

54F206

Our References:
CL 754, PL 22
August 6th, 1974.

SOIL INVESTIGATION REPORT
for
REASSESSMENT OF ELDREDGE, MASS.
(K.W. TILLON, Consulting Engineer.)
by
JOHN W. MCKEEAN, P.E.

As requested we carried out a soil investigation on the proposed bridge site, Dartmouth 3 and 4.

HISTORY OF INVESTIGATION

Nine penetration tests were performed at the locations shown on the location sketch. The results of these tests are shown on the accompanying sketch and are summarized as follows:

SUMMARY

Hole No.	Ground Surface	Water Level	S E V A R T I C H E S	
			2,000 lbs per cu. ft.	4,000 lbs per cu. ft. Natural
573	666	671	671	662
573	666	671	662	653
574	666	670	669	655
575	666	671	671	650
576	666	673	673	656
576	666	672	672	658
574	666	672	670	661
573	666	671	671	664
575	666	670	669	656

The major borings were performed at holes #3 and #4, with the following results:

August 6th, 1961.

Hole No. 1
Ground Surface Elevation - 871

0'	to	2'	Loamy very fine sand
2'	to	5'	Loamy very fine sand (Slightly Stony)
5'	to	5'	Loose brown gravel
5'	to	13'	Silty very fine saturated sand

Hole No. 6
Ground Surface Elevation - 871

0'	to	2'	Loamy very fine sand
2'	to	4'	Slightly loamy fine sand and gravel
4'	to	9'	Clean fine sand and gravel
9'	to	11' 6"	Silty very fine wet sand
11' 6"	to	22'	Silty very fine saturated sand

DISCUSSION

An examination of the percussion test summary table indicates that a safe bearing value of 4000 paf is reached in eight of the nine holes above elevation 869. An examination of percussion test diagram A, however, indicates that a safe bearing value of 4000 paf is not reached until elevation 861, which is five feet below the water table. A bearing value of 4000 paf is reached in hole K at elevation 871, but, unlike the other eight holes, the bearing value decreases to 2000 paf at elevation 863.

CONCLUSION

The peculiar result of test K, coupled with the fact that there is abundant evidence of springs to the west of this site, leads to the belief that basing footings at elevation 861 would probably prove difficult. Excavation below the water table in such sandy material would require shoring; furthermore, there is strong likelihood that boiling would occur in the bottom of such an excavation, leading to difficulties in basing concrete on stable soil. There is also the possibility of undermining of the Southeast portion of this embankment.

W.H. Wilson
W. H. Wilson, P. Eng.

54F-206C



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TORONTO 7, ONT.

Our Reference:
OP 7554 PC 311
August 18, 1954.

SOIL INVESTIGATION REPORT
for
DEPARTMENT OF HIGHWAYS, ONTARIO
(M. M. Dillon, Consulting Engineers.)
at
HIGHWAY 401, DORCHESTER #3 & #4.

As requested we carried out a further soil investigation at the proposed bridge site Dorchester #3 and #4 on Highway 401.

REPORT OF INVESTIGATION

No additional percussion tests were performed at the locations shown on the location sketch. The results of these tests are shown on the accompanying diagrams and are summarized as follows:

S U M M A R Y

E L E V A T I O N S

Hole No:	Ground Surface	2,000 lbs per sq. ft.	4,000 lbs per sq. ft.	Refusal	No. of Blows
10	876	873	872	861	200 for 5"
11	876	871	871	857	200 for 8"

DISCUSSION

The good results obtained on these two holes indicated that the condition encountered at Hole #4 of PC 299 must be of a local nature. It would appear that the footings could be designed as per the report PC 299. Though particular care should be exercised when excavating in the vicinity of Hole #4.

Further work done under PC 300 at the site of the proposed culvert at Dorchester #5 shows that the water found at elevation 866 on this site does not flow to the lower ground owing to a layer of impervious material.



August 18, 1954.

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However, this condition may be changed by work at the culvert site allowing the water to escape. Care should be exercised to see that any major change in the ground water conditions does not have a deleterious effect on either of the structures or on the approach hills.

The use of sand drains might be considered to ensure that consolidation due to the changed conditions would take place rapidly.

WHT Wilson
W. H. T. Wilson, P. Eng.

LOCATION SKETCH

#3

#9

#6

E HWY #2
401

E AVON RD.

#7

#5

#1

#8

#4

#10

21'-6"

51'

111'

PERCUSSION TEST
AUGER BORINGS



PC 299
PC 311

TELETYPE CORPORATION OF CANADA LTD

FAX

DEPARTMENT OF HIGHWAYS, OTTAWA

et

HIGHWAY 111, DIVISION ONE #2 & #4

FRANKI
COMPRESSED PILE COMPANY
OF CANADA LIMITED

PERCUSSION TEST DIAGRAM

FOR: DEPARTMENT OF HIGHWAYS

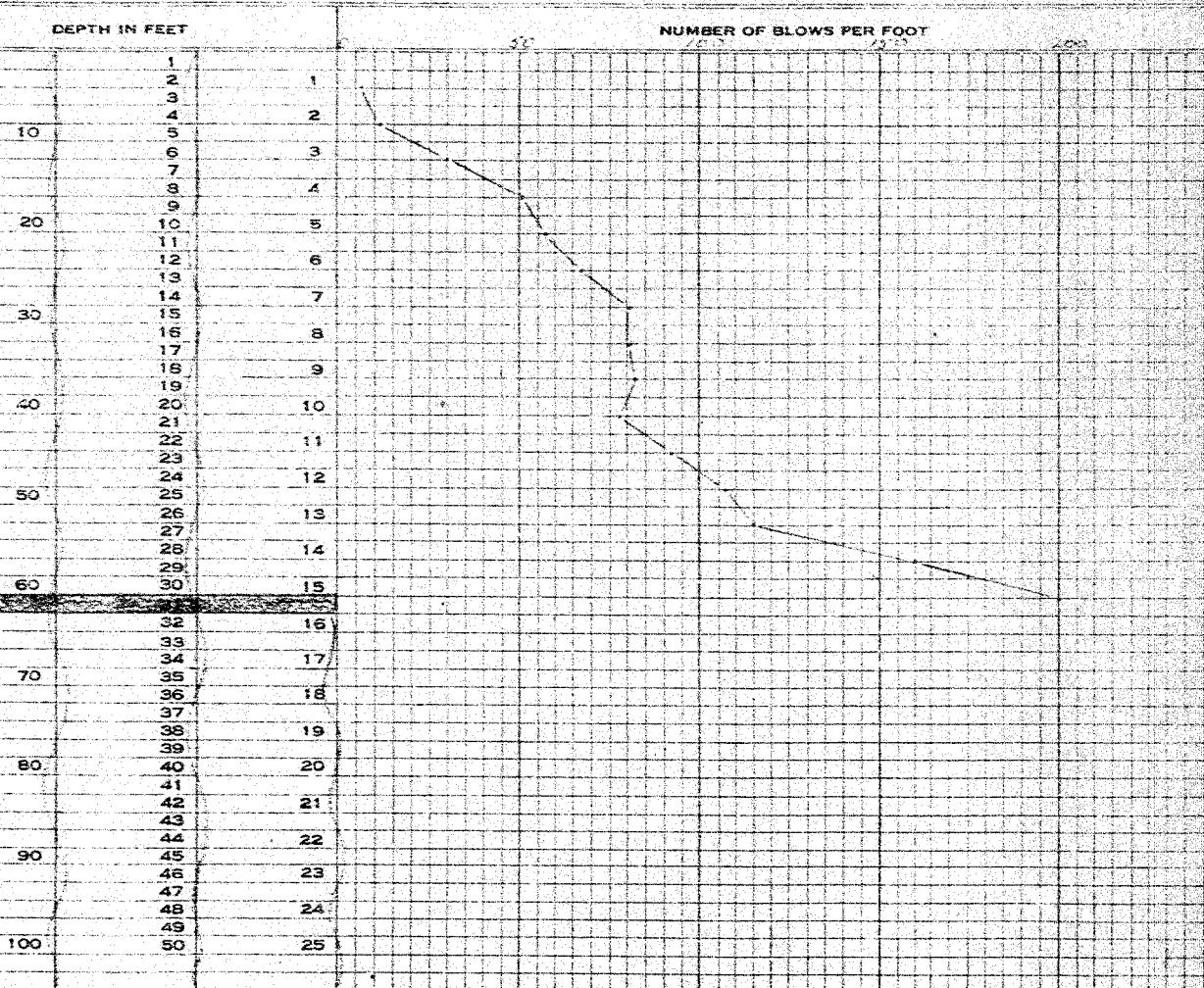
AT: HIGHWAY 401, DORCHESTER #3 & #4.

DATE: AUGUST 14, 1954.

JOB NO. FC 311

TEST NO.: 10

WEIGHT OF HAMMER 225# DROP 3 FT.



Ground Surface Elevation 876
 Refusal Elevation 861
 Number of Blows 200 for 5"

SIGNED

WPT White

FRANKI

COMPRESSED PILE COMPANY
OF CANADA LIMITED

PERCUSSION TEST DIAGRAM

FOR: DEPARTMENT OF HIGHWAYS

AT: HIGHWAY 101, DORCHESTER #3 & #4.

JOB NO. FC 311

DATE: AUGUST 13, 1954.

TEST NO.: 11

WEIGHT OF HAMMER 325#

DROP: 3 FT.

DEPTH IN FEET

NUMBER OF BLOWS PER FOOT

1	1
2	2
3	3
4	2
5	3
6	4
7	5
8	6
9	7
10	8
11	9
12	10
13	11
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95	93
96	94
97	95
98	96
99	97
100	98

G & T LTD. Ground Surface Elevation 876
 Refusal Elevation 857
 Number of Blows 200 for 8"

SIGNED

W.H. White

54-F-206C
Hwy. #401
DORCHESTER #3+4

