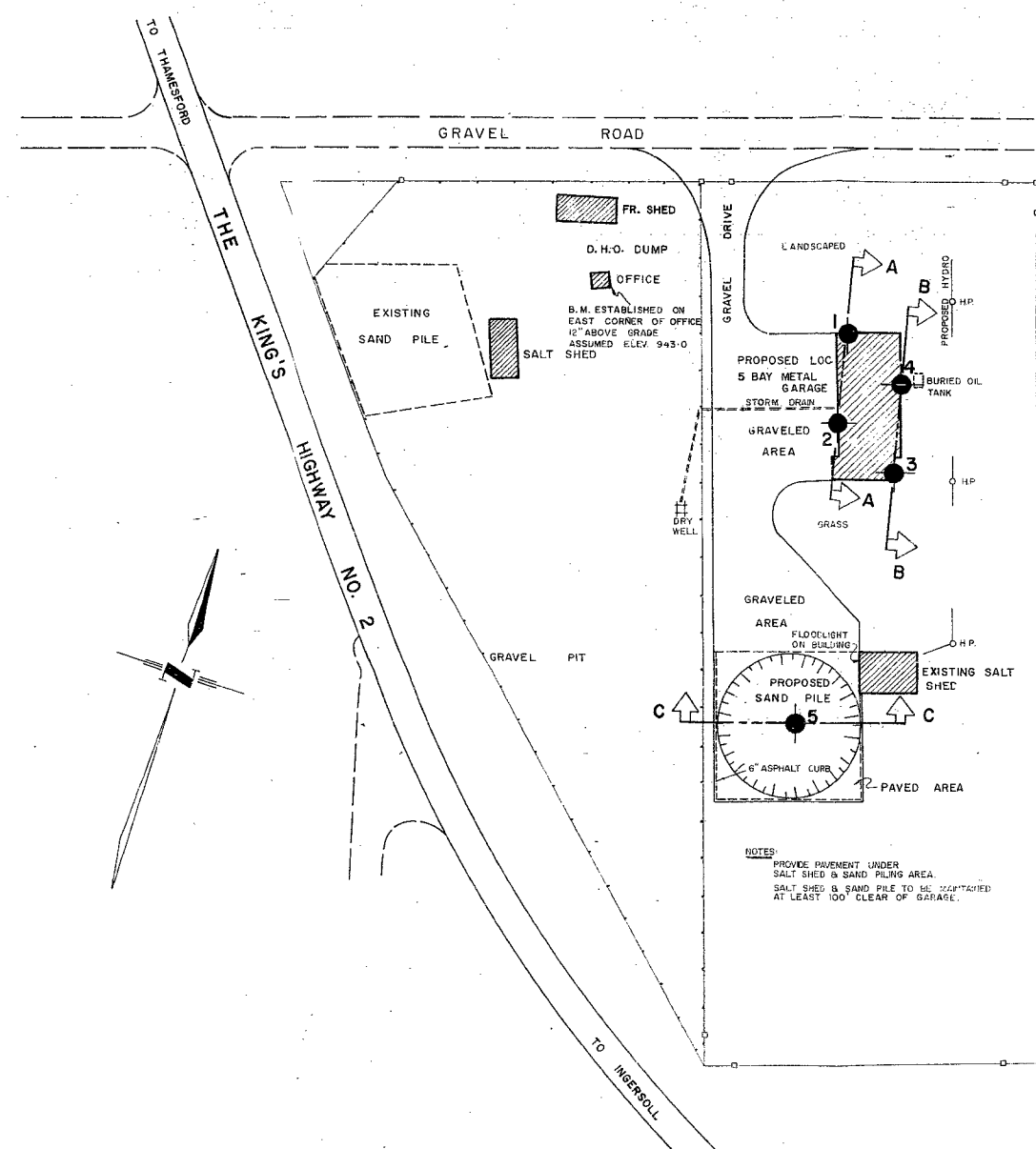


#62-F-22

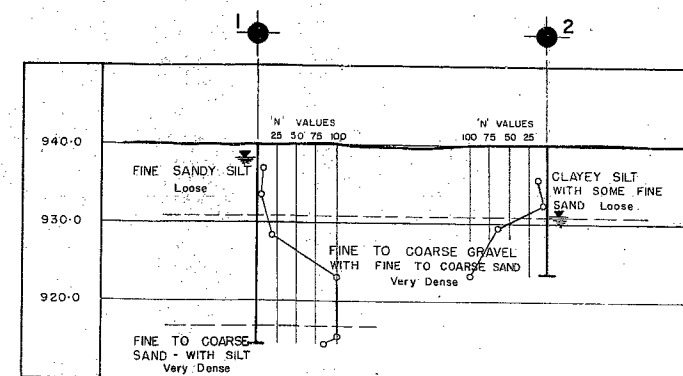
Hwy. #2 E

THAMES FORD

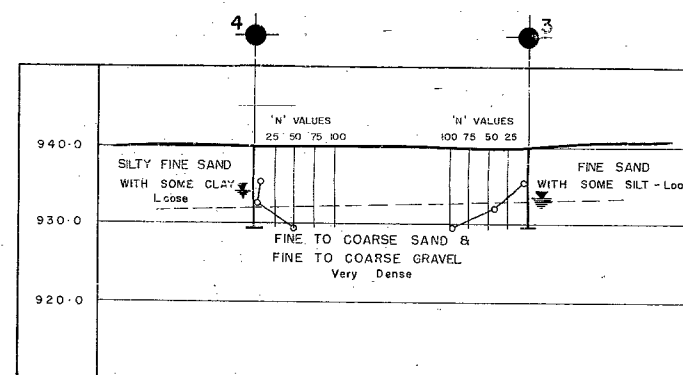
PROP. PATROL YD.



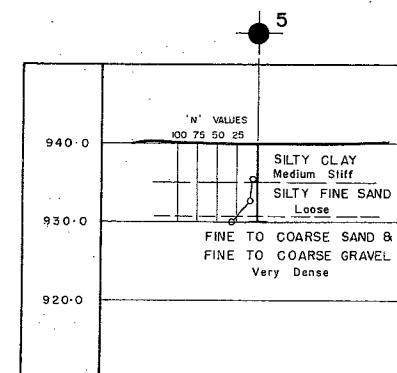
PLAN
SCALE IN FEET
50 25 0 50 100 150



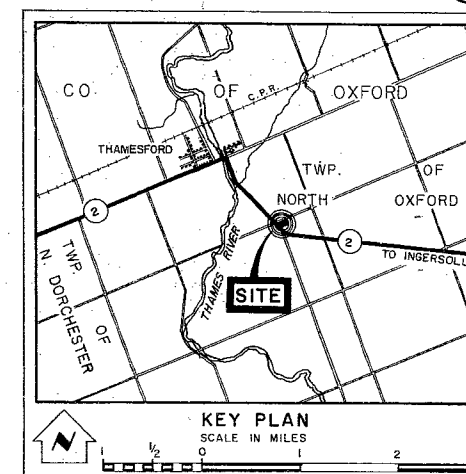
A - A
SCALE IN FEET
10 5 0 10 20 30



B - B
SCALE IN FEET
10 5 0 10 20 30



C - C
SCALE IN FEET
10 5 0 10 20 30



LEGEND			
	Bore Hole		
	Cone Penetration Hole		
	Bore & Cone Penetration Hole		
	Water Levels established at time of field investigation		
NO.	ELEVATION	STATION	OFFSET
1	940.0		
2	940.0		
3	940.0		
4	940.0		
5	940.0		

- NOTE -
The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence and may be subject to considerable error.

DEPARTMENT OF HIGHWAYS - ONTARIO MATERIALS & RESEARCH DIVISION - FOUNDATION SECTION			
THAMESFORD PATROL YARD			
ORIGINATED T. WIDDIS	DISTRICT NO. 2	DATE MAY 15, 1962	
DRAWN F. CLARK	W.P. NO. —	JOB NO. 62-F-22	
CHECKED <i>H. Widdis</i>	CONTRACT NO.	DRAWING NO.	
APPROVED <i>M. Scriver</i>			62-F-22 A

Mr. F. E. Cavell,
Superintendent,
Special Services Section.
Materials & Research Division.
(Foundation Section).

May 14, 1962.

D.H.O. FOUNDATION INVESTIGATION
REPORT.
W.J. 62-F-22 -- (W.P. Nil).

Attention: Mr. K. Hobby.

RE: PROPOSED PATROL YARD ON HWY. 2 AT THAMESFORD, LOT 24, CONC. II,
TWP. OF NORTH OXFORD, COUNTY OF OXFORD, DISTRICT NO. 2.

It is proposed to rebuild the D.H.O. patrol yard on Hwy. 2 at Thamesford. For design purposes a foundation investigation of the site was requested by the Special Services Section in a memo dated February 16, 1962.

To determine the subsoil conditions at the site five sampled boreholes were put down using a small auger machine. The locations and elevations of these boreholes, together with the inferred stratigraphical profiles are shown on the drawing 62-F-22A, which also shows the proposed layout of the patrol yard.

The subsoil at the site consists of a heterogeneous deposit varying from clayey silt with fine sand to silty fine sand. This deposit varies in thickness between 6.0 and 9.0 feet and has an average penetration resistance N of

cont'd. /2 ...

5 indicating that the deposit is loose.

Beneath this material is a deposit of very dense, fine to coarse gravel and sand. This deposit extends throughout the depth of a borehole drilled to 25.0 feet.

The water levels as recorded during the time of the field investigation are plotted on the drawing.

It is recommended that the loose clayey silt to silty fine sand be excavated and the buildings be supported on spread footings placed on the exposed fine to coarse gravel and sand. For a footing 2.5 feet wide a safe design load of 2.0 tons/sq. ft. may be used.

Dewatering of the excavation during construction should not prove a major problem.

For all service roads, parking lots and other areas to be paved or gravelled, the topsoil should be removed and replaced with 18" of acceptable sand cushion topped with 6" of G.B.C. class A.

Surfacing material for the roadways and parking areas should consist of a 2" binder of H.L. 6 and a 1½" wearing course of H.L. 3.

The field work was undertaken during the period from March 16, 1962 to March 20, 1962 by Mr. T. F. Widdis who also prepared the report under the supervision of Mr. M. Deveta.

cont'd. /3 ...

We trust that these recommendations are sufficient for your design work. However, should there be any additional questions you would like to discuss, please feel free to call on our Office.

AGS/tt

cc: Messrs. F. E. Cavell (2)
K. Hobby
H. A. Tregaskes
H. D. McMillan
H. C. Tackaberry
W. L. Fraser
T. J. Kovich
J. Roy
J. E. Gruspier
E. R. Saint
F. Norman
Foundations Office
Gen. Files ✓

A. G. Stermac

A. G. Stermac
Principal Foundation Engineer

APPENDIX I.