

Golder Associates Ltd.

309 Exeter Road, Unit #1
London, Ontario, Canada N6L 1C1
Telephone: (519) 652-0099
Fax: (519) 652-6299



**FOUNDATION INVESTIGATION REPORT
STRUCTURAL CULVERT
SITE 6-406-C
HIGHWAY 3 WIDENING
GWP 315-98-00
MINISTRY OF TRANSPORTATION - SOUTHWESTERN REGION**

Submitted to:

Delcan Corporation
1069 Wellington Road South, Suite 214
London, Ontario
N6E 2H6

DISTRIBUTION:

- 9 Copies - Delcan Corporation
- 2 Copies - Golder Associates Ltd.

March 12, 2007

06-1130-177-0-1
Geocres No. 40J2-93



TABLE OF CONTENTS

<u>SECTION</u>	<u>PAGE</u>
PART A – FOUNDATION INVESTIGATION REPORT	
1.0 INTRODUCTION.....	1
2.0 SITE DESCRIPTION.....	2
2.1 Site Geology	2
3.0 INVESTIGATION PROCEDURES.....	3
4.0 SUBSURFACE CONDITIONS.....	5
4.1 Site Stratigraphy	5
4.1.1 Topsoil and Fill.....	5
4.1.2 Clayey Silt Till	5
4.2 Groundwater Conditions	6
5.0 MISCELLANEOUS.....	7

LIST OF ABBREVIATIONS

LIST OF SYMBOLS

RECORD OF BOREHOLE SHEETS

FIGURE 1 - Key Plan

DRAWING 1 - Borehole Locations and Soil Strata

APPENDIX A - Laboratory Test Data

APPENDIX B - Site Photograph

1.0 INTRODUCTION

Golder Associates Ltd. (Golder Associates) has been retained by Delcan Corporation (Delcan) on behalf of the Ministry of Transportation, Ontario (MTO) to carry out foundation investigations as part of the detail design work for GWP 315-98-00. The project involves the first phase of the reconstruction and widening of Highway 3 (Talbot Trail) between Windsor and Leamington. The project limits extend along Highway 3 from just west of Essex Road 34 (Talbot Street North) easterly to just east of Essex Road 8 (Maidstone Avenue West) in Essex County, Ontario.

In conjunction with the widening, the scope of work for this project includes:

- Rehabilitation or replacement of selected culverts within the project limits;
- Slotted left turn lanes at all intersections;
- Revision or upgrading of illumination at four intersections;
- Revision or upgrading of traffic signals at two intersections;
- Replacement or relocation of existing traffic counting stations;
- Drainage improvements; and,
- Upgrading of existing signage.

Five structural culverts and four short span culverts are to be widened and two short span culverts are to be replaced.

This report addresses the foundation investigation for the extension of the south end of the structural culvert located on Highway 3 at Station 17+810 South Sandwich (Site 6-406-C). This culvert is to be extended by up to 5 metres.

The purpose of the foundation investigation is to determine the subsurface conditions at the location of the proposed works by drilling boreholes and carrying out in situ testing and laboratory testing on selected samples. The terms of reference for the scope of work are outlined in the MTO's Request for Proposal, in Golder Associates' proposal P61-3113-1 dated August 17, 2006 and our letter dated November 14, 2006. The work was carried out in accordance with our Quality Control Plan for Foundations Engineering dated September 18, 2006.

Delcan provided Golder Associates with a base plan and profile for this project in digital format which included the top elevations of the culverts.

2.0 SITE DESCRIPTION

GWP 315-98-00 extends along Highway 3 from 0.5 kilometres west of Essex Road 34 (Talbot Street North) within the Town of Tecumseh easterly to 0.6 kilometres east of Essex Road 8 (Maidstone Avenue West) in the Town of Essex. West of Manning Road, Highway 3 is within South Sandwich Township. East of Manning Road, it is within Maidstone Township. The chainage equation at Manning Road is $20+981.675 \text{ South Sandwich (SS)} = 10+000.000 \text{ Maidstone (M)}$. The west project limit is located at Station 17+700 SS and the east project limit is situated at Station 13+600 M.

Culvert Site 6-406-C is located at Station 17+810 SS on Highway 3 approximately 380 metres east of Essex Road 34. This culvert conveys flows of the West Branch of the Delisle Drain from the left/north side of Highway 3 to the right/south side. The location of the project is shown on the Key Plan, Figure 1, and in the photograph in Appendix B.

The land use in the vicinity of the site is predominantly agricultural. The adjacent topography is generally flat to slightly rolling with a ground surface elevation between 191 and 195 metres.

2.1 Site Geology

The site is situated on the Essex Clay Plain, a subregion of the physiographic region of southern Ontario known as the St. Clair Clay Plain.¹ This subregion is described as a beveled till plain with little relief that has been locally smoothed by shallow deposits of lacustrine clay deposited in depressions in the till. The prevailing soil type is reported to be the Brookston clay loam.

The available surficial geology mapping for the project area indicates that the predominant surficial soils are clayey silt till.² The till is reportedly underlain by limestone, dolomite and shale of the Middle Devonian era and by dolomite of the Upper Silurian era. The overburden thickness within the project area ranges from 27 to 41 metres.³

¹ L.J. Chapman and D.F. Putnam, 1984. *The Physiography of Southern Ontario*. Third Edition. Ontario Geological Survey, Special Volume 2.

² Vagners, U. J., 1972. *Quaternary Geology of the Windsor-Essex Area, (Western and Eastern Parts) Southern Ontario*. Ontario Department of Mines and Northern Affairs, Preliminary Maps P. 749 and P.750, Geological Series.

³ Vagners, U.J., Sado, E.V., and Yundt, S.E. 1973. *Drift Thickness of the Windsor-Essex Area (Western and Eastern Parts), Southern Ontario*, Ontario Division of Mines, Preliminary Maps P.814 and P.815, Drift Thickness Series.

3.0 INVESTIGATION PROCEDURES

The field work for this portion of the investigation was carried out on November 29 and 30, 2006, at which time two boreholes, numbered 9 and 10, were drilled in the area of the proposed culvert extension. Borehole 9 was advanced to a depth of 9.0 metres and borehole 10 to a depth of 7.5 metres.

The investigation for these two boreholes was carried out using a Deitrich 50 track-mounted power auger supplied and operated by a specialist drilling contractor. Samples of the overburden were obtained at intervals of 0.75 metres up to a depth of 4.6 metres and at 1.5 metre intervals below this depth using 50 millimetre outside diameter split spoon sampling equipment in accordance with the standard penetration test (SPT) procedures.

Groundwater conditions in the boreholes were observed throughout the drilling operations and these observations are provided on the corresponding Record of Borehole sheets. A standpipe piezometer was installed in borehole 10 to monitor the groundwater levels at this location. Boreholes were backfilled in accordance with current regulations, MTO recommended procedures and Ontario Regulation 128/03.

The field work was supervised on a full-time basis by an experienced member of our engineering staff who arranged for utility locates, directed the drilling, sampling and in-situ testing operations, logged the boreholes, and cared for the samples obtained. The soil samples were identified in the field, placed in labeled containers and transported to Golder Associates' London laboratory for further examination and testing. Index and classification tests consisting of water content determinations, grain size distribution analyses and Atterberg limits determinations were carried out on selected samples. The results of the field and laboratory testing are given on the Record of Borehole sheets and in Appendix A.

Temporary traffic control was carried out in accordance with the Ontario Traffic Manual, Temporary Conditions, Book 7, dated March 2001.

The as-drilled borehole locations and ground surface elevations are shown on the Record of Borehole sheets and on Drawing 1.

The table below summarizes the culvert location and the coordinates, ground surface elevations and depths of the associated boreholes.

<u>BOREHOLE</u>	<u>LOCATION (m)</u>		<u>GROUND SURFACE ELEVATION</u>	<u>BOREHOLE DEPTH</u>
	<u>Northing</u>	<u>Easting</u>	(m)	(m)
9	4 674 988.1	271 188.7	189.20	8.99
10	4 674 978.2	271 196.9	189.03	7.47

The existing culvert has the following characteristics:

<u>DIMENSIONS (m)</u>	<u>TOP ELEVATION (m)</u>		<u>CONSTRUCTION</u>
	(Lt)	(Rt)	
3.05 x 1.20 x 42.57	189.33	189.31	Concrete, rigid frame open footing

4.0 SUBSURFACE CONDITIONS

4.1 Site Stratigraphy

The detailed subsurface soil and groundwater conditions encountered in the boreholes together with the results of the in situ and laboratory testing carried out on selected samples are given on the attached Record of Borehole sheets following the text of this report and in Appendix A. The stratigraphic boundaries shown on the Record of Borehole sheets are inferred from non-continuous sampling and observations of drilling resistance and represent transitions between soil types rather than exact planes of geological change. Subsurface conditions will vary between and beyond the borehole locations.

In general, the boreholes drilled at the proposed culvert extension typically encountered topsoil and fill materials underlain by an extensive deposit of clayey silt till.

The locations of the boreholes are shown on the attached Drawing 1. A detailed description of the subsurface conditions encountered in the boreholes is provided on the Record of Borehole sheets and is summarized in the following sections.

4.1.1 Topsoil and Fill

Topsoil layers with an average thickness of 80 millimetres were encountered at ground surface in boreholes 9 and 10.

The topsoil layer at borehole 10 was underlain by a 1.3 metre thick layer of clayey silt fill from elevation 189.0 metres. The fill was firm with an N value of 7 blows per 0.3 metres.

4.1.2 Clayey Silt Till

Clayey silt till was encountered beneath the topsoil layer in borehole 9 from elevation 189.1 metres and beneath the fill layer in borehole 10 from elevation 187.7 metres. The results of particle size analyses conducted on three samples obtained from the standard penetration testing are shown in Figure A-1 in Appendix A.

The clayey silt till is stiff to hard with N values ranging from 13 blows to 46 blows per 0.3 metres. Water contents in the fill ranged from 15 to 21 per cent. The clayey silt till is of low plasticity based on an average plastic and liquid limits of 16 per cent and 30 per cent, respectively, and an average plasticity index of 14 per cent. The results of the Atterberg limits testing are shown on the Plasticity Chart, Figure A-2.

Although cobbles and boulders were not specifically encountered in either of the boreholes for this investigation, the presence of cobbles and boulders should be anticipated in the clayey silt till.

4.2 Groundwater Conditions

Groundwater conditions were observed during and on completion of drilling and sampling. Both boreholes were dry during and upon completion of drilling.

A standpipe piezometer was installed in borehole 10 to monitor the groundwater conditions. Two weeks after installation, on December 14, 2006, the groundwater level was measured at elevation 188.1 metres or at a depth of 0.9 metres below the ground surface. The most recent reading was obtained on February 26, 2007. On this date the groundwater level was at elevation 188.0 metres or 1.0 metres below the ground surface.

The water level in the West Branch of the Delisle Drain was at elevation 188.03 metres on November 30, 2006.

The groundwater levels are expected to fluctuate seasonally and are likely to be higher during periods of sustained precipitation or spring melt.

5.0 MISCELLANEOUS

The investigation was carried out using equipment supplied and operated by London Soil Test Limited which is an Ontario Ministry of Environment licensed well contractor. The field operations were supervised by Mr. Mike Arthur under the direction of Mr. David J. Mitchell. The laboratory testing was carried out at Golder Associates' London laboratory under the direction of Mr. Chris M. Sewell. The laboratory is an accredited participant in the MTO Soil and Aggregate Proficiency Program and is certified by the Canadian Council of Independent Laboratories for testing of Types C and D aggregates. This report was prepared by Ms. Dirka U. Prout, P. Eng. under the direction of the Project Manager, Mr. Philip R. Bedell, P. Eng. This report was reviewed by Mr. Fintan J. Heffernan, P. Eng., the Designated MTO Contact and Quality Control Auditor for this assignment.

GOLDER ASSOCIATES LTD.

Dirka U. Prout, P. Eng.
Geotechnical Engineer

Philip R. Bedell, P. Eng.
Principal

Fintan J. Heffernan, P. Eng.
Designated MTO Contact

DUP/PRB/FJH/cr
n:\active\2006\1130 - geotechnical\1130-100\06-1130-177 delcan - gwp 315-98-00 fdns - hwy 3\reports\(-1) site 6-406\mar 12 07 - (final) fdn rpt - structural culvert ss 17+810
hwy 3.doc

LIST OF ABBREVIATIONS

The abbreviations commonly employed on Records of Boreholes, on figures and in the text of the report are as follows:

I. SAMPLE TYPE

AS	Auger sample
BS	Block sample
CS	Chunk sample
SS	Split-spoon
DS	Denison type sample
FS	Foil sample
RC	Rock core
SC	Soil core
ST	Slotted tube
TO	Thin-walled, open
TP	Thin-walled, piston
WS	Wash sample

III. SOIL DESCRIPTION

(a) Cohesionless Soils

Density Index (Relative Density)	N Blows/300 mm or Blows/ft.
Very loose	0 to 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very dense	over 50

II. PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg. (140 lb.) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split spoon sampler for a distance of 300 mm (12 in.)

Consistency

	<u>kPa</u>	<u>psf</u>
Very soft	0 to 12	0 to 250
Soft	12 to 25	250 to 500
Firm	25 to 50	500 to 1,000
Stiff	50 to 100	1,000 to 2,000
Very stiff	100 to 200	2,000 to 4,000
Hard	over 200	over 4,000

(b) Cohesive Soils

Dynamic Cone Penetration Resistance; N_d :

The number of blows by a 63.5 kg. (140 lb.) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

PH: Sampler advanced by hydraulic pressure

PM: Sampler advanced by manual pressure

WH: Sampler advanced by static weight of hammer

WR: Sampler advanced by weight of sampler and rod

Piezo-Cone Penetration Test (CPT)

A electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (Q_t), porewater pressure (PWP) and friction along a sleeve are recorded electronically at 25 mm penetration intervals.

IV. SOIL TESTS

w	water content
w_p	plastic limit
w_l	liquid limit
C	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test ¹
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
D_R	relative density (specific gravity, G_s)
DS	direct shear test
M	sieve analysis for particle size
MH	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO_4	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V	field vane (LV-laboratory vane test)
γ	unit weight

Note: 1 Tests which are anisotropically consolidated prior to shear are shown as CAD, CAU.

LIST OF SYMBOLS

Unless otherwise stated, the symbols employed in the report are as follows:

I. General

π	3.1416
$\ln x$,	natural logarithm of x
\log_{10}	x or log x, logarithm of x to base 10
g	acceleration due to gravity
t	time
F	factor of safety
V	volume
W	weight

II. STRESS AND STRAIN

γ	shear strain
Δ	change in, e.g. in stress: $\Delta \sigma$
ϵ	linear strain
ϵ_v	volumetric strain
η	coefficient of viscosity
ν	poisson's ratio
σ	total stress
σ'	effective stress ($\sigma' = \sigma - u$)
σ'_{vo}	initial effective overburden stress
$\sigma_1, \sigma_2, \sigma_3$	principal stress (major, intermediate, minor)
σ_{oct}	mean stress or octahedral stress $= (\sigma_1 + \sigma_2 + \sigma_3)/3$
τ	shear stress
u	porewater pressure
E	modulus of deformation
G	shear modulus of deformation
K	bulk modulus of compressibility

III. SOIL PROPERTIES

(a) Index Properties

$\rho(\gamma)$	bulk density (bulk unit weight*)
$\rho_d(\gamma_d)$	dry density (dry unit weight)
$\rho_w(\gamma_w)$	density (unit weight) of water
$\rho_s(\gamma_s)$	density (unit weight) of solid particles
γ'	unit weight of submerged soil ($\gamma' = \gamma - \gamma_w$)
D_R	relative density (specific gravity) of solid particles ($D_R = \rho_s / \rho_w$) (formerly G_s)
e	void ratio
n	porosity
S	degree of saturation

(a) Index Properties (continued)

w	water content
w_l	liquid limit
w_p	plastic limit
I_p	plasticity index $= (w_l - w_p)$
w_s	shrinkage limit
I_L	liquidity index $= (w - w_p) / I_p$
I_C	consistency index $= (w_l - w) / I_p$
e_{max}	void ratio in loosest state
e_{min}	void ratio in densest state
I_D	density index $= (e_{max} - e) / (e_{max} - e_{min})$ (formerly relative density)

(b) Hydraulic Properties

h	hydraulic head or potential
q	rate of flow
v	velocity of flow
i	hydraulic gradient
k	hydraulic conductivity (coefficient of permeability)
j	seepage force per unit volume

(c) Consolidation (one-dimensional)

C_c	compression index (normally consolidated range)
C_r	recompression index (over-consolidated range)
C_s	swelling index
C_a	coefficient of secondary consolidation
m_v	coefficient of volume change
c_v	coefficient of consolidation
T_v	time factor (vertical direction)
U	degree of consolidation
σ'_p	pre-consolidation pressure
OCR	over-consolidation ratio $= \sigma'_p / \sigma'_{vo}$

(d) Shear Strength

τ_p, τ_r	peak and residual shear strength
ϕ'	effective angle of internal friction
δ	angle of interface friction
μ	coefficient of friction $= \tan \delta$
c'	effective cohesion
c_u, s_u	undrained shear strength ($\phi = 0$ analysis)
p	mean total stress $(\sigma_1 + \sigma_3)/2$
p'	mean effective stress $(\sigma'_1 + \sigma'_3)/2$
q	$(\sigma_1 + \sigma_3)/2$ or $(\sigma'_1 + \sigma'_3)/2$
q_u	compressive strength $(\sigma_1 + \sigma_3)$
S_t	sensitivity

- Notes:**
- 1 $\tau = c' + \sigma' \tan \phi'$
 - 2 shear strength $= (\text{compressive strength})/2$
 - * density symbol is ρ . Unit weight symbol is γ where $\gamma = \rho g$ (i.e. mass density x acceleration due to gravity)

RECORD OF BOREHOLE No 9

1 OF 1

METRIC

PROJECT 06-1130-177 LOCATION N 4674988.1 ; E 271188.7 ORIGINATED BY MA
G.W.P. 315-98-00 DIST HWY 3 BOREHOLE TYPE Power Auger, Solid Stem COMPILED BY LMK
DATUM GEODETIC DATE November 29, 2006 CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa										
								○ UNCONFINED	+	FIELD VANE								
189.20	GROUND SURFACE						● QUICK TRIAXIAL	×	LAB VANE	WATER CONTENT (%)								
0.08	TOPSOIL, clayey Brown CLAYEY SILT, some sand, trace gravel (TILL) Stiff to Hard Brown becoming Grey at about elev. 185.6m		1	SS	13													
			2	SS	37													
			3	SS	46													
			4	SS	37													
			5	SS	23													
			6	SS	25													
			7	SS	17													
			8	SS	16													
			9	SS	17													
180.21	END OF BOREHOLE																	
8.99	Borehole dry during drilling November 29, 2006.																	

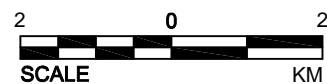
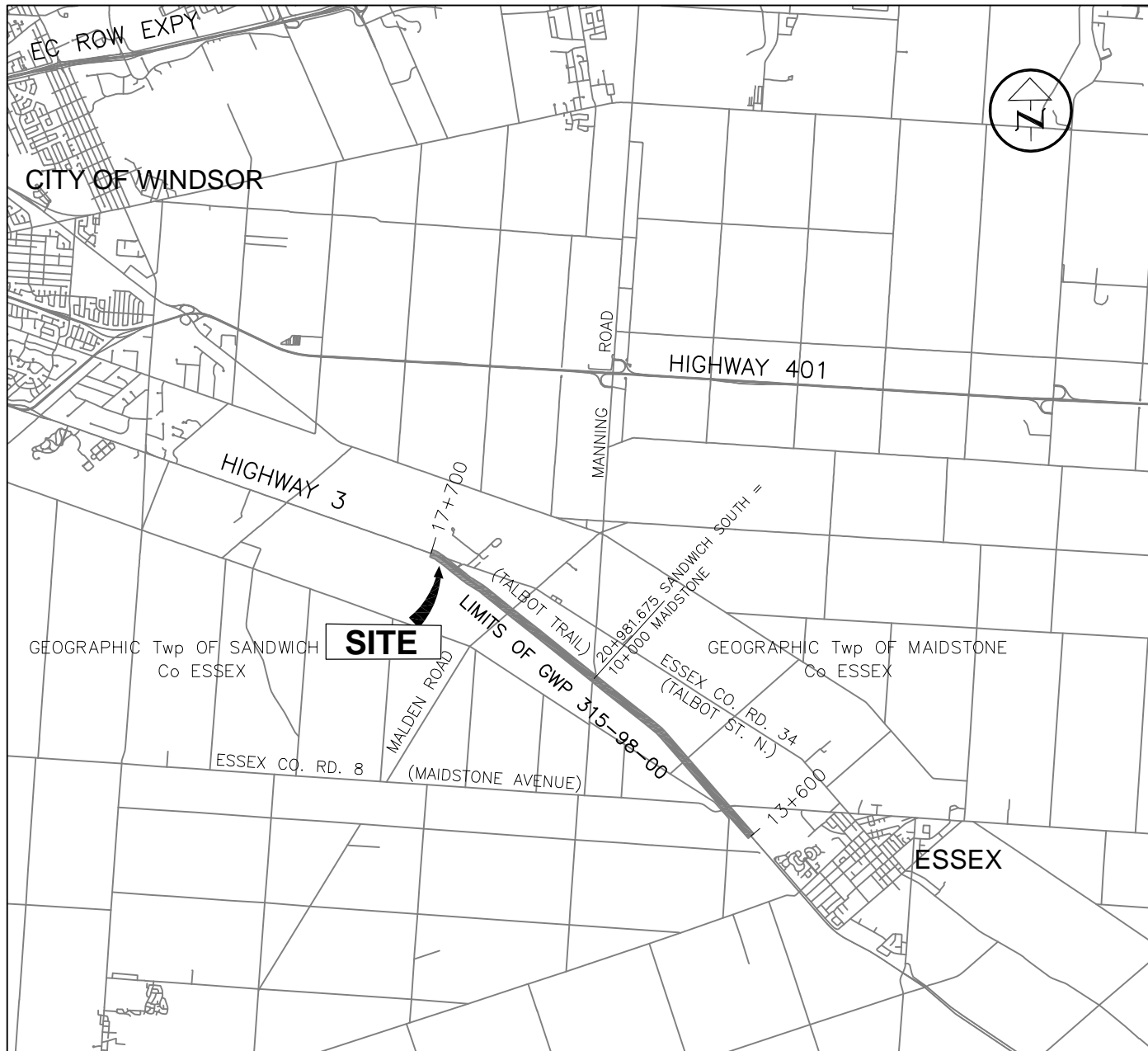
RECORD OF BOREHOLE No 10

1 OF 1

METRIC

PROJECT 06-1130-177
G.W.P. 315-98-00 LOCATION N 4674978.2 ; E 271196.9 ORIGINATED BY MA
DIST HWY 3 BOREHOLE TYPE Power Auger, Solid Stem COMPILED BY LMK
DATUM GEODETIC DATE November 30, 2006 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa									
								○ UNCONFINED	+ FIELD VANE	● QUICK TRIAXIAL	× LAB VANE	WATER CONTENT (%)					
189.03	GROUND SURFACE						20	40	60	80	100						
0.08	TOPSOIL, clayey Brown																
	FILL, clayey silt, trace sand, trace gravel and topsoil																
	Firm Brown		1	SS	7												
187.66																	
1.37	CLAYEY SILT, some sand, trace gravel (TILL) Stiff to Hard Brown becoming Grey at about elev. 185.7m		2	SS	20												
			3	SS	35												
			4	SS	30												
			5	SS	23												
			6	SS	24												
			7	SS	18												
			8	SS	15												
181.56																	
7.47	END OF BOREHOLE																
	Borehole dry during drilling November 30, 2006. and Standpipe dry to elev. 181.56m November 30, 2006																
	Water level measured in Standpipe at elev. 188.12m Dec. 14, 2006.																
	Water level measured in Standpipe at elev. 187.99m Feb. 26, 2007.																



NOTE

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ACCOMPANYING TEXT.

PROJECT

STRUCTURAL CULVERT - SITE 6-406-C
HIGHWAY 3 WIDENING
GWP 315-98-00

TITLE

KEY PLAN



**Golder
Associates**
LONDON, ONTARIO

PROJECT No. 06-1130-177-0-1

FILE No. 061130177-AA001

CADD WDF Dec. 13/06

CHECK

SCALE AS SHOWN REV. 0

FIGURE 1



SHEET



- 1

No.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100

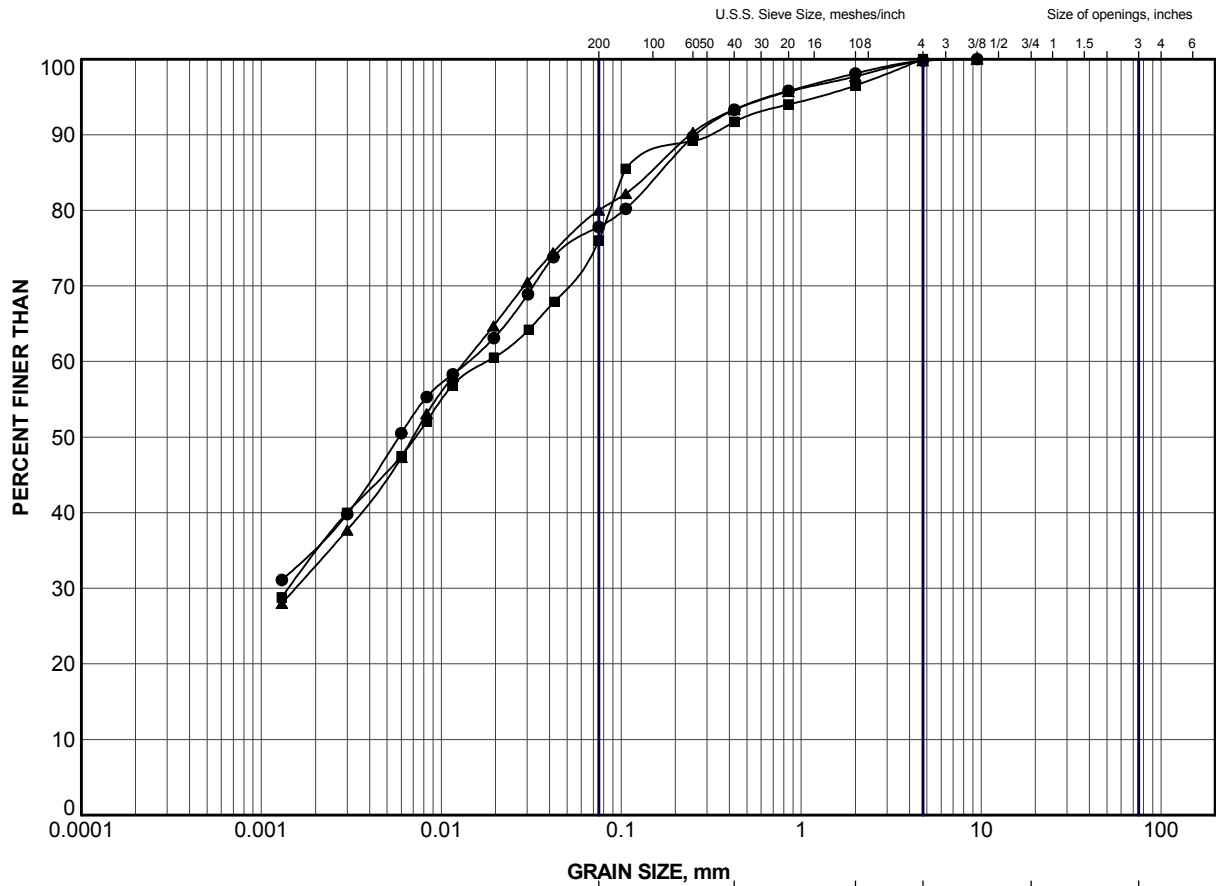
The boundaries between soil strata have been established only at borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

Base plans provided in digital format by DELCAN.



NO.	DATE	BY	REVISION	
Geocres No. 40J2-92				
HWY. 3		PROJECT NO. 06-1130-177-0-1		DIST.
SUBM'D.	DUP	CHKD.	DATE: Dec. 18/06	SITE: 6-406-C
DRAWN:	WDF	CHKD.	APPD.	DWG. 1


APPENDIX A
LABORATORY TEST DATA

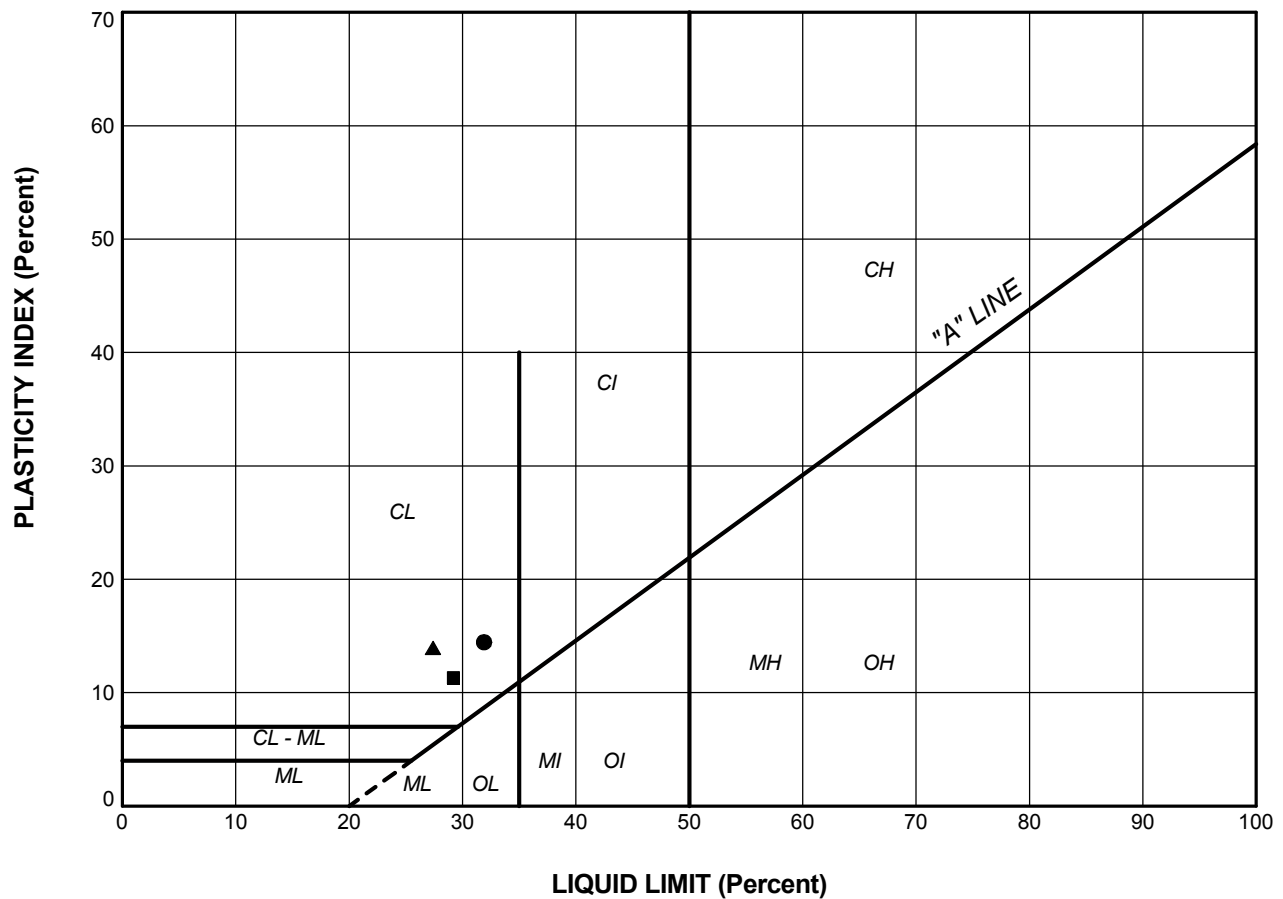


CLAY AND SILT	GRAVEL SIZE, mm						Cobble Size
	fine	medium	coarse	fine	coarse		
	SAND SIZE			GRAVEL SIZE			

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	9	4	185.9
■	9	8	181.4
▲	10	6	184.2

PROJECT					STRUCTURAL CULVERT - SITE 6-406-C HIGHWAY 3 WIDENING GWP 315-98-00				
TITLE					GRAIN SIZE DISTRIBUTION CLAYEY SILT (TILL)				
PROJECT No.		06-1130-177		FILE No.		06-1130-177.GPJ			
DRAWN		WDF		Dec 12/06		SCALE		N/A	
CHECK						REV.			
 Golder Associates LONDON, ONTARIO					FIGURE A-1				



PROJECT				STRUCTURAL CULVERT - SITE 6-406-C HIGHWAY 3 WIDENING GWP 315-98-00			
TITLE				PLASTICITY CHART			
PROJECT No.		06-1130-177		FILE No.		06-1130-177.GPJ	
DRAWN	WDF	Dec 12/06		SCALE	N/A		REV.
CHECK				FIGURE A-2			



APPENDIX B
SITE PHOTOGRAPH

March 2007

06-1130-177-0-1

SITE PHOTOGRAPH



Photo 1: Culvert Site 6-406-C (Station 17+810 SS). View of inlet and area of proposed extension.