
**Foundation Investigation and Design Final Report
Culin Creek Culvert Replacement
Highway 522
East Mills Township
G.W.P. 476-98-00
Geocres No. 31E-264**

Prepared for:

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Part 1 Foundation Investigation

1.1 Introduction

This submission presents the results of a geotechnical investigation completed by Trow Associates Inc. (Trow) for the replacement of the Culin Creek culverts (twin 1650 mm by 1000 mm by approximately 23.0 m long Corrugated Steel Pipe Arch (CSPA), located on Highway 522 at Stations 16+743 and 16+745 within East Mills Township. The culvert replacement is to consist of a pre-cast concrete box culvert 2400 mm wide by 1200 mm high and approximately 23.0 m long. Photographs of the site are included in Appendix A.

The purpose of this geotechnical investigation was to determine the existing soil conditions within the proposed construction limits by field investigation and laboratory testing.

The MTO's explanation of terms, abbreviations and symbols are included in Appendix C.

1.2 Site Description and Geological Setting

1.2.1 Site Description

The Culin Creek culverts are located in the East Mills Township at Stations 16+743 and 16+745 on Highway 522.

The site plan and cross section profile of the Culin Creek culverts are as shown on Sheets No. 1 and 2 in Appendix B.

The overall terrain in the area consists of undifferentiated igneous and metamorphic rock, exposed at the surface or covered by a discontinuous layer of drift. The vegetation in the area consists mainly of deciduous trees, some coniferous trees and smaller low lying shrubs and grass. The drainage in the area generally consists of roadside ditches which drain into Culin Creek.

1.2.2 Geological Setting

According to the Ontario Geological Survey (OGS) Maps 2544 and 2556, the site is located in the Mesoproterozoic era within the central gneiss belt, which falls under the mafic rocks, amphibolite, gabbro, diorite and mafic gneisses. The topography in the area consists of undulating bedrock outcrops separated by intervening marshy zones and wooded areas. As such, the surface soils in the area consist of intervening shallow organic deposits (peat), with fluvial deposits consisting of gravel, sand, silt and clay.

1.3 Investigative Procedures

1.3.1 General

The fieldwork for this project was carried out from June 20th to June 21st and from June 24th to June 25th, 2006. The investigation consisted of a total of 4 boreholes (BH-1 to BH-4). Borehole BH-1 was drilled as close to the culvert outlet that could safely be reached by the drill rig equipment and borehole BH-2 was drilled as close to the culvert inlet that could be safely reached by the drill rig equipment, to verify soil conditions below the existing culvert. Borehole BH-3 was drilled along the northwest side of the existing embankment and borehole BH-4 was drilled along the southwest side of the existing embankment, to verify embankment fill materials and soil conditions below the existing culvert.

All boreholes were advanced with a Mobile CME-55 track mounted drill rig equipped with continuous flight hollow stem augers and standard soil sampling equipment. All boreholes were advanced by Landcore Drilling.

From the drilling program, soil samples were obtained using a 51 mm (2 inch) outside diameter split spoon sampler in conjunction with Standard Penetration Tests (ASTM D 1586), at 0.75 m intervals for the upper 3.0 m and at 1.5 m intervals thereafter. The Standard Penetration Test "N" values were recorded and used to provide an assessment of the in-situ relative density of non-cohesive soils. Field vane tests were performed to determine the consistency of cohesive soils. All boreholes were backfilled with auger cuttings and sealed with bentonite pellets.

All fieldwork was supervised by a member of Trow's engineering staff who directed the drilling and sampling operations, logged the factual borehole data, and retrieved soil samples for subsequent laboratory testing and identification. All geodetic borehole elevations were determined in the field by Sutcliffe Rody Quesnel (SRQ). The location of the boreholes and geodetic elevations are shown on Sheet 1, with a cross-section of the boreholes on Sheets 2 and 3 in Appendix B.

1.4 Laboratory

The soil samples obtained in the field were carefully transported to our Sudbury laboratory and examined for further verification and classification. A laboratory testing program for the selected soil consisted of Natural Moisture Content Determination (LS 701), Particle Size Analyses (LS 702), Liquid Limit (LS 703) and Plastic Limit and Plasticity Index (LS 704).

The laboratory test results are summarized on the attached borehole logs in Appendix C, as well as in Appendix D.

1.5 Subsurface Conditions

1.5.1 General

The subsurface conditions encountered during the field investigation are summarized on the borehole logs located in Appendix C. The following is a description of the subsurface conditions encountered during the field investigation.

1.5.2 Stratigraphy, Culvert Outlet

In general, the stratigraphy within borehole BH-1 near the culvert outlet consisted of gravel fill, sand fill, overlying native sand, silty sand, silty clay, and sand till, which overlaid boulders and suspected bedrock.

Reclaimed Asphalt Product (RAP) gravel fill was encountered from ground surface to approximately 0.3 m below grade. The RAP gravel fill was black to brown in colour, dry, loose, poorly graded, fine to coarse grained and contained some fine to coarse grained sand. Underlying the RAP gravel fill was a 2.8 m thick layer of sand fill, which extended to a depth of 3.1 m below grade. The sand fill was brown to grey in colour, damp above 2.29 m depth and wet below, fine to coarse grained and contained trace fine grained gravel and some silt. The recorded uncorrected Standard Penetration Test (SPT) "N" values within the sand fill ranged from 0 to 3 blows per 300 mm, which were generally decreasing with depth, inferring a very loose material in relative density. Underlying the sand fill was a 2.1 m thick layer of sand, which extended to a depth of 5.2 m below grade. The sand was brown to grey in colour, wet, well graded, fine to coarse grained and contained some silt. The recorded uncorrected SPT "N" values within the sand were 0 blows per 300 mm inferring a very loose material in relative density.

Underlying the sand was a 1.1 m thick layer of silty sand, which extended to a depth of 6.3 m below grade. The silty sand was grey in colour, wet, well graded, and fine grained (sand). The recorded uncorrected SPT "N" value within the silty sand was 2 blows per 300 mm inferring a very loose material in relative density. Underlying the silty sand was a 1.3 m thick layer of silty clay, which extended to a depth of 7.6 m below grade. The silty clay was grey in colour, wet, of low plasticity and contained some silt. The field vane value for the silty clay was 25 kPa indicating a soft to firm material in consistency. However, it should be noted that field vanes performed on a clay material with silt (i.e. silty clay) may result in false elevated strength characteristics. Underlying the silty clay was a 0.8 m thick layer of sand, which extended to a depth of 8.4 m below grade. The sand was grey in colour, wet, well graded, fine grained and contained some silt. The recorded uncorrected SPT "N" value within the sand was 0 blows per 300 mm inferring a very loose material in relative density. Underlying the sand was a 0.6 m thick layer of sand till, which extended to a depth of 9.0 m below grade. The sand till was grey in colour, wet, poorly graded, fine to coarse grained and contained fine grained gravel and some silt. The recorded uncorrected SPT "N" value within the sand till was 23 blows per 300 mm inferring a compact material in relative density.

Underlying the sand till was a 1.6 m thick layer of boulders, which was cored from 9.0 to 10.6 m below grade. The boulders overlaid suspected bedrock, where SPT refusal was encountered (i.e. >100 blows per 300 mm). The boulders consisted mainly of granite and gneiss, which were up to 0.10 m in diameter.

1.5.3 Stratigraphy, Culvert Inlet

In general, the stratigraphy within borehole BH-2 near the culvert inlet consisted of sand fill, overlying native sand, silty sand and suspected bedrock.

Sand fill was encountered from ground surface to approximately 3.1 m below grade. The sand fill was brown in colour, damp above 1.52 m below grade and wet below, well graded, fine to coarse grained and contained trace to some fine to coarse grained gravel and trace to with silt. The recorded uncorrected SPT "N" values within the sand fill ranged from 0 to 4 blows per 300 mm, which were decreasing with depth, inferring a very loose material in relative density. Underlying the sand fill was a 1.5 m thick layer of sand, which extended to a depth of 4.6 m below grade. The sand was brown to grey in colour, wet, well graded, fine to coarse grained and contained trace fine grained gravel and some silt. The recorded uncorrected SPT "N" value within the sand was 0 blows per 300 mm inferring a very loose material in relative density. Underlying the sand was a 3.8 m thick layer of silty sand, which extended to a depth of 8.4 m below grade. The silty sand was brown to grey in colour, wet, well graded, fine to coarse grained (sand) and contained some fine to coarse grained gravel. The recorded uncorrected SPT "N" values within the silty sand ranged from 3 to 4 blows per 300 mm inferring a very loose material in relative density.

Bedrock was encountered underlying the silty sand in borehole BH-2 at approximately 8.4 m below existing grade. A 3 m core was taken from the bedrock. The bedrock consisted of granite and gneiss. The bedrock was fresh, intact, light grey to pink in colour, fine to medium grained and very strong. The Rock Quality Designation (RQD) was between 92 and 100%, indicating an excellent quality of rock. The RQD is also included in the attached borehole logs in Appendix C.

Borehole BH-2 was terminated at a depth of 11.4 m below grade.

1.5.4 Stratigraphy, Northwest Embankment

In general, the stratigraphy within borehole BH-3 along the existing northwest embankment consisted of sand fill overlying native sand, silt, silty clay, and sand till overlying suspected bedrock.

Sand fill was encountered from ground surface to approximately 3.1 m below grade. The sand fill was brown in colour, damp above 0.76 m depth and wet below, well graded, fine to coarse grained and contained trace fine grained gravel and some silt. The recorded uncorrected SPT "N" values within the sand fill ranged from 0 to 5 blows per 300 mm, which were decreasing with depth, inferring a loose to very loose material in relative density. Underlying the sand fill was a 3.2 m thick layer of sand, which extended to a depth of 6.3 m below grade. The sand was brown to grey in colour, wet, well graded, fine to coarse grained and contained trace fine to coarse grained gravel and some silt. The recorded uncorrected SPT "N" values within the sand ranged from 1 to 8 blows per 300 mm inferring a very loose to loose material in relative density.

Underlying the sand was a 0.3 m thick layer of silt, which extended to a depth of 6.6 m below grade. The silt was grey in colour, wet, well graded, with fine grained sand and trace clay. The recorded uncorrected SPT "N" value within the silt was 8 blows per 300 mm inferring a loose material in relative density. Underlying the silt was a 0.4 m thick layer of silty clay, which extended to a depth of 7.0 m below grade. The silty clay was grey in colour, wet, soft to firm in consistency (as determined in borehole BH-1), and of low plasticity. Underlying the silty clay was a 0.8 m thick layer of sand till, which extended to a depth of 7.8 m below grade. The sand till overlaid suspected bedrock, where SPT refusal was encountered (i.e. >100 blows per 300 mm). The sand till was grey in colour, wet, poorly graded, fine to coarse grained and contained trace fine to coarse grained gravel and trace silt. The recorded uncorrected SPT "N" value within the sand till was 100 blows per 300 mm inferring a very dense material in relative density.

1.5.5 Stratigraphy, Southwest Embankment

In general, the stratigraphy within borehole BH-4 along the existing southwest embankment consisted of sand fill overlying native sand, silty sand, silt, and sand till overlying suspected bedrock.

Sand fill was encountered from ground surface to approximately 3.1 m below grade. The sand fill was brown in colour, damp, well graded, fine to coarse grained, with fine to coarse grained gravel and contained trace silt. Uncorrected SPT "N" values within the sand fill ranged from 0 to 1 blow per 300 mm indicating a very loose material in relative density. Underlying the sand fill was a 1.6 m thick layer of sand, which extended to a depth of 4.7 m below grade. The sand was brown in colour, damp, well graded, fine to coarse grained and contained some fine to coarse grained gravel and some silt. The uncorrected SPT "N" value within the sand was 0 blows per 300 mm indicating a very loose material in relative density. Underlying the sand was a 0.5 m thick layer of silty sand, which extended to a depth of 5.2 m below grade. The silty sand was grey in colour, damp, well graded, fine grained (sand) and contained trace clay. The uncorrected SPT "N" value within the silty sand was 0 blows per 300 mm indicating a very loose material in relative density.

Underlying the silty sand was a 2.4 m thick layer of silt, which extended to a depth of 7.6 m below grade. The silt was grey in colour, damp above 5.6 m depth and wet below, with fine to coarse grained sand and contained trace clay. The recorded uncorrected SPT “N” value within the silt was 9 blows per 300 mm inferring a loose material in relative density. Underlying the silt was a 0.8 m thick layer of sand till, which extended to a depth of 7.9 m below grade. The sand till overlaid suspected bedrock, where SPT refusal was encountered (i.e. >100 blows per 300 mm). The sand till was grey in colour, wet, poorly graded, fine to coarse grained and contained trace fine to coarse grained gravel and trace silt. The recorded uncorrected SPT “N” value within the sand till was 100 blows per 300 mm inferring a very dense material in relative density.

1.6 Groundwater Conditions

Groundwater was observed in borehole BH-1 to BH-4 between Elevations 217.28 and 222.69 m. This infers a groundwater level near creek level at the time of the investigation. The lower water levels within the boreholes could be due to disturbance in the holes at the time of drilling and that the boreholes had not stabilized prior to backfilling. The higher groundwater level observed within borehole BH-3 at 222.7 m is likely due to trapped water within the existing fill. As such, for design purposes the groundwater level should be assumed to be equal to the creek water elevation, which was 221.17 m at the time of the investigation.

Seasonal variations in the water table should be anticipated, with higher levels occurring during wetter periods of the year (such as spring thaw and late fall) and lower levels during drier periods.

Part 2 Engineering Discussions and Recommendations

2.1 Introduction

The following subsections address the geotechnical design and construction considerations for the proposed Culin Creek culvert (2400 mm wide by 1200 mm high pre-cast concrete box culvert) located on Highway 522 at Stations 16+743 and 16+745 within East Mills Township. The new culvert is to be 23.0 m long

2.2 Culvert Replacement at Culin Creek Highway 522

It is understood by Trow that the existing twin 1650 mm by 1000 mm by 23 m long Corrugated Steel Pipe Arch (CSPA) culverts are to be replaced with a pre-cast concrete box culvert 2400 mm wide by 1200 mm high. The proposed invert will be placed at approximately the same invert as the existing culverts between elevations 220.73 m (southwest end - inlet) and 220.30 m (northeast end - outlet).

For the invert of the concrete box culvert to be founded at approximately the same invert as the existing culvert, the underside of the proposed culvert will be founded near Elevations 220.28 m (inlet) and 219.85 m (outlet), which accounts for the thickness of concrete (approximately 150 mm) and the bedding layer (minimum 300 mm). At these elevations the proposed culvert will be founded on the in-situ native sand material. The native sand material encountered between Elevations 219.83 (BH-2) and 220.40 m (BH-3) or 3.05 m below grade, had recorded uncorrected Standard Penetration Test (SPT) "N" values ranging from 0 to 2 blows per 300 mm, which infers a very loose material in relative density. The groundwater elevations that were observed within the boreholes were generally above the invert of the proposed culvert and therefore likely affected the relative density determinations from the Standard Penetration Tests in the field. As such, the lower SPT "N" values recorded in the vicinity of the proposed founding elevation, may not be relevant.

For the proposed culvert founded on in-situ native soil material or engineered fill at the elevations outlined above, a Factored Bearing Resistance at ULS of 125 kPa and a Factored Bearing Resistance at SLS of 50 kPa is recommended in accordance with the Canadian Highway Bridge Design Code, Section 6.7, Shallow Foundations. The recommended Factored Bearing Resistance at SLS of 50 kPa can only be achieved if the engineered fill material supporting the culvert (such as culvert bedding) is compacted to 100% of the Standard Proctor Maximum Dry Density (SPMDD).

Prior to the placement of the culvert, all fill material must be removed to the native material, which must be cleared of any soft, loose or disturbed soil. Any significantly loose areas are to be sub-excavated and replaced with Granular "A" or Granular "B" Type II (OPSS 1010) material compacted to a minimum of 100% of the Standard Proctor Maximum Dry Density (SPMDD). The groundwater level needs to be controlled below excavation levels to avoid disturbance, and any surface or groundwater seepage should be removed from within the excavation prior to the new culvert placement to allow placement of granular backfill in dry conditions.

Provided the existing highway grades are maintained, the anticipated maximum total settlements for the concrete box culvert are not expected to exceed 25 mm, assuming good construction practice. Any potential settlements within the underlying sand material are expected to occur during construction.

2.2.1 Culvert Bedding

The culvert bedding should consist of Granular "A" (OPSS 1010) with a minimum thickness of 300 mm beneath the culvert and extend a minimum of 300 mm horizontally on either side of the culverts edge and slope down at 1H:1V. The granular material should be compacted to 100% of the SPMDD in maximum 150 mm thick lifts and placed in dry conditions. Prior to placement of any engineered fill material, a non-woven geotextile (such as Terrafix 270R or equivalent) is to be placed between the native soils and the engineered fill to assist in placement and maintain the integrity of the granular materials. If construction proceeds during the winter months, the base of the trench should not be allowed to freeze prior to placing the bedding material. In areas where the base of the trench experiences loose or soft material, the area may have to be sub-excavated and replaced with Granular "A" or Granular "B" Type II material to stabilize the trench base.

Prior to placement of any fill material, the native sand and boulders are to be relatively level and visually inspected by a qualified engineer.

2.2.2 Culvert Backfill

Any organics and other deleterious material should be excavated as outlined in OPSD 803.010, attached in Appendix E. The culvert backfill should consist of stone free Granular "B", Type I or Granular "A" (OPSS 1010) placed in maximum 150 mm lifts kept at the same elevation on both sides of the culvert. The granular backfill should be compacted to 100% of SPMDD.

The culvert should be encased with a minimum of 300 mm of compacted material. Typical backfill diagrams are presented in Appendix E, OPSD 803.010. The minimum height of fill over the top of the culvert for heavy equipment during construction shall be 1000 mm, unless otherwise noted by the structural engineer. In addition the Contractor is to follow SP No. 902S01, regarding backfilling for structures.

2.2.3 Lateral Earth Pressure

Culvert walls and temporary shoring that may be required for excavation should be designed to resist lateral earth pressure. The expression for calculating lateral earth pressure is given by

$$p = K (\gamma h + q)$$

where p = Lateral earth pressure (kPa).

K = Coefficient of earth pressure.

γ = Unit weight of backfill (kN/m³).

h = Depth to point of interest (m).

q = Surcharge load acting adjacent to the wall at the ground surface (kPa).

The above expression does not take into account hydrostatic pressure, which must be included for the groundwater level at existing ground surface.

Table 1 below lists various earth pressure properties for given materials.

Table 1 - Material Types and Earth Pressure Properties

Material	Friction Angle ϕ (unfactored)	Coefficient of Active Earth Pressure (k_a)	Coefficient of Passive Earth Pressure (k_p)	Coefficient of Earth Pressure at Rest (k_0)	Unit Weight γ (kN/m ³)
Granular A	35°	0.27	3.7	0.43	22
Granular B Type I	32°	0.33	3	0.5	21
Granular B Type II	35°	0.27	3.7	0.43	21
Rock Fill	42°	0.2	5	0.33	21

Note: Values given for horizontal earth pressures are for horizontal backfill. For sloping backfill, the design requirements outlined in Sec C6.9.1(c) of the Canadian Highway Bridge Design Code should be used. A unit weight of $\gamma=21$ kN/m³ is based on well graded rockfill.

The mobilization of full active or passive resistance requires a measurable and perhaps significant wall movement or rotation. Therefore, unless the structural element can tolerate these deflections, the at-rest earth pressure should be used in design.

The effects of compaction surcharge should be taken into account in the calculations of active and at rest earth pressures. The lateral pressure due to compaction should be taken as at least 12 kPa at the surface, and its magnitude should be assumed to diminish linearly with depth to zero at the depth where the active (or at rest) pressure is equal to 12 kPa. This pressure distribution should be added to the calculated active (or at rest) pressure. Notwithstanding, lighter compaction equipment and smaller lifts should be used adjacent to walls to prevent overstraining.

2.2.4 Design Parameters

The design of the culvert is based on the following soil parameters as outlined in Table 2.

Table 2 - Material Types and Strength Parameters

Material	Friction Angle ϕ	Cohesion c' (kPa)	Unit Weight γ (kN/m ³)
Granular A	35°	0	22
Granular B Type I	32°	0	21
Granular B Type II	35°	0	21
Gravel Fill	35°	0	22
Sand / Sand Fill	32°	0	21
Silty Sand	30°	0	21
Silt	28°	1	20

2.2.5 Sliding Resistance

A friction angle, θ' , of 30° can be used for sliding resistance along the Granular "A" and a pre-cast concrete box culvert and 32 degrees for a cast in place concrete box culvert if applicable.

2.2.6 Erosion Protection Outlet

Rip-rap protection should be provided where the culvert discharges into the open creek. The rip-rap should extend approximately 5 m beyond the ends of the culvert and line the embankment slope to the spring line of the culvert. The size of the rip-rap is a function of the creeks hydrology. As a rule of thumb the thickness of the rip-rap should be a minimum of twice the median particle size, and 300 mm thick as a minimum. The rip-rap configuration at the creek bed should generally follow the OPSD 810.010, which is included in Appendix E of this report. Rip-rap placed at 1V:1H will be stable.

2.2.7 Erosion Protection Inlet

Rip-rap protection should be provided where the open creek enters the culvert. The rip rap should begin approximately 5 m before the culvert inlet and line the embankment slope to the spring line of the culvert. The size of the rip-rap is a function of the creeks hydrology. As a rule of thumb the thickness of the rip-rap should be a minimum of twice the median particle size, and 300 mm thick as a minimum. The rip-rap configuration at the creek bed should generally follow the OPSD 810.010, which is included in Appendix E of this report. Rip-rap placed at 1V:1H will be stable.

Where rip-rap is not present the embankment side slopes are to be vegetated with sodding, seeding or planting as necessary depending on the flow rate and volume. Should seeding be utilized, a 100 mm thick layer of topsoil should be placed along with a degradable erosion blanket to help minimize erosion until the vegetation begins to grow.

2.2.8 Clay Seal

A clay seal should be placed at the inlet of the proposed culvert, to prevent the migration of material along the face of the culvert, the formation of flow paths, and any potential internal erosion within the highway embankment. The following outlines the installation procedures and minimum material requirement of the clay seal:

- The clay seal should be placed against the constructed embankment, and subsequently protected by the inlet erosion protection extending a minimum of 1.0 m along the side of the culvert and extending out laterally 1.0 m from the culvert.
- The clay should be placed along the top and side of the culvert only. The clay must not be placed below the culvert.
- The clay should have a Liquid Limit greater than 50% and a Plasticity Index greater than 17.5%.
- The clay seal is to be place in maximum 150 mm thick lifts and compacted to 95% SPMDD within 2% of the optimum moisture content.

2.2.9 Stream Bed Rip-Rap

The Stream Bed rip-rap thickness is to be twice the median particle size, and/or 300 mm thick as a minimum as outlined by OPSD 810.010 included in Appendix E of this report.

2.2.10 Frost Protection

A frost penetration depth of up to 1.9 m can occur in open areas in the East Mills Township area without snow cover. The underlying sand and sand fill materials are considered to have a low susceptibility to frost heaving, according to the MTO Guidelines for Soil Frost Susceptibility. To minimize potential movements, the frost protection treatment as outlined in OPSD 803.030 and 803.031 included in Appendix E of this report should be applied.

2.2.11 Excavations

All excavations must be conducted in accordance with the Occupational Health and Safety Act and Regulations for Construction (OHSARC). The sand fill and sand material may be classified as Type 3 soil above the groundwater level and Type 4 soil below the groundwater level in conformance with the OHSARC. Excavations are expected to be below the groundwater levels measured during this investigation. To avoid disturbance of the founding materials and to allow placement of fill in dry conditions, groundwater must be controlled to below the proposed excavation levels.

Temporary excavation side slopes for Type 3 soil should not exceed 1H:1V. Temporary excavation side slopes in Type 4 soils should not exceed 3H:1V. There is a potential for sloughing to occur if the trench remains open for an extended period of time (i.e. 24-48 hours) or during a rainfall event. Therefore, it is recommended that excavations be supported by a trench box if they are to be open for an extended period of time or for rain events.

When excavations cannot be safely sloped to maintain stability during construction, suitably designed temporary shoring should be used. Systems such as steel sheet piles or steel "I" beam piles with timber lagging (soldier piles and lagging) can be employed for temporary excavation. It will be the Contractors responsibility to design a suitable temporary support system for the MTO review prior to installation. In addition the Contractor is to follow SP No. 902S01, regarding excavations for structures and SP No. 105S19, regarding protection systems (e.g. sheet piles or timber lagging).

2.2.12 Dewatering

The soils encountered below the groundwater table and within potential excavation depths consist of sand fill. The estimated hydraulic conductivity, "k" of the sand fill and native sand material is between 10^{-3} and 10^{-5} m/s.

Dewatering requirements will be governed by the water levels in the creek at the time of construction. It is the responsibility of the Contractor to propose a suitable dewatering system based on the time of construction, groundwater levels and creek flow conditions for prior approval of the MTO. The method used should not undermine the existing road.

Erosion and sediment control during culvert construction should be as per the MTC Drainage Manual, Volume 2. Silt fences and other sediment control measures should be included to protect the creek environment from the construction activities. All flow must be appropriately controlled during construction.

2.2.13 Construction Recommendations

In order to minimize the disruption to traffic, it is recommended that the replacement of the culverts through Highway 522, be conducted in two construction stages. Each stage will consist of removing and replacing the culverts on one side of the Highway at a time as to provide a throughway lane at all times.

Although the excavations are expected to remain stable at a slope of 1H:1V above the groundwater table and 3H:1V below the groundwater table, sloughing will occur if the trench remains open for an extended period of time. Where this may occur, it may be necessary to use temporary shoring. Suitably designed temporary shoring systems, such as sheet pile or soldier piles and lagging, can be used. It will be the Contractors responsibility to design a suitable temporary support system for the MTO review prior to installation. In addition, the contractor should follow SP No. 105S19, regarding protection systems.

Provided that the existing highway embankments are restored to as near as possible to the existing grades and embankment slopes, using an equivalent engineered fill material, a slope stability analysis is not required and any potential settlements will be negligible.

3.0 CLOSURE

This report has been prepared by D. Muldowney, B.Eng., and reviewed by T. Crilly M.Sc., P.Eng. and S. Gonsalves, M.Eng., P.Eng. Designated MTO Foundation Contact. The field investigation was conducted by Craig St Amant.

We trust this report is satisfactory for your purposes. Should you have any questions, please do not hesitate to contact this office.

Yours truly,

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Dist: Northland Engineering (1987) Limited (7)

APPENDIX A

Photographs



Photo #1 – Stations 16+744, East Mills Township, Facing Northeast
Photo taken on June 20th, 2006



Photo #2 – Station 16+744, East Mills Township, Facing Southwest
Photo taken on June 20th, 2006

APPENDIX B

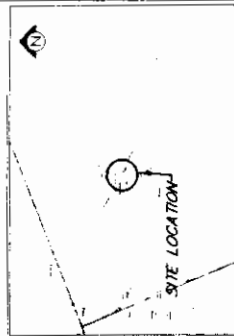
Drawings



Trow
ASSOCIATES INC.

CULIN CREEK
CULVERT REPLACEMENT
BORROW LOCATIONS

13.1-45



KEYPLAN

SECTION 4. INCORPORATION OF THE CHARTER

QUESTIONS

2015

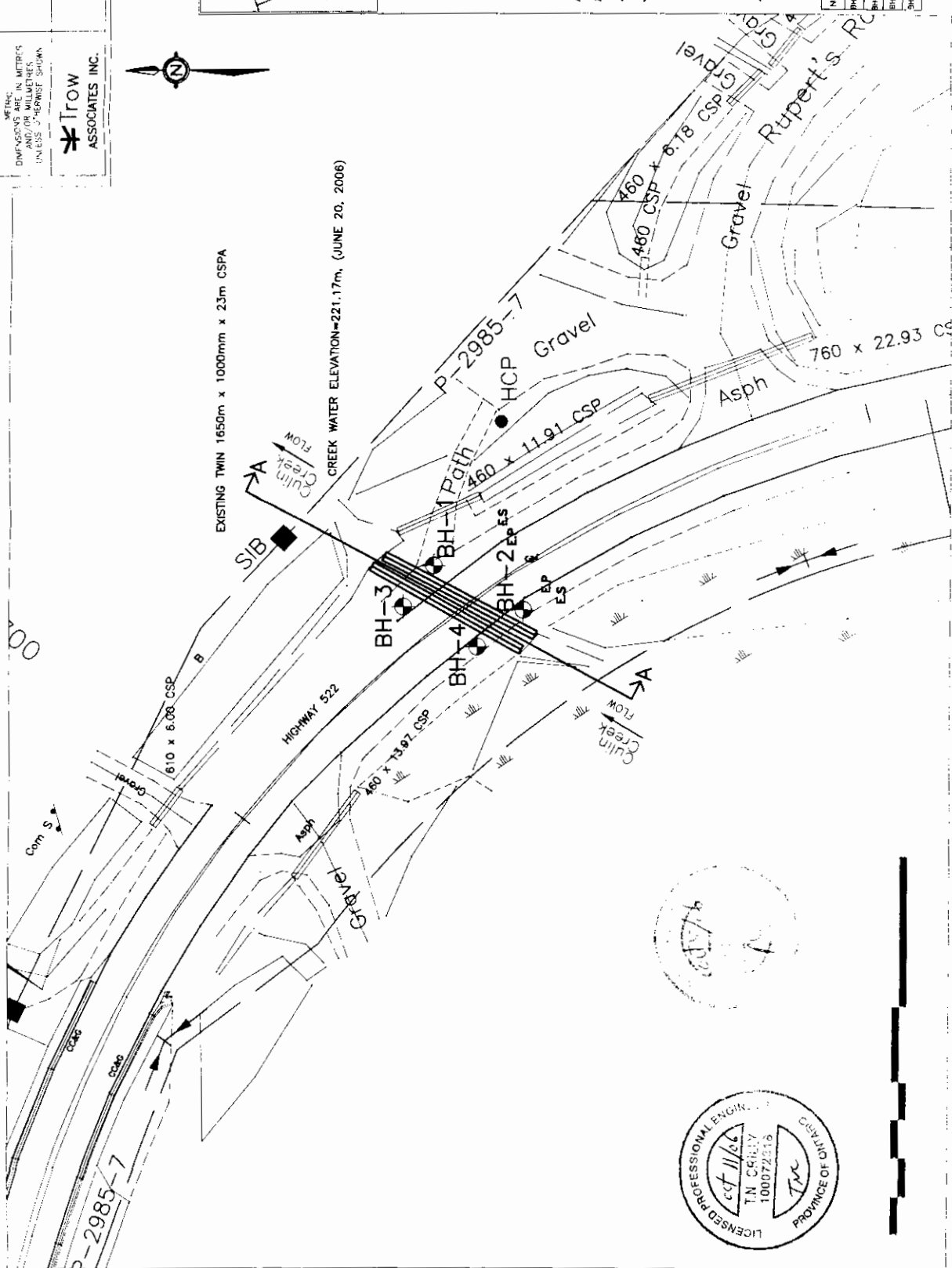
HIGHWAY CENTERLINE

GE OF SHOULDER

NOTES

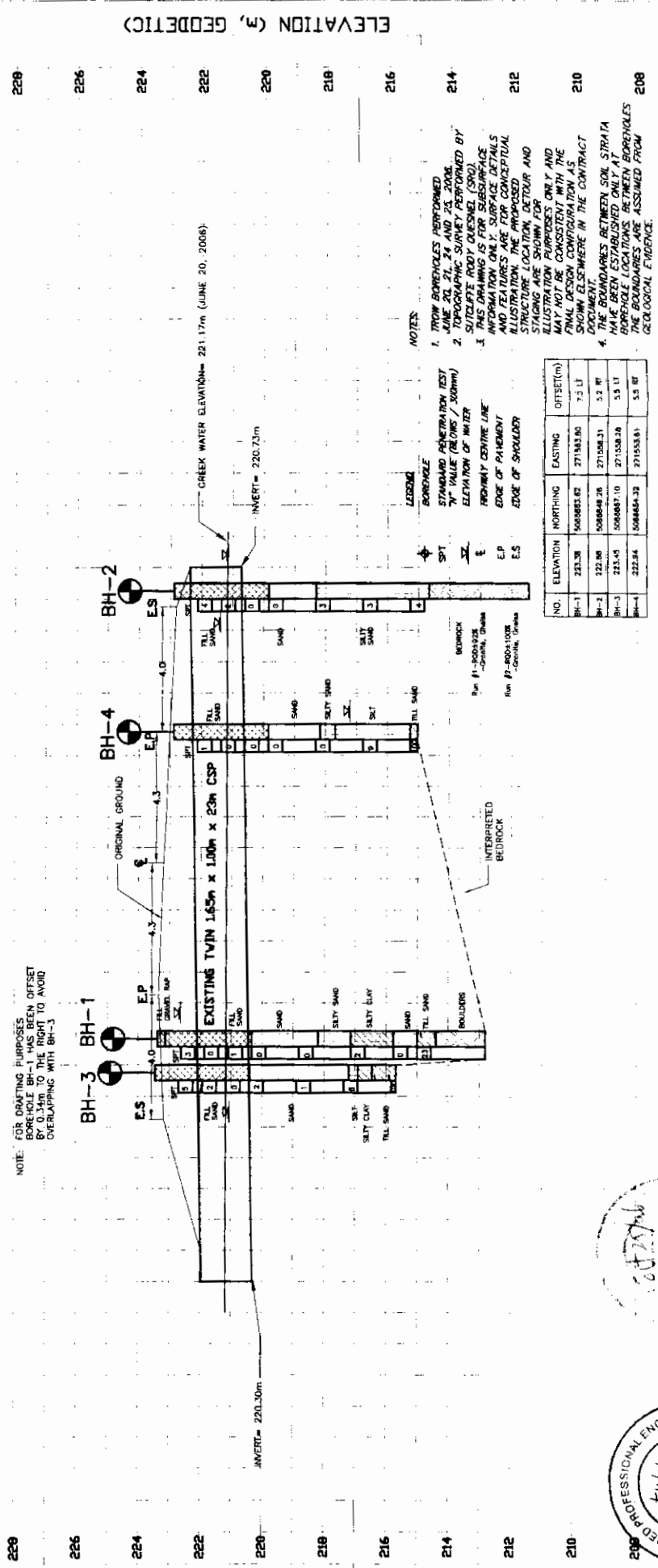
1. TOWN BORERHOLES PERFORMED BY
DATE 20, 21, 24 AND 25, 2006
2. TOPOGRAPHIC SURVEY PERFORMED BY
SUTCLIFFE ROOY QUESENE (SYO).
3. THIS DRAWING IS FOR SURFACE DETAILS
INFORMATION ONLY. SURFACE DETAILS
AND FEATURES ARE FOR CONCEPTUAL
ILLUSTRATION. THE PROPOSED
STATIONING, LENGTH, FOUR AND
EIGHTH CORNERS, SHOWING FOR
ILLUSTRATION PURPOSES ONLY AND
MAY NOT BE CONSISTENT WITH THE
FINAL DESIGN CONFIGURATION AS
SHOWN ELSEWHERE IN THE CONTRACT
DOCUMENT.
4. THE PROXIMITIES BETWEEN SOIL STRATA
AND BOREHOLE LOCATIONS ARE ASSUMED
THE BOREHOLE LOCATIONS BETWEEN BORERHOLES
THE BOUNDARIES ARE ASSUMED FROM
GEOLOGICAL EVIDENCE.

NO.	ELEVATION	NORTHING	EASTING	OFFSET(M)
BH-1	223.35	5086853.92	271543.60	7.0 LT
BH-2	222.86	5086848.26	271536.31	5.2 RT
BH-3	223.45	5086857.10	271556.28	5.5 LT
BH-4	222.34	5086854.32	271553.61	5.5 RT



NORTHEAST OUTLET

SOUTHWEST INLET



SECTION A - A
CULIN CREEK CULVERT PROFILE

07/27/06



APPENDIX C

Borehole Logs

N VALUE: THE STANDARD PENETRATION TEST (SPT) N VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 50MM O.D. SPLIT BARREL SAMPLER TO PENETRATE 0.3M INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5KG, FALLING FREELY A DISTANCE OF 0.76M. FOR PENETRATIONS OF LESS THAN 0.3M N VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N VALUE IS DENOTED THUS \bar{N} .

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (50MM O.D. 60° CONE ANGLE) DRIVEN BY 475 J IMPACT ENERGY ON 1" SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3M ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

c_u (kPa)	0 - 12	12 - 25	25 - 50	50 - 100	100 - 200	> 200
	VERY SOFT	SOFT	FIRM	STIFF	VERY STIFF	HARD

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS/0.3m)	0 - 5	5 - 10	10 - 30	30 - 50	> 50
	VERY LOOSE	LOOSE	COMPACT	DENSE	VERY DENSE

ROCKS ARE DESCRIBED BY THEIR COMPOSITION AND STRUCTURAL FEATURES AND / OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (RQD), FOR MODIFIED RECOVERY, IS:

RQD (%)	0 - 25	25 - 50	50 - 75	75 - 90	90 - 100
	VERY POOR	POOR	FAIR	GOOD	EXCELLENT

JOINTING AND BEDDING:

SPACING	50mm	50 - 300mm	0.3m - 1m	1m - 3m	> 3m
JOINTING	VERY CLOSE	CLOSE	MOD. CLOSE	WIDE	VERY WIDE
BEDDING	VERY THIN	THIN	MEDIUM	THICK	VERY THICK

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

SS SPLIT SPOON	TP THINWALL PISTON
WS WASH SAMPLE	OS OSTERBERG SAMPLE
ST SLOTTED TUBE SAMPLE	RC ROCK CORE
BS BLOCK SAMPLE	PH T/W ADVANCED HYDRAULICALLY
CS CHURCH SAMPLE	PM T/W ADVANCED MANUALLY
TW THINWALL OPEN	FS FOIL SAMPLE

STRESS AND STRAIN

u_w	kPa	PORE WATER PRESSURE
$\frac{u_w}{\sigma}$		PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ϵ	%	LINEAR STRAIN
$\epsilon_1, \epsilon_2, \epsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF LINEAR DEFORMATION
G	kPa	MODULUS OF SHEAR DEFORMATION
μ		COEFFICIENT OF FRICTION

MECHANICAL PROPERTIES OF SOIL

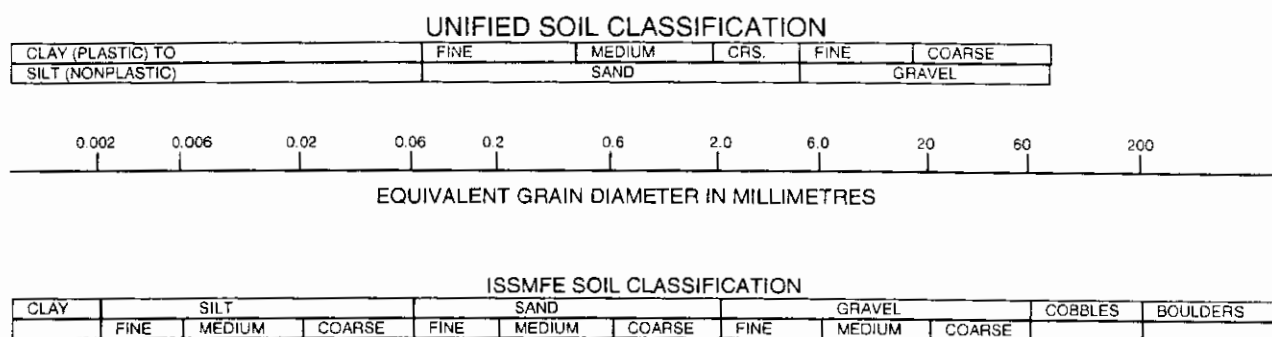
m_v	kPa ⁻¹	COEFFICIENT OF VOLUME CHANGE
C_c		COMPRESSION INDEX
C_s		SWELLING INDEX
C_α		RATE OF SECONDARY CONSOLIDATION
C_v	m ² /s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v		TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
ϕ'	°	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
ϕ_u	°	APPARENT ANGLE OF INTERNAL FRICTION
τ_R	kPa	RESIDUAL SHEAR STRENGTH
τ'_f	kPa	REMOULDED SHEAR STRENGTH
S_r		SENSITIVITY = $\frac{c_u}{\tau'_f}$

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m ³	DENSITY OF SOLID PARTICLES	e	1, %	VOID RATIO	e_{min}	1, %	VOID RATIO IN DENSEST STATE
γ_s	kN/m ³	UNIT WEIGHT OF SOLID PARTICLES	n	1, %	POROSITY	I_D		DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m ³	DENSITY OF WATER	w	1, %	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kN/m ³	UNIT WEIGHT OF WATER	S_r	%	DEGREE OF SATURATION	D_n	mm	n PERCENT - DIAMETER
ρ	kg/m ³	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u		UNIFORMITY COEFFICIENT
γ	kN/m ³	UNIT WEIGHT OF SOIL	w_p	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m ³	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m ³ /s	RATE OF DISCHARGE
γ_d	kN/m ³	UNIT WEIGHT OF DRY SOIL	I_p	%	PLASTICITY INDEX = $w_L - w_p$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m ³	DENSITY OF SATURATED SOIL	I_L		LIQUIDITY INDEX = $\frac{w - w_p}{I_p}$	i		HYDRAULIC GRADIENT
γ_{sat}	kN/m ³	UNIT WEIGHT OF SATURATED SOIL	I_C		CONSISTENCY INDEX = $\frac{w_p - w}{I_p}$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m ³	DENSITY OF SUBMERGED SOIL	e_{max}	1, %	VOID RATIO IN LOOSEST STATE	j	kN/m ²	SEEPAGE FORCE
γ'	kN/m ³	UNIT WEIGHT OF SUBMERGED SOIL						

Notes On Sample Descriptions

1. All sample descriptions included in this report follow the Unified Soil Classification System (USCS) as outlined by the Ministry of Transportation. Different classification systems may be used by others; one such system is the International Society for Soil Mechanics and Foundation Engineering (ISSMFE), as outlined in the Canadian Foundations Engineering Manual. Please note that, with the exception of those samples where a grain size analysis has been made, all samples are classified visually. Visual classification is not sufficiently accurate to provide exact grain sizing or precise differentiation between size classification systems.



2. **Fill:** Where fill is designated on the borehole log it is defined as indicated by the sample recovered during the boring process. The reader is cautioned that fills are heterogeneous in nature and variable in density or degree of compaction. The borehole description may therefore not be applicable as a general description of site fill materials. All fills should be expected to contain obstruction such as wood, large concrete pieces or subsurface basements, floors, tanks, etc., none of these may have been encountered in the boreholes. Since boreholes cannot accurately define the contents of the fill, test pits are recommended to provide supplementary information. Despite the use of test pits, the heterogeneous nature of fill will leave some ambiguity as to the exact composition of the fill. Most fills contain pockets, seams, or layers of organically contaminated soil. This organic material can result in the generation of methane gas and/or significant ongoing and future settlements. Fill at this site may have been monitored for the presence of methane gas and, if so, the results are given on the borehole logs. The monitoring process does not indicate the volume of gas that can be potentially generated nor does it pinpoint the source of the gas. These readings are to advise of the presence of gas only, and a detailed study is recommended for sites where any explosive gas/methane is detected. Some fill material may be contaminated by toxic/hazardous waste that renders it unacceptable for deposition in any but designated land fill sites; unless specifically stated the fill on this site has not been tested for contaminants that may be considered toxic or hazardous. This testing and a potential hazard study can be undertaken if requested. In most residential/commercial areas undergoing reconstruction, buried oil tanks are common and are generally not detected in a conventional geotechnical site investigation.
3. **Till:** The term till on the borehole logs indicates that the material originates from a geological process associated with glaciation. Because of this geological process the till must be considered heterogeneous in composition and as such may contain pockets and/or seams of material such as sand, gravel, silt or clay. Till often contains cobbles (75 to 200 mm) or boulders (over 200 mm). Contractors may therefore encounter cobbles and boulders during excavation, even if they are not indicated by the borings. It should be appreciated that normal sampling equipment cannot differentiate the size or type of any obstruction. Because of the horizontal and vertical variability of till, the sample description may be applicable to a very limited zone; caution is therefore essential when dealing with sensitive excavations or dewatering programs in till materials.

Notes On Soil Descriptions

4. The following table gives a description of the soil based on particle sizes. With the exception of those samples where grain size analyses have been performed, all samples are classified visually. The accuracy of visual examination is not sufficient to differentiate between this classification system or exact grain size.

Soil Classification		Terminology	Proportion
Clay and Silt	<0.075 mm		
Sand	0.075 to 4.75 mm	"trace" (e.g. Trace sand)	0% to 10%
Gravel	4.75 to 75 mm	"some" (e.g. Some sand)	10% to 20%
Cobbles	75 to 200 mm	with (e.g. with sand)	20% to 35%
Boulders	>200 mm	and (e.g. and sand)	35% to 50%

For a given material listed as an adjective (e.g. silty sand) means the predominant grain size is sand sized with 30 to 40% silt sized particles.

The compactness of Cohesionless soils and the consistency of the cohesive soils are defined by the following:

Cohesionless Soil		Cohesive Soil	
Compactness	Standard Penetration Resistance "N" Blows/ 0.3 m	Consistency	Undrained Shear Strength (kPa)
Very Loose	0 to 5	Very soft	<12
Loose	5 to 10	Soft	12 to 25
Compact	10 to 30	Firm	25 to 50
Dense	30 to 50	Stiff	50 to 100
Very Dense	Over 50	Very Stiff	100 to 200
		Hard	>200

5. ROCK CORING

Where rock drilling was carried out, the term RQD (Rock Quality Designation) is used. The RQD is an indirect measure of the number of fractures and soundness of the rock mass. It is obtained from the rock cores by summing the length of the core covered, counting only those pieces of sound core that are 100 mm or more length. The RQD value is expressed as a percentage and is the ratio of the summed core lengths to the total length of core run. The classification based on the RQD value is given below.

RQD Classification	RQD (%)
Very Poor Quality	<25
Poor Quality	25 to 50
Fair Quality	50 to 75
Good Quality	75 to 90
Excellent Quality	90 to 100

$$\text{Recovery Designation \% Recovery} = \frac{\text{Length of Core Per Run}}{\text{Total Length of Run}} \times 100$$



RECORD OF BOREHOLE No BH-1

SHEET 1 OF 1

METRIC

PROJECT NO. SO10242G LOCATION Cullin Creek Culvert - Hwy 522 Sta 16+744, 7.0m LT of Centerline ORIGINATED BY CS
DIST Parry Sound HWY 522 BOREHOLE TYPE Hollow Stem Auger COMPILED BY TA
DATUM Geodetic DATE 6/20/2006 CHECKED BY TC

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	SPT TEST (N-Value) ● DYNAMIC CONE PENETRATION 20 40 60 80 100	PLASTIC LIMIT PL	NATURAL WATER CONTENT W	LIQUID LIMIT LL	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES							
223.4 0.3	FILL GRAVEL RAP, black to brown, dry, loose, poorly graded, fine to coarse grained, some fine to coarse grained sand. FILL SAND, brown to grey, damp, very loose, well graded, fine to coarse grained, trace fine grained gravel, some silt. wet below ~ 2.29 m depth.		1	BAG		223						
			2	SS	3	222						
			3	SS	0	221						
			4	SS	1	220						
220.3 3.1	SAND, brown to grey, wet, very loose, well graded, fine to coarse grained, some silt.		5	SS	0	219						0 83 17 0
			6	SS	0	218						
218.2 5.2	SILTY SAND, grey, wet, very loose, well graded, sand fine grained.					217						
217.1 6.3	SILTY CLAY, grey, wet, soft to firm, low plasticity.		7	SS	2	216						
215.8 7.8	SAND, grey, wet, very loose, well graded, fine grained, some silt.		8	SS	0	215						
215.0 8.4	TILL SAND, grey, wet, compact, poorly graded, fine to coarse grained, trace fine grained gravel, some silt.		9	SS	23	214						
214.4 9.0	BOULDERS, Granite, Gneiss, up to 0.10 m in diameter.					213						
212.8 10.6	BOREHOLE TERMINATED AT ~ 10.57 m DEPTH DUE TO SPT REFUSAL ON SUSPECTED BOULDERS OR BEDROCK											

ON MOT 10242 - CULLIN CREEK GPJ ON MOT GDT 06/10/10



Trow Associates Inc.
1595 Clark Boulevard Ltd.
Brampton, Ontario L6T 4V1

RECORD OF BOREHOLE No BH-2

SHEET 1 OF 1

METRIC

PROJECT NO. SO10242G LOCATION Culin Creek Culvert - Hwy 522 Sta 16+744, 5.2m RT of Centerline ORIGINATED BY CS
DIST Parry Sound HWY 522 BOREHOLE TYPE Hollow Stem Auger COMPILED BY TA
DATUM Geodetic DATE 6/21/2006 CHECKED BY TC

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	SPT TEST (N-Value) • DYNAMIC CONE PENETRATION		PLASTIC LIMIT PL	NATURAL WATER CONTENT W	LIQUID LIMIT LL	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20 40 60 80 100					
222.9 0.0	FILL SAND, brown, damp, very loose, well graded, fine to coarse grained, some fine to coarse grained gravel, trace silt. wet below ~ 1.52 m depth. trace fine grained gravel, with silt below ~ 2.29 m depth.		1	BAG			222						5 68 27 0
			2	SS	4		221						
			3	SS	2		220						
			4	SS	0		219						
219.8 3.1	SAND, brown to grey, wet, very loose, well graded, fine to coarse grained, trace fine grained gravel, some silt.		5	SS	0		218						14 61 25 0
218.3 4.6	SILTY SAND, brown to grey, wet, very loose, well graded, sand fine to coarse grained, some fine to coarse grained gravel.		6	SS	3	217							
			7	SS	3	216							
			8	SS	4	215							
214.5 8.4	BEDROCK, Granite, Gneiss Run #1 At ~ 8.38-9.91m depth Rec=100% RQD=Excellent +/-92% High water return (85%), light grey Run #2 At ~ 9.91-11.43m depth Rec=100% RQD=Excellent +/- 100% High water return (75%), light grey						214						
211.5 11.4	BOREHOLE TERMINATED AT ~ 11.43 m DEPTH						213						

ON MOT 10242 - CULIN CREEK GPJ ON MOT GOT 06/10/10








Trow Associates Inc.
1595 Clark Boulevard Ltd.
Brampton, Ontario L6T 4V1

RECORD OF BOREHOLE No BH-3

SHEET 1 OF 1

METRIC

PROJECT NO. SO10242G LOCATION Culin Creek Culvert - Hwy 522 Sta 16+744, 5.5m LT of Centerline ORIGINATED BY CS
DIST Parry Sound HWY 522 BOREHOLE TYPE Hollow Stem Auger COMPILED BY TA
DATUM Geodetic DATE 6/24/2006 CHECKED BY TC

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	SPT TEST (N-Value) ● DYNAMIC CONE PENETRATION 		PLASTIC LIMIT PL	NATURAL WATER CONTENT w	LIQUID LIMIT LL	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	SHEAR STRENGTH kPa ○ UNCONFINED - FIELD VANE ⊗ QUICK TRIAXIAL x LAB VANE		WATER CONTENT (%)				
223.5 0.0	FILL SAND, brown, damp, loose, well graded, fine to coarse grained, trace fine grained gravel, some silt. wet below ~ 0.76 m depth. very loose below ~ 1.52 m depth.		1	BAG										
			2	SS	5									
			3	SS	2									
			4	SS	0									
220.4 3.1	SAND, brown to grey, wet, very loose, well graded, fine to coarse grained, trace fine grained gravel, some silt. trace coarse grained gravel below ~ 4.57 m depth.		5	SS	2									
			6	SS	1									
217.2			7	SS	8									
216.9 6.6	SILT, grey, wet, loose, well graded, with fine grained sand, trace clay.													
216.5 7.0	SILTY CLAY, grey, wet, soft to firm, low plasticity.													
215.7 7.8	TILL SAND, grey, wet, very dense, poorly graded, fine to coarse grained, trace fine to coarse grained gravel, trace silt. BOREHOLE TERMINATED AT ~ 7.77 m DEPTH DUE TO SPT REFUSAL ON SUSPECTED BOULDERS OR BEDROCK		8	SS	100									

3 x 3. Numbers refer to
Sensitivity

3% STRAIN AT FAILURE



Trow Associates Inc.
1595 Clark Boulevard Ltd.
Brampton, Ontario L6T 4V1

RECORD OF BOREHOLE No BH-4

SHEET 1 OF 1

METRIC

PROJECT NO. SO10242G LOCATION Culin Creek Culvert - Hwy 522 Sta 16+744, 5.5m RT of Centerline ORIGINATED BY CS
DIST Parry Sound HWY 522 BOREHOLE TYPE Hollow Stem Auger COMPILED BY TA
DATUM Geodetic DATE 6/25/2006 CHECKED BY TC

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	SPT TEST (N-Value) DYNAMIC CONE PENETRATION 20 40 60 80 100 SHEAR STRENGTH kPa ○ UNCONFINED - FIELD VANE x QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT PL NATURAL WATER CONTENT w LIQUID LIMIT LL WATER CONTENT (%)	UNIT WEIGHT γ kN/m³	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE						
222.9 0.0	FILL SAND, brown, damp, well graded, fine to coarse grained, with fine to coarse grained gravel, trace silt. trace fine grained gravel below ~ 0.28 m depth.		1	SS		222				
			2	SS		221				
			3	SS		220				
			4	SS		219				
219.9 3.1	SAND, brown, damp, very loose, well graded, fine to coarse grained, some fine to coarse grained gravel, some silt.		5	SS		218				0 59 37 5
218.2 4.7	SILTY SAND, grey, damp, very loose, well graded, sand fine grained, trace clay.		6	SS		217				
217.7 5.2	SILT, grey, damp, loose, well graded, with fine to coarse grained sand, trace clay. wet below ~ 5.60 m depth.		7	SS		216				0 24 71 5
215.3 7.9	TILL SAND, grey, wet, very dense, poorly graded, fine to coarse grained, trace fine to coarse grained gravel, trace silt. BOREHOLE TERMINATED AT ~ 7.85 m DEPTH DUE TO SPT REFUSAL ON SUSPECTED BOULDERS OR BEDROCK		8	SS	100					

ON MOT 10242 - CULIN CREEK GPJ ON MOT GDT 06/10/10

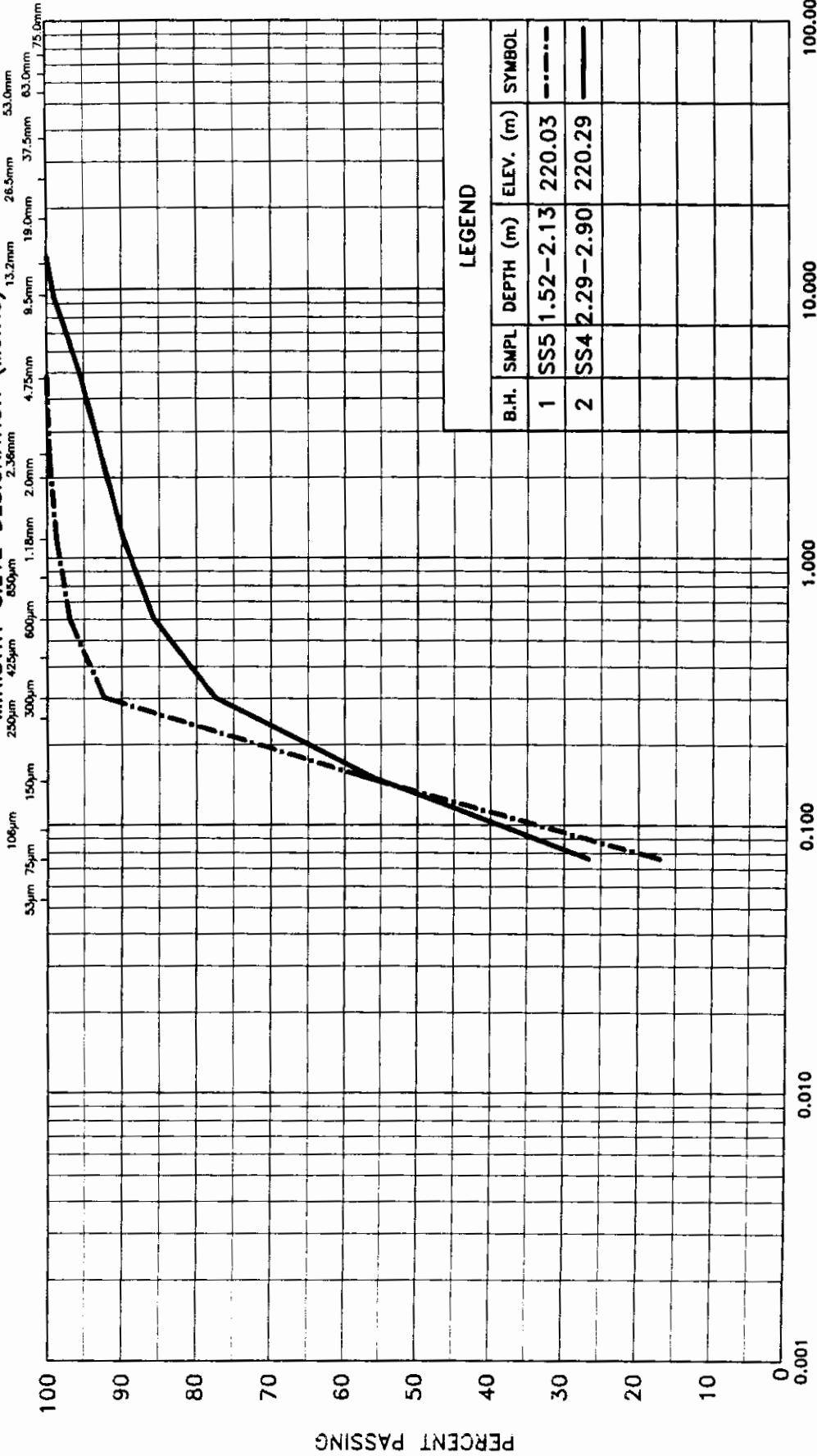
APPENDIX D

Laboratory Data

UNIFIED SOIL CLASSIFICATION

CLAY AND SILT			SAND			GRAVEL		
FINE			MEDIUM			FINE		
COARSE			COARSE			COARSE		

MINISTRY SIEVE DESIGNATION (Metric)



GRAIN SIZE DISTRIBUTION

SAND/SAND FILL

FIGURE No. D1

G.W.P. 478-98-00

CULIN CREEK CULVERT

REF. S010242G/C

Ministry of
Transportation

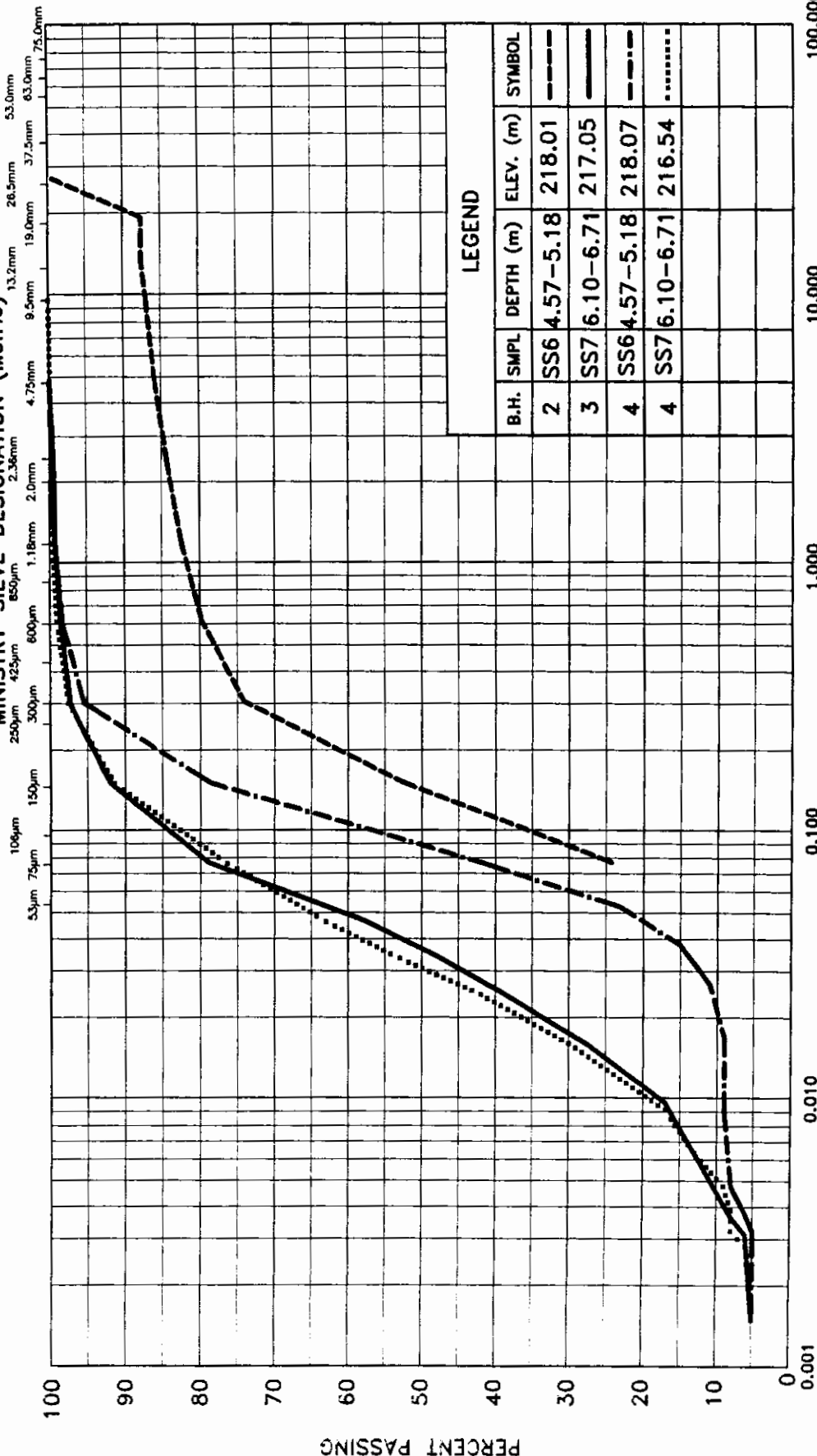


METRIC

UNIFIED SOIL CLASSIFICATION

CLAY AND SILT		SAND			GRAVEL		
		FINE	MEDIUM	COARSE	FINE	COARSE	

MINISTRY SIEVE DESIGNATION (Metric)



GRAIN SIZE DISTRIBUTION

SILT/SILTY SAND

FIGURE No. D2

G.W.P. 478-98-00

CULIN CREEK CULVERT

REF. S010242G/C

Ministry of
Transportation

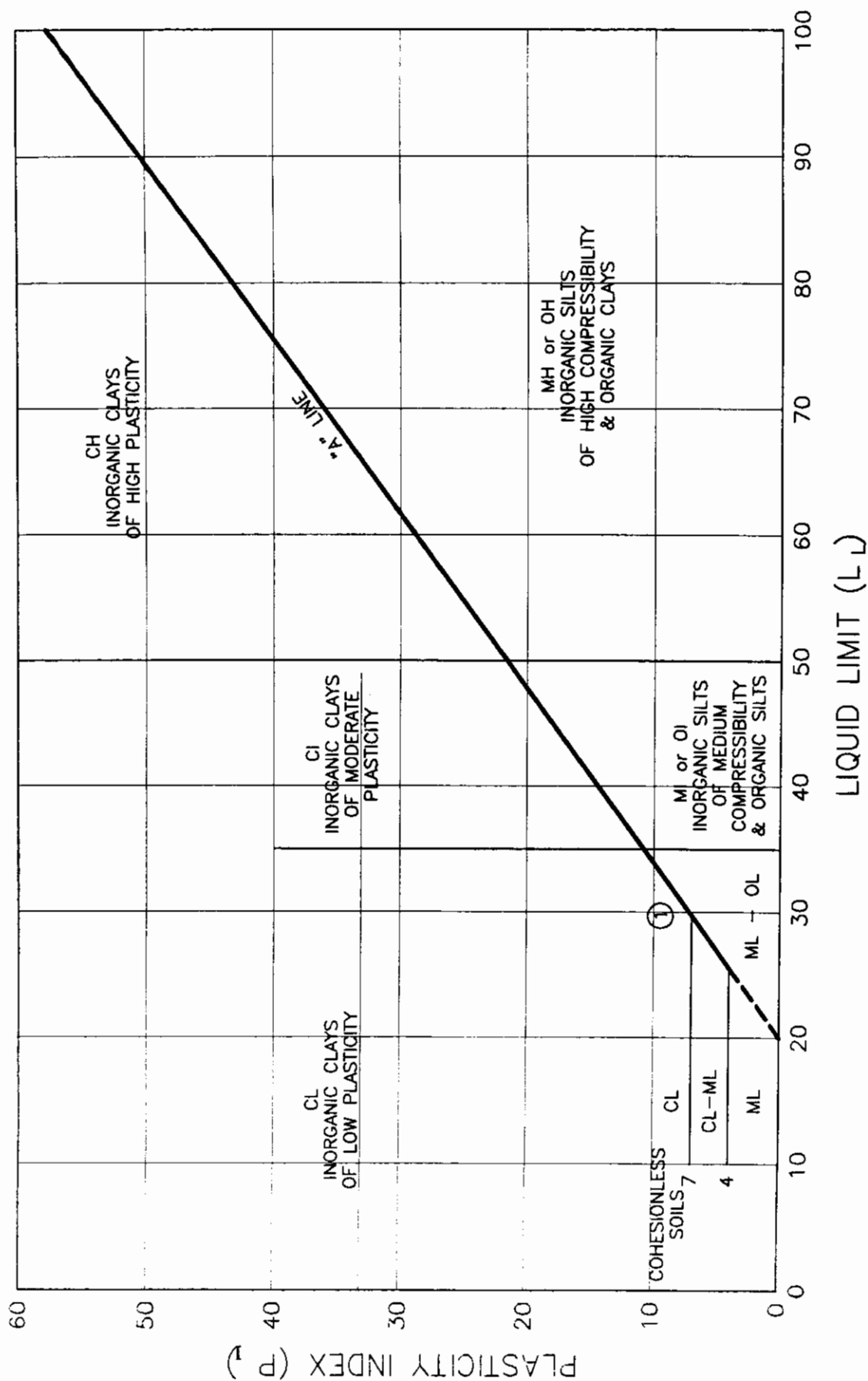


METRIC



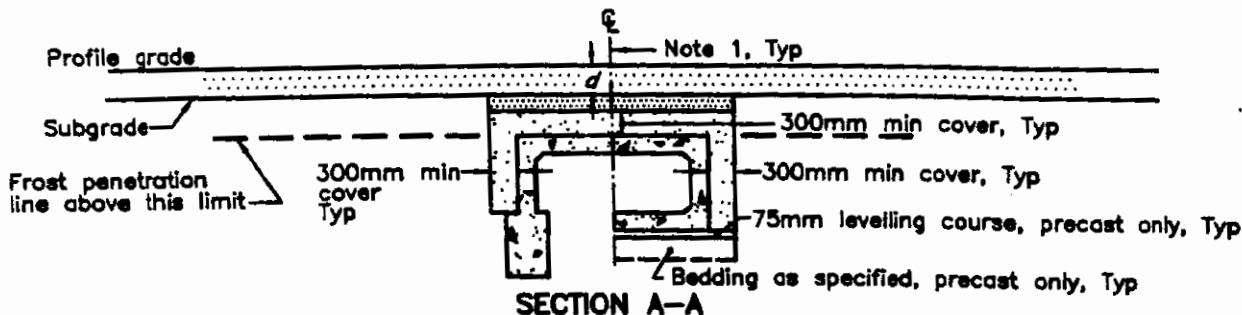
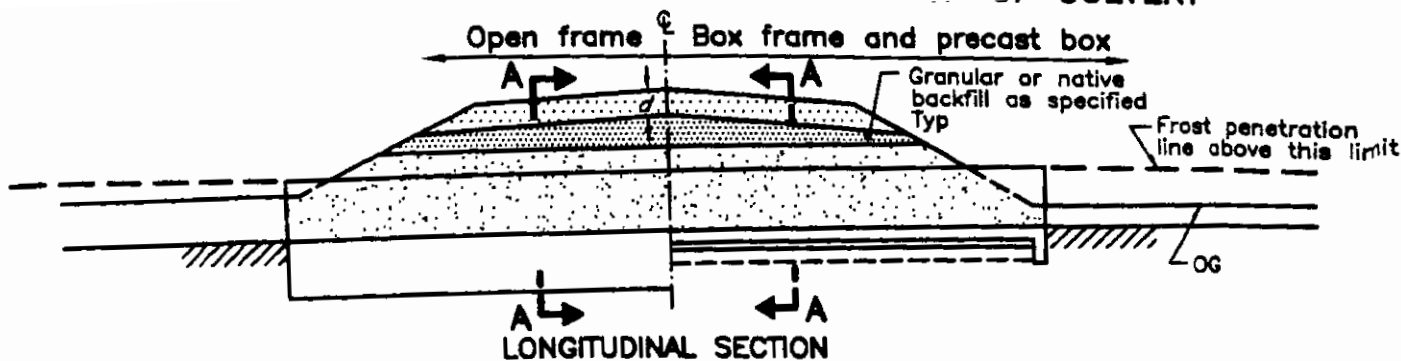
ATTERBERG LIMITS - O.P.S.S. PLASTICITY CHART

① BH-1, SS7 (6.10-6.71m)

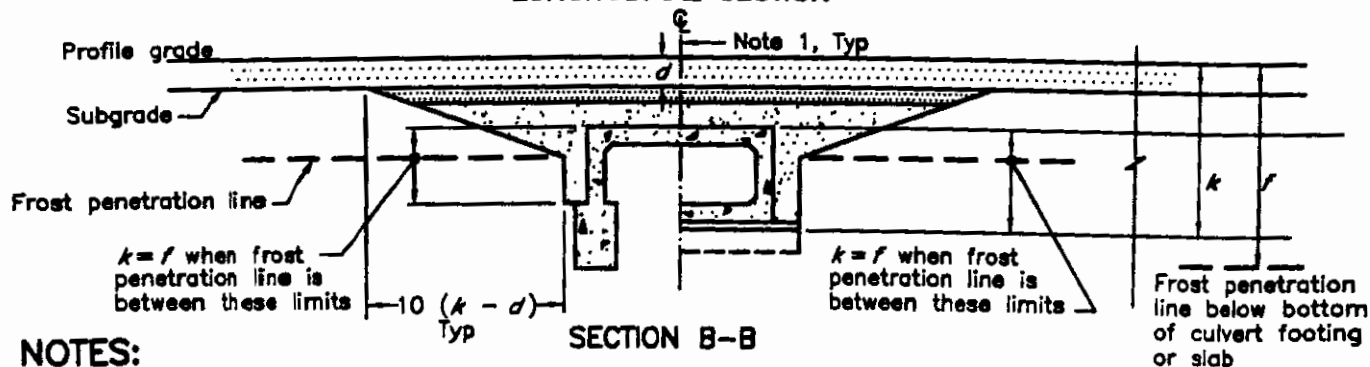
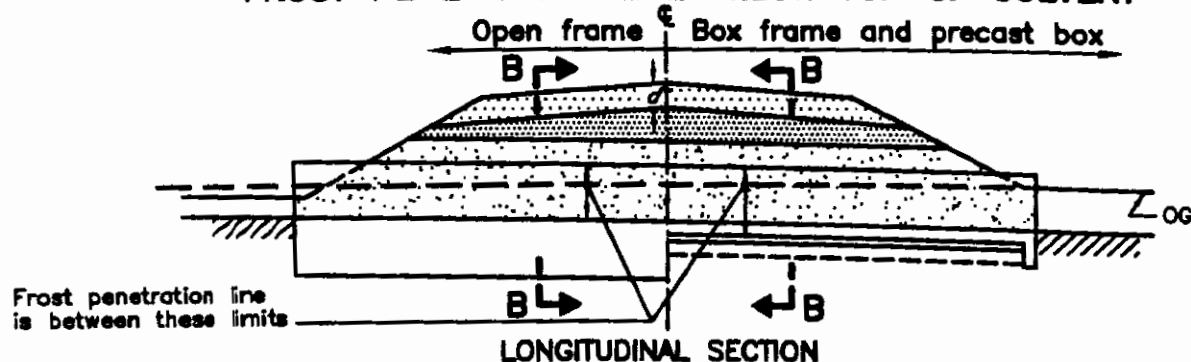


Appendix E

OPSD Specifications



FROST PENETRATION LINE BELOW TOP OF CULVERT



NOTES:

- 1 Condition of frost treatment symmetrical about centreline of culvert.
- A Bedding, levelling and cover material to be granular as specified.
- B This standard applies to rigid and non-rigid cast-in-place and precast concrete culverts.

C All dimensions are in millimetres unless otherwise shown.

LEGEND:

- d = depth of roadbed granular
 k = depth of frost treatment
 f = depth of frost penetration

ONTARIO PROVINCIAL STANDARD DRAWING

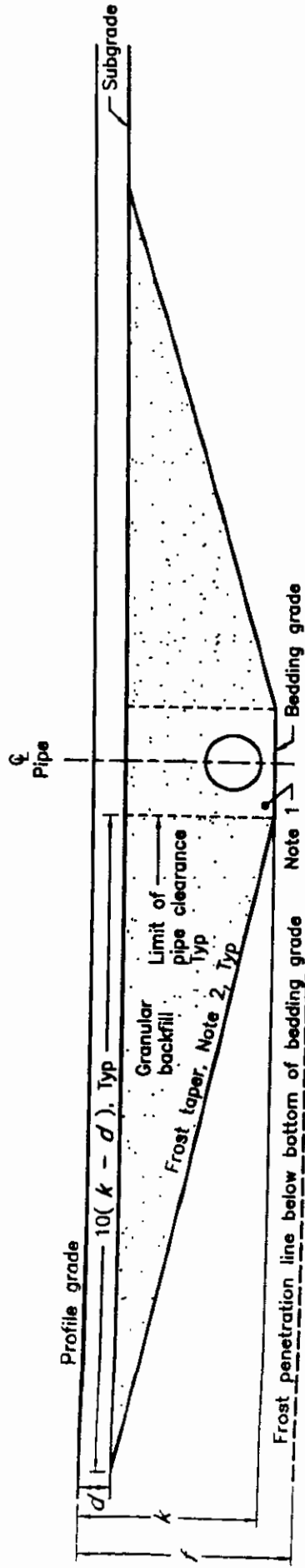
Nov 1999

Rev

**BACKFILL AND COVER
FOR CONCRETE CULVERTS**



OPSD - 803.010



FROST TREATMENT - RIGID AND FLEXIBLE PIPE

NOTES:

1 Pipe embedment or bedding, cover, and backfill according to:

- a) Flexible - OPSD-802.010, 802.013, 802.014, 802.020, 802.023, and 802.024
- b) Rigid - OPSD-802.030, 802.031, 802.032, 802.033, 802.034, 802.050, 802.051, 802.052, 802.053, and 802.054.

2 Frost tapers start at bedding grade.

A Frost tapers are not required in rock embankment.

LEGEND:

- d -depth of roadbed granular
- k -depth of frost treatment
- f -depth of frost penetration

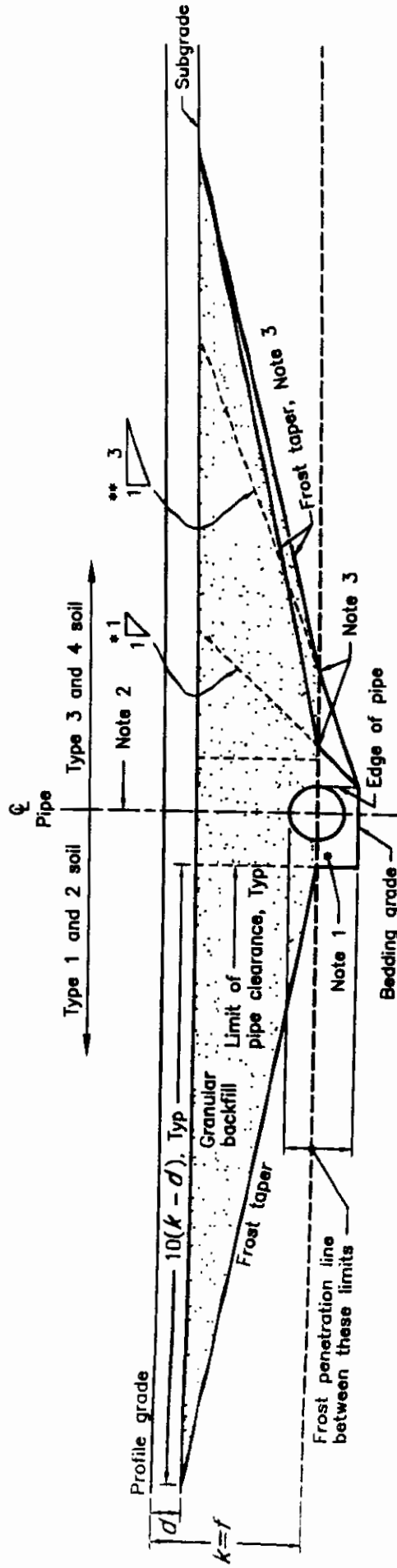
ONTARIO PROVINCIAL STANDARD DRAWING

FROST TREATMENT - PIPE CULVERTS
FROST PENETRATION LINE BELOW
BEDDING GRADE

Nov 2005 Rev 1



OPSD - 803.030



FROST TREATMENT - RIGID AND FLEXIBLE PIPE

NOTES:

- 1 Pipe embedment or bedding, cover, and backfill according to:
 - a) Flexible - OPSD-802.010, 802.013, 802.014, 802.020, 802.023 and 802.024
 - b) Rigid - OPSD-802.030, 802.031, 802.032, 802.033, 802.034, 802.050, 802.051, 802.052, 802.053, and 802.054
- 2 Condition of frost treatment symmetrical about centreline of pipe.
- 3 Frost tapers start at the intersection of the 1H:1V or 3H:1V slope and the frost penetration line.

A Frost tapers are not required in rock embankment.

B Frost tapers not required when frost line is above the top of pipe.

C Soil types as defined in the Occupational Health and Safety Act and Regulations for Construction Projects.

LEGEND:

- d - depth of roadbed granular
- k - depth of frost treatment
- f - depth of frost penetration
- * - Type 3 soil
- ** - Type 4 soil

ONTARIO PROVINCIAL STANDARD DRAWING

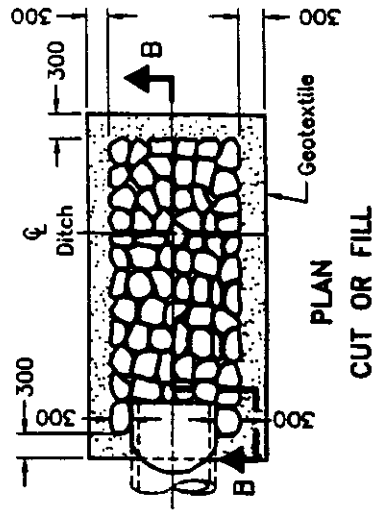
FROST TREATMENT - PIPE CULVERTS
FROST PENETRATION LINE BETWEEN
TOP OF PIPE AND BEDDING GRADE

Nov 2005

Rev 2

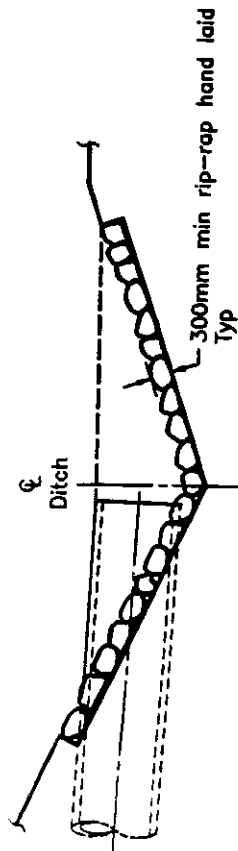


OPSD - 803.031



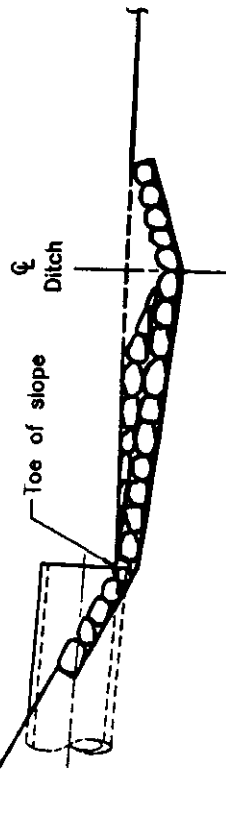
**PLAN
CUT OR FILL**

**PLAN
CUT OR FILL**



SECTION A-A CUT

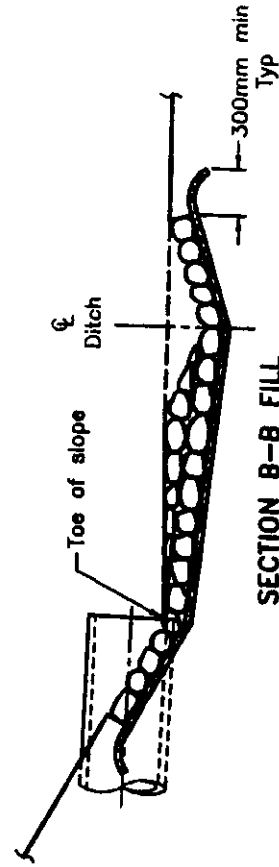
SECTION B-B CUT



SECTION A-A FILL

SECTION 8-8 FILL

TYPE B - WITH GEOTEXTILE



A All dimensions are in millimetres unless otherwise shown.

ONTARIO PROVINCIAL STANDARD DRAWING

Nov 2001	Rev 0
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OPSD - 810.010