

G.I.-30 SEPT. 1976

GEOCRES No. \_\_\_\_\_

DIST. 32 REGION \_\_\_\_\_

W.P. No. \_\_\_\_\_

CONT. No. \_\_\_\_\_

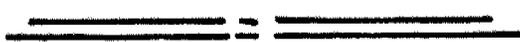
W. O. No. 95-11006

STR. SITE No. 14-446

HWY. No. 21

LOCATION Ravenswood Culvert

No of PAGES -         



OVERSIZE DRAWINGS TO BE INCLUDED WITH THIS REPORT. \_\_\_\_\_

REMARKS: \_\_\_\_\_

\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

# MEMORANDUM

To: A. Ho  
Head, Structural Section  
Southwestern Region

Date: December 14, 1995

Atten: S.A. Sidky

From: Pavements & Foundations Section  
Room 315, Central Building

Tel: 235-3731  
Fax: 235-5240

Re: Ravenswood Culvert at Sta. 19 + 787 Repair  
Highway 21, District 32, Chatham

In response to your letter dated October 31, 1995. The site was inspected on December 7, 1995.

It appears that the pavement is underlain by approximately 2.0 m clayey fill which is followed by native clayey soil. The parameters provided for the design of roadway protection are based on the site inspection and, no borehole was advanced at this location. If the design is based on undrained shear strength parameters, the following values may be used:

<u>Soil Boundary</u>	<u>Soil Type</u>	$\phi$	<u>Design Parameters</u>	
			<u>Cu (kPa)</u>	<u><math>\gamma</math> (kN/m<sup>3</sup>)</u>
El. 199.5 - El. 196.2	Clayey Fill	0	60	18.0
Below El. 196.2	Clay	0	100	19.0

If the construction period extends more than two to three months, it may be advisable to use effective strength parameters. The following effective strength parameters are recommended for the design.

<u>Soil Boundary</u>	<u>Soil Type</u>	$\phi$	<u>Design Parameters</u>	
			<u>c'(kPa)</u>	<u><math>\gamma</math> (kN/m<sup>3</sup>)</u>
El. 199.5 - El. 196.2	Clayey Fill	26	0	18
Below El. 196.2	Clay	28	0	19

*M. Vasavithasan*

M. Vasavithasan, P. Eng.  
Foundation Engineer  
for

T.C. Kim, P. Eng.  
Sr. Foundation Engineer

MV/TCK/mmj

# memorandum

RECEIVED

MAR 24 '95 AM

Brad-



Geotechnical Section, Southwestern Region, London

Planning & Design  
London Region

Fax: (519) 649-3108

To: A. E. Irving, Head  
Planning and Design Section  
Southwestern Region London

Date: March 21, 1995

Att: Brad Decker

WO 95-11006

Re: Highway 21, Ravenswood Culvert Repair, District #1

A soils investigation was conducted on March 14 at this site. An edge of pavement investigation was conducted left of centre line of Highway 21. Existing pavement structure was found to consist of 150 mm of hot mix over 150 mm Crushed Granular over 150 mm of Br F Sa which was underlain by hot mix pavement. The buried pavement extended to 5.0 m left of the existing surface course and was measured at 60 mm in depth. A brown silty clay was encountered in the outer fill slope. A log of test holes is included with this memo.

## RECOMMENDATIONS

### Removals

Assume existing Highway 21 pavement depth to be 150 mm. The pavement buried under the north shoulder is assumed to be 6.1 m wide and therefore would be partially located under existing pavement. Assume depth of hot mix to be 60 mm.

### Pavement Reinstatement

Provide for a new pavement structure at the culvert removal site to consist of:

- 40 mm HL4 surface
- 50 mm HL4 binder
- 50 mm HL4 lower binder
- 600 mm Granular "A"

Detour

The existing granular shoulders may be utilized as a base for the detour. For widenings beyond the inner edge of rounding provide for a minimum 300 mm depth of Granular "A". If a hot mix surface is required, recommend 50 mm HL4. Strip all topsoil under the detour. Assume a stripping depth of 80 mm.

Culvert

Foundation problems are not anticipated at this site. The repair section is to be constructed in the original location. Provide Granular "A" backfill as per O.P.S.D. 803.01 or 803.02. Assume F = 1.0 m.



Keith Helwig  
Pavement Design and Evaluation Officer  
For: E. Magni, Head  
Geotechnical Section  
Southwestern Region London  
(519) 681-1441 Ext 3255

KH11/cs

cc: P. Ginn  
A. Ho  
File K. Helwig (2)

GEOTECHNICAL SURVEY DATA

DATE OF SURVEY	TYPE OF SURVEY
95 03 14	Hand Equipment Investigation

NOTE

- . CONDITIONS AND PAVEMENT DEPTHS APPLY ONLY TO THE TIME OF THE SURVEY.
- . THE BOUNDARIES BETWEEN STRATA HAVE BEEN ESTABLISHED ONLY AT BOREHOLE LOCATIONS. BETWEEN BOREHOLES THE BOUNDARIES ARE ASSUMED.
- . SOILS ARE DESCRIBED ACCORDING TO THE M.T.O. CLASSIFICATION SYSTEM.
- . PAVEMENT CORE LOCATIONS WERE ESTABLISHED USING RANDOM NUMBERS UNLESS OTHERWISE SPECIFIED.
- . DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN. STATIONS IN KILOMETRES + METRES

ABBREVIATIONS

LSFH      Low Susceptibility to Frost Heave  
 MSFH      Moderate Susceptibility to Frost Heave  
 HSFH      High Susceptibility to Frost Heave  
 MWD      Maximum Wet Density  
 w          Water Content  
 w<sub>L</sub>      Liquid Limit  
 I<sub>p</sub>      Plasticity Index  
           = w<sub>L</sub> - w<sub>p</sub>  
 w<sub>opt</sub>     Optimum Water Content  
 w<sub>p</sub>      Plastic Limit  
 c<sub>u</sub>      Undrained Shear Strength  
 S<sub>t</sub>      Sensitivity  
           = Undrained shear strength  
           remoulded shear strength

W.P. ...District #1..... CONTRACT ..... HWY .21.....

Highway 21, Ravenswood  
March 14, 1995

## BOREHOLE LOG DATA

Offset From Centre Line Highway 21

### 19+798, 3.7 m Lt

0	-	150	Asph
150	-	300	Cr Gr
300	-	450	Br F Sa - Si Sa
450			N.F.P. Asph

### 19+798, 8.7 m Lt

0	-	340	Gr Gran, Very Wet
340	-	400	Asph
400			Cr Gran

### 19+800, 10.5 m Rt Datum -1.0

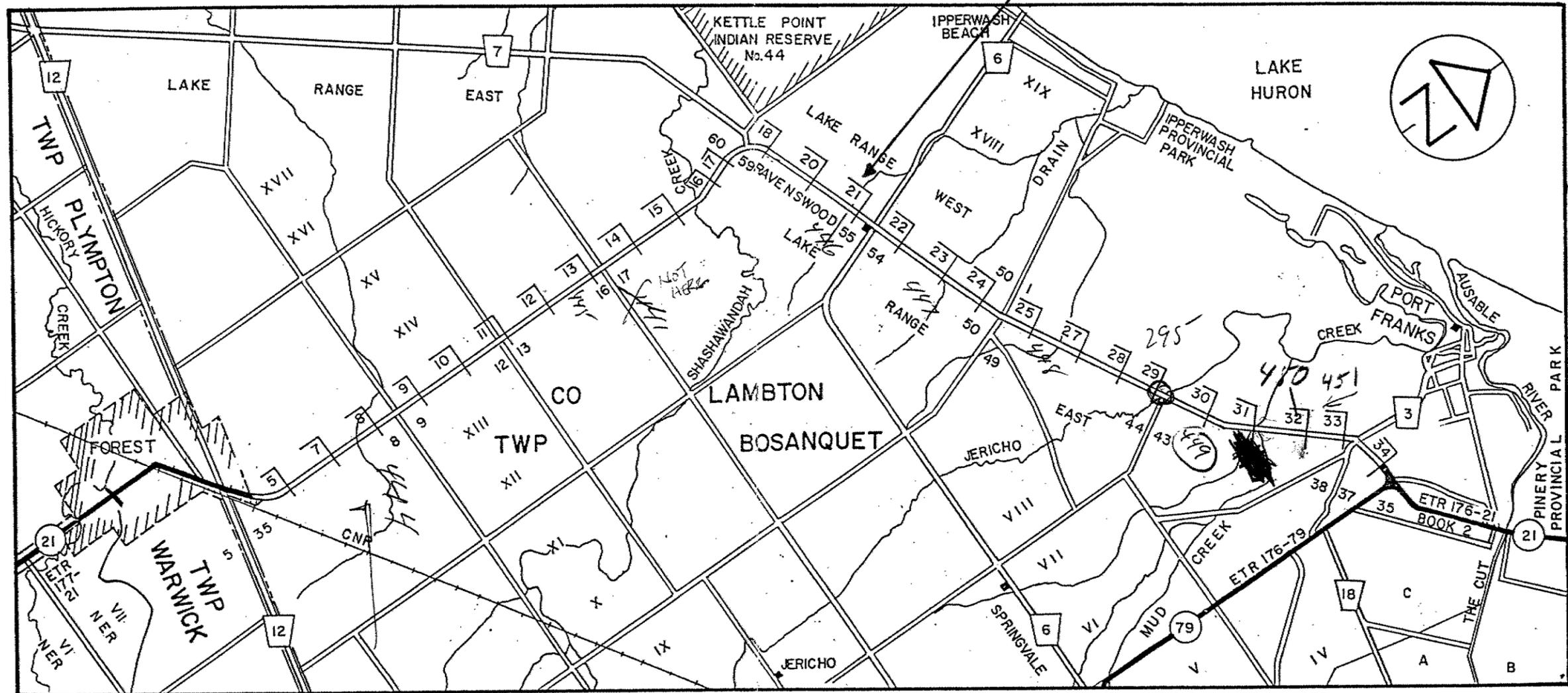
0	-	85	Tops
85	-	900	Br Si Cl Stiff
900	-	1.2	Br Si Cl with Sa Firm

METRIC

176-21

BOOK 1

Site



LARGE CURVE  
 3.80 x 1.80  
 (CONV. LINK?)  
443

# ENGINEERING & TITLE RECORDS

## KING'S HIGHWAY 21

TOWNSHIP

### BOSANQUET

COUNTY

### LAMBTON

10+000 TO 27+726.517

WO 95-11006



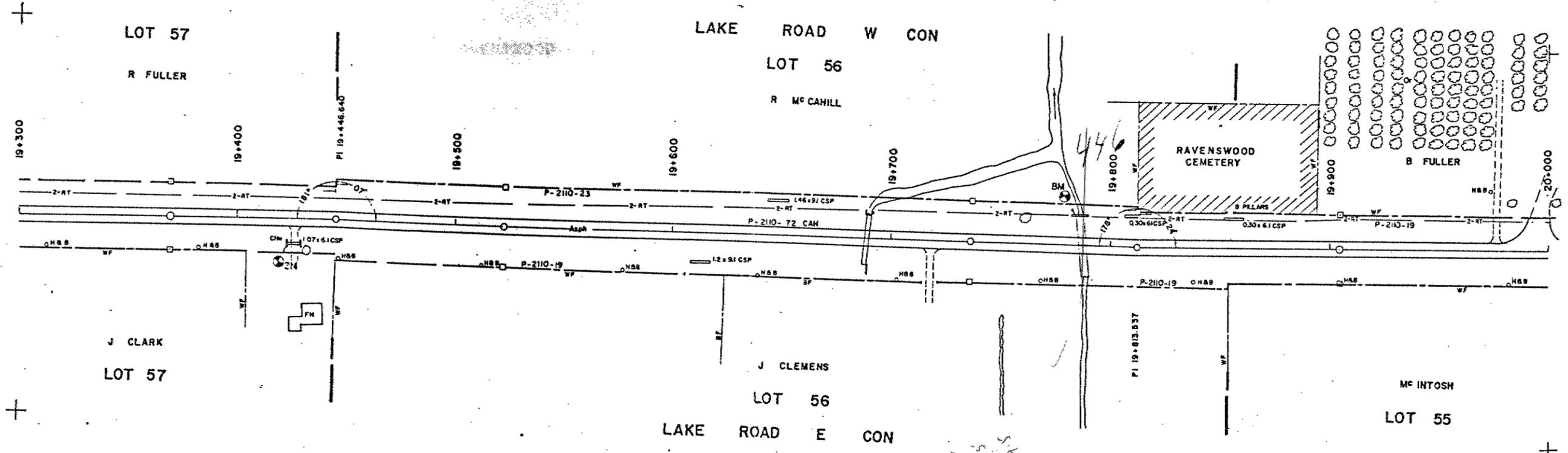
Ontario

Ministry of  
Transportation and  
Communications



**METRIC**

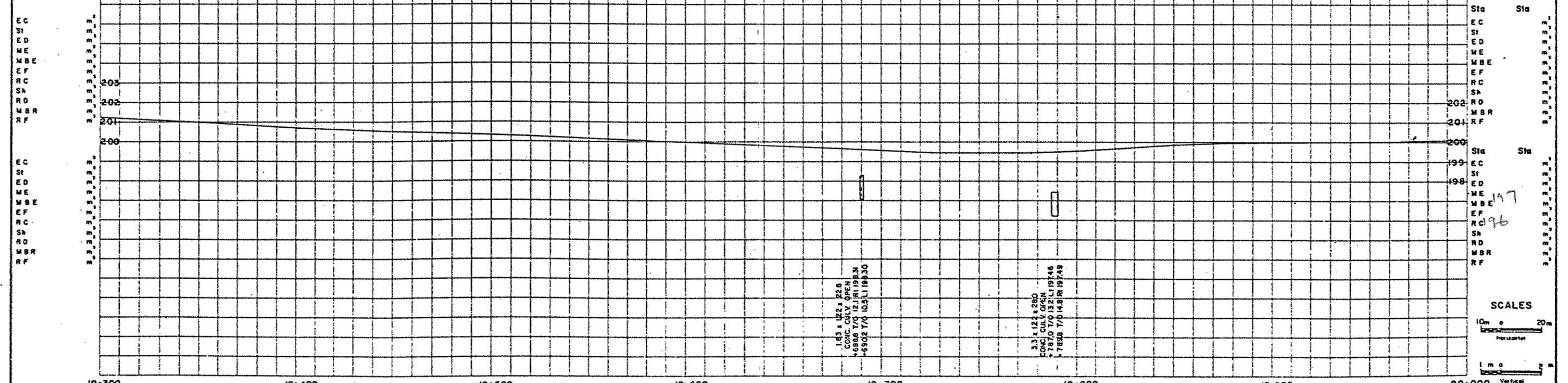
NOTE - DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN



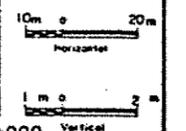
196.2  
197.5  
198.5

QUANTITIES

QUANTITIES



SCALES



Site appears to be between  
Sta. 19+773 & 19+794

fill is 2.0m.

terrain is undulating

may be sh<sub>1</sub> to very sh<sub>1</sub> clay

$C_u = 100$  kPa

$\sigma = 19 \text{ kN/m}^2$