIL GROUPS	GROUP SYMBOLS	PERMEABILITY WHEN COMPACTED	STRENGTH WHEN COMPACTED	COMPRESSIBILITY WHEN COMPACTED	WORKABILITY AS A CONSTRUCTION MATERIAL	SCOUR RESISTANCE	SUSCEPTIBILITY TO SURFICIAL EROSION	SUSCEPTIBILITY TO FROST ACTION	DRAINAGE CHARACTERISTICS
WELL-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	GW	PERVIOUS	EXCELLENT	NEGLECTIBLE	EXCELLENT	MEDIUM	NEGLECTIBLE	NEGLECTIBLE	EXCELLENT
POORLY GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	GP	VERY PERVIOUS	GOOD	NEGLECTIBLE	GOOD	MEDIUM	NEGLECTIBLE	NEGLECTIBLE	EXCELLENT
SILTY GRAVELS, POORLY GRADED GRAVEL- SAND-SILT MIXTURES	GM	SEMI-PERVIOUS TO IMPERVIOUS	GOOD	NEGLECTIBLE	GOOD	LOW TO MEDIUM	SLIGHT	SLIGHT	FAIR TO SEMI IMPERVIOUS
CLAYEY GRAVELS, POORLY GRADED GRAVEL-SAND-CLAY MIXTURES	GC	IMPERVIOUS	GOOD TO FAIR	VERY LOW	GOOD	MEDIUM	SLIGHT	NEGLECTIBLE TO SLIGHT	PRACTICALLY IMPERVIOUS
WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	SW	PERVIOUS	EXCELLENT	NEGLECTIBLE	EXCELLENT	LOW TO MEDIUM	SLIGHT	NEGLECTIBLE	EXCELLENT
POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	SP	PERVIOUS	GOOD	VERY LOW	FAIR TO GOOD	LOW TO MEDIUM	MODERATE	NEGLECTIBLE TO SLIGHT	EXCELLENT
SILTY SANDS, POORLY GRADED SAND-SILT MIXTURES	SM	SEMI-PERVIOUS TO IMPERVIOUS	GOOD	LOW	FAIR	LOW	MODERATE	SLIGHT TO MODERATE	FAIR TO SEMI IMPERVIOUS IMPERVIOUS
CLAYEY SANDS, POORLY GRADED SAND WITH SOME CLAY MIXTURES	SC	IMPERVIOUS	GOOD TO FAIR	LOW	GOOD	VERY LOW TO LOW	MODERATE TO SLIGHT	NEGLECTIBLE	PRACTICALLY IMPERVIOUS
INORGANIC SILTS AND SANDY SILTS OF SLIGHT PLASTICITY, ROCK FLOUR	ML	SEMI-PERVIOUS TO IMPERVIOUS	FAIR	MEDIUM	FAIR	VERY LOW	SEVERE	SEVERE	FAIR TO POOR
INORGANIC CLAYEY SILTS OF LOW PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, LEAN CLAYS	CL	IMPERVIOUS	FAIR	MEDIUM	GOOD TO FAIR	LOW TO MEDIUM	SLIGHT TO MODERATE	MODERATE TO SEVERE	PRACTICALLY IMPERVIOUS
ORGANIC SILTS OF LOW PLASTICITY	OL	SEMI-PERVIOUS TO IMPERVIOUS	POOR	MEDIUM	FAIR TO POOR	VERY LOW TO LOW	SEVERE	SEVERE	POOR
INORGANIC COMPRESSIBLE SILTS OF MEDIUM PLASTICITY	MI	SEMI-PERVIOUS TO IMPERVIOUS	FAIR	MEDIUM TO HIGH	FAIR TO POOR	LOW	MODERATE	MODERATE TO SEVERE	FAIR TO POOR
INORGANIC SILTY CLAYS OF MEDIUM PLASTICITY	CI	IMPERVIOUS	FAIR TO POOR	HIGH	FAIR	LOW TO MEDIUM	SLIGHT	MODERATE TO SEVERE	SEMI IMPERVIOUS TO PRACTICALLY
ORGANIC SILTY CLAY OF MEDIUM PLASTICITY	OI	SEMI-PERVIOUS TO IMPERVIOUS	POOR	HIGH	POOR	VERY LOW TO LOW	SEVERE	MODERATE TO SEVERE	POOR TO PRACTICALLY IMPERVIOUS
INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS	MH	SEMI-PERVIOUS TO IMPERVIOUS	FAIR TO POOR	HIGH	POOR	VERY LOW	MEDIUM	SEVERE	POOR
INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS	CH	IMPERVIOUS	POOR	HIGH	FAIR TO POOR	LOW TO MEDIUM	SLIGHT TO NEGLECTIBLE	NEGLECTIBLE	PRACTICALLY IMPERVIOUS
ORGANIC CLAYS OF HIGH PLASTICITY	OH	IMPERVIOUS	POOR	HIGH	POOR	LOW	MODERATE	NEGLECTIBLE TO SLIGHT	PRACTICALLY IMPERVIOUS
PEAT AND OTHER HIGHLY ORGANIC SOILS	Pt	-	-	-	-	LOW	SEVERE	-	FAIR TO GOOD

# RECORD OF BOREHOLE No. BH G17

G.W.P. 834-93-00	LOCATION Sta: 21+055, 4.0 m S of Culvert, 16.4 m Rt of CL of Rd. E 443127; N 4835074	1 OF 2	ORIGINATED BY JF
DIST Goderich HWY 21	BOREHOLE TYPE 150 mm diameter borehole (Solid Stem)	COMPILED BY SAL	
DATUM Geodetic	DATE 17 May 2011 - 17 May 2011	CHECKED BY SM	
PROJECT Rehabilitation of Highway 21, from Bayfield to Goderich, Ontario			JOB NO. TP110076

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	DEPTH m	ELEVATION m	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	SOIL VAPOUR READING PPM	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH (m)	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES				SHEAR STRENGTH kPa						
203.9 0.0	about 400 mm TOPSOIL		1	SS	6										
203.5 0.4	brown <b>Sandy Silt FILL</b> trace gravel and clay trace organic matter moist		2	SS	21										
202.5 1.4	light grey <b>SILT</b> some to with sand, trace clay and gravel, trace cobbles/boulders very dense moist to wet		3	SS	50/15										
			4	SS	50/13										
			5	SS	50/8										
			6	SS	50/13										
			7	SS	56/15										
			8	SS	52/15										

Continued Next Page

+<sup>3</sup>, ×<sup>3</sup>: Numbers refer to Sensitivity      ○ 3% STRAIN AT FAILURE

[illegible]

[illegible]

+ 3, × 3: Numbers refer to Sensitivity      ○ 3% STRAIN AT FAILURE

2 OF 2

G.W.P. 834-93-00	LOCATION Sta: 21+055, 3.0 m S of Culvert, 5.4 m Rt of CL of Rd, E 443116: N 4835073	ORIGINATED BY JF
DIST Goderich HWY 21	BOREHOLE TYPE 150 mm diameter borehole (Solid Stem)	COMPILED BY SAL
DATUM Geodetic	DATE 18 May 2011 - 18 May 2011	CHECKED BY SM
PROJECT Rehabilitation of Highway 21, from Bayfield to Goderich, Ontario	JOB NO.	TP110076

[illegible]

+ 3, × 3: Numbers refer to Sensitivity      ○ 3% STRAIN AT FAILURE



# RECORD OF BOREHOLE No. BH G19

G.W.P. 834-93-00		LOCATION Sta: 21+055, 3.0 m S of Culvert, 5.4 m Lt of CL of Rd, E 443105; N 4835080		1 OF 2	
DIST Goderich HWY 21		BOREHOLE TYPE 150 mm diameter borehole (Solid Stem)		ORIGINATED BY JF	
DATUM Geodetic		DATE 17 May 2011 - 17 May 2011		COMPILED BY SAL	
PROJECT Rehabilitation of Highway 21, from Bayfield to Goderich, Ontario				CHECKED BY SM	
				JOB NO. TP110076	

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	DEPTH m	ELEVATION SCALE m	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	SOIL VAPOUR READING PPM	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL			
ELEV DEPTH (m)	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES				SHEAR STRENGTH kPa										WATER CONTENT (%)		
									○ UNCONFINED	● QUICK TRIAXIAL	+ FIELD VANE	× LAB VANE	20						40	60	80
207.8																					
0.0	brown <b>Sand and Gravel FILL</b> some silt moist		1	SS	36									5 <sub>O</sub>							
207.2																					
0.6	brown <b>Sand FILL</b> trace to some silt and gravel		2	SS	26									10 <sub>O</sub>							
			3	SS	31									9 <sub>O</sub>							
			4	SS	5									13 <sub>O</sub>							
	pocket of organic matter in SS4																				
			5	SS	2									15 <sub>O</sub>							
	mixed with organic matter in SS5																				
	trace organic matter in SS6		6	SS	12									14 <sub>O</sub>							
201.5			7	SS	55/15									15 <sub>O</sub>							
6.2	grey <b>SILT</b> some to with sand, trace clay very dense moist to wet																				
	trace cobbles / boulders																				
			8	SS	50/10									15 <sub>O</sub>			2 18 75 5				
										</											

Continued Next Page

+ 3, × 3: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

# RECORD OF BOREHOLE No. BH G19

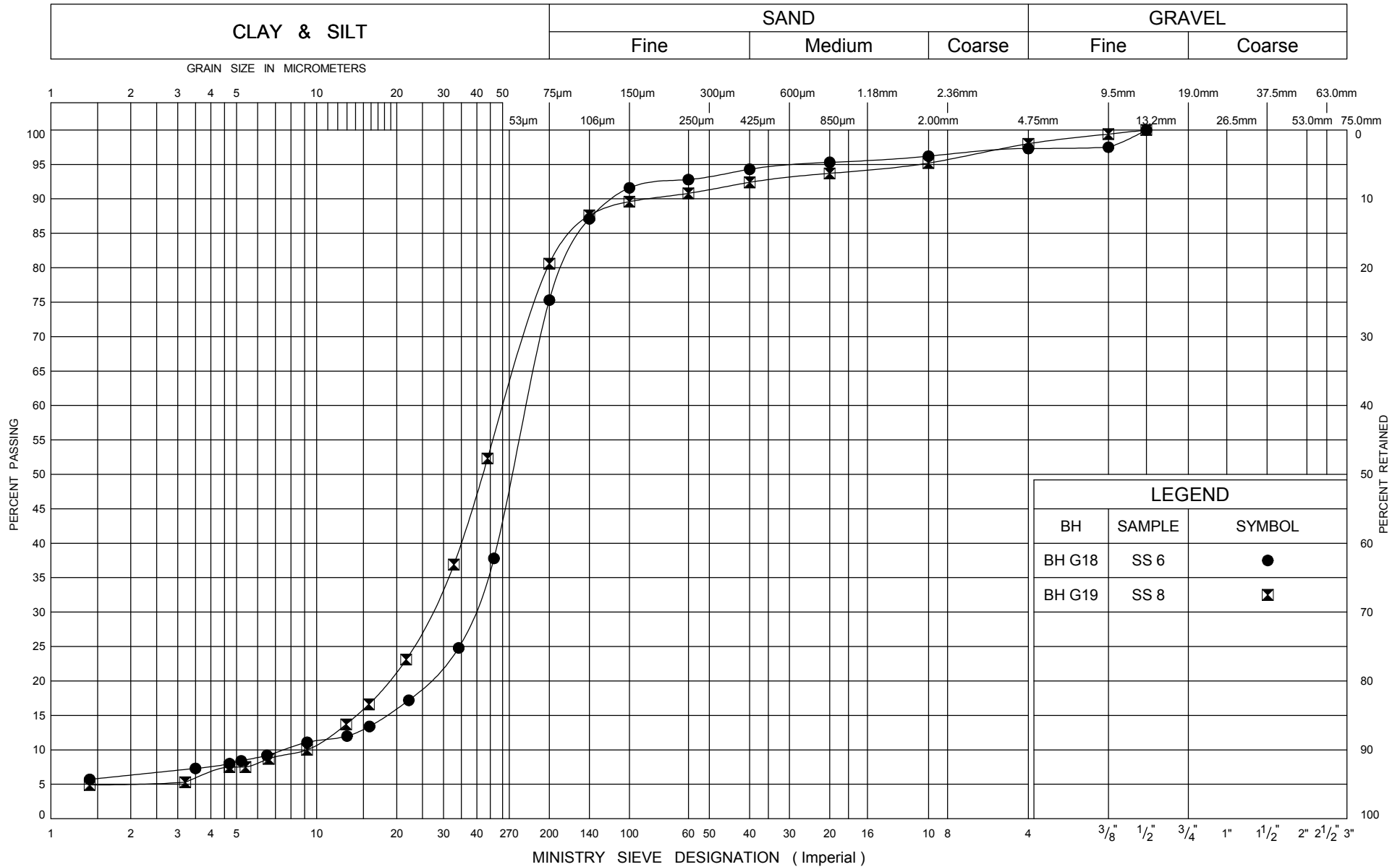
G.W.P. 834-93-00	LOCATION Sta: 21+055, 3.0 m S of Culvert, 5.4 m Lt of CL of Rd, E 443105; N 4835080	2 OF 2	ORIGINATED BY JF
DIST Goderich HWY 21	BOREHOLE TYPE 150 mm diameter borehole (Solid Stem)	COMPILED BY SAL	
DATUM Geodetic	DATE 17 May 2011 - 17 May 2011	CHECKED BY SM	
PROJECT Rehabilitation of Highway 21, from Bayfield to Goderich, Ontario			JOB NO. TP110076

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	DEPTH m	ELEVATION SCALE m	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	SOIL VAPOUR READING	REMARKS & GRAIN SIZE DISTRIBUTION (%)							
ELEV DEPTH (m)	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES				SHEAR STRENGTH kPa									WATER CONTENT (%)			PPM	GR	SA	SI	CL
									○ UNCONFINED	+ FIELD VANE	● QUICK TRIAXIAL	× LAB VANE	20					40	60	80					
			9	SS	50/8									12	○										
	trace cobbles / boulders							198																	
			10	SS	50/8									15	○										
	trace cobbles / boulders							197																	
196.3																									
11.4	grey CLAYEY SILT trace sand hard							196																	
			11	SS	50/13									14	○										
								195																	
			12	SS	50/13			194						14	○										
								193																	
192.4			13	SS	50/13									13	○										
15.4	End of Borehole																								
	Groundwater level on 17 May 2011 was 8.1 m depth																								

**APPENDIX B**

**LABORATORY TEST RESULTS**

## UNIFIED SOIL CLASSIFICATION SYSTEM

Ministry of  
Transportation

Ontario

## GRAIN SIZE DISTRIBUTION

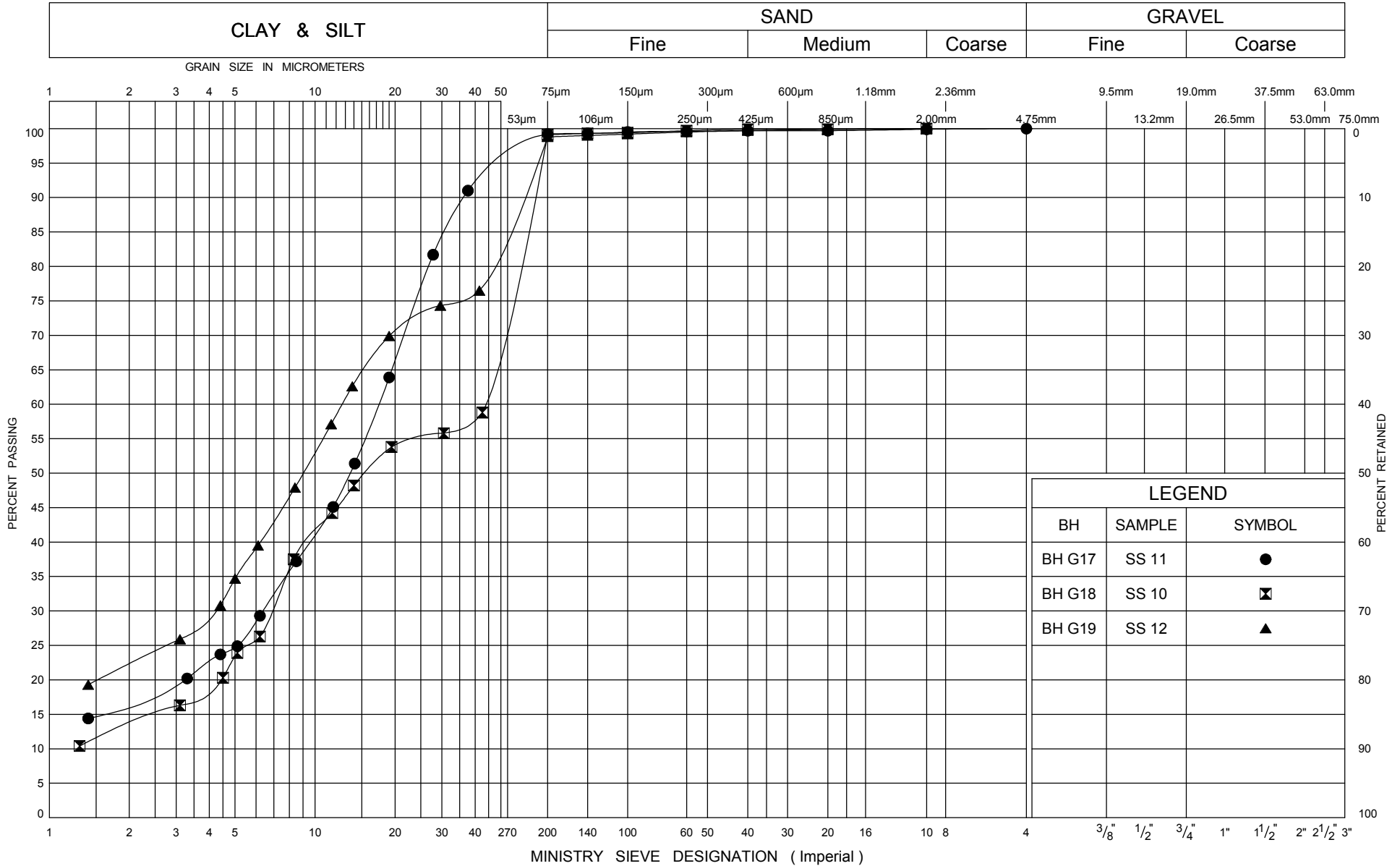
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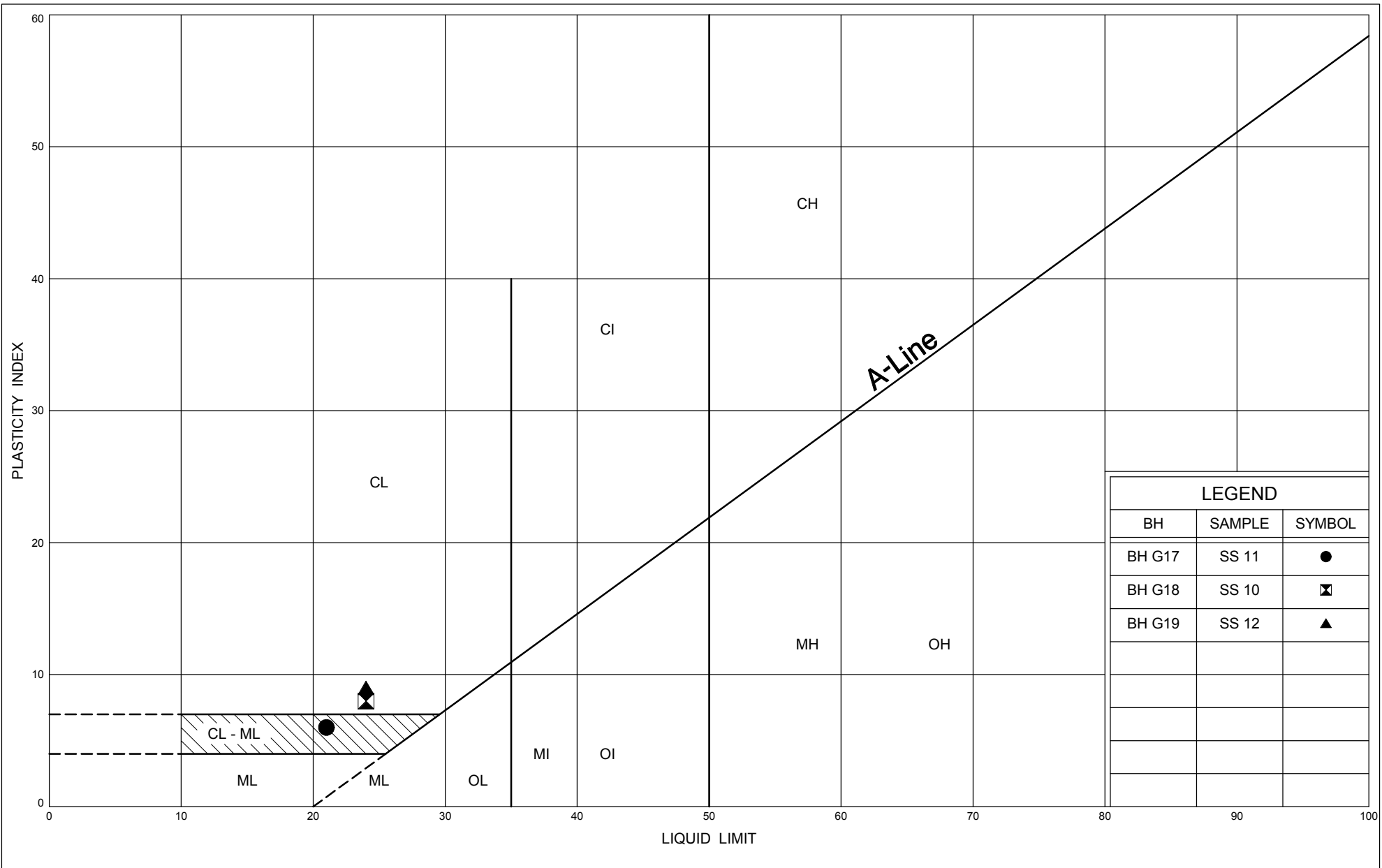
Figure No. B1

G.W.P. 834-93-00

Culvert at Sta. 21+055, Hwy 21, Bayfield to Goderich

# UNIFIED SOIL CLASSIFICATION SYSTEM





Your Project #: TP110076.5  
 Site: HWY21 (7 CULVERTS)  
 Your C.O.C. #: 32091

**Attention: Shami Malla**

AMEC Earth & Environmental Ltd  
 Scarborough  
 104 Crockford Blvd  
 Scarborough, ON  
 CANADA M1R3C3

Report Date: 2011/06/06

## CERTIFICATE OF ANALYSIS

**MAXXAM JOB #: B175937**

**Received: 2011/05/27, 17:25**

Sample Matrix: Soil  
 # Samples Received: 7

Analyses	Quantity	Date Extracted	Date Analyzed	Laboratory Method	Method Reference
Chloride (20:1 extract)	4	N/A	2011/06/02	CAM SOP-00463	
Chloride (20:1 extract)	3	N/A	2011/06/04	CAM SOP-00463	
Conductivity	7	N/A	2011/06/02	CAM SOP-00414	APHA 2510
pH CaCl2 EXTRACT	6	2011/06/02	2011/06/02	CAM SOP-00413	SM 4500 H
pH CaCl2 EXTRACT	1	2011/06/03	2011/06/03	CAM SOP-00413	SM 4500 H
Resistivity of Soil	7	2011/05/30	2011/06/02	CAM SOP-00414	APHA 2510
Sulphate (20:1 Extract)	7	N/A	2011/06/04	CAM SOP-00464	EPA 375.4

\* RPDs calculated using raw data. The rounding of final results may result in the apparent difference.

\* Results relate only to the items tested.

Encryption Key

Please direct all questions regarding this Certificate of Analysis to your Project Manager.

GINA BAYBAYAN,  
 Email: GBAYBAYAN@maxxam.ca  
 Phone# (905) 817-5766

=====

Maxxam has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per section 5.10.2 of ISO/IEC 17025:2005(E), signing the reports. For Service Group specific validation please refer to the Validation Signature Page.

Total cover pages: 1

Maxxam Job #: B175937  
Report Date: 2011/06/06

AMEC Earth & Environmental Ltd  
Client Project #: TP110076.5  
Project name: HWY21 (7 CULVERTS)

### RESULTS OF ANALYSES OF SOIL

Maxxam ID		JQ4509	JQ4510	JQ4511		JQ4512		JQ4513		JQ4514		
Sampling Date		2011/05/16	2011/05/16	2011/05/16		2011/05/16		2011/05/18		2011/05/17		
	Units	BH G7 / SS2	BH G10 / SS4	BH G12 / SS4	QC Batch	BH G13 / SS2	QC Batch	BH G15 / SS2	QC Batch	BH G17 / SS3	RDL	QC Batch
<b>Calculated Parameters</b>												
Resistivity	ohm-cm	2600	530	1100	2502843	3200	2502843	6000	2502843	3200		2502843
<b>Inorganics</b>												
Soluble (20:1) Chloride (Cl)	ug/g	140	970	470	2508305	120	2506767	<20	2506767	49	20	2506767
Conductivity	umho/cm	389	1870	951	2506690	316	2506690	166	2506690	317	2	2506690
Available (CaCl2) pH	pH	7.61	7.50	7.60	2506893	7.81	2506893	7.61	2508147	7.93		2506893
Soluble (20:1) Sulphate (SO4)	ug/g	<20	<20	<20	2508307	<20	2506764	<20	2506764	120	20	2506764

Maxxam ID		JQ4515		
Sampling Date		2011/05/17		
	Units	BH G21 / SS2	RDL	QC Batch
<b>Calculated Parameters</b>				
Resistivity	ohm-cm	1900		2502843
<b>Inorganics</b>				
Soluble (20:1) Chloride (Cl)	ug/g	270	20	2506767
Conductivity	umho/cm	532	2	2506690
Available (CaCl2) pH	pH	7.74		2506893
Soluble (20:1) Sulphate (SO4)	ug/g	<20	20	2506764

RDL = Reportable Detection Limit  
QC Batch = Quality Control Batch



Maxxam Job #: B175937  
Report Date: 2011/06/06

AMEC Earth & Environmental Ltd  
Client Project #: TP110076.5  
Project name: HWY21 (7 CULVERTS)

Package 1	20.3°C
-----------	--------

Each temperature is the average of up to three cooler temperatures taken at receipt

**GENERAL COMMENTS**

Maxxam Job #: B175937  
Report Date: 2011/06/06

AMEC Earth & Environmental Ltd  
Client Project #: TP110076.5  
Project name: HWY21 (7 CULVERTS)

### QUALITY ASSURANCE REPORT

QC Batch	Parameter	Date	Matrix Spike		Spiked Blank		Method Blank		RPD		QC Standard	
			% Recovery	QC Limits	% Recovery	QC Limits	Value	Units	Value (%)	QC Limits	% Recovery	QC Limits
2506690	Conductivity	2011/06/02					<2	umho/cm	2.4	35	103	75 - 125
2506764	Soluble (20:1) Sulphate (SO4)	2011/06/04	102	75 - 125	97	85 - 115	<20	ug/g	NC	35		
2506767	Soluble (20:1) Chloride (Cl)	2011/06/02	109	75 - 125	98	85 - 115	<20	ug/g	NC	35		
2508305	Soluble (20:1) Chloride (Cl)	2011/06/04	95	75 - 125	103	85 - 115	<20	ug/g	5.0	35		
2508307	Soluble (20:1) Sulphate (SO4)	2011/06/04	111	75 - 125	95	85 - 115	<20	ug/g	NC	35		

N/A = Not Applicable

RPD = Relative Percent Difference

Duplicate: Paired analysis of a separate portion of the same sample. Used to evaluate the variance in the measurement.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate sample matrix interference.

QC Standard: A blank matrix to which a known amount of the analyte has been added. Used to evaluate analyte recovery.

Spiked Blank: A blank matrix to which a known amount of the analyte has been added. Used to evaluate analyte recovery.

Method Blank: A blank matrix containing all reagents used in the analytical procedure. Used to identify laboratory contamination.

NC (RPD): The RPD was not calculated. The level of analyte detected in the parent sample and its duplicate was not sufficiently significant to permit a reliable calculation.

## Validation Signature Page

**Maxxam Job #: B175937**

---

The analytical data and all QC contained in this report were reviewed and validated by the following individual(s).

A handwritten signature in black ink, appearing to read "Cristina Carriere", is written over a horizontal line.

CRISTINA CARRIERE, Scientific Services

=====

Maxxam has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per section 5.10.2 of ISO/IEC 17025:2005(E), signing the reports. For Service Group specific validation please refer to the Validation Signature Page.

## **APPENDIX C**

### **SITE PHOTOGRAPHS**



**Photograph No. 1:** View of existing culvert inlet.



**Photograph No. 2:** View inside the culvert.





**Photograph No. 3:** View embankment at existing culvert outlet.