

#58-F-225-C

W.P.#929-58

OTTAWA,

QUEENSWAY &

WOODROFFE AVE.

BA 771
58F 225C

DE LEUW, CATHER & COMPANY
OF CANADA LIMITED
CONSULTING ENGINEERS
TORONTO OTTAWA

226 SPARKS STREET
OTTAWA 4, ONTARIO
CENTRAL 3-9663

July 29, 1958.

Mr. F.I. Hewson,
Consultant Liaison Engineer,
Bridge Design Office,
Department of Highways,
280 Davenport Road,
Toronto 2, Ontario.

Dear Sir:

Re: Bridge No. **2** at Woodroffe Ave. W.P. No. 929-58
Queensway Ottawa, District 9

We enclose herewith 3 copies of McRostie and
Associates soils foundation report No. SF347 for the above
structure.

Yours truly,

DE LEUW, CATHER & CO. OF CANADA LIMITED.

Leon J. Marshall

Leon J. Marshall, P. Eng.,
Senior Structural Engineer.

LJM/pr

MCROSTIE & ASSOCIATES

CONSULTING ENGINEERS AND SURVEYORS

OTTAWA 1
CANADA

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FOUNDATION REPORT BRIDGE NO.21. Field Work

Six boreholes were completed on the site in the locations shown on plate one. Two inch split barrel samples were recovered for visual classification and standard penetration tests were performed in the boreholes. Two inch thin walled tube samples were taken in the cohesive soil layers and small penetrometer tests were made. The underlying limestone was diamond drilled, cores recovered for inspection and a record kept of core recovery percentages. A careful watch was kept for drops or discontinuities and for loss of water during drilling operations.

Groundwater levels were observed during the programme.

2. Observations

The surface elevations at boreholes are uniform except that those at the Northerly limit of the Queensway are from 1 to 2 feet lower. There is a uniform depth of 1 foot of top soil over the site. The limestone occurs at approximately elevation 256 at all boreholes except in the case of borehole No.4 at the Southwest corner of the site where it is 5 feet lower. Below the top soil stiff to hard silty fissured clay $1\frac{1}{2}$ to $5\frac{1}{2}$ feet in depth is found. Below the clay and overlying the limestone there is from 4 to 10 feet of loose to dense till. The underlying rock is shaly limestone of the Ottawa formation. In two of the boreholes, namely numbers 2 and 6, a drop of about one inch was noted. The bedding thickness of the shaly limestone averaged from 2 to 4 inches in thickness.

At boreholes 1 and 5 at the West side of Woodroffe Avenue there was no overnight groundwater whereas at the remaining boreholes the ground water was approximately 11 feet below the ground surface. This level can be considered as near the seasonal high except for seasons of heavy precipitation during which the level would be nearer to the ground surface.

3. Recommendations

3.1 Foundation Type

With rock nine feet from the surface at hole No.1 (Northwest corner) and only thin layers of dense soils of above the rock, it appears that the structure must be on rock at this corner. For any but special structures and certainly for rigid frame structures, if one corner is supported on rock the remainder of the structure should be also on rock. For these reasons it would appear that piers or short piles to rock are the most likely suitable types of foundation for the proposed structure.

3.2 Rock and Soil Strengths

Bearing capacities can be assigned to the materials encountered as below:

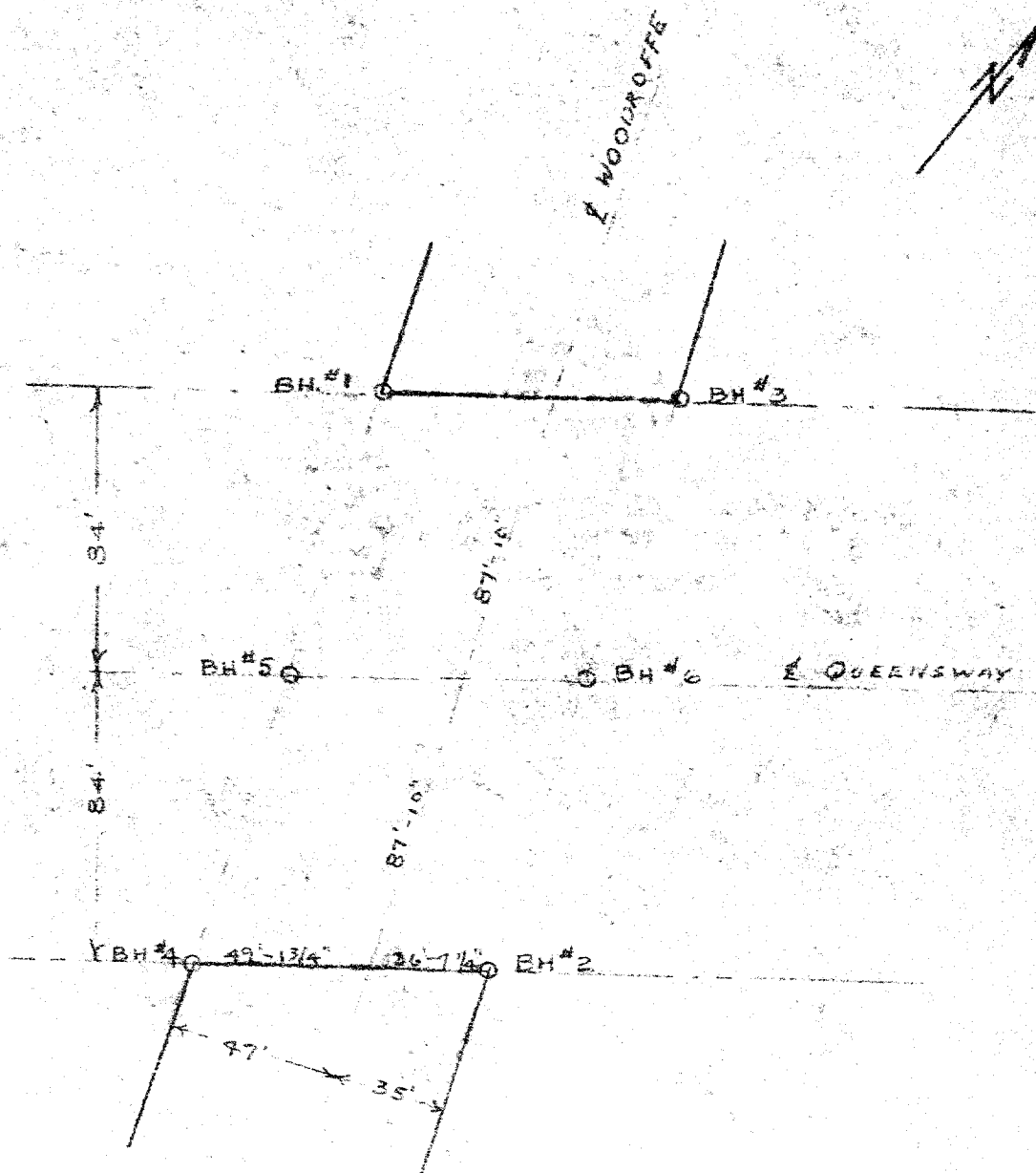
Till below elevation 238	-	4,000 pounds per sq. ft. except at hole six. Also see recommendations concerning foundation type.
Rock at elevation 236 to elevation 231	-	30,000 pounds per sq. ft. but confirmation of lack of seams or drops required.

4. Construction Precautions

The occurrence of drops or discontinuities in the drilling program indicates the possibility of the existence of seams of unconsolidated material imbedded between layers of shaly limestone. If these are 5 feet or more below the footing levels, they are not a cause

for concern.

A program of construction drilling to check that drops do not occur in the first five feet below heavily loaded piers or columns should be considered. If drops are detected a decision to grout the possible open spaces in these drops or to lower the excavation below them can be made pending upon the conditions existing at the time.



McROSTIE & ASSOCIATES
CONSULTING ENGINEERS

BOREHOLE LOCATIONS
QUEENSWAY AT WOODROFFE
BRIDGE No 2

SCALE 1" = 50'

PLATE 1

See SK-II-36

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SOIL PROFILE AND SUMMARY OF LABORATORY TESTS

QUEENSWAY AT WOODROFFE
BRIDGE No. 2

ELEVATION OF GROUND SURFACE (ZERO DEPTH) 245.7 - GEODETIC

HOLE No. 1

REMARKS REF. B.M. No. 11-G PL. 252-3

DATE MAY 16, 1958

UNCONFIRMED COMPRESSIVE STRENGTH KIPS/FT. ²	SMALL SCALE PENETROMETER KIPS/FT. ²	STANDARD PENETRATION BLOWS/FT.	SAMPLE NUMBER	DESCRIPTION OF SOIL	DEPTH IN FEET	ELEVATION	PENETRATION TEST	
							LB. HAMMER	NO CASING
							INCH DROP	INCH DIA. ROD
							BLOWS PER FOOT	
GROUND SURFACE							NO OVERNIGHT WATER	

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SOIL PROFILE AND SUMMARY OF LABORATORY TESTS

QUEENSWAY AT WOODROFFE
BRIDGE No.2

ELEVATION OF GROUND SURFACE (ZERO DEPTH) 247.0 - GEODETIC

REMARKS SEE PLATE 2

HOLE NO.

3

DATE MAY 21 1958

UNCONFINED COMPRESSIVE STRENGTH KIPS/FT. ²	SMALL SCALE PENETROMETER KIPS/FT.	STANDARD PENETRATION BLOWS/FT.	SAMPLE NUMBER	DESCRIPTION OF SOIL	DEPTH IN FEET	ELEVATION	PENETRATION TEST	
							LB. HAMMER	NO CASING
							INCH DROP	INCH DIA. ROD
							BLOWS PER FOOT	
				GROUND SURFACE				
				TOP SOIL	0	247.0		
					1.2	246.0		
				VERY STIFF, FISSURED SILTY BROWNISH-GRAY CLAY	2			
60	5.3	5	3-1		4			
	4.6							
		1 for 6"	3-2A		5.5	241.5		
		4	3-2B	LOOSE TILL	6			
		8						
		10 for 6"	3-3		8.5	238.5		
				MEDIUM DENSE TILL				
		37 for 6"			10.0	237.0		
		35 for 4"		DENSE TILL	10.8	236.2		
							OVERNIGHT WATER LEVEL 10.6 FT.	
				SHALY LIMESTONE (DRILLED) CORE RECOVERY 86% BEDDING THICKNESS 2"	12			
					12.5	232.5		
				SHALY LIMESTONE CORE RECOVERY 98% BEDDING THICKNESS 3"	16			
					18			
					19.7	227.3		
				LIMESTONE (DRILLED) - CORE RECOVERY 100% - BEDDING 3"	20	225.9		
					21.1			
				BOTTOM OF HOLE	22			

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SOIL PROFILE AND SUMMARY OF LABORATORY TESTS

QUEENSWAY AT WOODROFFE
BRIDGE No. 2

ELEVATION OF GROUND SURFACE (ZERO DEPTH) 247.9 - GEODETIC
REMARKS SEE PLATE 2

HOLE NO.

4

DATE MAY 21-22, 1958

UNCONFINED COMPRESSIVE STRENGTH KIPS/FT. ²	SMALL SCALE PENETROMETER KIPS/FT.	STANDARD PENETRATION BLOWS/FT.	SAMPLE NUMBER	DESCRIPTION OF SOIL	DEPTH IN FEET	ELEVATION	PENETRATION TEST				
							LB. HAMMER		NO CASING		
							INCH DROP		INCH DIA. ROD		
							BLOWS PER FOOT				
				GROUND SURFACE							
				TOP SOIL	0	247.9					
					1.0	246.9					
				VERY STIFF FISSURED SILTY	2						
6.0	53	5	4-1	BROWNISH-GRAY CLAY	4.0	243.9					
	4.8			STIFF FISSURED SILTY							
24	2.2	3	4-2	BROWNISH-GRAY CLAY	6						
	2.0				6.5	241.4					
		8	4-3	LOOSE TILL	8						
		19			10.0	237.9					
		25	4-4	MEDIUM DENSE TILL	11.0						
					11.7						
				DENSE							
				SANDY TILL	14						
		90	4-5		16.7						
					16.7	231.2					
				LIMESTONE (DRILLED)	18						
				CORE RECOVERY 95%	20						
				BEDDING THICKNESS 3"	20.8	226.1					
					22						
				SHALY LIMESTONE							
				DRILLED-CORE RECOVERY 65%	24						
				PORTION OF CORE COULD NOT BE RETRIEVED	26						
				BEDDING THICKNESS 2"	27.0	220.9					
					28						
				BOTTOM OF HOLE							
	</										

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SOIL PROFILE AND SUMMARY OF LABORATORY TESTS

QUEENSWAY AT WOODROFFE
BRIDGE No. 2

ELEVATION OF GROUND SURFACE (ZERO DEPTH) 247.8

REMARKS SEE PLATE 2

HOLE NO.

5

DATE MAY 22-25 1958

UNCONFINED COMPRESSIVE STRENGTH KIPS/FT. ²	SMALL SCALE PENETROMETER KIPS/FT. ²	STANDARD PENETRATION BLOWS/FT.	SAMPLE NUMBER	DESCRIPTION OF SOIL	DEPTH IN FEET	ELEVATION	PENETRATION TEST	
							LB. HAMMER INCH DROP	NO CASING INCH DIA. ROD
							BLOWS PER FOOT	
GROUND SURFACE							NO OVERNIGHT WATER	
				TOP SOIL	0	247.8		
					1.5	246.8		
6.0	5.6	6	5-1	VERY STIFF, FISSURED	2			
	5.4			SILTY	4			
4.8	4.4	4	5-2	BROWNISH-GRAY CLAY	6	241.3		
	4.2				6.5			
		2007	5-3	DENSE	8			
				SANDY TILL	10	237.3		
		115			10.5			
				SHALY LIMESTONE (DRILLED)	12			
				CORE RECOVERY 93%	14			
				BEDDING THICKNESS 2"	14			
					15.8	232.0		
				LIMESTONE (DRILLED)	16			
				CORE RECOVERY 91%	18			
				BEDDING THICKNESS 4"	20			
					21.7	226.1		
				BOTTOM OF HOLE	22			
							% WATER CONTENT	
							PLATE 6	

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SOIL PROFILE AND SUMMARY OF LABORATORY TESTS

QUEENSWAY AT WOODROFFE
BRIDGE No. 2

ELEVATION OF GROUND SURFACE (ZERO DEPTH) 247.7 - GEODETIC
REMARKS SEE PLATE 2

HOLE NO.

6

DATE MAY 20-21-1958

UNCONFINED COMPRESSIVE STRENGTH KIPS/FT. ²		SMALL SCALE PENETROMETER KIPS/FT. ²	STANDARD PENETRATION BLOWS/FT.	SAMPLE NUMBER	DESCRIPTION OF SOIL	DEPTH IN FEET	ELEVATION	PENETRATION TEST			
								LB. HAMMER	NO CASING		
								INCH DROP	INCH DIA. ROD		
								BLOWS PER FOOT			
GROUND SURFACE											
					TOP SOIL	0	247.7				
					VERY STIFF, FISSURED SILTY BROWNISH-GRAY CLAY	2	246.7				
6.2	6.3	6	6-1			4	243.7				
	6.0				MEDIUM DENSE	6					
		14	6-2								
		13	6-3		TILL	8					
		15				10					
		3 for 2	6.4			11.2	236.5	← OVERNIGHT WATER LEVEL 11.2 FT.			
					SHALY LIMESTONE (DRILLED) CORE RECOVERY 78% BEDDING THICKNESS 2"	12					
					SHALY LIMESTONE 1" DRP - WATER LOST AT 16"-4"	14					
					BEDDING THICKNESS 2"	15.2	232.4				
					LIMESTONE BEDDING THICKNESS 2"	16					
						18					
						20	226.9				
						20.5					
					BOTTOM OF HOLE	22					
								% WATER CONTENT			
								PLATE			
								7			