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FOUNDATION INVESTIGATION AND DESIGN REPORT
CULVERT REPLACEMENT
STATION 16+427, HIGHWAY 401
TOWNSHIP OF ROCHESTER
HIGHWAY 401 RECONSTRUCTION
GWP 63-00-00, AGREEMENT NO. 3004-E-0006
MINISTRY OF TRANSPORTATION - SOUTHWESTERN REGION
Site 6-478C

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LIST OF ABBREVIATIONS

LIST OF SYMBOLS

RECORDS OF BOREHOLES

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PART A

**FOUNDATION INVESTIGATION REPORT
CULVERT REPLACEMENT
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1.0 INTRODUCTION

Golder Associates Ltd. (Golder Associates) has been retained by Dillon Consulting Limited on behalf of the Ministry of Transportation, Ontario (MTO) to provide foundations engineering services as part of the detail design work for the section of Highway 401 described by GWP 63-00-00. This section of Highway 401 is some 9.9 kilometres in length and extends from 2.5 kilometres east of Essex Road 27 easterly to 1.2 kilometres west of Highway 77 in the Townships of Rochester and Tilbury West, County of Essex, Ontario.

The purpose of this portion of the foundation investigation was to determine the subsurface conditions for the replacement of the culvert located at Station 16+427, Highway 401, Township of Rochester, by drilling boreholes, carrying out in-situ tests and laboratory tests on selected samples. The terms of reference for the scope of work are outlined in the MTO's request for proposal and Golder Associates proposal P41-3106, dated December 24, 2004. The work was carried out in accordance with our Quality Control Plan for Foundation Engineering Detail Design Services dated March 9, 2005.

2.0 SITE DESCRIPTION

2.1 General

GWP 63-00-00 comprises the reconstruction and widening of some 9.9 kilometres of Highway 401 extending from 2.5 kilometres east of Essex Road 27 in the Township of Rochester easterly to 1.2 kilometres west of Highway 77 in the Township of Tilbury West, County of Essex, Ontario. The location of the project is shown on the Key Plan, Figure 1. The project chainage extends from Highway 401 Station 13+000, Township of Rochester to Station 12+700, Township of Tilbury West.

This report addresses the subsurface conditions for the proposed culvert replacement at Station 16+427, Highway 401 in the Township of Rochester. The location of the subject culvert is shown on the Key Plan, Figure 1. The existing culvert is understood to be a non-rigid frame open footing culvert. The culvert has a span of 3.66 metres, a height of 1.52 metres and a length of 88.69 metres.

This section of Highway 401 is currently a four lane divided freeway with a depressed grass median. In each direction, two 3.35 metre wide lanes with 3.58 metre outer shoulders and 4.57 metre wide inner shoulders are present.

The topography in the area of the site is generally flat. The areas outside of the Highway 401 paved surfaces are well vegetated with grasses. The primary land use in the area is agricultural with some residential areas along French Line Road.

2.2 Site Geology

The project lies within the Essex Clay Plain, a subregion of the physiographic region of southern Ontario known as the St. Clair Clay Plains, identified in "The Physiography of Southern Ontario" by Chapman and Putnam (1984). The clay plain is described as a till plain that has been smoothed by shallow deposits of lacustrine clay which settled in the depressions of the till. The prevailing soil type is reportedly the Brookston clay.

Based on the Ontario Department of Mines and Northern Affairs Preliminary Maps P.749 and P.750 entitled "Quaternary Geology of the Windsor-Essex Area" Western and Eastern Parts, respectively, the project area is reportedly located in predominantly clayey silt till.

Based on the available bedrock geology mapping, the subcropping bedrock consists of limestone of the Dundee formation of Middle Devonian age.

3.0 INVESTIGATION PROCEDURES

The field work for this investigation was carried out between July 27 and 29, 2005 at which time three boreholes were drilled at the locations indicated on Drawing 1.

The as-drilled borehole locations, ground surface elevations and borehole depths are as follows:

BOREHOLE	LOCATIONS (m)		GROUND SURFACE	BOREHOLE DEPTH
	Northing	Easting	ELEVATION (m)	
201	4677069	293188	182.83	9.60
202	4677055	293181	183.34	9.60
203	4677135	293184	183.38	9.60

The soil stratigraphy encountered in the boreholes is shown on the attached Record of Borehole sheets and on Drawing 1.

The boreholes were advanced using an all terrain vehicle mounted power auger supplied and operated by a specialist drilling contractor. Samples of the overburden were obtained at suitable intervals of depth using 50 millimetre outside diameter split spoon sampling equipment in accordance with the standard penetration test (SPT) procedures. Groundwater conditions were observed in the boreholes throughout the drilling operations. All of the boreholes were backfilled in accordance with current regulations and MTO recommended procedures.

The field work was supervised on a full-time basis by experienced members of our engineering staff who arranged for underground utility locates, directed the drilling, sampling and in situ testing operations, logged the boreholes and cared for the samples obtained. The soil samples were identified in the field, placed in labeled containers and transported to Golder Associates' London laboratory for further examination and routine testing. Index and classification tests consisting of water content determinations, grain size distribution analyses and Atterberg limits determinations were carried out on selected samples. The results of the field and laboratory testing are given on the Record of Borehole sheets and in Appendix A.

Temporary traffic control was provided in accordance with the Ontario Traffic Manual, Book 7, dated March 2001.

4.0 SUBSURFACE CONDITIONS

4.1 Site Stratigraphy

The detailed subsurface soil and groundwater conditions encountered in the boreholes and the results of the in situ and laboratory testing are provided on the attached Record of Borehole sheets following the text of this report and in Appendix A. The stratigraphic boundaries shown on the Record of Borehole sheets are inferred from non-continuous sampling and observations of drilling resistance and may represent transitions between soil types rather than exact planes of geological change. Further, the subsurface conditions may vary significantly between and beyond the borehole locations.

In summary, the boreholes drilled for the proposed culvert extensions encountered topsoil and fill materials overlying silty clay till.

A detailed description of the subsurface conditions encountered in the boreholes is provided on the Record of Borehole sheets and is summarized in the following sections.

4.1.1 Topsoil and Fill Materials

Topsoil was encountered at ground surface in borehole 201. The topsoil was 0.3 metres thick at the borehole location.

Fill materials were encountered beneath the topsoil in borehole 201 and at ground surface in boreholes 202 and 203. In boreholes 202 and 203, about 0.8 to 2.9 metres of loose to compact granular fill associated with the Highway 401 N/S-W and W-N/S ramp shoulder structures were present. The granular fill materials had N values, as determined in the standard penetration testing, of 6 to 17 blows per 0.3 metres with in situ water contents of 4 to 16 per cent.

In boreholes 201 and 202 at elevation 182.6 metres, stiff clayey fill materials were encountered. The clayey fill materials were 1.1 to 1.5 metres thick. The clayey fill materials had measured N values of 8 to 14 blows per 0.3 metres with in situ water contents of 8 to 25 per cent.

4.1.2 Silty Clay Till

Beneath the fill materials, all of the boreholes encountered stiff to hard silty clay till. The surface of the till was encountered between elevation 180.5 and 181.5 metres. Borehole 203 was terminated in the silty clay till after exploring it for 6.7 metres. Where fully penetrated in boreholes 201 and 202, the silty clay till was 5.6 to 6.2 metres thick. The silty clay till had N values of 9 to 37 blows per 0.3 metres with natural water contents of 10 to 25 per cent and an average of about 20 per cent. The silty clay till had corresponding average plastic and liquid

limits of 18 and 34 per cent, respectively, based on five Atterberg limits determinations indicating an inorganic clay of low to intermediate plasticity.

Grain size distribution curves for samples of the silty clay till recovered from the standard penetration testing are provided on Figure A-1. The Atterberg limits data is provided on Figure A-3.

4.1.3 Silty Clay

Beneath the silty clay till in borehole 201, stiff silty clay was encountered at elevation 175.8 metres. Borehole 201 was terminated in the silty clay after exploring it for about 1.6 metres. The silty clay had measured N values of 10 and 12 blows per 0.3 metres with a natural water content of 26 per cent.

A grain size distribution curve for a sample of the silty clay recovered from the standard penetration testing is provided on Figure A-2.

4.1.4 Clayey Silt Till

Beneath the silty clay till in borehole 202, hard clayey silty till was encountered at elevation 174.8 metres. Borehole 202 was terminated in the clayey silt till after exploring it for about 1.1 metres. The silty clay had a measured N value of 35 blows per 0.3 metres.

4.2 Groundwater Conditions

Groundwater conditions were observed in the boreholes during drilling. Groundwater was encountered in borehole 201 about 4.1 metres below ground surface or at elevation 178.7 metres during drilling on July 27, 2005. The remainder of the boreholes were dry during drilling. This information is summarized below:

<u>BOREHOLE</u>	<u>GROUND SURFACE ELEVATION (m)</u>	<u>ENCOUNTERED GROUNDWATER ELEVATION (m)</u>
201	182.83	178.7
202	183.34	Dry
203	183.38	Dry

Based on the conditions encountered in the boreholes, the long-term groundwater level is


estimated to be at approximately elevation 179 metres. Seasonal variation in groundwater levels should be expected.

The water level in the watercourse was not determined.


5.0 MISCELLANEOUS

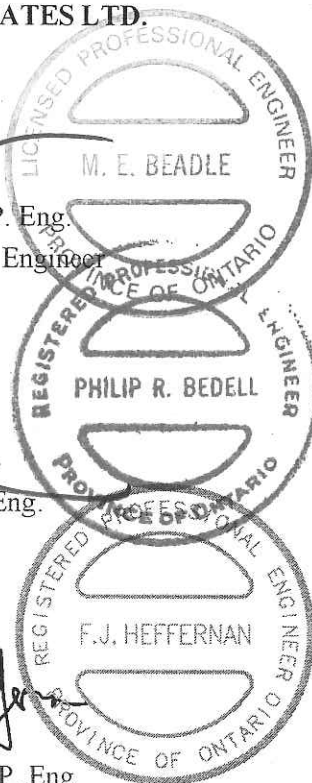
This investigation was carried out using equipment supplied and operated by Lantech Drilling Services Inc., an Ontario Ministry of Environment licensed well contractor. The field operations were supervised by Mr. Michael Arthur and Mr. Dan Babcock under the direction of Mr. David J. Mitchell. The laboratory testing was carried out at Golder Associates' London laboratory under the direction of Mr. Chris M. Sewell. The laboratory is an accredited participant in the MTO Soil and Aggregate Proficiency Program and is certified by the Canadian Council of Independent Laboratories for testing Types C and D aggregates. This report was prepared by Mr. Michael E. Beadle, P. Eng. under the direction of the Project Manager, Mr. Philip R. Bedell, P. Eng. This report was reviewed by Mr. Fintan J. Heffernan, P. Eng., the Designated MTO Contact and Quality Control Auditor for this assignment.

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PART B

**FOUNDATION DESIGN REPORT
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MINISTRY OF TRANSPORTATION – SOUTHWESTERN REGION**

December 2005

05-1130-031-1-3

Golder Associates

6.0 ENGINEERING RECOMMENDATIONS

6.1 General

This section of the report provides our recommendations on the foundation aspects of the design of the proposed culvert replacement at Station 16+427, Highway 401, Township of Rochester. The recommendations are based on our interpretation of the factual information obtained during the investigation. It should be noted that the interpretation and recommendations are intended for use only by the design engineer. Where comments are made on construction they are provided only in order to highlight those aspects which could affect the design of the project. Those requiring information on aspects of construction should make their own interpretation of the factual information provided as it may affect equipment selection, proposed construction methods and scheduling.

The existing culvert is understood to be a non-rigid frame open footing culvert. The existing culvert has a span of 3.66 metres, a height of 1.52 metres and a length of 88.69 metres. It is proposed to replace this culvert with a new, non-rigid frame open footing culvert with a span of 4.00 metres and a height of 1.52 metres.

6.2 Foundations

The subsurface conditions encountered in the boreholes drilled for this investigation typically consist of fill overlying silty clay till.

Based on the information provided, the proposed new culvert will be founded as follows:

<u>PROPOSED DIMENSIONS</u> (m)	<u>MEASURED GROUNDWATER LEVEL</u> (m)	<u>ANTICIPATED FOUNDING ELEVATION</u> (m)	<u>FOUNDING SOIL</u>
4.00 x 1.52	178.7	179.7	Silty clay till

The culvert excavation should penetrate the existing fill materials and be terminated in the very stiff to hard silty clay till at about elevation 179.7 metres. This elevation is about 1 metre above the encountered groundwater level in borehole 201. Assuming a maximum allowable settlement of 25 millimetres and a footing width of 1.0 metres, culvert foundations constructed at this level may be designed using a factored bearing resistance at ultimate limit states (ULS) of 300 kilopascals (kPa) and a geotechnical resistance at serviceability limit states (SLS) of 200 kPa.

Temporary diversion of surface water flow may be required during installation depending on the prevailing weather conditions at the time of construction.

6.2.1 Lateral Earth Pressures for Design

The lateral pressures acting on the proposed culvert will depend on the backfill soils and, where used, the type and method of placement of the backfill materials behind the wall, as well as the subsequent lateral movement of the structure. The following recommendations are made concerning the design of the culvert walls in accordance with the Canadian Highway Bridge Design Code (CHBDC).

Backfill behind the culvert should be carried out as per Ontario Provincial Standard Drawing (OPSD) 802.02. Backfill behind the culvert walls should consist of select, free draining granular fill meeting the requirements of Granular A or Granular B Type III but with less than 5 per cent passing the 0.075 millimetre sieve.

Where backfill soils are placed and compacted behind the walls, a compaction surcharge equal to 12 kPa should be included in the lateral earth pressures for structural design in accordance with Section 6.9.3 of the CHBDC. Compaction equipment should be used in accordance with OPSS 501.06.

Heavy compaction equipment should not be used adjacent to the walls and roof of the culvert sections. The height of backfill adjacent to the culvert walls should be maintained equal on both sides of the structures during all stages of backfill placement.

The granular fill should be placed in a zone with a width equal to at least 1.2 metres behind the culvert walls. For walls backfilled as noted above, the following unfactored parameters may be assumed:

	<u>Granular A</u>	<u>Granular B</u> <u>Type III</u>
Fill unit weight:	22 kN/m ³	21 kN/m ³
Coefficients of lateral earth pressure:		
'active', K_a	0.31	0.27
'at-rest', K_o	0.47	0.43

If the wall support allows lateral yielding (unrestrained structure), active earth pressures may be used in the geotechnical design of the structure. If the culvert wall support does not allow lateral yielding (restrained structure), at-rest pressures should be assumed for geotechnical design. The unfactored coefficient of passive pressure for the portion of the culvert wall and footing below invert may be taken as 3.1. Resistance to sliding may be based on an angle of internal friction of 30 degrees between the concrete footings and the silty clay till subgrade materials.

6.2.2 Construction Considerations

The founding soils are sensitive to disturbance and softening due to water seepage and/or ponding. If cast-in-place culverts are to be constructed, placement of a working slab of lean concrete will be required at the base of the culvert excavations for the footing area. Exposure without protection of the working slab will result in softening of the founding soils. The cleaned base of the excavation should be inspected by qualified geotechnical personnel prior to placing the lean concrete. It is recommended that the footing excavation be carried out such that the final 0.5 metres of excavation is completed with the geotechnical personnel on site and the lean concrete be placed immediately after footing inspection.

Erosion protection for the culvert/wing wall backfill should be provided, as appropriate. Consideration could be given to using suitable non-woven geotextile and rip rap, as required, to the high water level to provide erosion protection based on hydraulic requirements. In addition, sediment control such as silt fences and erosion control blankets may be required during construction as well as diversion of the water to mitigate migration of fine soil particles into the water course.

6.2.3 Excavations and Temporary Cut Slopes

Based on the subsurface conditions encountered in the boreholes, the base of excavations will be above the measured groundwater level. Temporary open cut slopes should be maintained no steeper than 1 horizontal to 1 vertical, but may need to be locally flattened, as required, in the fill materials.

Surficial water seepage into the excavations should be expected at the culvert site, and will be heavier during periods of sustained precipitation. In addition to diverting any culvert flows, pumping from well filtered sumps located at the base of the excavations may be required to provide groundwater control during foundation excavations.

The consideration with respect to protection of the founding soils, however, as given in Section 6.2.2 under the heading Construction Considerations must be recognized. Sumps should be maintained outside of the actual footing limits. Surface water runoff should be directed away from the excavations at all times. The appropriate Non Standard Special Provision (NSSP) should be included in the contract documents to address these issues.

Where space is restricted and will not permit open cuts, a temporary support system should be installed to support the sides of the excavation and permit the use of vertical cuts. The temporary support system could consist of soldier piles and lagging where H-piles would be driven to a suitable depth and horizontal lagging installed as the excavation proceeds. Support to the soldier

pile and lagging wall system could be in the form of struts and walers in the case of footing excavations or rakers and anchors in the case of roadway protection excavations.

The design of braced soldier pile and lagging walls should be based on a rectangular earth pressure distribution using the design parameters given below. Where the support to the wall is provided by anchors or rakers, the wall design should be based on a triangular earth pressure distribution using the design parameters given below. The raker/anchor support must be designed to accommodate the loads applied from earth pressures and surcharge pressures from area, line or point loads as well as the impact of sloping ground behind the system.

Unfactored triangular earth pressure distribution (p in kN/m^2 ; increasing with depth), can be calculated as follows:

$$p = K_a \gamma H$$

where H = the height of the excavation at any point in metres
 K_a = 0.3 for level ground behind excavation
 γ = soil unit weight = 20 kN/m^3

Unfactored rectangular earth pressure distribution (p in kN/m^2 ; constant with depth), can be calculated as follows:

$$p = K \gamma H$$

where H = the height of the excavation
 K = 0.3 for level ground behind excavation
 γ = soil unit weight = 20 kN/m^3

Passive toe restraint to the soldier piles may be determined using a triangular pressure distribution acting over an equivalent width equal to three times the pile socket diameter. The coefficient of passive lateral earth pressure, K_p , for the socket within the very stiff to hard silty clay till may be taken as 3.5. The soil unit weight should be taken as 20 kN/m^3 and the unit weight of water should be taken as 9.8 kN/m^3 .

The temporary excavation support system should be designed and constructed in accordance with MTO Special Provision 539S01. The lateral movement of the temporary shoring system should meet Performance Level 2 as specified in SP 539S01.


6.3 Inlet and Outlet Seals and Camber

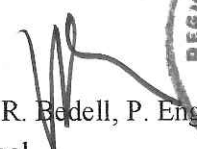
Based on our understanding that an open footing culvert will be constructed, inlet and outlet seals are not considered necessary. Based on the anticipated height of fill over the culvert, a camber is not necessary.

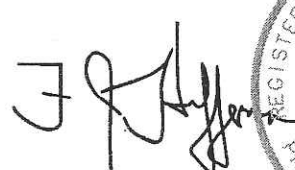
7.0 MISCELLANEOUS

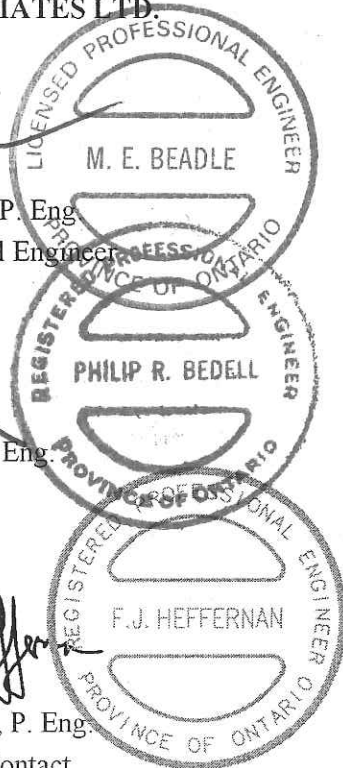
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LIST OF ABBREVIATIONS

The abbreviations commonly employed on each "Record of Borehole", on the figures and in the text of the report, are as follows:

I. SAMPLE TYPES

<i>AS</i>	auger sample
<i>CS</i>	chunk sample
<i>DO</i>	drive open
<i>DS</i>	Denison type sample
<i>FS</i>	foil sample
<i>RC</i>	rock core
<i>SC</i>	soil core
<i>ST</i>	slotted tube
<i>TO</i>	thin-walled, open
<i>TP</i>	thin-walled, piston
<i>WS</i>	wash sample
<i>SS</i>	split spoon

II. PENETRATION RESISTANCES

Dynamic Penetration Resistance:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 0.3 m (12 in.).

Standard Penetration Resistance, N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) drive open sampler for a distance of 0.3 m (12 in.).

<i>WH</i>	sampler advanced by static weight-weight, hammer
<i>PH</i>	sampler advanced by hydraulic force
<i>PM</i>	sampler advanced by manual force

III. SOIL DESCRIPTION

(a) Cohesionless Soils

Relative Density	"N" Blows/0.3 m or Blow/ft.
Very loose	0 to 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very dense	over 50

(b) Cohesive Soils

Consistency	<u>kPa</u>	"Cu" = "Su" <u>psf.</u>
Very soft	0 to 12	0 to 250
Soft	12 to 25	250 to 500
Firm	25 to 50	500 to 1000
Stiff	50 to 100	1000 to 2000
Very stiff	100 to 200	2000 to 4000
Hard	over 200	over 4000

IV. SOIL TESTS

<i>C</i>	consolidation test
<i>H</i>	hydrometer analysis
<i>M</i>	sieve analysis
<i>MH</i>	combined analysis, sieve and hydrometer ¹
<i>Q</i>	undrained triaxial ²
<i>R</i>	consolidated undrained triaxial ²
<i>S</i>	drained triaxial
<i>U</i>	unconfined compression
<i>V</i>	field vane test
<i>Chem</i>	chemical analysis

NOTES:

1. Combined analyses when 5 to 95 per cent of the material passes the No. 200 sieve.
2. Undrained triaxial tests in which pore pressures are measured are shown as Q or R.

LIST OF SYMBOLS

I. GENERAL

π	= 3.1416
e	= base of natural logarithms 2.7183
$\log_e a$ or $\ln a$	natural logarithm of a
$\log_{10} a$ or $\log a$	logarithm of a to base 10
t	time
g	acceleration due to gravity
V	volume
W	weight
m	mass
M	moment
F	factor of safety

II. STRESS AND STRAIN

u	pore pressure
σ	normal stress
σ'	normal effective stress (σ is also used)
τ	shear stress
ε	linear strain
ε_{sy}	shear strain
ν	Poisson's ration (μ is also used)
E	modulus of linear deformation (Young's modulus)
G	modulus of shear deformation
K	modulus of compressibility
η	coefficient of viscosity

III. SOIL PROPERTIES

(a) Unit weight

γ	unit weight of soil (bulk density)
γ_s	unit weight of solid particles
γ_w	unit weight of water
γ_d	unit dry weight of soil (dry density)
γ'	unit weight of submerged soil
G_s	specific gravity of solid particles $G_s = \gamma_s/\gamma_w$
e	void ratio
n	porosity
w	water content
S_r	degree of saturation

(b) Consistency

w_L	liquid limit
w_p	plastic limit
I_p	plasticity index
w_s	shrinkage limit
I_L	liquidity index = $(w - w_p)/I_p$
I_C	consistency index = $(w_L - w)/I_p$
e_{max}	void ratio in loosest state
e_{min}	void ratio in densest state
D_r	relative density = $(e_{max} - e)/(e_{max} - e_{min})$

(c) Permeability

h	hydraulic head or potential
q	rate of discharge
v	velocity of flow
i	hydraulic gradient
κ	coefficient of permeability
j	seepage force per unit volume

(d) Consolidation (one-dimensional)

m_v	coefficient of volume change = $-\Delta e/(1+e)\Delta\sigma'$
C_c	compression index = $-\Delta e/\Delta\log_{10}\sigma'$
c_v	coefficient of consolidation
T_F	time factor = $c_v t/d^2$ (d , drainage path)
U	degree of consolidation

(e) Shear strength

τ_f	shear strength	$\left. \begin{array}{l} \text{in terms} \\ \text{of effective} \\ \text{stress} \end{array} \right\} \tau_f = c' + \sigma' \tan \phi$
c'	effective cohesion intercept	
ϕ'	effective angle of shearing resistance, or friction	
S_u	apparent cohesion*	$\left. \begin{array}{l} \text{in terms of} \\ \text{total stress} \end{array} \right\} \tau_f = cu + \sigma \tan \phi_u$
ϕ_u	apparent angle of shearing resistance, or friction	
μ	coefficient of friction	
S_t	sensitivity	

*For the case of a saturated cohesive soil, $\phi_u = 0$ and the undrained shear strength $\tau_f = S_u$ is taken as half the undrained compressive strength.

PROJECT 05-1130-031-1-3

RECORD OF BOREHOLE No 201

1 OF 1

METRIC

G.W.P. 63-00-00

LOCATION N 4677069.4 ; E 293188.0

ORIGINATED BY D.B

DIST 1

HWY 401




BOREHOLE TYPE POWER AUGER/HOLLOW STEM AUGERS

COMPILED BY DCH/WDF

DATUM GEODETIC

DATE July 27, 2005

CHECKED BY *[Signature]*

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT			PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa								WATER CONTENT (%)		
								○ UNCONFINED + FIELD VANE										
								● QUICK TRIAXIAL × LAB VANE										
182.83	GROUND SURFACE					20	40	60	80	100	10	20	30					
0.00	TOPSOIL, clayey																	
182.56	Black																	
0.27	FILL, silty clay, trace sand, trace gravel		1	SS	14													
181.46	Stiff Grey																	
1.37	SILTY CLAY, trace sand, trace gravel (TILL)		2	SS	15													
	Very Stiff to Hard																	
	Brown to grey at elev. 179.2m		3	SS	22													
			4	SS	37													
			5	SS	31													
			6	SS	22													
		7	SS	13														
		8	SS	18														
175.82	SILTY CLAY, trace sand		9	SS	12													
7.01	Stiff Grey																	
			10	SS	10													
173.23	END OF BOREHOLE																	
9.60	Groundwater encountered during drilling at elev. 178.73 July 27, 2005																	

ON MTO 05-1130-031-1-3.GPJ ON MOT.GDT 12/14/05

PROJECT 05-1130-031-1-3

RECORD OF BOREHOLE No 202

1 OF 1

METRIC

G.W.P. 63-00-00

LOCATION N 4677054.8 ; E 293180.7

ORIGINATED BY D.B

DIST 1 HWY 401

BOREHOLE TYPE POWER AUGER/HOLLOW STEM AUGERS

COMPILED BY DCH/WDF

DATUM GEODETIC

DATE July 29, 2005

CHECKED BY *WY*

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT			PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa							
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE	WATER CONTENT (%)						
183.34	GROUND SURFACE						20 40 60 80 100								
0.00 183.04	FILL, sand and gravel, crushed														
0.30 182.58	FILL, sand and gravel, some silt														
0.76	FILL, silty clay, some sand, trace gravel Stiff Brown and grey		1	SS	9										
			2	SS	8										
181.05 2.29	SILTY CLAY, trace sand, trace gravel (TILL) Stiff to Very Stiff Brown to grey at elev. 178.9m		3	SS	9										
			4	SS	17										
			5	SS	24										
			6	SS	15										
			7	SS	12										
			8	SS	9										
			9	SS	12										
174.81 8.53	CLAYEY SILT, trace sand, trace gravel (TILL) Hard Grey														
173.74 9.60	END OF BOREHOLE		10	SS	35										
	Borehole dry during drilling July 29, 2005														

ON_MTO 05-1130-031-1-3.GPJ ON_MOT.GDT 12/14/05

RECORD OF BOREHOLE No 203

1 OF 1

METRIC

PROJECT 05-1130-031-1-3

G.W.P. 63-00-00

LOCATION N 4677134.6 E 293183.7

ORIGINATED BY D.B

DIST 1 HWY 401

BOREHOLE TYPE POWER AUGER/HOLLOW STEM AUGERS

COMPILED BY DCH/WDF

DATUM GEODETIC

DATE July 29, 2005

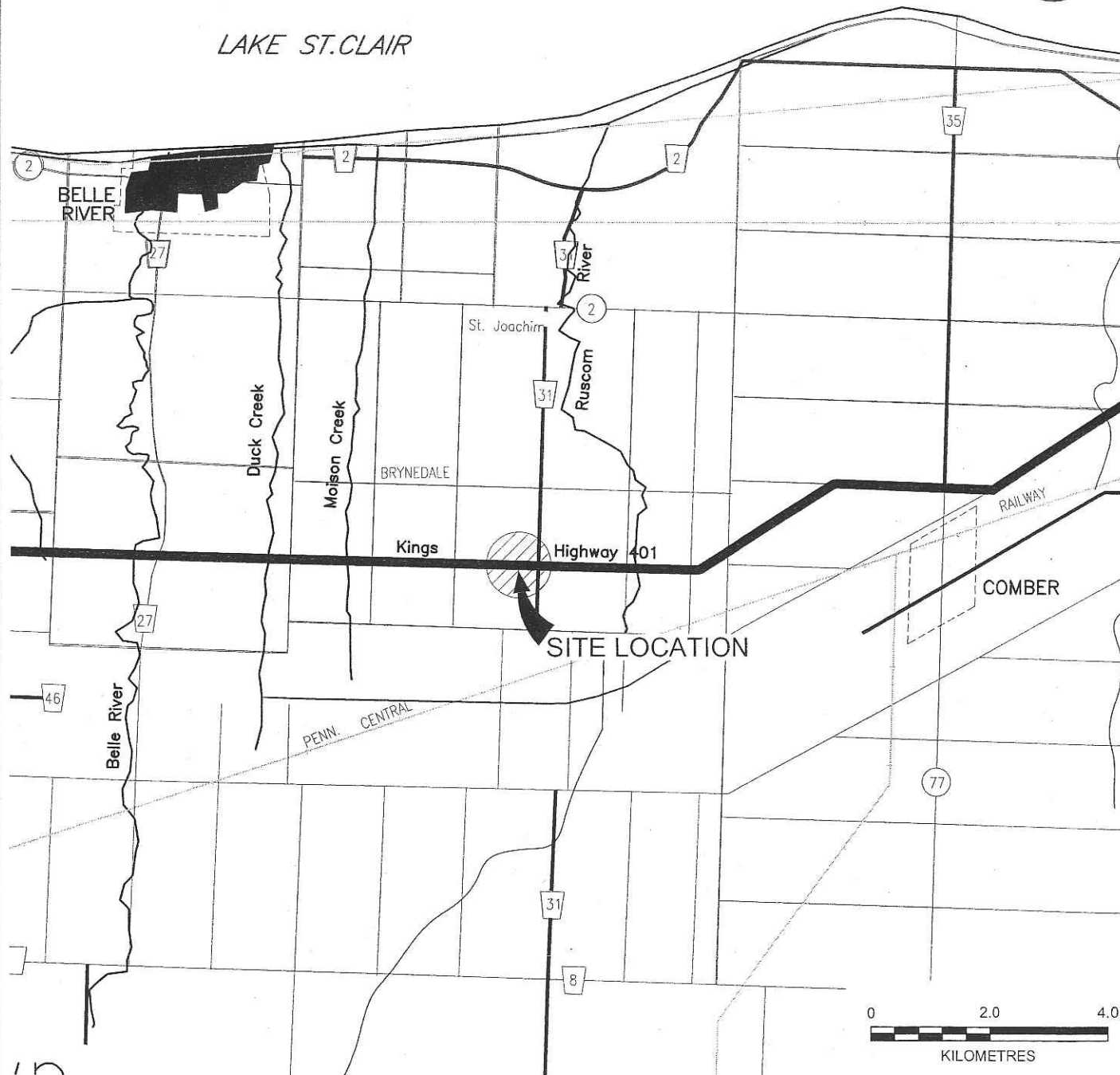
CHECKED BY *[Signature]*


SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT			PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa								WATER CONTENT (%)		
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE										
								20 40 60 80 100	20 40 60 80 100	10 20 30								
183.38	GROUND SURFACE																	
0.00 183.08	FILL, sand and gravel, crushed																	
0.30	FILL, sand and gravel, some silt Compact Brown						183											
182.31			1	SS	17													
1.07	FILL, silty clay, some sand, trace gravel Very Stiff Brown						182											
1.21	FILL, sand and gravel, some silt Compact to Loose Brown		2	SS	10													
			3	SS	6		181											
180.48																		
2.90	SILTY CLAY, trace sand, trace gravel (TILL), trace organic material at 5.5m depth Stiff to Very Stiff Brown to grey at elev. 177.2m		4	SS	13		180											
			5	SS	25									0 19 44 37				
			6	SS	21		179											
			7	SS	10		178											
			8	SS	10		177											
			9	SS	9		176							9 12 37 42				
			10	SS	11													
							175											
173.78			11	SS	17		174											
9.60	END OF BOREHOLE																	
	Borehole dry during drilling July 29, 2005																	

ON_MTO 05-1130-031-1-3.GPJ ON_MOT.GDT 12/14/05



LAKE ST. CLAIR



PROJECT		CULVERT REPLACEMENT STA. 16+427, TWP ROCHESTER GWP 63-00-00, HIGHWAY 401	
TITLE		KEY PLAN	
PROJECT No. 05-1130-031-1-3		FILE No. 051130031-1-16+427-F001	
CADD	WOF/DCH	Sept. 08/05	SCALE AS SHOWN REV. 0
CHECK	<i>[Signature]</i>		
 Golder Associates LONDON, ONTARIO		FIGURE 1	

METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES
UNLESS OTHERWISE SHOWN

DIST HWY. 401
CONT. No. 2005-3046
WP No. 63-00-10



HIGHWAY 401
STATION 16+427
ROCHESTER TOWNSHIP
BOREHOLE LOCATIONS & SOIL STRATA

SHEET



Golder Associates Ltd.
LONDON, ONTARIO, CANADA



KEY PLAN
0 2,000 4,000m
METRES

LEGEND

- Borehole
- N Blows/0.3m (Std. Pen. Test, 475 j/blow)
- ≡ WL encountered during drilling
- DRY Borehole dry during drilling

No.	ELEVATION (metres)	CO-ORDINATES	
		NORTH	EAST
201	182.83	4 677 069.4	293 188.0
202	183.34	4 677 054.8	293 180.7
203	183.38	4 677 134.6	293 183.7

NOTES

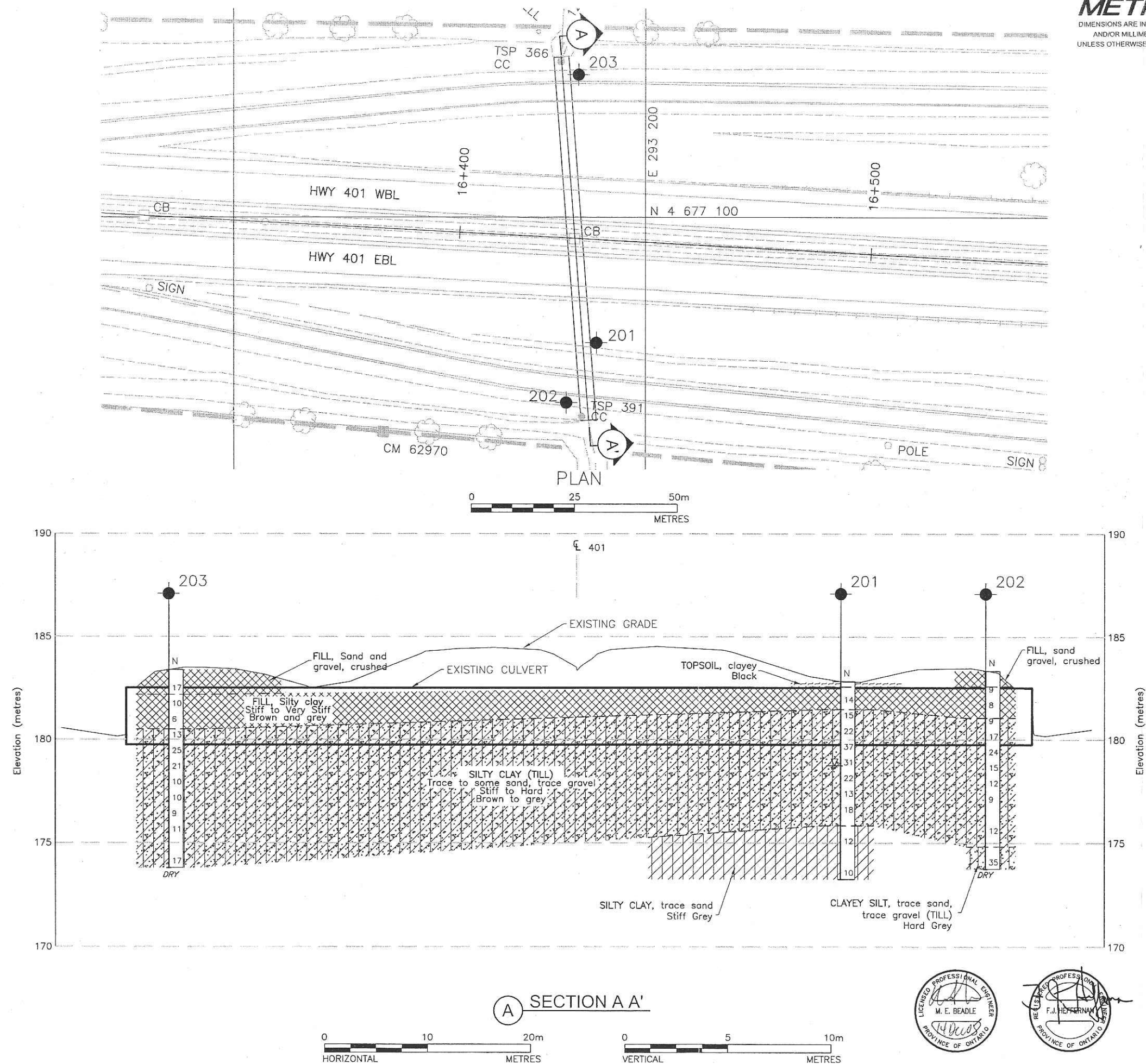
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

This drawing is for subsurface information only. The proposed structure details are shown for illustration purposes only and may not be consistent with the final design configuration as shown elsewhere in the Contract Documents.

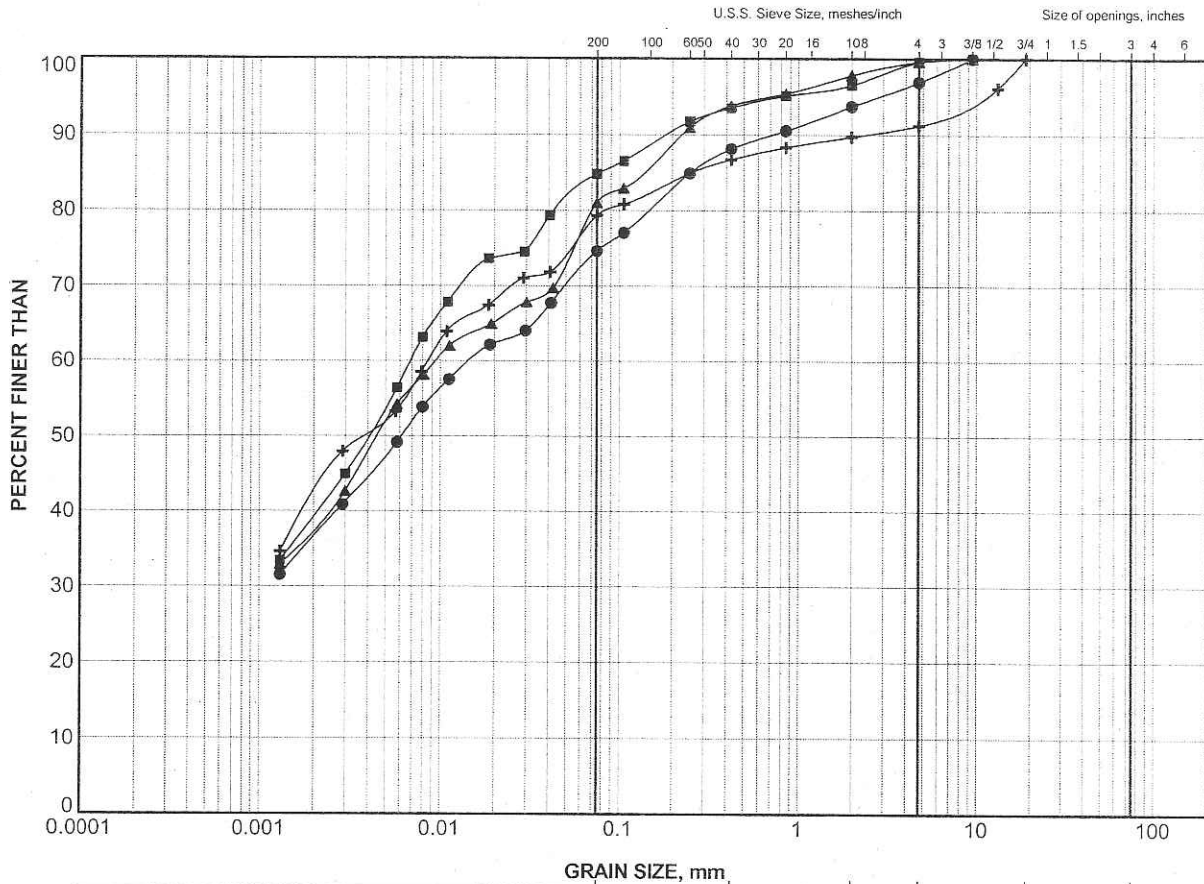
REFERENCE

REFERENCE : BASE DRAWINGS SUPPLIED BY DILLON CONSULTING

NO.	DATE	BY	REVISION
Geocres No.	40J2-68		
HWY. No.	401	PROJECT NO:	051-130031-1-3
SUBM'D.	—	CHKD:	DATE: Sept. 08/05
DRAWN:	DCH	CHKD:	APPD.
			DWG. 1



APPENDIX A
LABORATORY TEST DATA (FIGURES A-1 TO A-3)



CLAY AND SILT						Cobble Size
	fine	medium	coarse	fine	coarse	
	SAND SIZE			GRAVEL SIZE		

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	201	5	178.8
■	202	6	178.5
▲	203	5	179.3
+	203	9	176.3

PROJECT

CULVERT REPLACEMENT STA. 16+427,
TWP ROCHESTER
GWP 63-00-00, HIGHWAY 401

TITLE

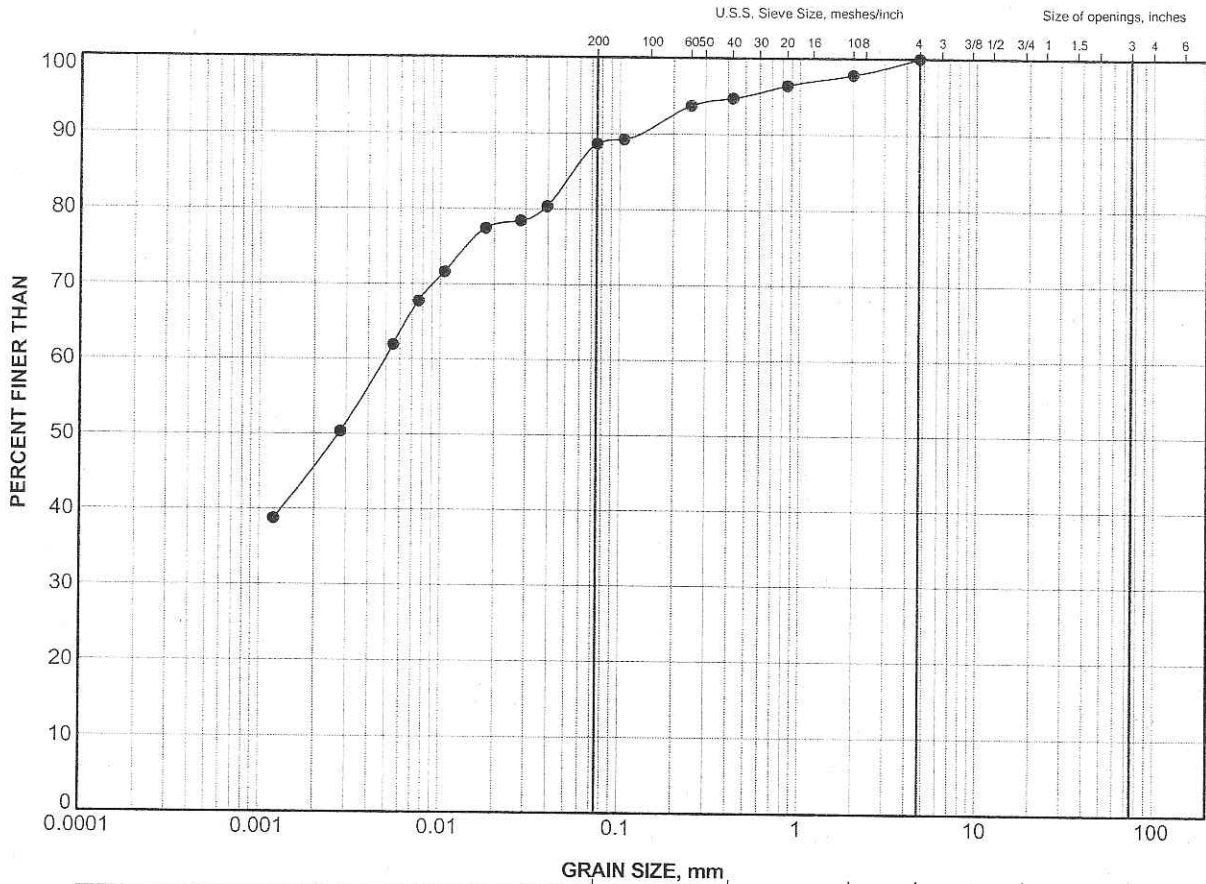
GRAIN SIZE DISTRIBUTION SILTY CLAY TILL



**Golder
Associates**
LONDON, ONTARIO

PROJECT No.	05-1130-031-1-3	FILE No.	05-1130-031-1-3.GPJ
DRAWN	WDF	Sep 08/05	SCALE N/A REV.
CHECK			


FIGURE A-1



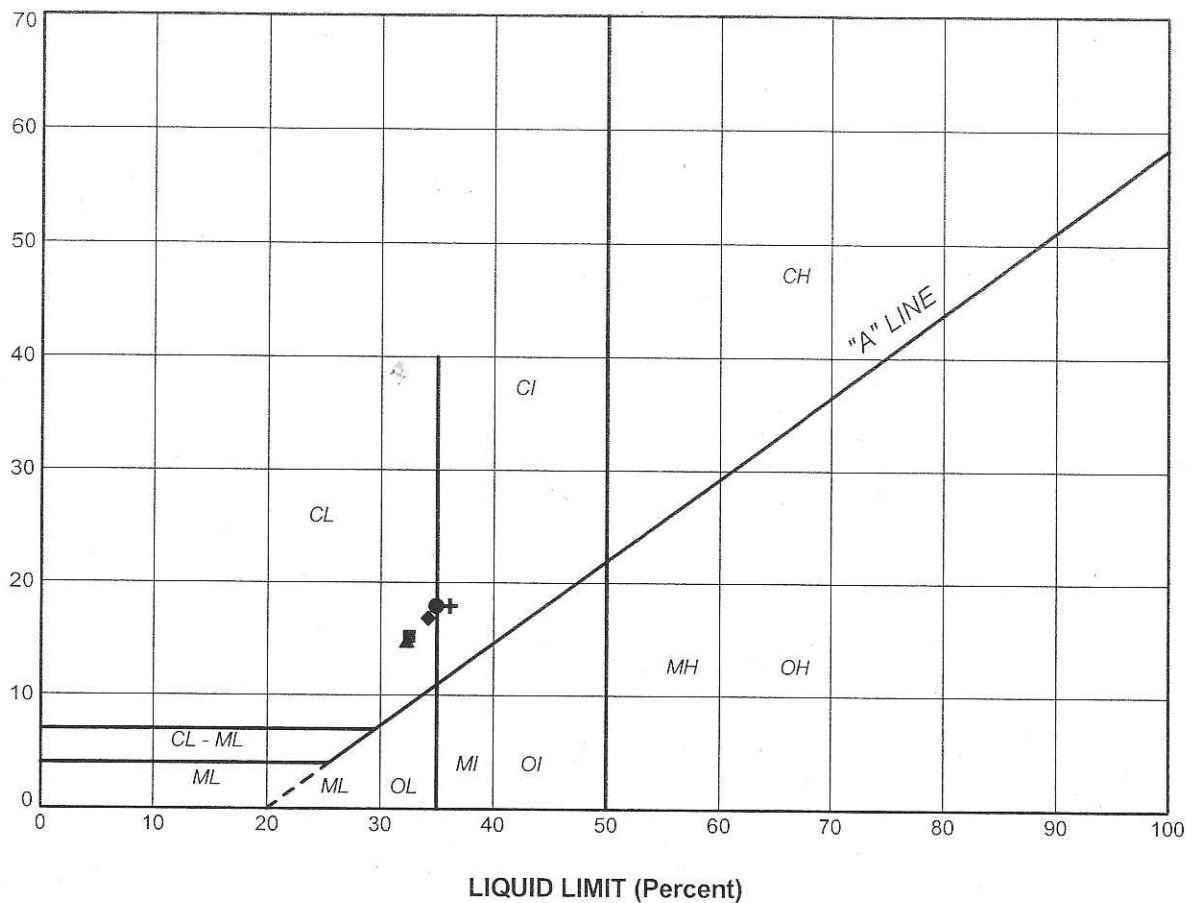
CLAY AND SILT	SAND SIZE, mm					Cobble Size
	fine	medium	coarse	fine	coarse	
	SAND SIZE			GRAVEL SIZE		

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	201	9	175.0

PROJECT		CULVERT REPLACEMENT STA. 16+427, TWP ROCHESTER GWP 63-00-00, HIGHWAY 401			
TITLE		GRAIN SIZE DISTRIBUTION SILTY CLAY			
PROJECT No.		05-1130-031-1-3		FILE No. 05-1130-031-1-3.GPJ	
DRAWN		WDF		Sep 08/05	
CHECK		[Signature]		SCALE N/A REV.	
 Golder Associates LONDON, ONTARIO		FIGURE A-2			

PLASTICITY INDEX (Percent)




SOIL TYPE
C = Clay
M = Silt
O = Organic

PLASTICITY
L = Low
I = Intermediate
H = High

LEGEND

SYMBOL	BOREHOLE	SAMPLE	LL(%)	PL(%)	PI
●	201	5	34.9	17.0	17.9
■	202	6	32.5	17.3	15.2
▲	202	8	32.3	17.5	14.8
+	203	5	36.1	18.2	17.9
◆	203	9	34.2	17.4	16.8

PROJECT				CULVERT REPLACEMENT STA. 16+427, TWP ROCHESTER GWP 63-00-00, HIGHWAY 401			
TITLE							
PLASTICITY CHART							
PROJECT No.		05-1130-031-1-3		FILE No.		05-1130-031-1-3.GPJ	
DRAWN		WDF		SCALE		N/A	
CHECK		WDF		REV			
		Sep 08/05					
 Golder Associates LONDON, ONTARIO				FIGURE A-3			