



THURBER ENGINEERING LTD.

**FINAL
FOUNDATION INVESTIGATION AND DESIGN REPORT
HIGHWAY 11 UNDERPASS STRUCTURE
MUSKOKA RD 3, DISTRICT OF MUSKOKA
AGREEMENT NO. 5017-E-0003
Site No.: 42-169**

G.W.P. 5336-11-00

Geocres No.: 31E-393

Report to:

McIntosh Perry Consulting Engineers Limited

Latitude: 45.312632°
Longitude: -79.245535°

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PART 1. FACTUAL INFORMATION

1 INTRODUCTION

This section of the report presents the factual findings obtained from a foundation investigation completed at the Muskoka Road 3 crossing of Highway 11, located approximately 4 km south of Highway 60 within the District of Muskoka. Thurber Engineering Limited (Thurber) carried out the current field investigation as a sub-consultant to McIntosh Perry Consulting Engineers Ltd. (MPCE) under Assignment No. 5017-E-0003.

The purpose of this investigation was to explore the subsurface conditions at the site and, based on the data obtained, to provide a borehole location plan, records of boreholes, stratigraphic profile, laboratory test results and a written description of the subsurface conditions. A model of the subsurface conditions influencing design and construction was developed in the course of the current investigation. The following historical foundation investigation report was obtained from the online Geocres library and reviewed in preparation of this report.

Foundation Investigation Report, W.P. 74-74-06, Hwy 11 District 11, Huntsville, Muskoka Road No. 3, South Junction to Huntsville, 2.6 Miles South of Hwy. 60 [Geocres No. 31E-79]

2 SITE DESCRIPTION

The project assignment includes an underpass structure that is a two span cast in place post tensioned voided concrete slab bridge. The existing underpass conveys Muskoka Road 3 in a west – east alignment at an approximate skew of 4 degrees over Highway 11.

The underpass (Structure No.42-169) has two spans of 36.6 m each, an overall width varying from 11.6 to 13.1 m and a road width varying from 9.1 to 10.6 m. The clearance under the structure is approximately 4.6 m. The structure is understood to have been constructed in 1980 and rehabilitated in 2000. The foundations are documented to consist of H-piles driven to refusal.

At the location of the underpass structure, Highway 11 includes two driving lanes plus a speed change lane in each direction. Highway 11 has a rural cross-section, paved

shoulders and a grassed median. Ramp intersection are located on Muskoka Road 3 approximately 100 to 150 m from both ends of the structure.

The approach fill height is approximately 7.3 m with the Muskoka Road 3 road surface at elevation 329.2 m. The existing Muskoka Road 3 embankment slopes are inclined at approximately 3.2H:1V. Reinforced concrete barrier walls with railing are situated on each side of the deck as guardrails.

The land adjacent to Highway 11 is vegetated with grasses and trees. Adjacent to the highway right-of-way, are primarily commercial properties. Traffic volumes on this section of Highway 11 are understood to be 18,200 AADT (2016).

Select photographs showing the existing conditions in the area of the underpass at the time of the field investigation are included in Appendix D for reference.

3 SITE INVESTIGATION AND FIELD TESTING

The site investigation and field testing program was carried out on April 27th, 2018. The field investigation consisted of advancing two boreholes, identified as 18-1 and 18-2 near the abutments of the structure. The drilling was carried out using a truck mounted CME 55 drill rig equipped with hollow stem augers. Prior to commencement of drilling, utility clearances were obtained in the vicinity of the borehole locations.

Soil samples were obtained at selected intervals using a split spoon sampler in conjunction with Standard Penetration Testing (SPT). The boreholes were sampled to depths of 16.5 and 18.0 m below the existing ground surface (elev. 312.9 and 311.1 m) in Boreholes 18-1 and 18-2, respectively. The drilling and sampling operations were supervised on a full time basis by an experienced member of Thurber's technical staff. The drilling supervisor logged the boreholes and processed the recovered soil samples for transport to Thurber's Ottawa geotechnical laboratory for further examination and testing.

The approximate borehole locations are shown on the Borehole Locations and Soil Strata Drawing included in Appendix A. The coordinates and elevations of the boreholes from the current investigation are provided on this drawing and on the individual Record of Borehole sheets. The northing and easting (MTM zone 10), elevation, and termination depth of the boreholes are summarized below in Table 3-1. The borehole elevations were surveyed relative to benchmark HCM 820120090 (elev. 329.535 m), shown on Drawing B-625-11-10 dated April 20, 2017, provided by MPCE, with a Nikon-AP-8 with an accuracy of +/- 1.5 mm. Horizontal locations were measured relative to existing site features.

Table 3-1: Borehole Summary

Borehole No.	Drilled Location	Northing (m)	Easting (m)	Ground Surface Elevation (m)	Termination Depth (m)
18-1	West of Structure, EB Lane	5 019 219.7	324 701.2	329.3	16.5
18-2	East of Structure, WB Lane	5 019 209.0	324 804.6	329.1	18.0

Following completion of the field investigation the boreholes were backfilled in accordance with MOE requirements (O.Reg. 903, as amended). All boreholes were backfilled with granulars within the depth of pavement structure and capped with 150 mm of cold patch asphalt to reinstate the traveling surface.

4 LABORATORY TESTING

The recovered soil samples were subjected to visual identification and to natural moisture content determination. Selected samples were also subjected to gradation analysis (hydrometer and/or sieve) and Atterberg Limit testing. The results of these tests are summarized on the Record of Borehole sheets included in Appendix B. One sample of soil recovered from within each Borehole was selected and submitted for analytical testing of corrosivity parameters. All laboratory test results are provided in Appendix C.

5 DESCRIPTION OF SUBSURFACE CONDITIONS

Details of the encountered soil stratigraphy are presented on the Record of Borehole sheets included in Appendix B and the Borehole Location and Soil Strata drawing included in Appendix A. A general description of the stratigraphy, based on the conditions encountered in the boreholes, is given in the following paragraphs. However, the factual data presented on the Record of Borehole sheets takes precedence over this general description for interpretation of the site conditions. It must be recognized that the soil and groundwater conditions may vary between and beyond borehole locations.

In general terms, the site was found to be underlain by a pavement structure and sand to silty sand fill overlying native silt to silty clay to clayey silt and silty sand. Bedrock was not encountered within the depth of the current investigation.

A review of the historic Geocres report indicated elevation discrepancies with the current investigation and benchmarks, thus the information has not been included in the discussion provided herein.

5.1 Embankment

5.1.1 Asphalt

Both boreholes were drilled through the existing Muskoka Road 3 approach embankments and encountered a layer of asphalt at the surface with a thickness of 100 to 150 mm.

5.1.2 Fill: Sand

Encountered below the asphalt was a layer of granular fill consisting of sand with varying amounts of silt and gravel. The underside depth of the granular fill ranged from 6.1 to 7.2 m (elev. 321.9 to 323.2 m) below the existing roadway.

The SPT tests conducted in this fill gave N-values typically ranging from 3 to 40 blows indicating a relative density of very loose to dense.

Recorded moisture contents ranged from 1 to 8%. The results of grain size analyses conducted on two samples of the sand fill are summarized below and are illustrated on Figure C1 in Appendix C.

Soil Particle	Percentage (%)
Gravel	0 – 19
Sand	76 – 89
Silt & Clay	5 – 11

5.1.3 Fill: Silty Sand

Encountered below the sand fill in Borehole 18-2 was a layer of silty sand fill. The thickness of this fill deposit was 3.0 m with a base elevation of 318.9 m.

The SPT tests conducted in this silty sand layer gave N-values of 3 and 5 blows indicating a relative density of very loose to loose.

The recorded moisture contents were approximately 23%. Atterberg Limits testing conducted on one sample of the silty sand indicate this material to be non-plastic. The results of a grain size analysis conducted on one sample of the silty sand indicated the material to consist of 0% gravel, 52% sand, 38% silt and 10% clay, and the results are summarized on the Record of Borehole sheet in Appendix B and are illustrated on Figure C1 in Appendix C.

5.2 Silty Clay (CL-ML)

Below the west embankment fill in Borehole 18-1 and the silty sand in Borehole 18-2 (see Section 5.1.3) was a native deposit of silty clay with varying amounts of sand. The silty clay deposit was 1.5 to 4.1 m thick and extended to a base elevation ranging from 317.4 to 319.1 m.

The SPT tests conducted in the silty clay layer gave N-values ranging from 5 to 35 indicating a firm to hard consistency.

Recorded moisture contents ranged from 16 to 39%. The results of grain size analysis conducted on two samples of the silty clay are summarized below and are illustrated on Figure C2 in Appendix C.

Soil Particle	Percentage (%)
Gravel	0 – 2
Sand	5 – 10
Silt	72 – 74
Clay	18 – 19

The results of Atterberg Limits testing completed on two samples of this material indicated a liquid limit ranging from 22 to 25, a plastic limit ranging from 17 to 21, and a plasticity index ranging from 4 to 5. The laboratory results indicate that the silty clay has low plasticity

(CL- ML). The results are summarized on the Record of Borehole sheets in Appendix B and the Atterberg Limits graph is included in Figure C5 of Appendix C.

5.3 Clayey Silt (MI)

Encountered below the silty clay in Borehole 18-2 (see Section 5.2) was a native deposit of clayey silt. The thickness of this clayey silt deposit was 4.6 m and it extended to a base elevation of 312.8 m.

In-situ shear vane test results indicated varying undrained shear strength ranging from 27 to 81 kPa indicating a firm to stiff consistency. SPT tests gave N-values ranging from Weight of Hammer to 15.

Recorded moisture contents ranged from 34 to 44%. The results of grain size analysis conducted on one sample of the clayey silt indicated the material to consist of 0% gravel, 1% sand, 50% silt and 49% clay, and the results are summarized on the Record of Borehole sheet in Appendix B and are illustrated on Figure C2 in Appendix C.

The results of Atterberg Limits testing completed on one sample of this material indicated a liquid limit of 37, a plastic limit of 26, and a plasticity index of 11. The laboratory results indicate that the clayey silt has intermediate plasticity (MI). The results are summarized on the Record of Borehole sheet in Appendix B and the Atterberg Limits graph is included in Figure C5 of Appendix C.

5.4 Silt (ML)

Encountered below the silty clay in Borehole 18-1 and the clayey silt in Borehole 18-2 was a silt deposit. Borehole 18-2 was terminated within this layer at a base elevation of 311.1 m. The thickness of this layer in Borehole 18-1 was 4.6 m with a base elevation of 314.5 m.

The SPT tests conducted in the silt layer gave N-values ranging from 8 to 18 indicating a relative density of loose to compact.

Recorded moisture contents ranged from 24 to 32%. Atterberg Limits testing conducted on one sample of the silt indicate this material to be non-plastic. The results of grain size analysis conducted on one sample of the silt indicated this material to consist of 0% gravel, 2% sand, 71% silt and 27% clay and the results are summarized on the Record of Borehole sheet in Appendix B and illustrated on Figure C3 in Appendix C.

5.5 Silty Sand (SM) with gravel

Borehole 18-1 encountered a silty sand with gravel deposit below the silt. The borehole was terminated within this layer at a base elevation of 312.9 m.

The SPT tests conducted in this silty sand layer gave N-values of 18 and 22 blows indicating a relative density of compact.

The recorded moisture contents ranged from 14 to 28%. Atterberg Limits testing conducted on one sample of the silty sand indicate this material to be non-plastic. The results of a grain size analysis conducted on one sample of the silty sand indicated the material to consist of 16% gravel, 48% sand, 33% silt and 3% clay, and the results are summarized on the Record of Borehole sheet in Appendix B and illustrated on Figure C4 in Appendix C.

5.6 Groundwater

The groundwater level measured in both boreholes during drilling operations on April 27th, 2018 was recorded at an elevation of approximately 322.0 to 322.1 m.

These observations are considered short term and it should be noted that the groundwater level at the time of construction could be different and seasonal fluctuations of the groundwater level are to be expected. In particular, the groundwater level may be at a higher elevation after periods of significant and/or prolonged precipitation events.

5.7 Analytical Testing

Two samples of soil were submitted to Paracel Laboratories in Ottawa, Ontario for analysis of pH, water soluble sulphate, sulphide and chloride concentrations, resistivity and conductivity. The analysis results are summarized in the table below:

Borehole (sample)	Depth (mbgs)	Sulphate (µg/g)	pH (-)	Resistivity (Ohm-cm)	Conductivity (uS/cm)	Chloride (µg/g)	Sulphide %
18-1 (SS10)	7.6 – 8.2	11	6.39	16,400	61	21	<0.02
18-2 (SS10)	7.6 – 8.2	9	6.44	6,310	158	83	<0.02

6 MISCELLANEOUS

Borehole locations were selected by Thurber relative to the bridge abutments and other site features. The as-drilled locations and ground surface elevation of the boreholes were measured by Thurber following completion of the field program. Base plan drawings and survey benchmarks were provided by MPCE.

George Downing Estate Drilling Ltd. of Hawkesbury, Ontario supplied and operated the drilling equipment to conduct the drilling, soil sampling, in-situ testing and borehole decommissioning. The field investigation was supervised on a full time basis by Mr. Sean O'Bryan, of Thurber. Overall supervision of the field investigation program was provided by Mr. Stephen Peters, P.Eng.

Routine geotechnical laboratory testing was completed by Thurber's laboratory in Ottawa, Ontario. Analytical testing was completed by Paracel Laboratories in Ottawa, Ontario.

Interpretation of the factual data and preparation of this report were carried out by Miss Katya Edney, P.Eng. and Mr. Stephen Peters, P.Eng. The report was reviewed by Dr. Fred Griffiths, P.Eng. and Dr. P.K. Chatterji, P.Eng. a Designated Principal Contact for MTO Foundation Projects.

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PART 2. ENGINEERING DISCUSSION AND RECOMMENDATIONS

7 INTRODUCTION

This section of the report provides an interpretation of the factual data from Part 1 of this report and presents geotechnical recommendations to assist the project team in the design of the proposed underpass rehabilitation works at the Muskoka Road 3 crossing of Highway 11, located approximately 4 km south of Highway 60 within the District of Muskoka. The discussion and recommendations presented in this report are based on the information provided by McIntosh Perry Consulting Engineers Ltd. (MPCE) and on the factual data obtained during the course of the investigation.

This foundation investigation and design report with the interpretation and recommendations are intended for the use of the Ministry of Transportation and shall not be used or relied upon for any other purposes or by any other parties including the construction or design-build contractor. The construction or design-build contractor must make their own interpretation based on the factual data in Part 1 of the report. Where comments are made on construction, they are provided only in order to highlight those aspects which could affect the design of the project. Contractors must make their own interpretation of the factual information provided as it may affect equipment selection, proposed construction methods and scheduling.

In general terms, the site was found to be underlain by a pavement structure and sand to silty sand fill overlying native silt to silty clay to clayey silt and silty sand. Bedrock was not encountered within the depth of the current investigation. The short-term groundwater level was recorded during drilling operations on April 27th, 2018 at an elevation of approximately 322.0 to 322.1 m.

The underpass (Structure No.42-169) is understood to have been constructed in 1980 and rehabilitated in 2000. The foundations are documented to consist of H-piles driven to refusal.

7.1 Proposed Structure Rehabilitations

At the time of preparation of this Foundation Investigation and Design Report, the proposed rehabilitation of the underpass structure, as per the General Arrangement Drawing dated May 2018, is to include abutment bearing replacements, concrete patching to interior face of barrier wall, concrete sealing to interior and exterior face of barriers, sidewalk, and east

abutment, waterproofing and paving, reconstructing concrete end dams and replacing the expansion joints and repairing deteriorated concrete. It is a possibility that the pier may be retrofitted to withstand new crash guidelines.

It is understood that the modifications will not add any further loading to the existing foundations.

8 GEOTECHNICAL RECOMMENDATIONS

The approximate embankments are approximately 7.3 m in height above the adjacent ground level, with a clearance of approximately 4.6 m above Highway 11. At each end of the bridge structure, temporary protection systems are being considered to allow excavation behind the abutments while maintaining one lane of traffic. In accordance with the RFP, geotechnical recommendations are provided herein for temporary protection systems and reinstatement of the highway embankments.

8.1 Excavation

All temporary excavation must be carried out in accordance with the Occupational Health and Safety Act (OHSA). For the purposes of OHSA, the existing fills above the water table may be classified as Type 3. The existing fills and native cohesionless soils below the water table are classified as Type 4 soil.

Newly placed granular fill constructed in accordance with OPSS 501 and the rehabilitation contract can be considered to be Type 2 soil.

It is anticipated that there will be space restrictions and excavations will need to be carried out within a protection system. Further discussion is presented in Section 8.2.

8.2 Temporary Protection Systems

Temporary Protection Systems will likely be required during construction and must be implemented in accordance with OPSS.PROV 539 and designed for Performance Level 2 (maximum 25 mm horizontal deflection). The actual pressure distribution acting on the shoring system is a function of the construction sequence and the relative flexibility of the wall and these factors must be considered when designing the shoring system. An interlocking sheet pile system or a soldier pile and lagging system are considered to be two feasible options. Bracing may be required depending on the depth of temporary excavation to limit lateral movement of the TPS.

A native deposit of stiff silty clay was encountered just below the embankment. This deposit is sensitive to disturbance and vibrations. Using vibratory methods could induce settlement of the bridge approach embankments. Vibratory equipment should not be permitted at this site for installation or removal of the temporary protections system. Suggested wording for an NSSP is provided in Appendix E. Although not encountered within the boreholes, the Contractor should be aware that cobbles or obstructions could be present in the fill.

Lateral earth pressure coefficients, under fully mobilized conditions, that can be used in design of the protection system installed through embankment fill and culvert backfill are provided in Section 8.3. The lateral earth pressure coefficients for the existing native clay are given below:

Native Clay

$$\begin{aligned}\gamma &= 19 \text{ kN/m}^3 && \text{(use submerged unit weight for soil below groundwater level)} \\ K_A &= 0.35 \\ K_P &= 2.9\end{aligned}$$

Temporary protection systems are the responsibility of the Contractor and should be designed by a licensed Professional Engineer experienced in such designs and retained by the Contractor. The designer must undertake an assessment of the foundation soils ability to support the weight of cranes and/or other construction equipment used during installation of the protection systems and rehabilitation works.

It is recommended that the TPS should be cut off and left in place in accordance with OPSS 539.

8.3 Lateral Earth Pressures

Lateral earth pressures parameters provided in Table 8-1 and in the text below are based on the assumption that the backfill is fully drained so that there are no unbalanced hydrostatic pressures. If adequate drainage cannot be confirmed, the potential for buildup of hydrostatic pressures should be considered in design.

Lateral earth pressures acting on vertical structures should be computed in accordance with the CHBDC but generally are given by the following expression:

$$p_h = K * (\gamma h + q)$$

where:

$$\begin{aligned}p_h &= \text{horizontal pressure on the wall at depth } h \text{ (kPa)} \\ K &= \text{earth pressure coefficient (see table below)} \\ \gamma &= \text{unit weight of retained soil (must adjust for groundwater level)} \\ h &= \text{depth below top of fill where pressure is computed (m)} \\ q &= \text{value of any surcharge (kPa)}\end{aligned}$$

A lateral earth pressure due to backfill compaction should be added to the calculated lateral earth pressure in accordance with Clause 6.12.3 of the CHBDC. Typical earth pressure coefficients for backfill are shown in Table 8-1.

Table 8-1. Static Earth Pressure Coefficients with Horizontal Backfill

Condition	Earth Pressure Coefficient (K)			
	OPSS Granular A or OPSS Granular B Type II $\phi = 35^\circ$, $\gamma = 22.8 \text{ kN/m}^3$	OPSS Granular B Type I $\phi = 32^\circ$, $\gamma = 21.2 \text{ kN/m}^3$	OPSS SSM and Existing Sand Fill $\phi = 30^\circ$, $\gamma = 21.0 \text{ kN/m}^3$	Native Silt $\phi = 28^\circ$, $\gamma = 19.0 \text{ kN/m}^3$
Active, K_A (Movement away from Soil Mass)	0.27	0.31	0.33	0.36
At Rest, K_O (Non-Yielding Wall)	0.43	0.47	0.50	0.53
Passive, K_P (Movement towards Soil Mass)	3.7	3.3	3.0	2.8
Soil Group(*)	"medium dense sand"	"loose to medium dense sand"	"loose sand"	"loose sand"

Note: (*) for use with Figure C6.16 of the Commentary to the CHBDC.

The parameters in the table above correspond to full mobilization of active and passive earth pressures and require certain relative movements between the wall and adjacent soil to produce these conditions. The values to be used in design can be assessed from Figure C6.16 of the Commentary to the CHBDC using the soil group designation as outlined in Table 8-1. Active pressures should be used for unrestrained walls. For rigid structures, it is recommended that at-rest horizontal earth pressures be used for design. Where ground surfaces are sloped behind the walls, the coefficients provided in the Table 8-1 are not applicable and must be re-estimated.

8.4 Embankment Design and Reinstatement

8.4.1 Embankment Reconstruction

It is recommended that where the existing embankment has been removed as part of the rehabilitation work, that it be reinstated in accordance with OPSS 902 and consist of free draining, non- frost susceptible granular materials such as Granular A or Granular B Type II material meeting the requirements of OPSS.PROV 1010 and the fill should be placed and compacted as per OPSS.PROV 501.

Pavement structure reinstatement should follow the recommendations provided in the Pavement Design Report (completed by others).

Heavy compaction equipment, used adjacent to the bridge abutments, must be restricted in accordance with OPSS.PROV 501. Care must be exercised when compacting the fill adjacent to the walls in order not to damage the structures. Embankment reconstruction after bridge rehabilitation should be carried out in accordance with OPSS.PROV 206.

The embankment should be reinstated with side slopes of 2H:1V (or flatter).

8.4.2 Embankment Settlement and Stability

The condition of the existing embankment slopes was examined in the field during the field investigation and no evidence of instability (tension cracks etc.) was noted at that time.

It is understood that the existing embankment geometry will not change following rehabilitation and therefore no permanent grade raise or embankment widening is proposed. Provided proper construction methods are used, no long term or global stability issues are anticipated for embankments reinstated at this site. Material stockpiling above the existing grades is a temporary construction measure and the stability implications are the responsibility of the Contractor. The selection and placement of construction equipment (such as cranes) are also the Contractor's responsibility.

As no permanent grade raise is anticipated negligible settlement is expected to occur in the soils underlying the reinstated approach fills.

The magnitude of the embankment compression constructed with granular materials is in the order of 0.5% of the embankment height and is expected to occur during and following fill placement.

8.5 Cement Type and Corrosion Potential

Analytical tests were completed to determine the potential for degradation of the concrete in the presence of soluble sulphates and the potential for corrosion of exposed steel. The concentration of soluble sulphate provides an indication of the degree of sulphate attack that is expected for concrete in contact with soil and groundwater at the site. Soluble sulphate concentrations less than 1000 µg/g generally indicate that a low degree of sulphate attack is expected for concrete in contact with soil and groundwater. The class of concrete selected should consider the effects of road de-icing salts.

The pH, resistivity and chloride concentration provide an indication of the degree of corrosiveness of the sub-surface environment. The tests results provided in Section 5.7 may be used to aid in the selection of coatings and corrosion protection systems for buried steel objects. The corrosive effects of road de-icing salts should also be considered.

9 CONSTRUCTION CONSIDERATIONS

9.1 Surface and Groundwater Control

Excavation for the rehabilitation is not expected to intersect the groundwater. Embankment reinstatement and structure backfilling required as part of the underpass structure rehabilitation must be carried out in the dry. The Contractor must be prepared to control the surface water flow and seepage from water perched in the approach fills at this site to permit construction in a dry and stable excavation. Temporary surface water control measures will be required to remain operational during construction until the structure rehabilitation is completed and backfilled.

9.2 Scour Protection and Erosion Control

Based on the subsurface conditions encountered in the boreholes, the embankment materials soils are considered to have low susceptibility to erosion as per the Wischmeier Nomograph. The native soils at the toe and below the embankments are considered to have moderate susceptibility to erosion.

Slope protection and drainage measures will be required to ensure the long-term surficial stability of the reinstatement of the embankment slopes. Slope vegetation should be established as soon as possible after completion of the embankment fills in order to control surficial erosion in general accordance with OPSS.PROV 804. The contractor should provide silt fences and erosion control blankets, as required, throughout the duration of the construction to prevent silt/sediment from running off the site as per OPSS 805.

10 CONSTRUCTION CONCERNS

Potential construction concerns include, but are not necessarily limited to:

- The Contractor's selection of construction equipment and methodology must include assessment of the capability of the existing embankment to support the proposed construction equipment and any temporary fill.

The successful performance of the rehabilitated structure will depend largely upon good workmanship and quality control during construction.

11 CLOSURE

Engineering analysis and preparation of this report were carried out by Miss Katya Edney, P.Eng. and Mr. Stephen Peters, P.Eng. The report was reviewed by Dr. Fred Griffiths, P.Eng and Dr. P.K. Chatterji, P.Eng a Designated Principal Contact for MTO Foundation Projects.

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Report Prepared By:

for *7/24/19*

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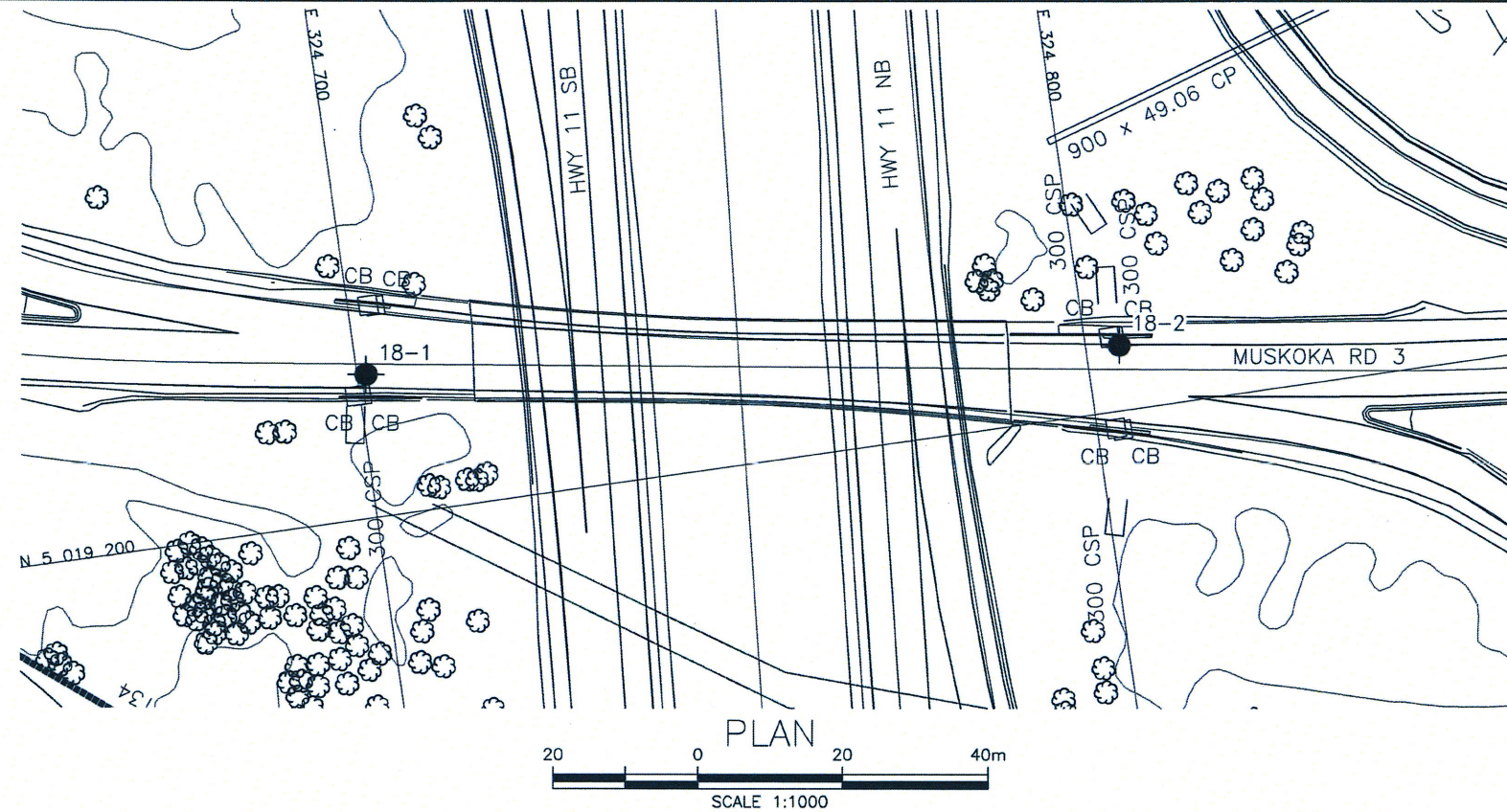
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Appendix A.

Borehole Location Plan and Stratigraphic Drawings

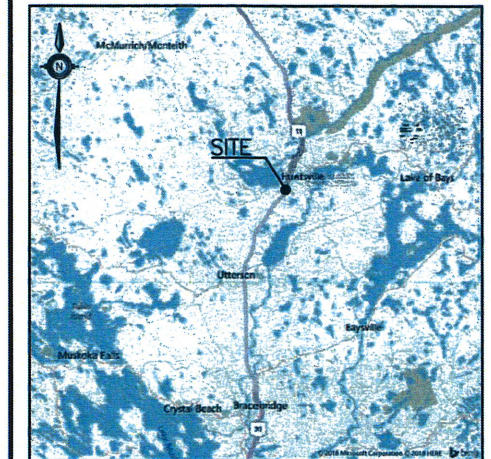
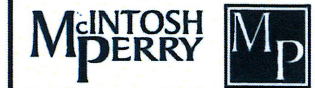


METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES
UNLESS OTHERWISE SHOWN



CONT No
GWP No 5336-11-00

HIGHWAY 11
MUSKOKA ROAD 3
UNDERPASS
BOREHOLE LOCATIONS AND SOIL STRATA



KEYPLAN LEGEND

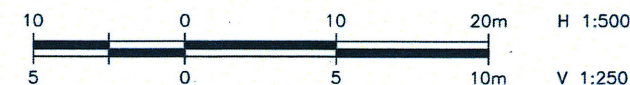
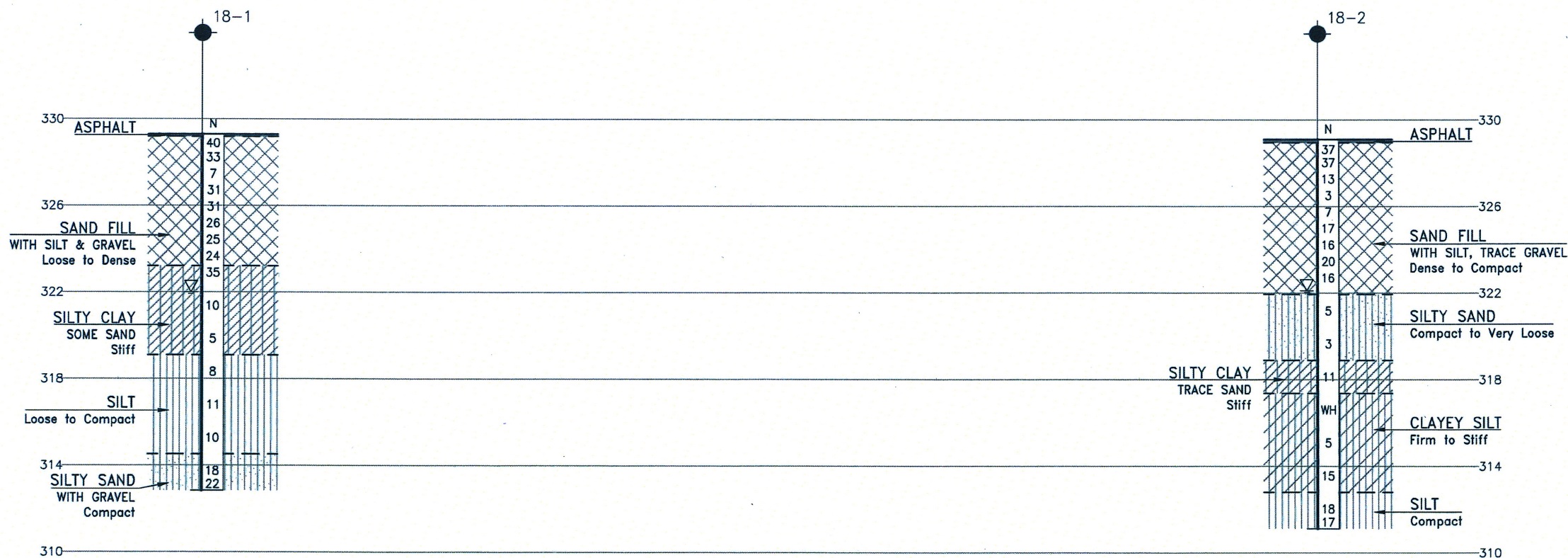
- ◆ Borehole
- ◆ Borehole & Cone
- N Blows /0.3m (Std Pen Test, 475J/blow)
- CONE Blows /0.3m (60° Cone, 475J/blow)
- PH Pressure, Hydraulic
- W Water Level
- ↑ Head Artesian Water
- Piezometer
- 90% Rock Quality Designation (RQD)
- A/R Auger Refusal

NO	ELEVATION	NORTHING	EASTING
18-1	329.3	5 019 219.7	324 701.2
18-2	329.1	5 019 209.0	324 804.6

-NOTES-

- The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.
- This drawing is for subsurface information only. Surface details and features are for conceptual illustration.
- Coordinate system is MTM NAD 83 Zone 10.

GEOCRES No. 31E-393



REVISIONS	DATE	BY	DESCRIPTION
DESIGN KE	CHK SP	CODE	LOAD
DRAWN AN	CHK KE	DATE	JUL 2018

Appendix B.

Record of Borehole Sheets



SYMBOLS, ABBREVIATIONS AND TERMS USED ON TEST HOLE RECORDS

TERMINOLOGY DESCRIBING COMMON SOIL GENESIS

Topsoil	mixture of soil and humus capable of supporting vegetative growth
Peat	mixture of fragments of decayed organic matter
Till	unstratified glacial deposit which may include particles ranging in sizes from clay to boulder
Fill	material below the surface identified as placed by humans (excluding buried services)

TERMINOLOGY DESCRIBING SOIL STRUCTURE:

Desiccated	having visible signs of weathering by oxidization of clay materials, shrinkage cracks, etc.
Fissured	having cracks, and hence a blocky structure
Varved	composed of alternating layers of silt and clay
Stratified	composed of alternating successions of different soil types, e.g. silt and sand
Layer	> 75 mm in thickness
Seam	2 mm to 75 mm in thickness
Parting	< 2 mm in thickness

RECOVERY:

For soil samples, the recovery is recorded as the length of the soil sample recovered.

N-VALUE:

Numbers in this column are the field results of the Standard Penetration Test: the number of blows of a 63.5 kg hammer falling 0.76 m, required to drive a 50 mm O.D. split spoon sampler 0.3 m into undisturbed soil. For samples where insufficient penetration was achieved and N-value cannot be presented, the number of blows are reported over the sampler penetration in millimetres (e.g. 50/75).

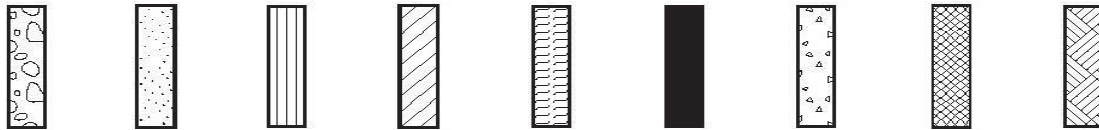
DYNAMIC CONE PENETRATION TEST (DCPT):

Dynamic cone penetration tests are performed using a standard 60 degree apex cone connected to an "A" size drill rods with the same standard fall height and weight as the Standard Penetration Test. The DCPT value is the number of blows of the hammer required to drive the cone 0.3 m into the soil. The DCPT is used as a probe to assess soil variability.



STRATA PLOT:

Strata plots symbolize the soil and bedrock description. They are combinations of the following basic symbols. The dimensions within the strata symbols are not indicative of the particle size, layer thickness, etc.



Boulders
Cobbles
Gravel Sand Silt Clay Organics Asphalt Concrete Fill Bedrock

TEXTURING CLASSIFICATION OF SOILS

Classification	Particle Size
Boulders	Greater than 200 mm
Cobbles	75 – 200 mm
Gravel	4.75 – 75 mm
Sand	0.075 – 4.75 mm
Silt	0.002 – 0.075 mm
Clay	Less than 0.002 mm

TERMS DESCRIBING CONSISTENCY (COHESIVE SOILS ONLY)

Descriptive Term	Undrained Shear Strength (kPa)
Very Soft	12 or less
Soft	12 – 25
Firm	25 – 50
Stiff	50 – 100
Very Stiff	100 – 200
Hard	Greater than 200

NOTE: Clay sensitivity is defined as the ratio of the undisturbed strength over the remolded strength.

SAMPLE TYPES

SS	Split spoon samples
ST	Shelby tube or thin wall tube
DP	Direct push sample
PS	Piston sample
BS	Bulk sample
WS	Wash sample
HQ, NQ, BQ etc.	Rock core sample obtained with the use of standard size diamond coring equipment

TERMS DESCRIBING CONSISTENCY (COHESIONLESS SOILS ONLY)

Descriptive Term	SPT “N” Value
Very Loose	Less than 4
Loose	4 – 10
Compact	10 – 30
Dense	30 – 50
Very Dense	Greater than 50

MODIFIED UNIFIED SOIL CLASSIFICATION

Major Divisions		Group Symbol	Typical Description
COARSE GRAINED SOIL	GRAVEL AND GRAVELLY SOILS	GW	Well-graded gravels or gravel-sand mixtures, little or no fines.
		GP	Poorly-graded gravels or gravel-sand mixtures, little or no fines.
		GM	Silty gravels, gravel-sand-silt mixtures.
		GC	Clayey gravels, gravel-sand-clay mixtures.
	SAND AND SANDY SOILS	SW	Well-graded sands or gravelly sands, little or no fines.
		SP	Poorly-graded sands or gravelly sands, little or no fines.
		SM	Silty sands, sand-silt mixtures.
		SC	Clayey sands, sand-clay mixtures.
FINE GRAINED SOILS	SILT AND CLAY SOILS $W_L < 35\%$	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
		OL	Organic silts and organic silty-clays of low plasticity.
	SILT AND CLAY SOILS $35\% < W_L < 50\%$	MI	Inorganic compressible fine sandy silt with clay of medium plasticity, clayey silts.
		CI	Inorganic clays of medium plasticity, silty clays.
		OI	Organic silty clays of medium plasticity.
	SILT AND CLAY SOILS $W_L > 50\%$	MH	Inorganic silts, micaceous or diatomaceous fine sandy of silty soils, elastic silts.
		CH	Inorganic clays of high plasticity, fat clays.
		OH	Organic clays of high plasticity, organic silts.
HIGHLY ORGANIC SOILS		Pt	Peat and other organic soils.

Note - W_L = Liquid Limit



EXPLANATION OF ROCK LOGGING TERMS

ROCK WEATHERING CLASSIFICATION

Fresh (FR)	No visible signs of weathering.
Fresh Jointed (FJ)	Weathering limited to surface of major discontinuities.
Slightly Weathered (SW)	Penetrative weathering developed on open discontinuity surfaces, but only slight weathering of rock materials.
Moderately Weathered (MW)	Weathering extends throughout the rock mass, but the rock material is not friable.
Highly Weathered (HW)	Weathering extends throughout the rock mass and the rock is partly friable.
Completely Weathered (CW)	Rock is wholly decomposed and in a friable condition, but the rock texture and structures are preserved.

TERMS

Total Core Recovery: (TCR)	Core recovered as a percentage of total core run length.
Solid Core Recovery: (SCR)	Percent ratio of solid core of full cylindrical shape recovered. Expressed with respect to the total length of core run.
Rock Quality Designation: (RQD)	Total length of sound core recovered in pieces 0.1 m in length or larger, as a percentage of total core length
Unconfined Compressive Strength: (UCS)	Axial stress required to break the specimen.
Fracture Index: (FI)	Frequency of natural fractures per 0.3 m of core run.

DISCONTINUITY SPACING

Bedding	Bedding Plane Spacing
Very thickly bedded	Greater than 2 m
Thickly bedded	0.6 to 2 m
Medium bedded	0.2 to 0.6 m
Thinly bedded	60 mm to 0.2 m
Very thinly bedded	20 to 60 mm
Laminated	6 to 20 mm
Thinly laminated	Less than 6 mm

STRENGTH CLASSIFICATION

Rock Strength	Approximate Uniaxial Compressive Strength (MPa)
Extremely Strong	Greater than 250
Very Strong	100 – 250
Strong	50 – 100
Medium Strong	25 – 50
Weak	5 – 25
Very Weak	1 – 5
Extremely Weak	0.25 – 1

RECORD OF BOREHOLE No 18-1

1 OF 2

METRIC

GWP# 5336-11-00 LOCATION Lat: 45.312647°, Long: -79.246187° HWY 11 BOREHOLE TYPE CME55 Truck with HSA ORIGINATED BY SOB
 DATUM Geodetic DATE 2018.04.27 - 2018.04.27 COMPILED BY KE
 CHECKED BY SP

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				GR	SA	SI	CL
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE	20	40	60	80	100	W _p		W	W _L		
329.3																			
0.0																			
0.1	ASPHALT (100 mm)																		
	SAND with silt and gravel loose to dense brown FILL		1	SS	40		329						○						
			2	SS	33		328						○						
			3	SS	7		327						○						
			4	SS	31		326						○						
			5	SS	31		325						○						
			6	SS	26		324						○						
			7	SS	25		323						○						
			8	SS	24		322						○						
323.2																			
6.1	SILTY CLAY (CL-ML) some sand stiff, grey-brown		9	SS	35		321						○						
			10	SS	10		320						○						
			11	SS	5														

Continued Next Page

+³, ×³: Numbers refer to
Sensitivity

20
15
10
(%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-1

2 OF 2

METRIC

GWP# 5336-11-00 LOCATION Lat: 45.312647°, Long: -79.246187°
HWY 11 BOREHOLE TYPE CME55 Truck with HSA ORIGINATED BY SOB
DATUM Geodetic DATE 2018.04.27 - 2018.04.27 COMPILED BY KE
CHECKED BY SP

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa							WATER CONTENT (%)		
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE							w _P w w _L		
Continued From Previous Page							20	40	60	80	100	20	40	60			
319.1																	
10.2	SILT (ML) loose to compact, grey						319										
			12	SS	8		318						○			0 2 71 27 Non-Plastic	
			13	SS	11		317						○				
							316										
			14	SS	10		315						○				
314.5																	
14.8	SILTY SAND (SM) with gravel compact, brown-grey						314						○				
			15	SS	18								○				
			16	SS	22								○			16 48 33 3 Non-Plastic	
312.9							313										
16.5	End of Borehole Water level during drilling operations at 7.3 mbgs in HSA (elev. 322.0 m)																

DOUBLE LINE 20244_MUSKOKARD3.GPJ 2012TEMPLATE(MTO).GDT 19/8/19

RECORD OF BOREHOLE No 18-2

1 OF 2

METRIC

GWP# 5336-11-00 LOCATION Lat: 45.312548°, Long: -79.244869°
HWY 11 BOREHOLE TYPE CME55 Truck with HSA ORIGINATED BY SOB
DATUM Geodetic DATE 2018.04.27 - 2018.04.27 COMPILED BY KE
CHECKED BY SP

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				GR	SA	SI	CL
								20	40	60	80	100	W _p	W					
329.1																			
0.0		ASPHALT (150 mm)					329												
0.2		SAND trace gravel dense to loose, brown FILL																	
			1	SS	37														
			2	SS	37														
			3	SS	13														
			4	SS	3														
		5	SS	7															
325.3																			
3.8		SAND with silt compact, brown-grey FILL																	
			6	SS	17														
			7	SS	16														
			8	SS	20														
			9	SS	16														
321.9																			
7.2		SILTY SAND compact to very loose grey-brown to grey FILL																	
			10	SS	5														

Continued Next Page

+³, ×³: Numbers refer to
Sensitivity





20
15
10
(%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-2

2 OF 2

METRIC

GWP# 5336-11-00 LOCATION Lat: 45.312548°, Long: -79.244869° HWY 11 Underpass at Muskoka Rd 3, MTM Zone 10: N 5 019 209.0 E 324 804.6 ORIGINATED BY SOB
 HWY 11 BOREHOLE TYPE CME55 Truck with HSA COMPILED BY KE
 DATUM Geodetic DATE 2018.04.27 - 2018.04.27 CHECKED BY SP

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa							
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE							
								WATER CONTENT (%)							
Continued From Previous Page															
318.9	SILTY CLAY (CL-ML) trace sand stiff, grey					319									
10.2			12	SS	11	318									
317.4	CLAYEY SILT (MI) firm to stiff, grey					317									
11.7			13	SS	WH	316									
			14	SS	5	315									
			15	SS	15	314									
						313									
312.8	SILT (ML) compact, grey					312									
16.3			16	SS	18										
			17	SS	17										
311.1	End of Borehole Water level during drilling operations at 7.0 mbgs in HSA (elev. 322.1 m)														
18.0															

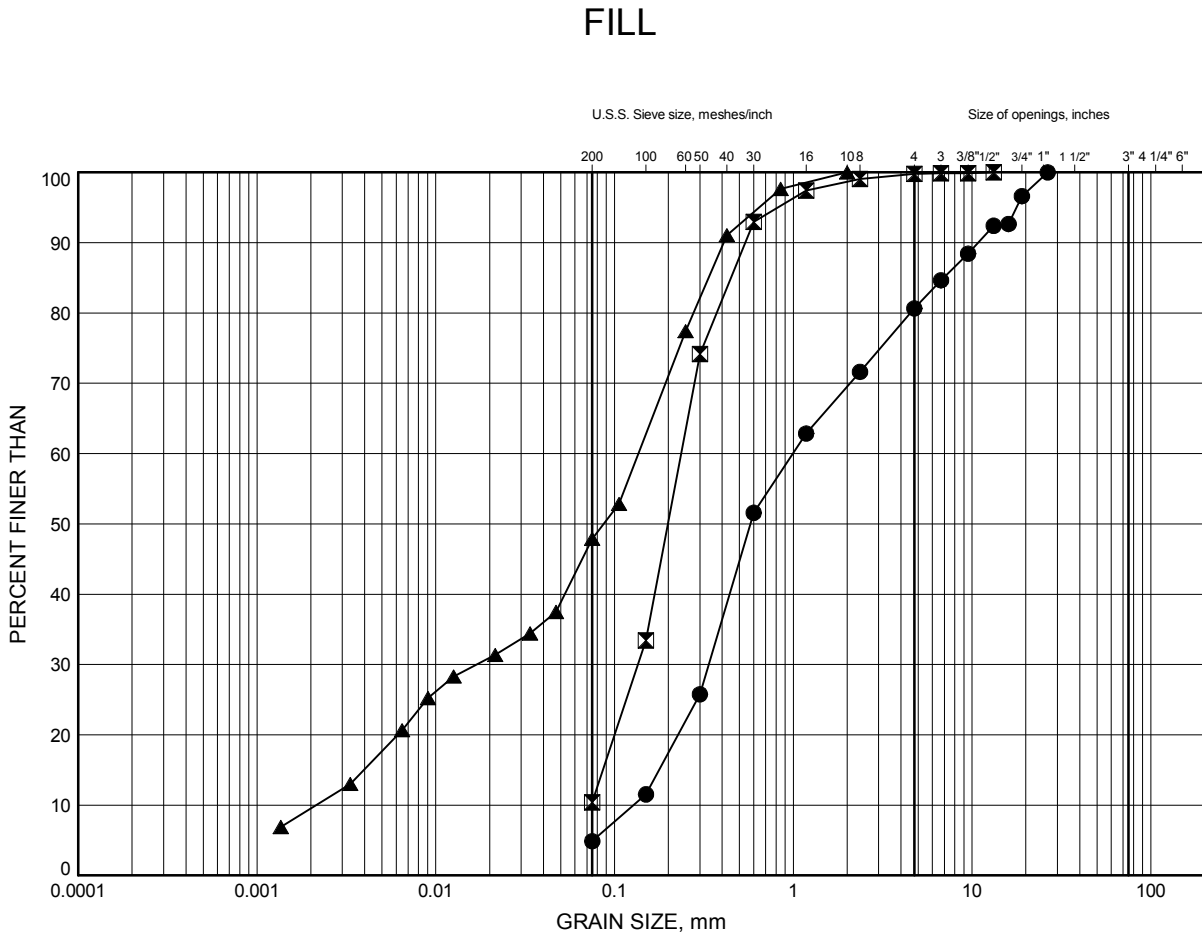
DOUBLE LINE 20244_MUSKOKARD3.GPJ 2012TEMPLATE(MTO).GDT 19/8/19

+³, ×³: Numbers refer to Sensitivity 20 15 10 5 0 (%) STRAIN AT FAILURE

Appendix C.
Laboratory Testing

Hwy's 11 and 118 - Muskoka Road 3
GRAIN SIZE DISTRIBUTION

FIGURE C1



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	4.1	325.2
⊠	18-2	4.9	324.2
▲	18-2	9.4	319.6

Date August 2019
 GWP# 5336-11-00

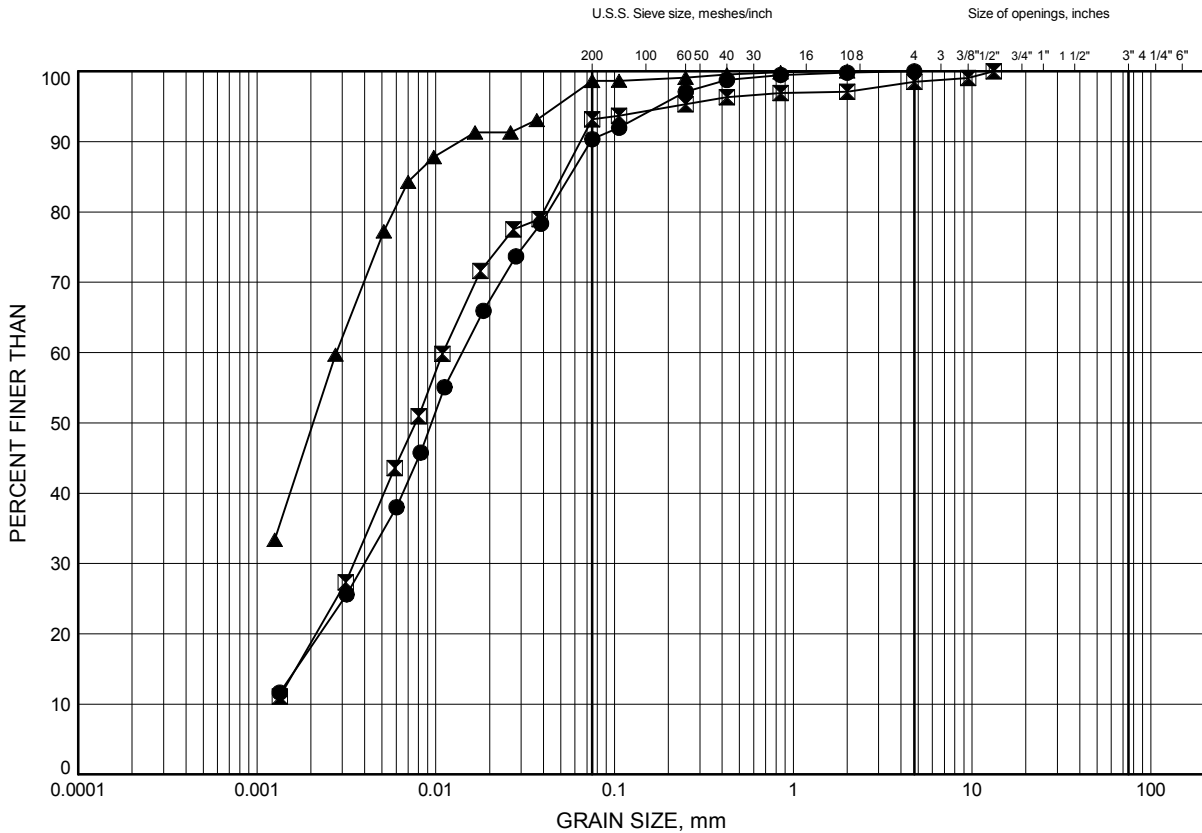


Prep'd KE
 Chkd. SP

Hwy's 11 and 118 - Muskoka Road 3
GRAIN SIZE DISTRIBUTION

FIGURE C2

SILTY CLAY TO CLAYEY SILT



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	6.4	322.9
⊠	18-2	11.0	318.1
▲	18-2	12.5	316.6

Date August 2019

GWP# 5336-11-00

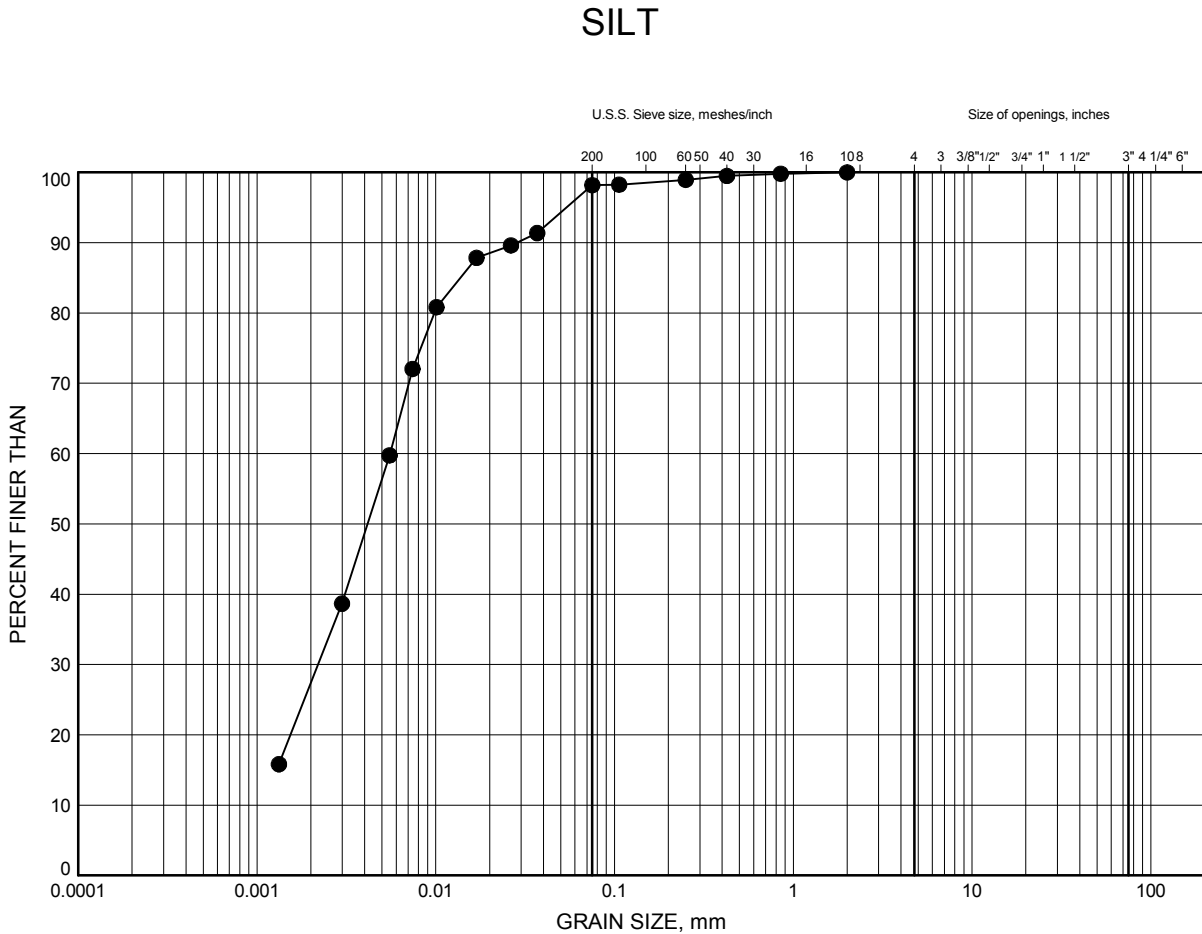


Prep'd KE

Chkd. SP

Hwy's 11 and 118 - Muskoka Road 3
GRAIN SIZE DISTRIBUTION

FIGURE C3



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	11.0	318.4

Date August 2019
 GWP# 5336-11-00

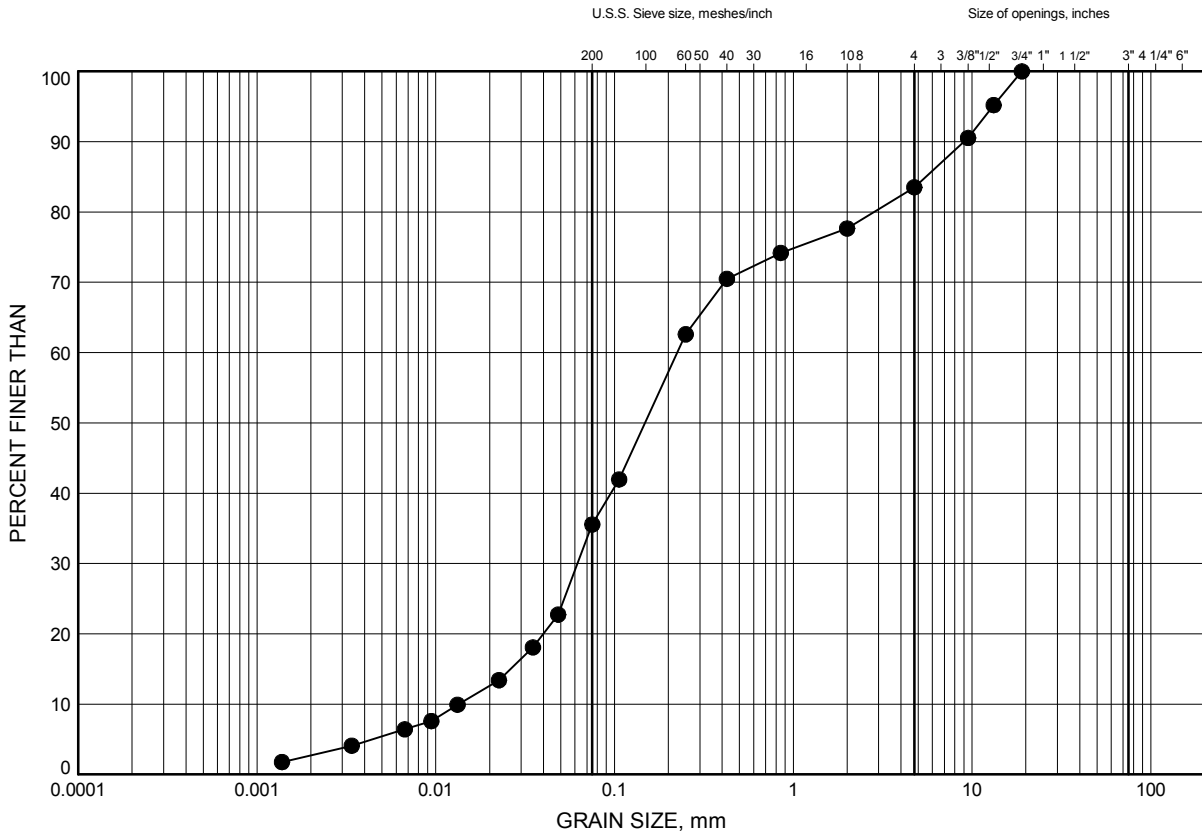


Prep'd KE
 Chkd. SP

Hwy's 11 and 118 - Muskoka Road 3
GRAIN SIZE DISTRIBUTION

FIGURE C4

SILTY SAND with gravel



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	16.2	313.2

Date August 2019
 GWP# 5336-11-00



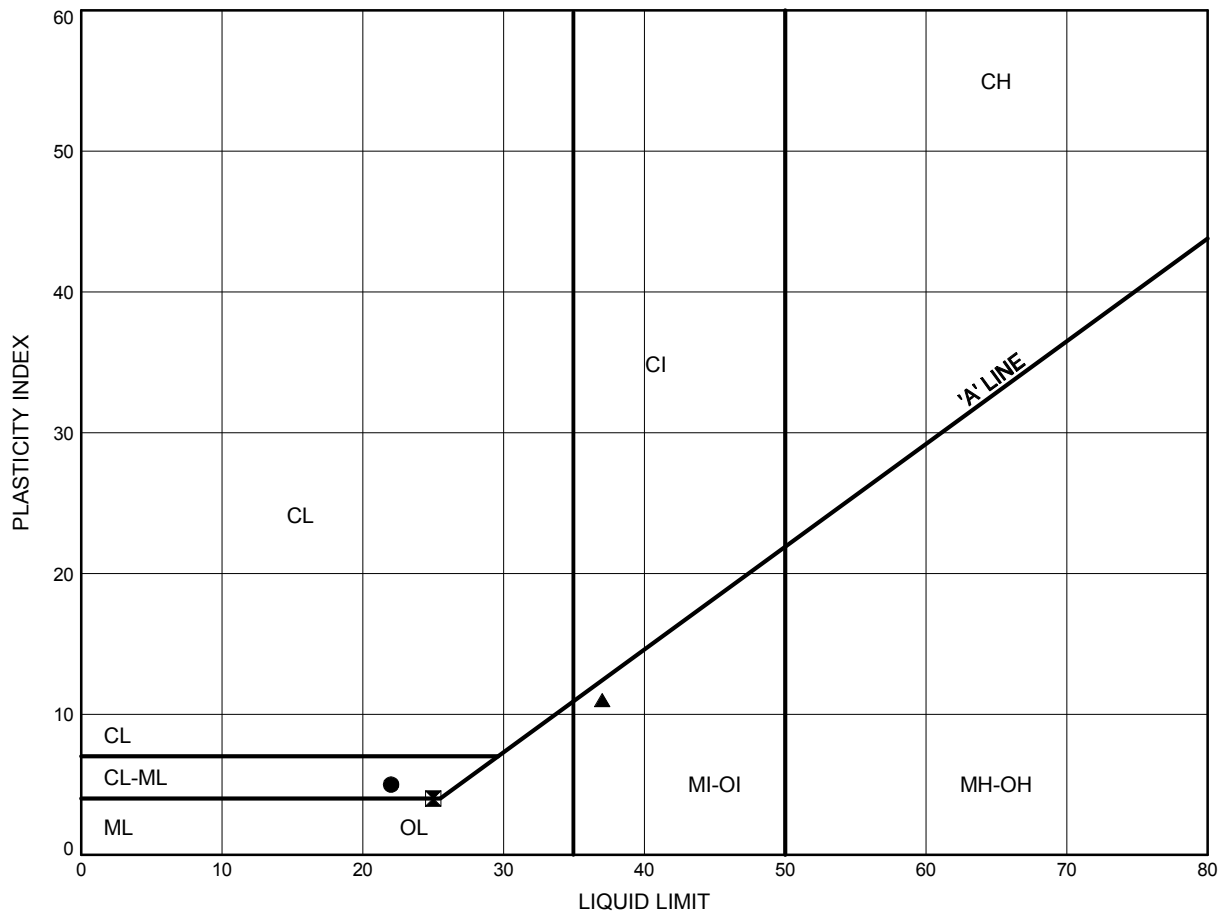
Prep'd KE
 Chkd. SP

Hwy's 11 and 118 - Muskoka Road 3

ATTERBERG LIMITS TEST RESULTS

FIGURE C5

SILTY CLAY to CLAYEY SILT



LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	6.4	322.9
⊠	18-2	11.0	318.1
▲	18-2	12.5	316.6

Date August 2019

GWP# 5336-11-00



Prep'd KE

Chkd. SP

Certificate of Analysis

Thurber Engineering Ltd.

2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya Edney

Client PO:
Project: HWY 11 + 118
Custody: 39845

Report Date: 10-May-2018
Order Date: 4-May-2018

Order #: 1818669

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1818669-01	Road 117 18-1'ss13 40-42'
1818669-02	Road 117 18-2 'SS12 35-37'
1818669-03	Fraserburg '18-1SS10 22'6-24'6"
1818669-04	Fraserburg'18-2 SS12A 35-36'6"
1818669-05	Road 2 '18-1 SS9 20-22'
1818669-06	Road 2 '18-2 SS10 25-27'
1818669-07	Bullens '18-1 SS11 23'3"-25'3"
1818669-08	Road 3 18-1 SS10 25-27'
1818669-09	Road 3 18-2 SS10 25-27'
1818669-10	Siding 18-2SS5 10-12'
1818669-11	Siding 18-3 SS5 10-12'

Approved By:



Dale Robertson, BSc
Laboratory Director

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018

Order Date: 4-May-2018

Project Description: HWY 11 + 118

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC, water extraction	8-May-18	8-May-18
Conductivity	MOE E3138 - probe @25 °C, water ext	8-May-18	9-May-18
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	7-May-18	7-May-18
Resistivity	EPA 120.1 - probe, water extraction	8-May-18	9-May-18
Solids, %	Gravimetric, calculation	7-May-18	7-May-18

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018

Order Date: 4-May-2018

Project Description: HWY 11 + 118

Client ID:	Road 117 18-1'ss13 40-42'	Road 117 18-2 'SS12 35-37'	Fraserburg '18-1SS10 22'6-24'6"	Fraserburg'18-2 SS12A 35-36'6"
Sample Date:	04/30/2018 09:00	04/30/2018 09:00	04/29/2018 09:00	04/29/2018 09:00
Sample ID:	1818669-01	1818669-02	1818669-03	1818669-04
MDL/Units	Soil	Soil	Soil	Soil

Physical Characteristics

% Solids	0.1 % by Wt.	83.9	83.9	68.9	70.1
----------	--------------	------	------	------	------

General Inorganics

Conductivity	5 uS/cm	133	234	469	262
pH	0.05 pH Units	5.84	6.14	5.56	5.32
Resistivity	0.10 Ohm.m	75.0	42.7	21.3	38.1

Anions

Chloride	5 ug/g dry	82	113	246	120
Sulphate	5 ug/g dry	12	9	51	10

Client ID:	Road 2 '18-1 SS9 20-22'	Road 2 '18-2 SS10 25-27'	Bullens '18-1 SS11 23'3"-25'3"	Road 3 18-1 SS10 25-27'
Sample Date:	04/28/2018 09:00	05/01/2018 09:00	04/21/2018 09:00	04/27/2018 09:00
Sample ID:	1818669-05	1818669-06	1818669-07	1818669-08
MDL/Units	Soil	Soil	Soil	Soil

Physical Characteristics

% Solids	0.1 % by Wt.	87.0	72.7	77.5	80.3
----------	--------------	------	------	------	------

General Inorganics

Conductivity	5 uS/cm	218	1780	400	61
pH	0.05 pH Units	6.41	5.76	7.44	6.39
Resistivity	0.10 Ohm.m	45.8	5.61	25.0	164

Anions

Chloride	5 ug/g dry	124	1170	23	21
Sulphate	5 ug/g dry	7	10	200	11

Client ID:	Road 3 18-2 SS10 25-27'	Siding 18-2SS5 10-12'	Siding 18-3 SS5 10-12'	-
Sample Date:	04/27/2018 09:00	04/24/2018 09:00	04/23/2018 09:00	-
Sample ID:	1818669-09	1818669-10	1818669-11	-
MDL/Units	Soil	Soil	Soil	-

Physical Characteristics

% Solids	0.1 % by Wt.	82.5	79.5	72.3	-
----------	--------------	------	------	------	---

General Inorganics

Conductivity	5 uS/cm	158	2120	428	-
pH	0.05 pH Units	6.44	6.34	6.13	-
Resistivity	0.10 Ohm.m	63.1	4.71	23.4	-

Anions

Chloride	5 ug/g dry	83	1590	154	-
Sulphate	5 ug/g dry	9	19	76	-

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018

Order Date: 4-May-2018

Project Description: HWY 11 + 118

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	5	ug/g						
Sulphate	ND	5	ug/g						
General Inorganics									
Conductivity	ND	5	uS/cm						
Resistivity	ND	0.10	Ohm.m						

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018

Order Date: 4-May-2018

Project Description: HWY 11 + 118

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	55.9	5	ug/g dry	55.6			0.7	20	
Sulphate	23.4	5	ug/g dry	22.9			2.1	20	
General Inorganics									
Conductivity	443	5	uS/cm	424			4.4	6.2	
pH	7.77	0.05	pH Units	7.77			0.0	10	
Resistivity	22.6	0.10	Ohm.m	23.6			4.4	20	
Physical Characteristics									
% Solids	98.2	0.1	% by Wt.	98.0			0.2	25	

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018

Order Date: 4-May-2018

Project Description: HWY 11 + 118

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	149	5	ug/g	55.6	93.2	78-113			
Sulphate	119	5	ug/g	22.9	95.8	78-111			

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO:

Report Date: 10-May-2018
Order Date: 4-May-2018
Project Description: HWY 11 + 118

Qualifier Notes:

Login Qualifiers :

Container(s) - Bottle and COC sample ID don't match -

*Applies to samples: Road 117 18-1'ss13 40-42', Road 117 18-2 'SS12 35-37', Fraserburg '18-1SS10 22'6-24'6',
Bullens '18-1 SS11 23'3"-25'3"*

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable

ND: Not Detected

MDL: Method Detection Limit

Source Result: Data used as source for matrix and duplicate samples

%REC: Percent recovery.

RPD: Relative percent difference.

Soil results are reported on a dry weight basis when the units are denoted with 'dry'.

Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

Subcontracted Analysis

Thurber Engineering Ltd.
2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya Edney

Tel: (613) 247-2121
Fax: (613) 247-2185

Paracel Report No **1818669**
Client Project(s): **HWY 11 + 118**
Client PO:
Reference: **Standing Offer**
CoC Number: **39845**

Order Date: 04-May-18
Report Date: 15-May-18

Sample(s) from this project were subcontracted for the listed parameters. A copy of the subcontractor's report is attached

Paracel ID	Client ID	Analysis
1818669-01	Road 117 18-1'ss13 40-42'	Sulphide, solid
1818669-02	Road 117 18-2 'SS12 35-37'	Sulphide, solid
1818669-03	Fraserburg '18-1SS10 22'6-24'6"	Sulphide, solid
1818669-04	Fraserburg'18-2 SS12A 35-36'6"	Sulphide, solid
1818669-05	Road 2 '18-1 SS9 20-22'	Sulphide, solid
1818669-06	Road 2 '18-2 SS10 25-27'	Sulphide, solid
1818669-07	Bullens '18-1 SS11 23'3"-25'3"	Sulphide, solid
1818669-08	Road 3 18-1 SS10 25-27'	Sulphide, solid
1818669-09	Road 3 18-2 SS10 25-27'	Sulphide, solid
1818669-10	Siding 18-2SS5 10-12'	Sulphide, solid
1818669-11	Siding 18-3 SS5 10-12'	Sulphide, solid

**SGS Canada Inc.**

P.O. Box 4300 - 185 Concession St.
Lakefield - Ontario - K0L 2H0
Phone: 705-652-2000 FAX: 705-652-6365

Paracel Laboratories

Attn : Dale Robertson

300-2319 St.Laurent Blvd.
Ottawa, ON
K1G 4K6,

Phone: 613-731-9577
Fax:613-731-9064

15-May-2018

Date Rec. : 08 May 2018
LR Report: CA13203-MAY18
Reference: Project#: 1818669

Copy: #1

CERTIFICATE OF ANALYSIS

Final Report

Sample ID	Sample Date & Time	Sulphide %
1: Analysis Start Date		14-May-18
2: Analysis Start Time		13:09
3: Analysis Completed Date		14-May-18
4: Analysis Completed Time		14:54
5: QC - Blank		< 0.02
6: QC - STD % Recovery		101%
7: QC - DUP % RPD		ND
8: RL		0.02
9: Road 117 18-1'ss13 40-42'	30-Apr-18	< 0.02
10: Road 117 18-2 'SS12 35-37'	30-Apr-18	< 0.02
11: Fraserburg '18-1SS10 22'6-24'6"	29-Apr-18	< 0.02
12: Fraserburg '18-2 SS12A 35-36'6"	29-Apr-18	< 0.02
13: Road 2 '18-1 SS9 20-22'	28-Apr-18	< 0.02
14: Road 2 '18-2 SS10 25-27'	01-May-18	< 0.02
15: Bullens '18-1 SS11 23'3"-25'3"	21-Apr-18	0.40
16: Road 3 18-1 SS10 25-27'	27-Apr-18	< 0.02
17: Road 3 18-2 SS10 25-27'	27-Apr-18	< 0.02
18: Siding 18-2SS5 10-12'	24-Apr-18	< 0.02
19: Siding 18-3 SS5 10-12'	23-Apr-18	< 0.02

RL - SGS Reporting Limit

Carrie Greenlaw
Project Specialist
Environmental Services, Analytical

Appendix D.

Site Photographs



Photo 1. Looking north along Highway 11 from East of Muskoka Road 3 Underpass Structure (May 9th, 2018)



Photo 2. Muskoka Road 3 Underpass Structure from east (May 9th, 2018)



Photo 3. Looking east along Muskoka Road 3 (May 9th, 2018)



Photo 4. Looking west along Muskoka Road 3 (April 27th, 2018)

Appendix E.

List of Special Provisions and OPSS Documents Referenced in this Report

1. The following Special Provisions and OPSS Documents are referenced in this report:

OPSS.PROV 206	Construction Specification for Grading
OPSS.PROV 501	Construction Specification for Compacting
OPSS.PROV 539	Construction Specification for Temporary Protection Systems
OPSS.PROV 804	Construction Specification for Seed and Cover
OPSS 805	Construction Specification for Temporary Erosion and Sediment Control Measures
OPSS 902	Construction Specification for Excavating and Backfilling Structures
OPSS.PROV 1010	Material Specification for Aggregates Base, Subbase, Select Subgrade, and Backfill Material

2. Suggested text for a NSSP on "Installation of Temporary Protection System"

Vibratory equipment is not permitted for installation or removal of temporary protection systems. Temporary protection systems shall be cut off and remain in place in accordance with OPSS.PROV 539.