

62-F-98

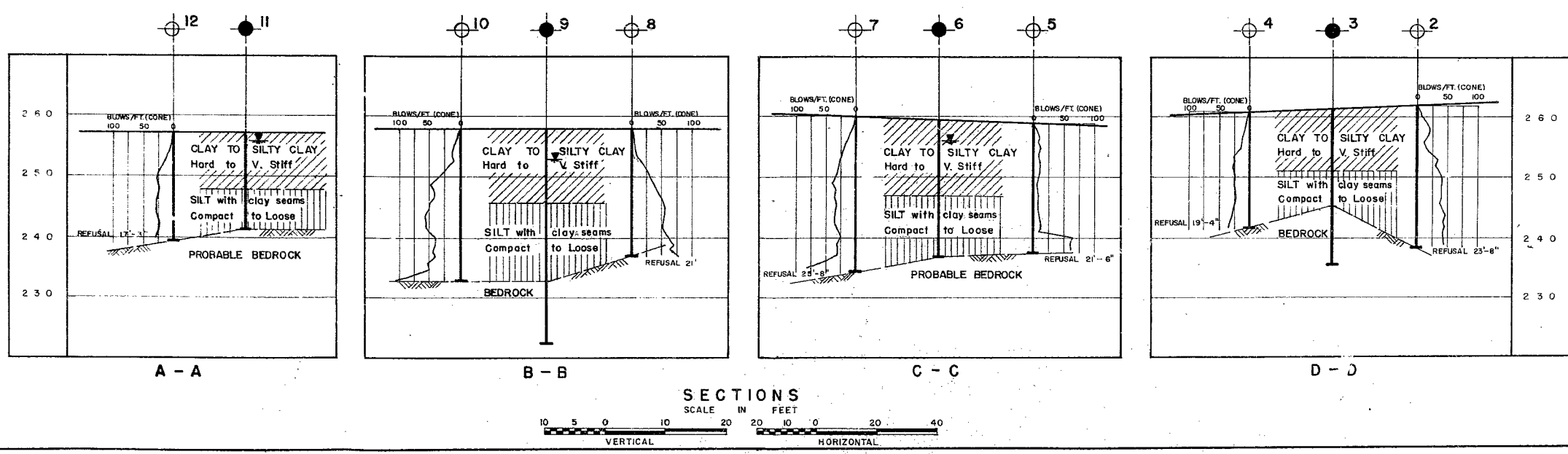
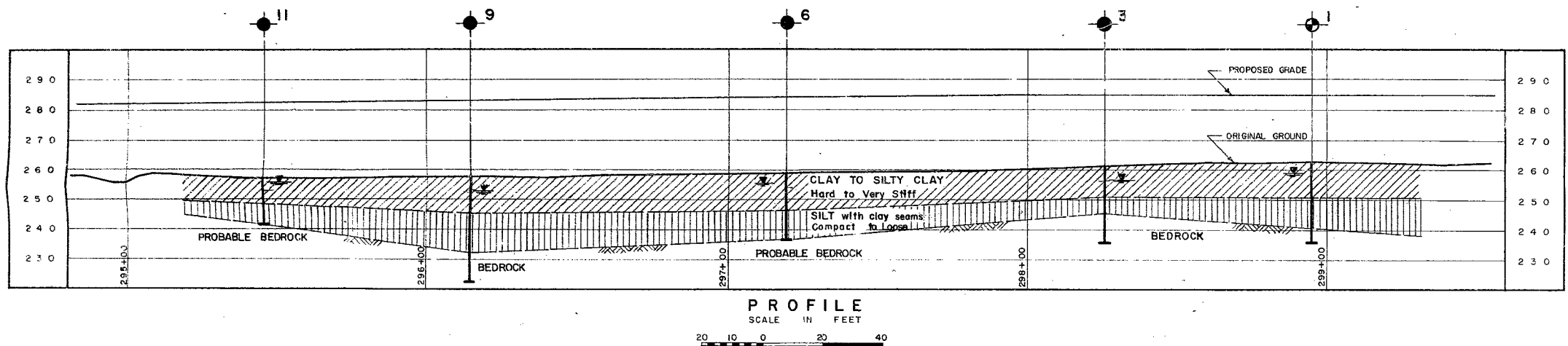
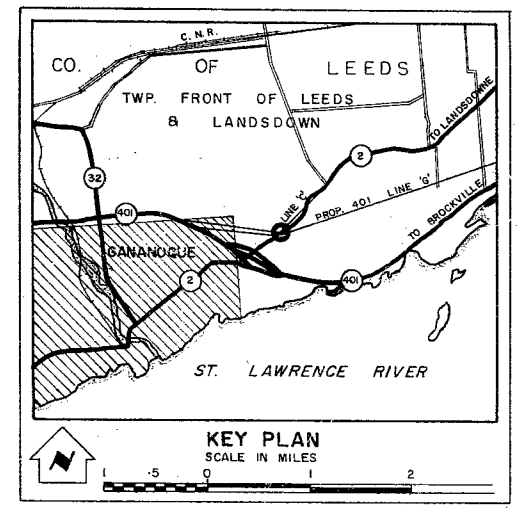
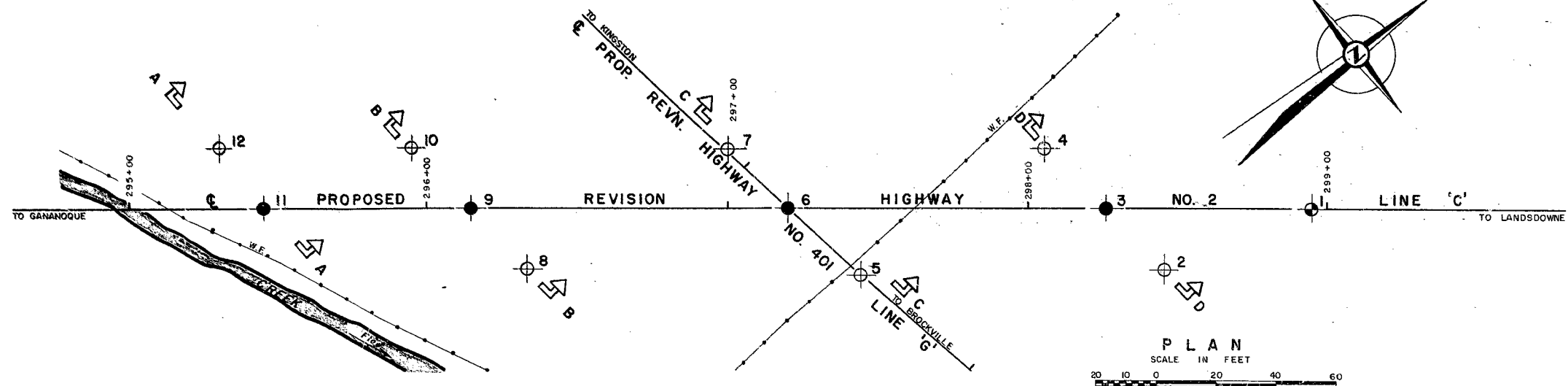
W.P.# 173-61

Hwy. # 2 E.

Hwy. # 401

1/2 MILES E. OF

GANANOQUE



LEGEND			
	Bore Hole		
	Cone Penetration Hole		
	Bore & Cone Penetration Hole		
	Water Levels established at time of field investigation, Aug. 1962		

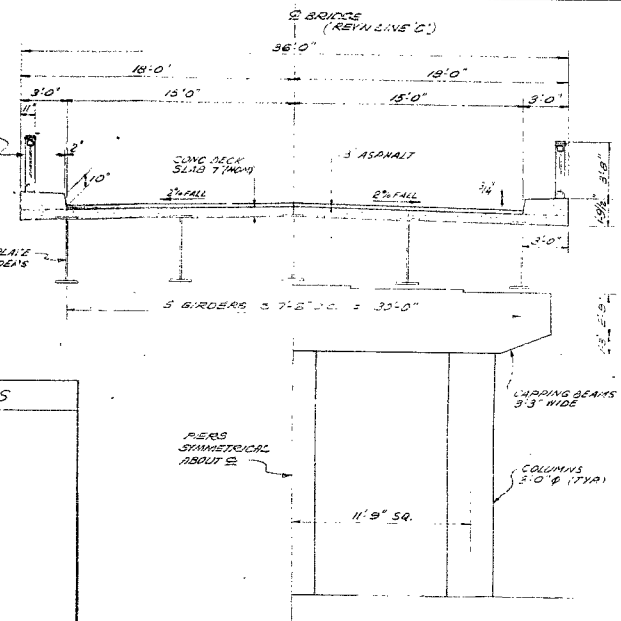
NO.	ELEVATION	STATION	OFFSET
1	263.0	298+95	0
2	262.0	298+45	20' RT.
3	261.5	298+26	0
4	261.0	298+05	20' LT.
5	259.0	297+44	22' RT.
6	259.5	297+20	0
7	260.0	297+00	20' LT.
8	258.0	296+33	20' RT.
9	258.0	296+15	0
10	258.0	295+95	20' LT.
11	257.0	295+45	0
12	257.0	295+30	20' LT.

NOTE
The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence and may be subject to considerable error.

DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH SECTION

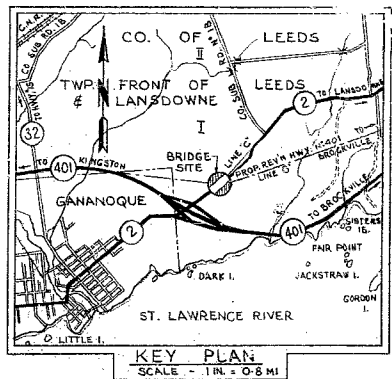
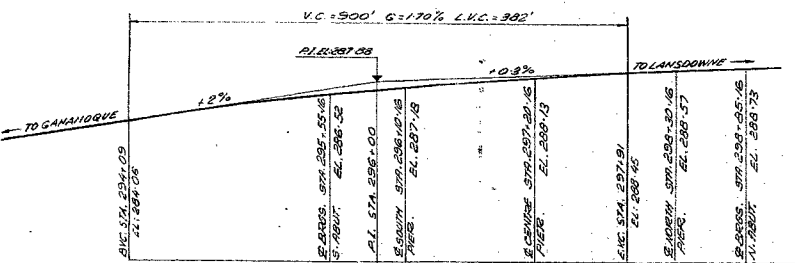
**HIGHWAY NO. 2 LINE 'C' REVISION
AND
HIGHWAY NO. 401 LINE 'G' REVISION
AT GANANOQUE**

ORIGINATED S. CHERRINGTON	DISTRICT NO. 3	DATE 30 AUGUST 1962
DRAWN D. MUMFORD	W.P. NO. 173-81	JOB NO. 62-F-98
CHECKED <i>[Signature]</i>	CONTRACT NO.	DRAWING NO.
APPROVED <i>[Signature]</i>		62-F-98 A



N°	LIST OF DRAWINGS

NOTES:

[illegible][illegible]

DEPARTMENT OF HIGHWAYS ONTARIO BRIDGE DIVISION			
HWY. 2 INTERCHANGE UNDERPASS AT GANANOQUE			
KING'S HIGHWAY No. 401		DIST. No. 8	
CO. LEEDS		CON. I	
TWP. FRONT OF LEEDS & LANSLOWNE LOT 19			
PRELIMINARY GENERAL ARRANGEMENT			
APPROVED _____ BRIDGE ENGINEER		SITE No. _____	W.P. No. 173-61
DESIGN DRAWING	CHECK B. P. O.	CHECK X. F.	CONTRACT Nos.
DATE	LOADING	DRAWING No.	D-5169-PI

Damas & Smith Limited • Consulting Engineers • Toronto

Mr. A. M. Toye,
Bridge Engineer.
Materials & Research Division,
(Foundation Section)

September 5, 1962.

D.H.O. FOUNDATION INVESTIGATION
REPORT.
W.J. 62-F-98 -- W.P. 173-61.

Attention: Mr. S. McCombie.

Re: Proposed Structure - Hwy. #2, Line 'C',
Station 297+20 and Hwy. #401, Line 'G',
Station 61+20, 1/2 Mi. E. of Gananoque,
District #8.

Attached, we are forwarding to you, our detailed foundation investigation report dealing with existing subsoil conditions at the above structure site.

We believe you will find the factual data and recommendations contained therein, self-explanatory. However, should further information be desired, please do not hesitate to contact our Office.

AGS/MdeF
Attach.

cc: Messrs. A. M. Toye (2)
H. A. Tregaskes
H. D. McMillan
J. Ford
E. A. Cash
J. E. Gruspier
T. J. Kovich
J. Roy
E. R. Saint
F. Norman
A. Watt
Foundations Office ✓
Gen. Files.

A. G. Stermac
A. G. Stermac,
PRINCIPAL FOUNDATION ENGINEER

TABLE OF CONTENTS

1. INTRODUCTION.
 2. DESCRIPTION OF SITE.
 3. FIELD AND LABORATORY WORK.
 4. SUBSOIL CONDITIONS:
 - 4.1) General.
 - 4.2) Hard to Med. Stiff Brown Clay to Silty Clay.
 - 4.3) Compact to Loose Grey Silt containing Thin Layers of Clay.
 - 4.4) Bedrock.
 5. GROUND WATER CONDITIONS.
 6. DISCUSSION AND RECOMMENDATIONS:
 - 6.1) General.
 - 6.2) Structure.
 - 6.3) Approach Fills.
 7. SUMMARY.
 8. MISCELLANEOUS.
-

FOUNDATION INVESTIGATION

For

Proposed Structure - Hwy. #2, Line 'C',
Station 297+20 and Hwy. #401, Line 'B',
Station 61+20, 1/2 Mi. E. of Gananoque,
District #8.

W.J. 62-F-98 ----- W.P. 173-61

1. INTRODUCTION:

A request was received from the Bridge Location Section dated July 19, 1962, for a foundation investigation at the site of a proposed structure to carry Hwy. #2, Line 'C' over Hwy. #401, Line 'B'.

A field investigation to determine the subsoil conditions at the site of the proposed structure was carried out by this Section.

Presented in this report are the results of this investigation, together with the recommendations pertaining to the design of the structure foundations.

2. DESCRIPTION OF SITE:

The proposed site is located about 100 yards North of existing Hwy. #2 and one-half mile East of the Town of Gananoque.

The site is located in a relatively flat meadow surrounded by rolling countryside with numerous rock outcrops of igneous and metamorphic rock.

Physiographically, the area is located in a section known as the "Leeds Knobs and Flats". Typically, this area consists of

cont'd. /2 ...

2. DESCRIPTION OF SITE: (cont'd.) ...

knobs of granite and other Precambrian rocks with the low areas being filled with clay left by the Champlain Sea.

3. FIELD AND LABORATORY WORK:

The field work consisted of 5 sampled boreholes and 8 dynamic cone penetration tests.

Samples were recovered at required depths by means of a 2" O.D. split-spoon sampler. The dimensions of the split-spoon sampler and the energy used in driving it, conform to the requirements of the Standard Penetration Test. Driving energy to advance the 2-inch diameter cone was 350 ft.-lbs. per blow.

AXT rock core samples were obtained in B.H.'s 1, 3 & 9.

The locations and elevations of all boreholes are shown on Dwg. 62-F-98A which accompanies this report.

4. SUBSOIL CONDITIONS:

4.1) General:

The soil stratigraphy at the site was found to be generally uniform. Detailed descriptions of the soil types encountered in each boring, are given in Appendix I of this report. The estimated stratigraphical profile of Dwg. 62-F-98A is based upon this information.

From ground level downwards, the various soil types encountered, are as follows:

cont'd. /3 ...

4. SUBSOIL CONDITIONS: (cont'd.) ...

4.2) Hard to Med. Stiff Brown Clay to Silty Clay:

A surface layer of hard to med. stiff brown silty clay varying between 9 and 12' in thickness, was found in all sampled boreholes.

'N' values in the material ranged from 53 blows/ft. at the upper part, to a low of 15 blows/ft. near the bottom of the layer.

Representative values for this material are noted below:

	<u>Range</u>	<u>Average Values</u>
w _L	55% to 41%	45%
w _p	31% to 21%	24%
w	25% to 30%	27%
γ	124 to 131 p.c.f.	127 p.c.f.
'N'	53 to 15 blows/ft.	30 blows/ft.
Shear Strength	- 3000 to 5000 p.s.f.	4000 p.s.f.

The Atterberg limits show the clay is of high plasticity near the surface, but decreases in plasticity with depth.

4.3) Compact to Loose Grey Silt containing Thin Layers of Clay:

This layer varies in thickness between 6 and 13', and lies between the surface crust of brown clay to silty clay and the underlying bedrock.

cont'd. /4 ...

4. SUBSOIL CONDITIONS: (cont'd.) ...

4.3) Compact to Loose Grey Silt containing Thin Layers of Clay: (cont'd.) ...

'N' values in this material varied between 8 to 14 blows per ft. which would classify the material as being loose to compact. This deposit contains occasional thin layers of clay approx. 1/4" thick.

The moisture content of the silt was found to be approx. 22%. Due to lack of sufficient material, Atterberg limits were not carried out on the clay layers, but one moisture content test was carried out and a value of 31.7% was obtained. Visual examination of the clay layers indicated the material appeared to be dark grey, highly plastic and with a brittle texture.

4.4) Bedrock:

The bedrock was found to vary between 15 ft. and 25 ft. below the surface (el. 245 to el. 233).

All boreholes were advanced to rock contact. In B.H. #1, 5' of rock core (in AXT size) was taken with 100% recovery; 10' of AXT core was taken in B.H.'s 3 and 9 with 95% recovery in B.H. #3, and 100% recovery in B.H. #9.

The bedrock was found to be a sound granite in B.H.'s 1 and 3, and sound quartzite in B.H. #9.

5. GROUND WATER CONDITIONS:

The ground water level was found to vary between 1 and 5' below the surface.

The ground water elevations taken at the time of investigation, are shown on the borehole logs and Dwg. 62-F-98A.

cont'd. /5 ...

6. DISCUSSION AND RECOMMENDATIONS:

6.1) General:

It is proposed to construct a 4-span structure just East of the Town of Gananoque, to carry the relocated Hwy. #2 over relocated Hwy. #401.

The subsoil consists of 9 - 12' of hard to very stiff brown clay overlying 6 to 23' of compact to loose grey silt containing thin layers of clay.

This material is underlain by sound bedrock.

6.2) Structure:

Spread footings placed 5 ft. below ground surface are capable of supporting 2 T.S.F. with resultant settlements not exceeding 1".

If higher loads are desired, the structure should be supported on end-bearing piles driven to bedrock. For a 12" Ø timber pile, a design load of 20 T/pile may be used. Such piles should be treated if their cut-off is above the lowest known ground water elevation. For a 12 $\frac{3}{4}$ " O.D. steel tube pile driven to bedrock, a design load of 60 T/pile can be used.

6.3) Approach Fills:

No stability problems are anticipated with a fill height of 25' having 2:1 side slopes.

Since the footing excavation will extend below the free water table, some seepage may occur; however, this should be easily handled by ordinary pumping methods.

cont'd. /6 ...

7. SUMMARY:

The subsoil at the site consists of a clay overlying a deposit of silt. This material is underlain by bedrock.

Spread footings placed 5 ft. or below the ground surface, are capable of supporting 2 T.S.F. with settlements not greater than 1 inch.

For higher loadings, 12" Ø timber piles, or 12 $\frac{3}{4}$ " O.D. steel tube piles driven to bedrock, are capable of supporting 20 and 60 Tons/pile, respectively.

No major dewatering problems are anticipated.

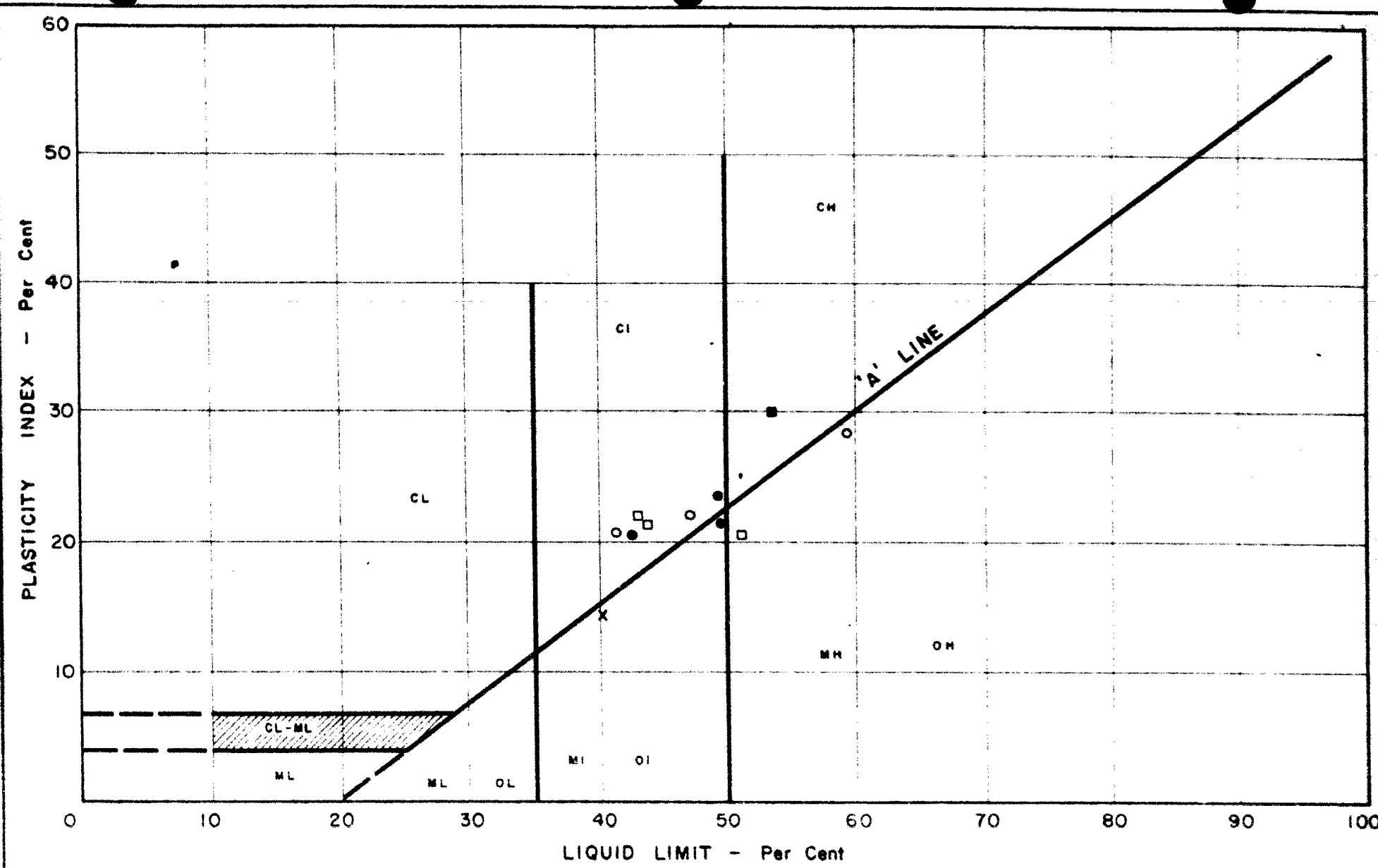
Stability problems are not anticipated provided the approaches are constructed with side slopes 2 horizontal to 1 vertical.

8. MISCELLANEOUS:

The field work, performed during the period from August 9 to August 17, together with the preparation of this report, was undertaken by Mr. G. G. Cherrington. The investigation was carried out under the general supervision of Mr. M. Devata, who also reviewed the report.

September 1962.

APPENDIX I.



NOTES	BORE HOLE	1 - ●
	"	3 - ■
	"	6 - ○
	"	9 - □
	"	11 - X

DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH DIVISION
PLASTICITY CHART
Job No. 62-F-98 W.P. No. 173-61
Location HWY. 2 - PROP. LINE 'G' AT GANANOQUE

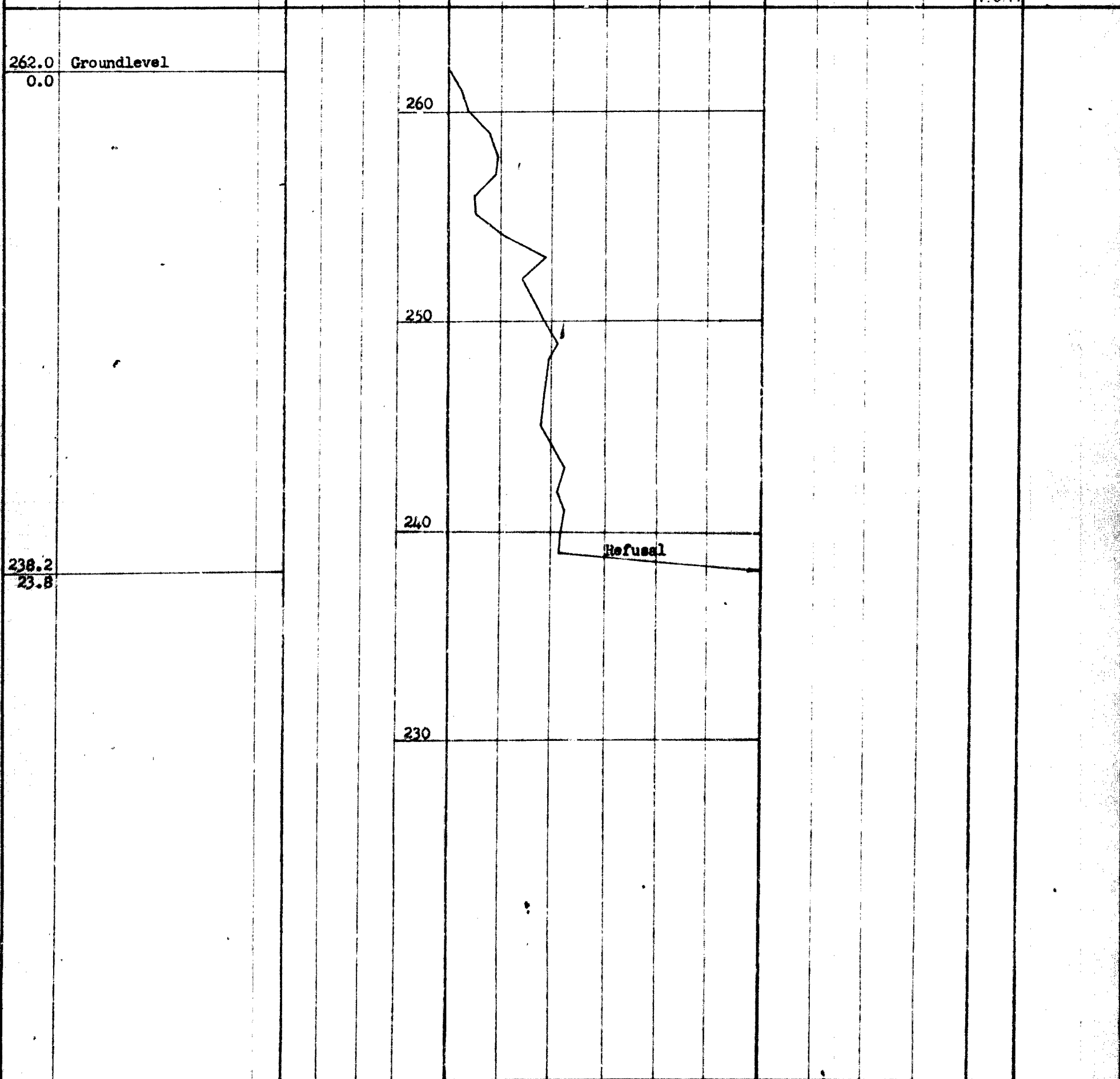
FOUNDATION SECTION

JOB 62-F-98 LOCATION Sta. 298/95 E ORIGINATED BY M.D.
W.P. 173-61 BORING DATE Aug. 9, 1962. COMPILED BY M.D.
DATUM G.S.C. BOREHOLE TYPE Washboring. CHECKED BY H.S.

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE					LIQUID LIMIT ——— WL PLASTIC LIMIT ——— WP WATER CONTENT ——— W			BULK DENSITY P.C.F.	REMARKS		
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV. SCALE	BLOWS / FOOT	20	40	60	80	100	WP			WL	
263.0 0.0	Groundlevel														W.L. from observations in borehole.		
	Silty clay-Hard to v. stiff-Brown.		1	SS	53		260								124.0	P 259.0 L 4.0	
			2	SS	49												
			3	SS	29												127.0
250.5 12.5	Silt with clay seams compact-grey.						250										
			4	SS	11												
			5	SS	11												
241.0 22.0	Bedrock-sound granite 100% recovery.						240										
236.0 27.0	End of borehole.																

FOUNDATION SECTION

0.0



DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH DIVISION

RECORD OF BOREHOLE NO. 4

FOUNDATION SECTION




JOB 62-F-98 LOCATION Sta. 298+05 20' Rt. of E ORIGINATED BY MD.
W.P. 173-61 BORING DATE Aug. 14, 1962. COMPILED BY M.D.
DATUM G.S.C. BOREHOLE TYPE Dynamic Cone Penetration Test. CHECKED BY H.S.

[illegible]

FOUNDATION SECTION

27.5 End of Conehole

DEPT. HIGHWAYS - ONTARIO RESEARCH DIVISION	RECORD OF BOREHOLE NO. 6	FOUNDATION SECTION
JOB _____	LOCATION <u>Sta. 297/20 E</u>	ORIGINATED BY <u>M.D.</u>
BORING DATE <u>Aug. 15, 1962.</u>		COMPILED BY <u>M.D.</u>
DA _____	BOREHOLE TYPE <u>Washboring.</u>	CHECKED BY <u>HR</u>

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE						LIQUID LIMIT ——— w _L PLASTIC LIMIT ——— w _P WATER CONTENT ——— w			BULK DENSITY P.C.F.	REMARKS	
ELEV DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV. SCALE	BLOWS / FOOT 20 40 60 80 100					SHEAR STRENGTH P.S.F.					w _P w w _L WATER CONTENT %
259.5	Ground level					260											
0.0	Silty clay - Hard to v. stiff-brown to grey.			1	SS	23											
				2	SS	20	250										
			3	SS	15												
12.5	Silt with clay seams compact.		4	SS	13												
				5	SS	13	240										
22.5	Probable bedrock. End of borehole.					230											

gwl 256.0
3.5

115.0

131.0

Clay&Silt
97%
Sand 3%

swl 256.0
3.5

115.0

131.0

Clay & Silt
97%
Sand 3%

FOUNDATION SECTION

CHECKED BY AK

[illegible]

FOUNDATION SECTION

ORIGINATED BY H.D.

COMPILED BY _____ M.D.

CHECKED BY

[illegible]

[illegible]

FOUNDATION SECTION

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE		LIQUID LIMIT ——— WL		BULK DENSITY P.C.F.	REMARKS	
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV. SCALE	BLOWS / FOOT	SHEAR STRENGTH P.S.F.	PLASTIC LIMIT ——— WP			WATER CONTENT ——— W
WATER CONTENT %												
257.0	Groundlevel					260						
0.0	Silty clay-Hard-Brown.		1	SS	34	250						
248.0	Silt with clay seams-Loose.		2	SS	8							
241.5	Probable bedrock.		3	SS	>15							
15.5	End of borehole.					240						

[illegible]

MEMORANDUM

~~BA 1525~~
BA 1511-1

To: Mr. A. M. Toye,
Bridge Engineer,
Bridge Division.

FROM: Mr. A. G. Stermac,
Principal Foundation Engr.,
Foundation Section,
Materials & Research Division.

Attention: Mr. S. McCombie.

DATE: October 31, 1962.

OUR FILE REF.

IN REPLY TO

SUBJECT:

D.H.O. FOUNDATION INVESTIGATION REPORT -
Proposed Structure - Hwy. #2, Line 'C',
Station 297+20 and Hwy. #401, Line 'G',
Station 61+20, 1/2 Mi. E. of Gananoque,
District #8 - W.J. 62-F-98 -- W.P. 173-61.

FILE

Attached, we are forwarding log sheets to be
inserted in your copy(s) of the above report
which was submitted to you on Sept. 26, 1962.

AGS/MdeF
Encls.

for Mr. de Foux (sic)
A. G. Stermac,
PRINCIPAL FOUNDATION ENGINEER

cc: Messrs. A. M. Toye (2)
H. A. Tregaskes
H. D. McMillan
J. Ford
E.A. Cash
J.E. Gruspier
T.J. Kovich
J. Roy
E.R. Saint
F. Norman
A. Watt
Foundations Office
Gen. Files.

BA 1511-~~1~~
BA 1525

Mr. A. M. Teye,
Bridge Engineer,
Bridge Division.

Attention: Mr. S. McCombie.

Mr. A. G. Stermac,
Principal Foundation Engr.,
Foundation Section,
Materials & Research Division.

October 31, 1962.

D.H.O. FOUNDATION INVESTIGATION REPORT -
Proposed Structure - Hwy. #2, Line 'C',
Station 297+20 and Hwy. #401, Line 'G',
Station 61+20, 1/2 Mi. E. of Gananoque,
District #8 - W.J. 62-F-98 -- W.P. 173-61.

Attached, we are forwarding log sheets to be
inserted in your copy(s) of the above report
which was submitted to you on Sept. 26, 1962.

AGS/MdeF
Encls.


(Handwritten signature: A. G. Stermac)
A. G. Stermac,
PRINCIPAL FOUNDATION ENGINEER

cc: Messrs. A. M. Teye (2) ✓
H. A. Tregaskes
H. D. McMillan
J. Ford
E.A. Cash
J.E. Gruspier
T.J. Kovich
J. Roy
E.R. Saint
F. Norman
A. Watt
Foundations Office
Gen. Files.

FOUNDATION SECTION

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE		LIQUID LIMIT ——— w_L		BULK DENSITY γ P.C.F.	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV. SCALE	BLOWS / FOOT	PLASTIC LIMIT ——— w_p	WATER CONTENT ——— w		
						SHEAR STRENGTH P.S.F.		WATER CONTENT %			
262.0 0.0	Groundlevel					260					
						250					
						240					
238.2 23.8						230					

DEPARTMENT OF HIGHWAYS - ONTARIO MATERIALS & RESEARCH DIVISION		RECORD OF BOREHOLE NO. 3		FOUNDATION SECTION	
JOB <u>62-F-98</u>		LOCATION <u>Sta. 298/26 E</u>		ORIGINATED BY <u>M.D.</u>	
W.P. <u>173-61</u>		BORING DATE <u>Aug. 13, 1962.</u>		COMPILED BY <u>M.D.</u>	
DATUM <u>G.S.C.</u>		BOREHOLE TYPE <u>Washboring.</u>		CHECKED BY <u>H.S.</u>	

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE						LIQUID LIMIT ——— w _L PLASTIC LIMIT ——— w _p WATER CONTENT ——— w			BULK DENSITY P.C.F.	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV SCALE	BLOWS / FOOT 20 40 60 80 100					SHEAR STRENGTH P.S.F. w _p w _p w _L 20 40 60				
261.5	Groundlevel														Waterlevel from observation in borehole.	
0.0	Silty clay-V. stiff Brown to grey.						260									
			1	SS	17											
252.0			2	SS	14										Clay 23% Silt 75% Sand 2%	
9.5	Silt with clay seams compact-loose.					250										
		3	SS	8												
245.0			4	SS	29											
16.5	AXT core-95% recovery, Sound granite.															
						240										
235.4																
26.1	End of borehole.															
						230										

DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH DIVISION

RECORD OF BOREHOLE NO. 1

FOUNDATION SECTION

JOB 62-F-98 LOCATION Sta. 298+95 E ORIGINATED BY M.D.
W.P. 173-61 BORING DATE Aug. 9, 1962. COMPILED BY M.D.
DATUM G.S.C. BOREHOLE TYPE Washboring. CHECKED BY H.S.

SOIL PROFILE			SAMPLES			ELEV SCALE	DYNAMIC PENETRATION RESISTANCE					LIQUID LIMIT — WL PLASTIC LIMIT — WP WATER CONTENT — W			BULK DENSITY Y P.C.F.	REMARKS	
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT		BLOWS / FOOT	20	40	60	80	100	W	WL			
263.0	Groundlevel																
0.0	Silty clay-Hard to v. stiff-Brown.		1	SS	53												
			2	SS	49												
			3	SS	29												
250.5	Silt with clay seams compact-grey.		4	SS	11												
12.5																	
			5	SS	11												
241.0	Bedrock-sound granite 100% recovery.																
22.0																	
236.0	End of borehole.																
27.0																	

W.L. from observations in borehole.

259.0
4.0

Refusal

FOUNDATION SECTION

261.0	Groundlevel
0.0	
244.7	
19.3	End of Conshole.

FOUNDATION SECTION

259.0
0.0

SOIL PROFILE			SAMPLES		ELEV SCALE	DYNAMIC PENETRATION RESISTANCE	LIQUID LIMIT — %L		BULK DENSITY P.C.F.	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE		BLOWS / FOOT	BLOWS / FOOT	PLASTIC LIMIT — %P		
							20 40 60 80 100	WATER CONTENT — %		
							SHEAR STRENGTH P.S.F.	WD WL		
								WATER CONTENT %		
259.0 0.0						260				
						250				
						240				
237.5 27.5 End of Conehole							Refusal			
						230				

Clay&Silt 97%
Sand 3%

FOUNDATION SECTION

CHECKED BY AK

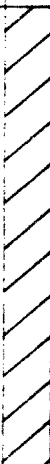



SOIL PROFILE			SAMPLES			ELEV. SCALE	DYNAMIC PENETRATION RESISTANCE		LIQUID LIMIT ——— WL		BULK DENSITY	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT		BLOWS / FOOT	SHEAR STRENGTH P.S.F.	PLASTIC LIMIT ——— WP	WATER CONTENT ——— W		
260.0	Groundlevel					260						
0.0												
						250						
						240						
234.2												
25.8	End of conehole.											
						230						

DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH DIVISION

RECORD OF BOREHOLE NO. 9

FOUNDATION SECTION

JOB 62-F-98 LOCATION Sta. 296+15 E ORIGINATED BY M.D.
W.P. 173-61 BORING DATE Aug. 16, 1962. COMPILED BY M.D.
DATUM G.S.C. BOREHOLE TYPE Washboring. CHECKED BY ✓

SOIL PROFILE			SAMPLES			DYNAMIC PENETRATION RESISTANCE					LIQUID LIMIT ——— w _L			BULK DENSITY P.C.F.	REMARKS	
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT	ELEV SCALE	BLOWS / FOOT					PLASTIC LIMIT ——— w _p				
							20 40 60 80 100					WATER CONTENT ——— w				
							SHEAR STRENGTH P.S.F.					w _p w w _L				
							WATER CONTENT %					20 40 60				
258.0	Groundlevel					260										
0.0	Silty clay-Hard brown to grey brown.		1	SS	38											
			2	SS	36	250										
			3	SS	22											
245.5	Silt with clay seams compact.		4	SS	19											
12.5						240										
			5	SS	10											
232.8	Bedrock AXT core-100% recovery Sound Quartzite.					230										
25.2																
222.5	End of borehole.					220										
35.5																

vwl 253.0
5.0
Clay&Silt
88%
Sand 12%

Clay&Silt
98%
Sand 2%

v.w.l. 253.0
5.0
Clay&Silt
88%
Sand 12%Clay&Silt
98%
Sand 2%

JOB 62-F-98

LOCATION Sta. 295+95 20' Lt. of C

ORIGINATED BY M.D.

W. P. 173-61

BORING DATE Aug. 17, 1962.

COMPILED BY M.D.




DATUM G.S.C.

BOREHOLE TYPE Dynamic Cone Penetration Test.

CHECKED BY

SOIL PROFILE			SAMPLES			ELEV SCALE	DYNAMIC PENETRATION RESISTANCE BLOWS / FOOT	Liquid Limit ——— W _L	Plastic Limit ——— W _P	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT		20 40 60 80 100	WATER CONTENT ——— W	W _P W _L	
258.0 0.0						260				
						250				
						240				
25.1	End of Conehole.					230				

FOUNDATION ACTION

SOIL PROFILE			SAMPLES			ELEV. SCALE	DYNAMIC PENETRATION RESISTANCE		LIQUID LIMIT ——— WL		BULK DENSITY	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE	BLOWS / FOOT		BLOWS / FOOT	SHEAR STRENGTH P.S.F.	PLASTIC LIMIT ——— WP	WATER CONTENT ——— W		
257.0	Groundlevel					260						
0.0	Silty clay-Hard-Brown.											
			1	SS	34	250						
248.0												
9.0	Silt with clay seams-Loose.		2	SS	8							
241.5	Probable bedrock.		3	SS	>15							
15.5	End of borehole.					240						

FOUNDATION SECTION

ORIGINATED BY H.D.

COMPILED BY M.D.

CHECKED BY _____

SOIL PROFILE			SAMPLES		ELEV. SCALE	DYNAMIC PENETRATION RESISTANCE	LIQUID LIMIT ——— % _L	BULK DENSITY	REMARKS
ELEV. DEPTH	DESCRIPTION	STRAT. PLOT	NUMBER	TYPE		BLOWS / FOOT	BLOWS / FOOT		
						20 40 60 80 100	WATER CONTENT ——— %		
						SHEAR STRENGTH P.S.F.	% _P ——— % _L		
							WATER CONTENT %		
							20 40 60		
					260				
257.0									
0.0									
					250				
					240				
239.8									
17.2	End of Conshole.					Refusal			

DEPARTMENT OF HIGHWAYS ONTARIO

MEMORANDUM

To: Mr. A.G. Stermac,
Principal Foundations Engineer,
Room 107, Lab. Bldg.,

FROM: A.P. Watt

DATE: February 22, 1963.

OUR FILE REF.

IN REPLY TO

62-F-98

SUBJECT: W.P. 173-61, Bridge Site 17-137,
Hwy. 2 Interchange U'Pass
at Gananoque,
Hwy. 401, Dist. 8.

Enclosed please find one copy of the preliminary
plan D- 5169- P1 for the above structure.

The designer appears to have complied with the
requirements of the foundation report but we would
appreciate any comments you wish to make.

APW:db

Apwatt
A.P. Watt, 3506
Bridge Location Engineer.

1) What contact pressure is being used?

*Re removed by (removed) on May 9/63
M. A. Watt
April 1963*