

62-F-72

W.P. #591-56

HWY. #33

COLLINS BAY

FAILURE AREA

29

Mr. J. S. Graspier,
Regional Soils Engineer,
Kingston, Ontario.

Materials & Research Division,
(Foundation Section)

W.P. 581-57-1-1

June 19, 1962.

Re: Collins Bay Failure Area,
Hwy. #33, near Kingston,
Cont. 60-129 - Dist. #8.

J.S. 62-F-1

Attached, we are sending you the report dealing
with the failure area on Hwy. #33 at Collins Bay, East of
Kingston.

We believe that the investigation has provided
enough information to enable the explanation of what has
actually happened and why it had occurred. Because of the
very heterogeneous and irregular stratification and the
nature of the predominant subsoil material (muck), any
quantitative analysis and prediction of developments is
unwarranted and would serve no purpose. It is our opinion
that the problem has ceased to be a stability one, and has
become a maintenance problem.

Should there be any additional information that
you would require, please feel free to contact our office.

AGC/WdF
attach.

G. Sternac
G. C. Sternac,
PRINCIPAL FOUNDATION ENGINEER

cc: Mr. G. A. Wrong
Foundations Office
Gen. Files.

REPORT ON

COLLINS BAY FAILURE AREA
HWY. #33, NEAR KINGSTON,
CONT. 60-129, DISTRICT #8.

After Hwy. #33 was widened and resurfaced, some cracks appeared on the road surface at Collins Bay, approximate Station 98+00 to 100+00. The nature of the cracks indicated that some vertical and lateral movements must have taken place. Although appearance of cracks when road widening and grade adjustment have been completed is very often to be expected because of differential settlements resulting from uneven loading conditions, the cracks at Collins Bay were somewhat large and it was thought reasons for that should be established. Also, it was thought, if the investigation would show that any remedial measures should be required, they should be undertaken as soon as possible.

In order to provide the answer and explanation to the above question, a number of boreholes were put down in the problem area. Nine power auger boreholes and three diamond drill boreholes were completed. Their respective locations are shown on the accompanying drawing. On the same drawing, the subsoil conditions - i.e., the different materials and the stratification as revealed by the borings are also presented.

Cross section A shows the rough subsoil stratification parallel to the road centre line. None of these power auger borings has encountered any rock fill, only very soft tuck underlain by silty clay, soft in the upper portion and becoming stiffer

with depth. The contact between the muck and the silty clay is very irregular. The borings were carried down to refusal, but the cause of refusal was not established.

Cross section B-B is perpendicular to the road's centre line and encompasses two diamond drill boreholes - No. 3 and 2) and one power auger borehole (No. 6P). There the difference in thickness of rock fill and very soft compressible muck is obvious. A more equalized condition is to be expected under the area of borehole 3 than 2. Apart from that, the very soft muck in the area of borehole 6P provides only little lateral support and therefore, movements in this direction are explainable.

Cross section C-C, being also at right angle to the road's centre line, encompasses one diamond drill borehole - (No. 1) and one power auger hole (No. 3P). No borehole was put down on the north side of the road which is performing satisfactorily. The diamond drill borehole has revealed a very irregular stratification which resulted from only a partial displacement of the soft muck by the rock fill. Here again, due to the presence of trapped muck and small lateral support, the appearance of cracks can be explained.

Because the amount of settlements decreases as time goes on and the shear strength of the subsoil in this particular case increases with time, the conditions are improving and barring any unusual or unpredictable development, equilibrium will be reached. It is practically impossible on the basis of

available data and information, to make any predictions as to when stabilization will be reached - i.e., when the movements will cease. It is our opinion that the problem has now become one of maintenance.

The field work was carried out by the Kingston Regional Soils personnel while the preparation of the drawing and the report was the responsibility of the Foundation Section.

June 1962.

Re: Treatment
Permitted, Hwy 23
Lineal 201
6.1 to 6.4File A-23
with report

MEMORANDUM

To: Mr. W. Neillipovitz,
Staff Engineer of the
Executive Section,
Administration Building.

ATTENTION:

OUR FILE REF.

FROM: Soils Section,
Materials and Testing Office.

DATE: October 19, 1970.

IN REPLY TO

SUBJECT: Highway #33, Contract 70-174
Collins Bay East Limits Easterly, 0.2 Miles
District 8, Kingston. (W.J. 62-F-72) (Cont: 60-129)

You requested that we review the treatment proposed on the above mentioned contract which has now been cancelled. It is suggested that this work be included with the follow-up grading, drainage, Granular base and paving job, W.P. 25-67 Kingston west limits westerly to Collins Bay.

It was intended under this contract to excavate and pre-load the swamp area on the proposed widening between Stations 96+00 and 115+00 in the Township of Kingston. By this pre-loading it was hoped that major settlements in the embankment widening through the swamp area could be overcome.

Mr. Rutka, Mr. Stermac and I have reviewed the treatment proposed and it appears that there are two reasonable alternatives:
1) to call this small contract again in early spring of 1971, or
2) to include this work with the remainder of the construction proposed between Kingston and Collins Bay.

If this latter alternative was selected, it would be desirable to specify by Special Provision that the pre-loading through the swamp area be carried out as one of the first operations of the contract.

Since high bid prices might again occur if this small contract is called, we would recommend that the second alternative be selected.

Cont'd.../2

Mr. W. Neilipovitz

- 2 -

October 19, 1970

Highway #33, Contract 70-174
Collins Bay East Limits Easterly, 0.2 Miles
District 8, Kingston

Without early preloading settlements can be expected through this swamp. Therefore, we would recommend that only binder course paving be required over the widening through this swamp area. The asphalt surface course and curb and gutter should be placed at a later date after the settlements have occurred.



G. A. Wrong,
Principal Soils Engineer.

GAW/sd

cc:- J. Walters
H. Tregaskés
V. Snell
J. Gruspier
S. Markiewicz
A. Stermac✓
File

Ald

MEMORANDUM

TO: Mr. V. A. Snell
District Engineer
District 8, Kingston

FROM: Materials and Testing Office
Kingston

ATTENTION:

DATE: May 10th, 1971

OUR FILE REF.

IN REPLY TO

SUBJECT:

Contract IB 32-70, Highway 33,
Fill Widening at Collins Bay (W.P. 25-67)

DIST. 8

62-F-72

or 62-F-104

Considerable settlement has occurred on a section of the rock fill widening at Collins Bay. From Station 97+25 to Station 98+ distortions on the gravel surface south of the existing asphalt pavement are approximately 12" to 18" in depth owing to settlement of the rock fill that was placed during the winter months. From Station 98+60 to Station 99+35 there is an approximate 2" pavement elevation differential on either side of a wide meandering longitudinal crack on the north side of centreline.

It is anticipated that the settlement is due to thawing of the ice beside the fill which provided some lateral confinement immediately after construction, thawing of ice and snow within the rock fill such that rearrangement and consolidation of rock fill fragments has occurred, and to consolidation of the underlying soft clay foundation material. It is difficult to ascertain the amount each of these factors has contributed. Considerable movement was noted as the ice in the bay thawed.

In order to establish settlement rates, plates were placed on the fill widening and stubby nail control points were established on the existing pavement soon after construction was completed. However, owing to the amount of settlement and use of the fill widening grade as a car park most of these control points have been lost or destroyed. In conjunction with the construction staff, we will establish a new set of control points (with protection) such that the settlement rate associated with consolidation of the foundation material can be ascertained and taken into account when the remainder of the construction is completed this year. Pending these measurements, it may be advisable to leave the surface course pavement off this section for a certain period of time after the main construction is carried out.

Owing to the present condition of the fill widening grade, it is recommended that restoration be effected by adding required Granular 'A' from Station 97+25 to Station 98+ and by regrading

the entire gravel grade. An old guide post, showing in the vicinity of Station 97+30, should be removed in conjunction with this repair. In addition, the fore mentioned elevation differential and wide longitudinal crack on the existing pavement should be repaired with the bituminous cold mix sufficient to eliminate any traffic hazard.

A. M. Batten

A. M. Batten
Senior Soils Supervisor

for: J. E. Gruspier
AMB:mgn Regional Materials Engineer

cc: A. G. Sternac ✓
G. A. Wrong
H. B. McKay

CONTRACT NO 70-174

OCT 2. 1970

NOTE :

J. WALTER ADVISED THAT THE SUCCESSFUL CONTRACTOR WAS WAY ABOVE OUR ESTIMATE. COULD THIS CONTRACT BE CANCELLED AND THE WORK DONE NEXT YEAR BY THE OVERALL CONTRACT? HOW MUCH IS GAINED BY STAGING I.E. FILL BUILDINGS IN 1970, PAVING IN 1971?

THE AREA WAS A MAINTENANCE PROBLEM UNTIL THIS YEAR. FOR THE ORIGINAL CONSTRUCTION NO SUBSOIL INVESTIGATION WAS CARRIED OUT AND MUCK WAS PARTIALLY DISPLACED AND PARTIALLY TRAPPED. THIS CREATED SETTLEMENTS REQUIRING MAINTENANCE. (SEE REPORT 1962)

IT CAN NOT BE EXPECTED THAT THE UNDERLYING SOIL NOT SETTLE EVEN IF ALL MUCK IS EXCAVATED AND DISPLACED. MAINTENANCE WILL BE NECESSARY BECAUSE:

- (1) THE TRAPPED MUCK UNDER THE PRESENT EMBANKMENT SLOPE WILL FURTHER COMPRESS UNDER THE NEW FILL, AND
- (2) THE UNDERLYING CLAY BELOW THE NEW FILL WILL ALSO COMPRESS.

CONSEQUENTLY, ROD WALTER ADVISED THE UNDERSTANDING IS CONSTRUCTED A FEW YEARS OF MAINTENANCE HAD TO BE RECKONED WITH.

KES.

PROVINCE OF ONTARIO



DEPARTMENT OF HIGHWAYS

HON. GEORGE E. GOMME
MINISTER

A.T.C. MCNAB
DEPUTY MINISTER

H.W. ADCOCK
ASST. DEPUTY MINISTER
(Engineering)

J. WALTER
DIRECTOR OF DESIGN



PROVINCE OF ONTARIO
DEPARTMENT OF HIGHWAYS
DESIGN BRANCH

Station 95+90 to Station 104+50

Length (miles) 0.16

Survey Plan Nos. B-19-11

Survey Profile Nos. C-19-38

Soil Profile Nos. 33K8-5

Bridge Drawings Nos. _____

W. P. No. 35-70-01 Contract No. 70-174

Work of GRADING AND DRAINAGE

Hwy. No. 33 District No. 8 - KINGSTON

Location COLLINS BAY EAST LIMITS EASTERLY 0.2 MILES.

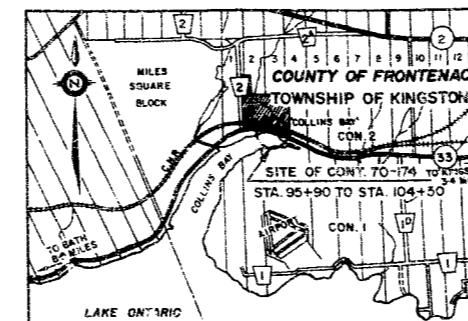
Township of KINGSTON County of FRONTENAC

Date Senior Project Design Engineer (Roads)

Date Senior Project Design Engineer (Bridges)

Date Regional Road Design Engineer

TOTTEN, SIMS, HUBICKI ASSOCIATES LIMITED
CONSULTING ENGINEERS
TORONTO WHITBY COBOURG KINGSTON



KEY PLAN

SCALE
0 miles 0 miles

Date Bridge Engineer

Date Road Design Engineer

INDEX

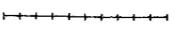
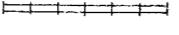
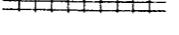
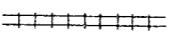
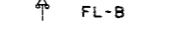
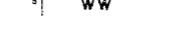
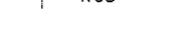
LEGEND

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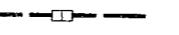
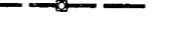
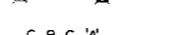
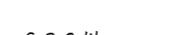
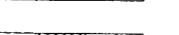
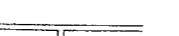
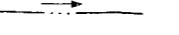
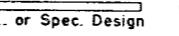
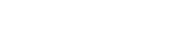
REFERENCE POINTS

- DHM Old style Dept. of Highways conc. mon.
- CM Other survey conc. monument markers
- SIBM 1" Square iron bar with a cap
- DHRPM Department of Highways rock post
- $\frac{1}{2}'' \text{ } \# \text{ IB}$ Odd size iron bars or pipe or iron tube
- $\frac{1}{2}'' \text{ } \# \text{ IP}$
- $\frac{1}{2}'' \text{ SIB}$ Odd size square iron bar
- WS Wood stake
- Co-ordinated monument
- Cut cross
- Cut vee
- RIB 1" or $\frac{3}{4}'' \text{ } \varnothing$ round iron bars as planted by Engineering Surveys
- SIB 1" square iron bars as planted by Land Surveys

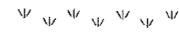
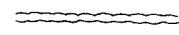
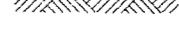
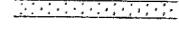
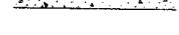
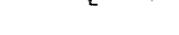
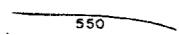
RAILWAYS

-  400' or 200' Scale, single track
-  400' or 200' Scale, double track (or more)
-  100', 50', 40' Scale, single track. Gauge to scale (4'-7 1/2")
-  100', 50', 40' Scale, double track (or more) Gauge to scale (4'-7 1/2")
-  GFL Gate and flashing light
-  GFL-B Gate flashing light and bell
-  FL Flashing light
-  FL-B Flashing light and bell
-  WW WIG-WAG
-  WW-B WIG-WAG and bell
-  RCS Railway crossing sign

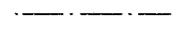
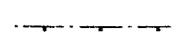
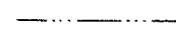
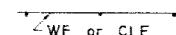
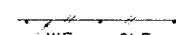
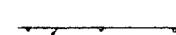
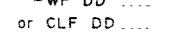
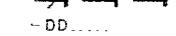
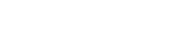
DRAINAGE

-  Manhole, Catch Basin or Ditch inlet
-  Double Manhole, Catch Basin or Ditch inlet
-  Manhole solid cover
-  Catch Basin or Manhole to be removed
-  C & G 'A' Curb and Gutter type (1"=100')
-  C & G 'A' Curb and Gutter type (1"= 50' and larger scales)
-  Curb and Gutter transition
-  Dropped Curb
-  Gutter outlet 90°
-  Gutter outlet 45°
-  Existing ditches, watercourses, creeks etc. Width 5' or less
-  Ditch to be constructed
-  Existing ditches, watercourses, creeks etc. Width over 5'
-  Flow arrow
-  Culvert to be constructed
-  Culvert with headwall to be constructed
-  Culvert existing
-  Culvert existing with headwalls
-  Storm sewer to be constructed

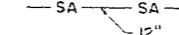
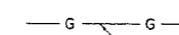
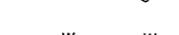
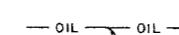
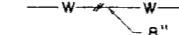
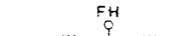
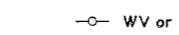
MISCELLANEOUS (CONTINUED)

-  Sodding
-  Hedge
-  Trees or fruit trees
-  Rock section in profile
-  Pavement to be removed
-  Paved shoulder
-  Granular subbase section
-  Granular 'A' section
-  Concrete section
-  Highway centre line with or without centre line symbol
-  Contour lines
- Elevation on high side of line intervals:
 - $1'' = 20'$
 - $1'' = 40'$
 - $1'' = 50'$
 - $1'' = 100'$ 2 Feet or 5 Feet
 - $1'' = 200'$ 5 Feet
- Every fifth contour is a thick line

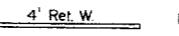
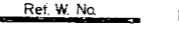
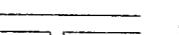
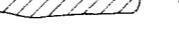
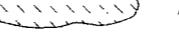
RIGHT OF WAY, FENCES, ETC.

-  Right of Way or D.H.O. property limit (no fence)
-  Right of Way or D.H.O. property limit including fence, 1 foot inside R/W
-  Permanent easement
-  Temporary easement
-  Existing fence
-  Fence to be removed
-  Fence to be erected
-  WF or CLF
-  WF or CLF
-  Guide Rail existing
-  Guide Rail to be removed
-  Guide Rail to be erected
-  DD....

UTILITIES

-  SA SA 12" Sanitary sewer
-  G G 3" Gas main
-  W W 8" Water main
-  B B Underground Bell cable
-  L L Underground lighting cable
-  H H Underground hydro cable
-  OIL OIL 12" Oil pipe line
-  ST ST 18" Storm sewer, existing
-  W W Utility to be removed
-  FH FH Fire hydrant
-  H or B or T Hydro, Bell or Telegraph poles
-  WV or GV Water valve or Gas valve
-  GM Gas meter

MISCELLANEOUS

-  4' Ret. W. Existing retaining wall with height
-  Ref. W. No. Retaining wall to be constructed, with reference number
-  5' Sidewalks with widths
-  Area to be cleared
-  Area to be grubbed
-  Area to be cleared and grubbed
-  Edge of Lake or River
-  Swamp and edge of swamp

508-3 Use decimal point of written elevation to indicate point of spot elevation

➡ Traffic direction arrow

● Well

□ Mail box

○ LS Light standard

○ LSS Light standard with sign

● TS Traffic signal

↑ Highway sign

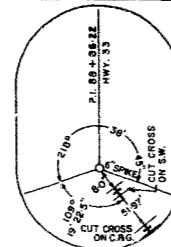
■ OSF Overhead sign footing

■ OS Overhead signs

ABBREVIATIONS

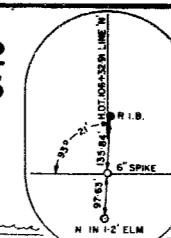
CON. 2
LOT 2

'COLLINS BAY



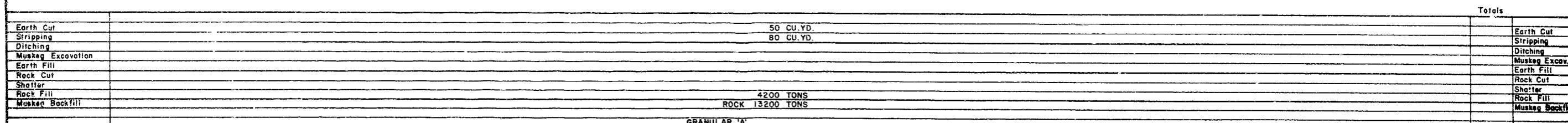
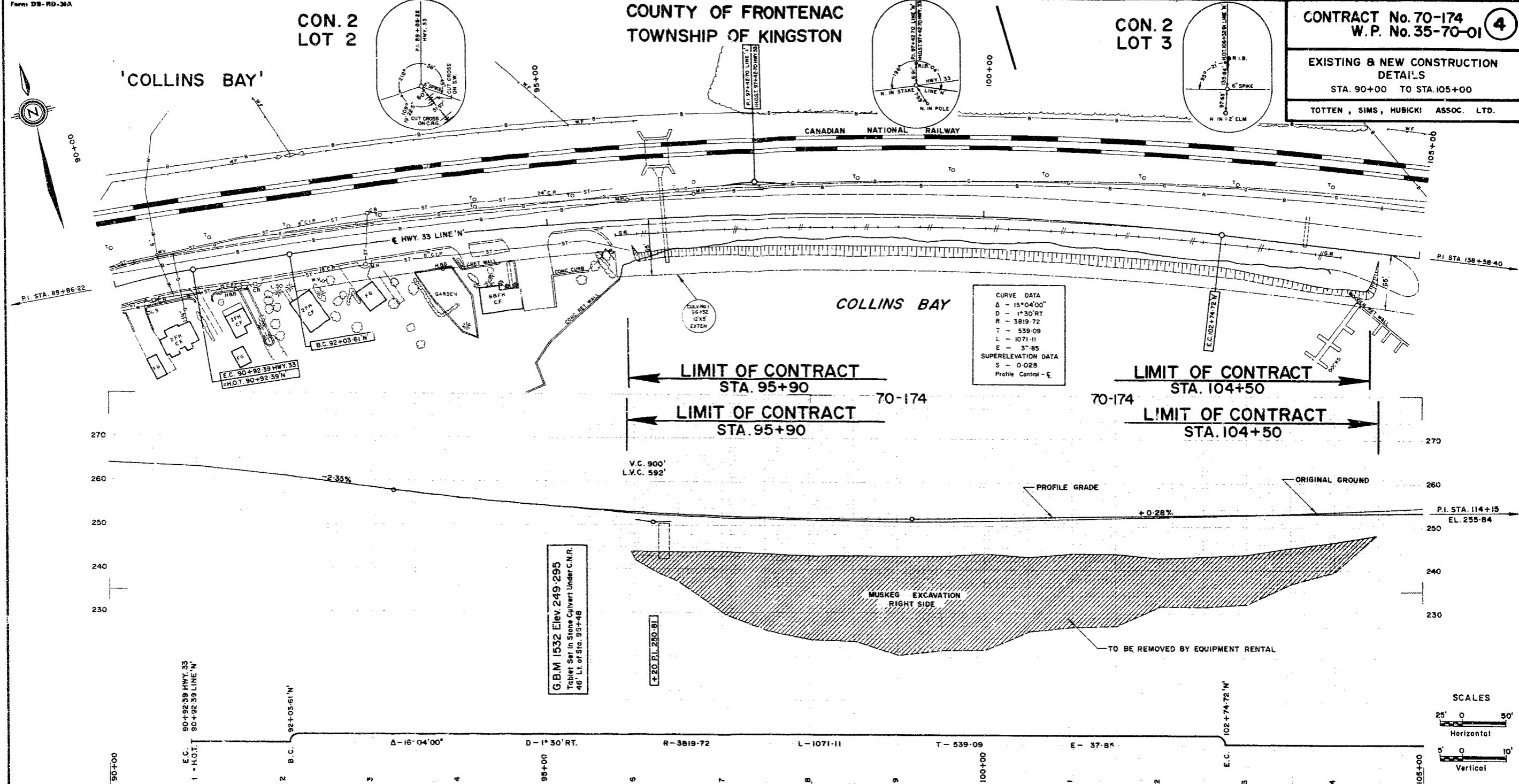
**COUNTY OF FRONTENAC
TOWNSHIP OF KINGSTON**

CON. 2
LOT 3



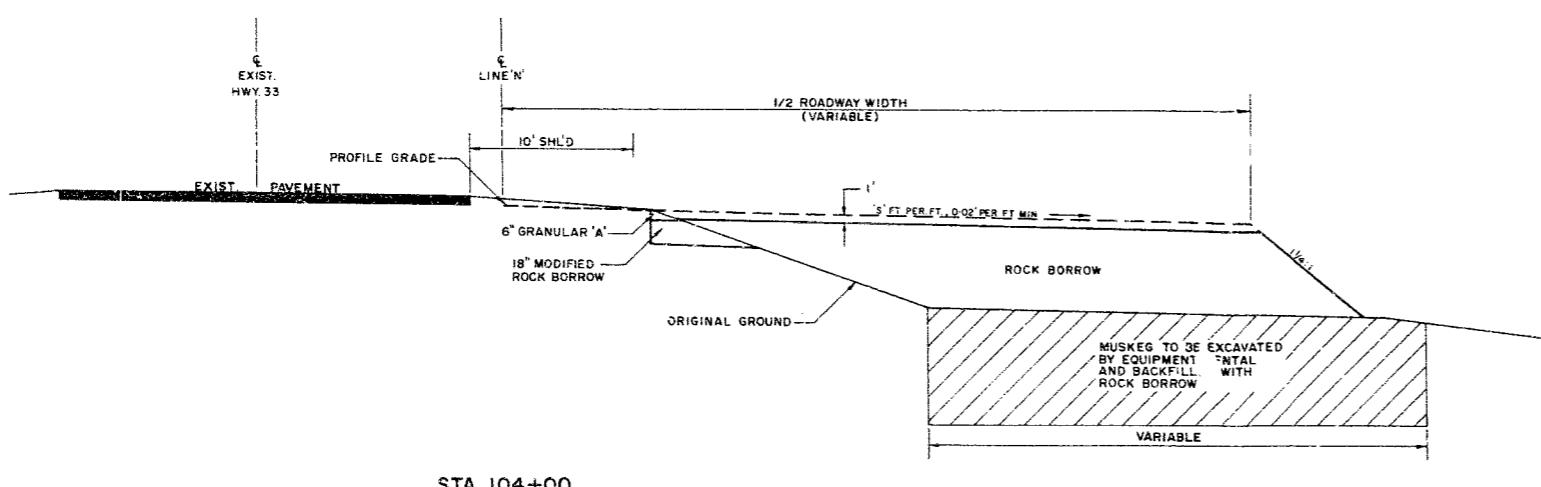
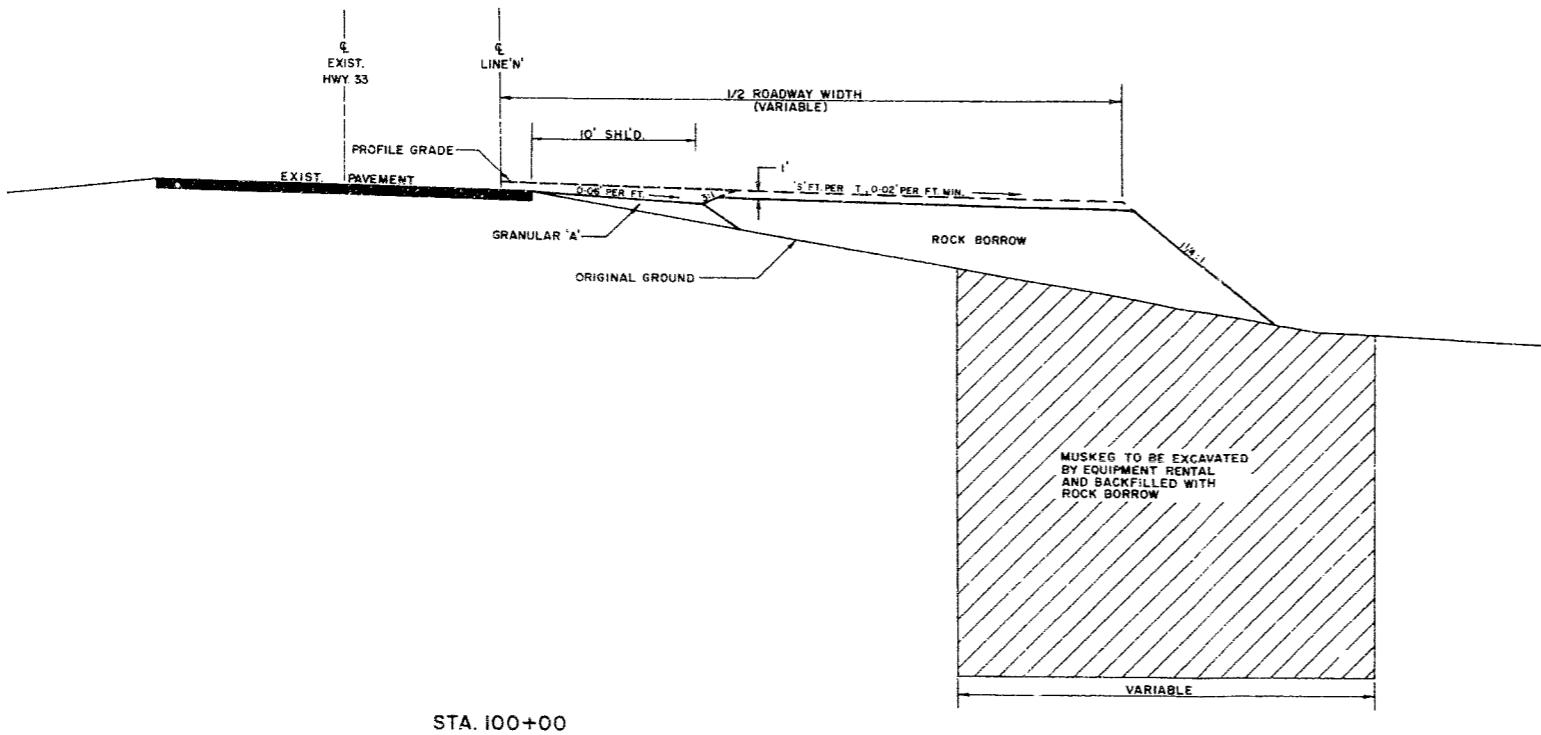
CONTRACT No. 70-174
W.P. No. 35-70-C

**EXISTING & NEW CONSTRUCTION
DETAILS**



GRADING SECTIONS
STA. 95+95 TO STA. 104+45

TOTTON, SIMS, HUBICKI ASSOC. LTD.

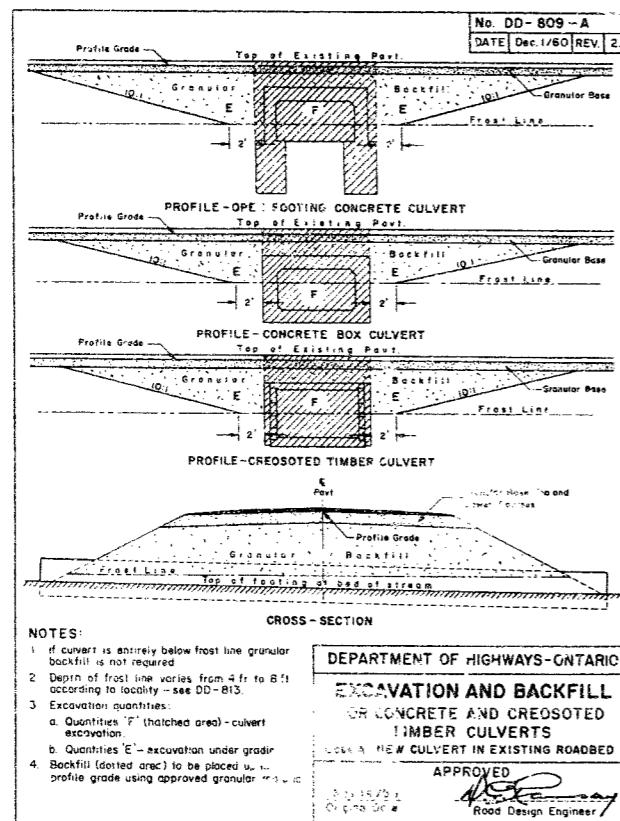
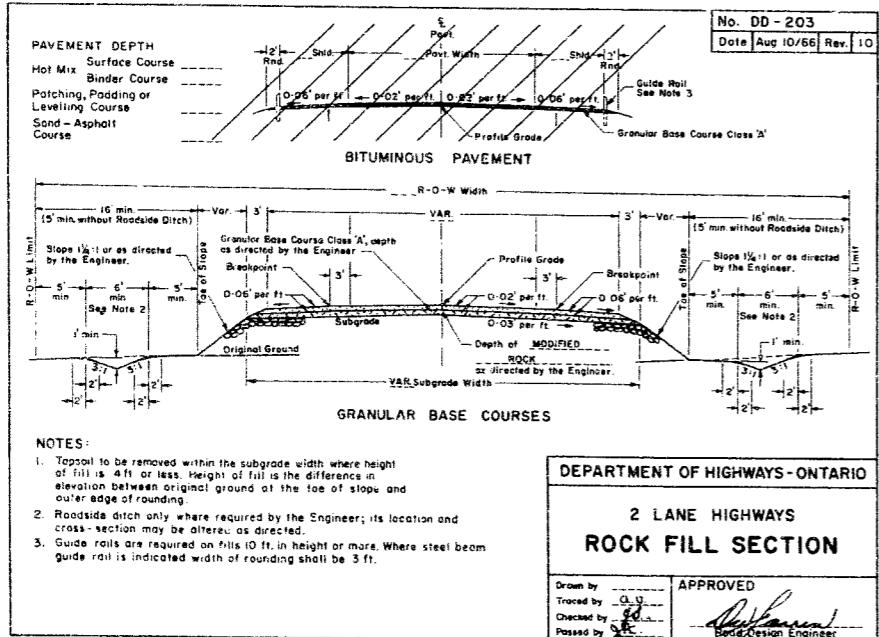


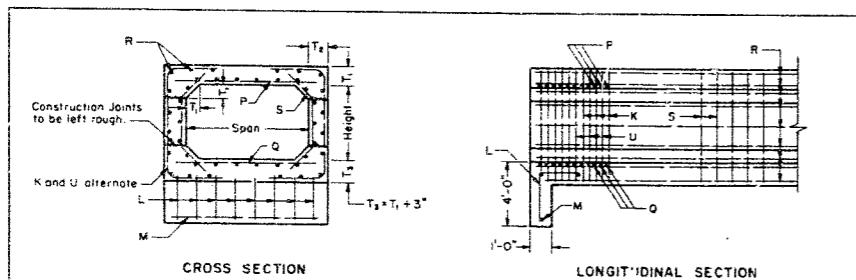
OFFSETS FROM LINE 'N' TO EDGE OF ROADWAY

STA.	OFFSET
96+00	39.0
+25	36.2
+50	36.5
+75	36.7
97+00	37.0
+25	37.2
+50	37.5
+75	37.7
98+00	38.0
+25	38.0
+50	38.0
+75	38.0
99+00	38.0
+25	38.0
+50	38.0
+75	38.0
100+00	38.0
+25	38.0
+50	38.0
+75	38.0
101+00	38.0
+25	38.0
+50	38.0
+75	38.0
102+00	38.0
+25	38.0
+50	38.0
+75	38.0
103+00	38.3
+25	40.1
+50	42.0
+75	43.8
104+00	45.7
+25	47.5

NOTE:

THESE SECTION TO BE USED
IN CONJUNCTION WITH STANDARDS
DD-203 AND SD-4-37





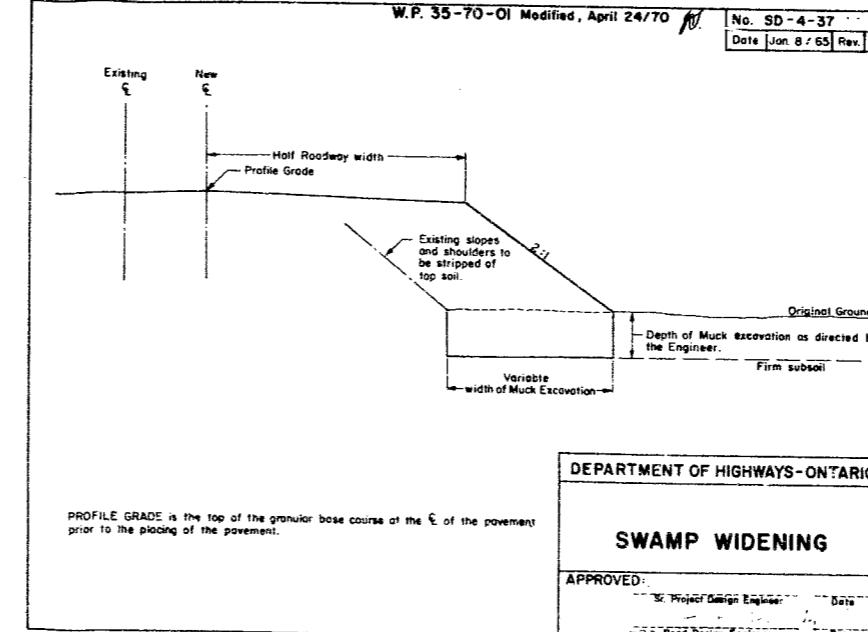
Size of Culvert	Item	Fill on Deck in feet:										Approx. Steel per cu. yd. of Conc.
		0	2	4	6	8	10	12	14	16	18	
in 6 x 3	T ₁	8½"					Unlimited Fill					
in 6 x 6	T ₂	10"										
in 6 x 3	Conc. cu. yd. per lin. ft.	.70										
in 6 x 4		.76										
in 6 x 5		.82										
in 6 x 6		.88										
in 8 x 4	T ₁	8½"										
in 8 x 6	T ₂	10"										
in 8 x 4	Conc. cu. yd. per lin. ft.	.88										
in 8 x 5		.94										
in 8 x 6		1.00										
in 10 x 4	T ₁	8½"	8½"	9"	9½"	10"	10½"	10½"	—	Unlimited Fill		
in 10 x 6	T ₂	10"	10"	10½"	11"	11½"	12½"	13"	13"	"		
in 10 x 4	Conc. cu. yd. per lin. ft.	1.00	1.00	1.06	1.08	1.14	1.22	1.28	1.28	"		
in 10 x 5		1.07	1.07	1.13	1.15	1.21	1.30	1.36	1.36	"		
in 10 x 6		1.13	1.13	1.19	1.22	1.28	1.37	1.43	1.43	"		
in 10 x 7		1.19	1.19	1.26	1.28	1.35	1.43	1.52	1.52	"		
in 10 x 8		1.25	1.25	1.32	1.35	1.42	1.53	1.60	1.60	"		
in 12 x 5	T ₁	9½"	9½"	10"	10½"	11"	12"	12½"	13"	"		
in 12 x 10	T ₂	12"	12"	12"	13"	14"	15"	15"	16½"	17"	"	
in 12 x 5	Conc. cu. yd. per lin. ft.	1.37	1.37	1.37	1.44	1.49	1.60	1.79	1.86	1.94	1.94	
in 12 x 6		1.44	1.44	1.44	1.54	1.69	1.79	1.89	1.97	2.04	2.04	
in 12 x 7		1.52	1.52	1.52	1.62	1.77	1.88	1.98	2.07	2.15	2.15	
in 12 x 8		1.59	1.59	1.59	1.70	1.66	1.79	2.09	2.17	2.25	2.25	
in 12 x 9		1.66	1.66	1.66	1.78	1.95	2.07	2.19	2.27	2.36	2.36	
in 12 x 10		1.74	1.74	1.74	1.84	2.03	2.16	2.28	2.37	2.46	2.46	
in 14 x 5	T ₁	10½"	11"	11½"	12"	12½"	13"	14"	14½"	15"	15½"	
in 14 x 10	T ₂	13"	13½"	14"	14½"	15½"	16½"	18"	19"	19½"	20½"	
in 14 x 5	Conc. cu. yd. per lin. ft.	1.65	1.73	1.81	1.89	1.99	2.10	2.28	2.39	2.47	2.55	
in 14 x 6		1.74	1.82	1.90	1.96	2.05	2.20	2.39	2.51	2.60	2.71	
in 14 x 7		1.82	1.90	1.98	2.07	2.18	2.30	2.51	2.62	2.72	2.84	
in 14 x 8		1.90	1.98	2.07	2.16	2.28	2.40	2.62	2.74	2.84	2.96	
in 14 x 9		1.98	2.07	2.16	2.25	2.37	2.50	2.73	2.86	2.96	3.09	
in 14 x 10		2.06	2.15	2.24	2.34	2.47	2.61	2.84	2.98	3.08	3.22	

Refer also to Standard DD-802

NOTES:

- Structure to be built in accordance with D.H.O. Specifications Form 9 and the Special Provisions as outlined in the Tender Form.
- All concrete work to conform to Section 9-04 of D.H.O. Form 9. Class of concrete: 3000 p.s.i.
- Depending on size of culvert and height of fill the size of bars P, K and U varies from No. 1 to No. 5 and the spacing varies approximately from 6" to 10". S bars - No. 5, spacing usually double that of P R bars - No. 5, spacing 1'-0" or in floor and 2'-0" oc in deck and sides. L bars - No. 5, spacing 1'-0" oc. M bars - No. 4, ties.
- No concrete may be placed in any footing until depth of excavation and character of the foundation have been approved by the Engineer.
- All exposed corners to have one inch chamfer.
- Fill must be placed against both sides of the culvert simultaneously and evenly, as directed by the Engineer.

Drawn by	J. A. K.	APPROVED	APPROVED
Checked by	J. A. K.	APPROVED	APPROVED
Printed by	R. D. G.	Printed Date	Printed Date
STANDARD CONCRETE CULVERTS RIGID FRAME BOX TYPE FOR ESTIMATING PURPOSES ONLY			



DEPARTMENT OF HIGHWAYS-ONTARIO

SWAMP WIDENING

APPROVED:

By Project Design Engineer Date

By Road Design Engineer Date

10

☒ Shrinkage factor for earth and bulking factor for rock have been applied to the quantities shown.

CONCRETE AND TIMBER CULVERTS

CONTRACT No. 70-174

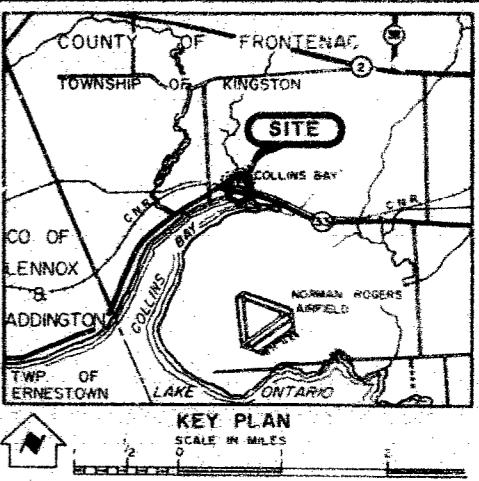
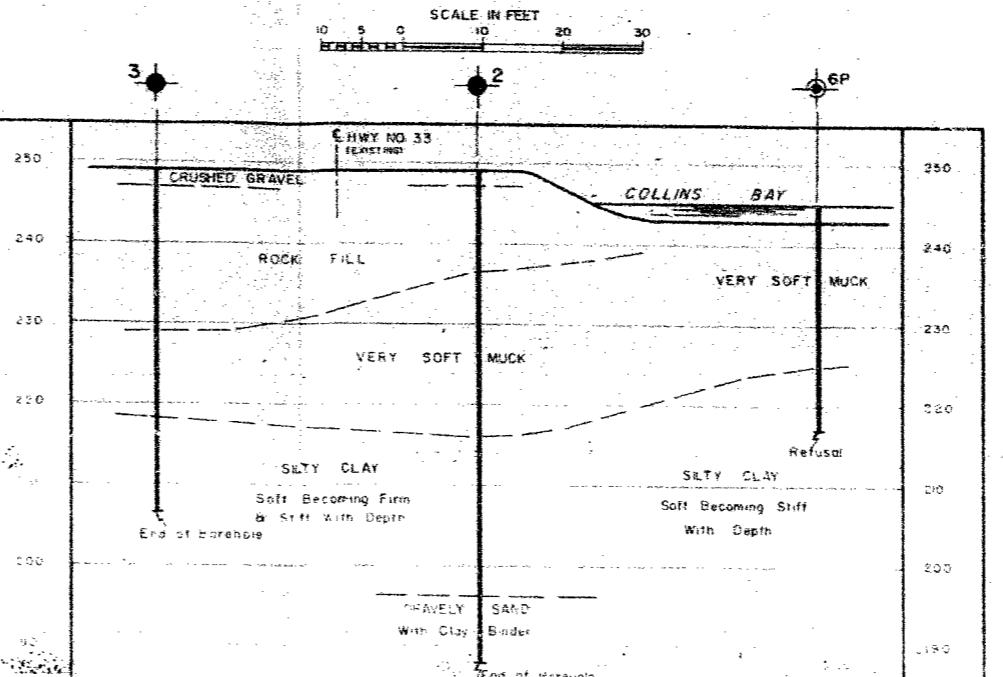
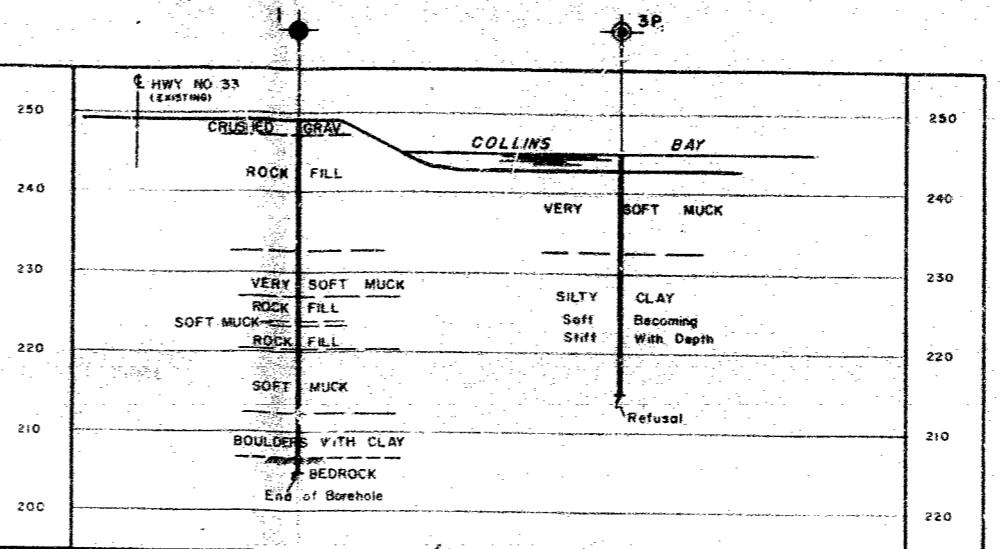
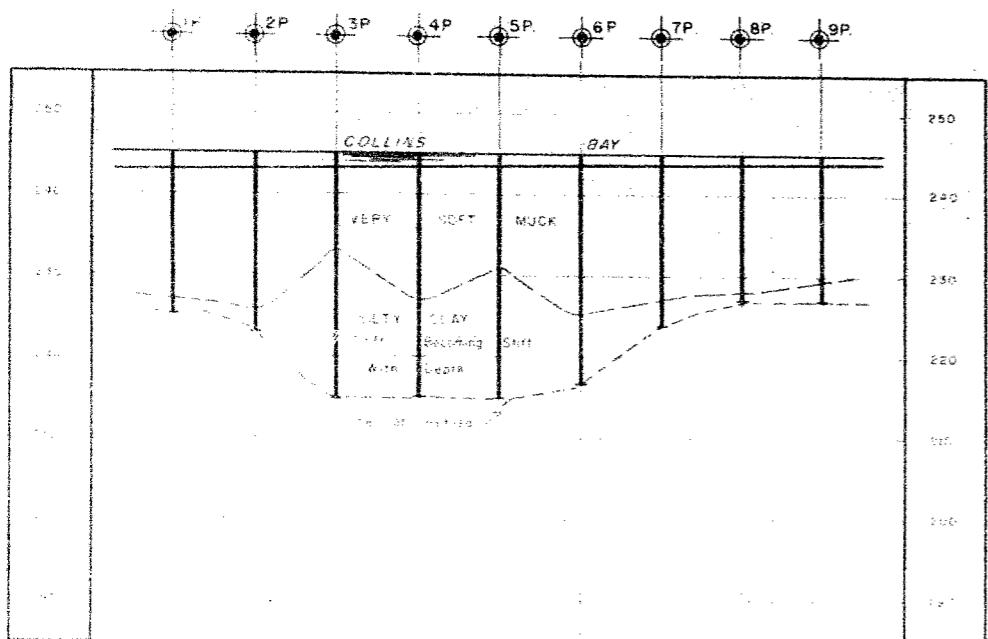
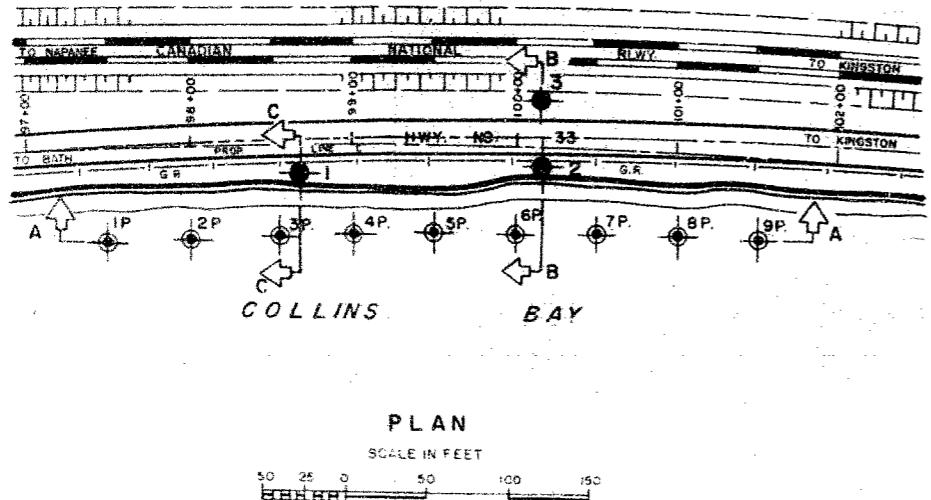
9

DEPARTMENT OF HIGHWAYS ONTARIO

MISCELLANEOUS I

CONTRACT No. 70-174

10



LEGEND

- Bore Hole
- Cone Penetration Hole
- Bore & Cone Penetration Hole
- ▼ Water Levels established at time of field investigation
- Power Auger Hole

NO.	ELEVATION	STATION	OFFSET
1	249.0	98+60	22 RT
2	249.0	100+5	18 RT
3	249.0	100+15	20 LT
4P	245.0	97+50	60 RT
2P	"	98+00	"
3P	"	98+55	"
4P	"	99+00	"
5P	"	99+50	"
6P	"	100+00	"
7P	"	100+50	"
8P	"	101+00	"
9P	"	101+50	"

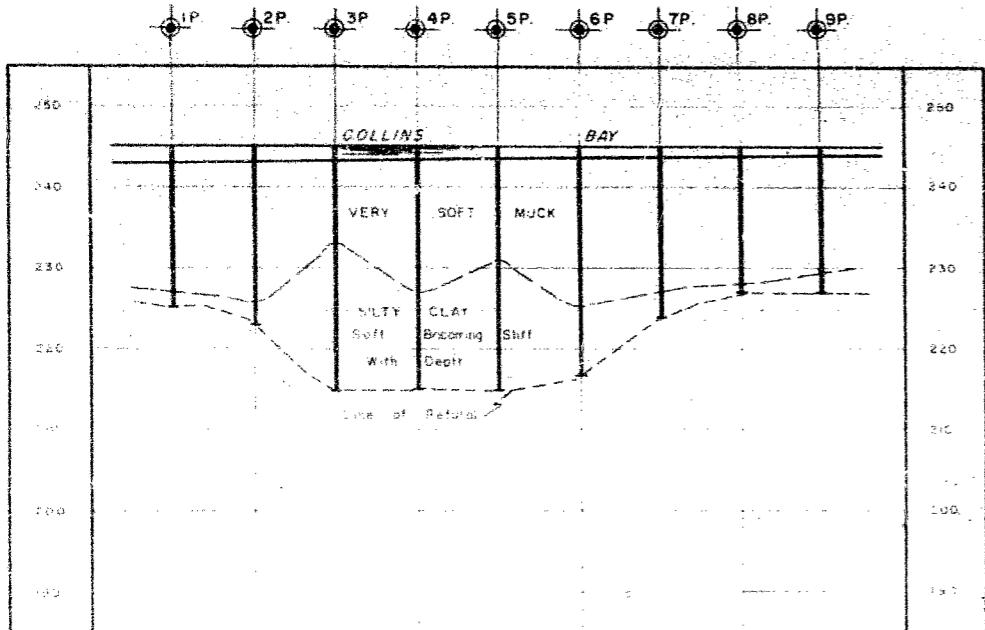
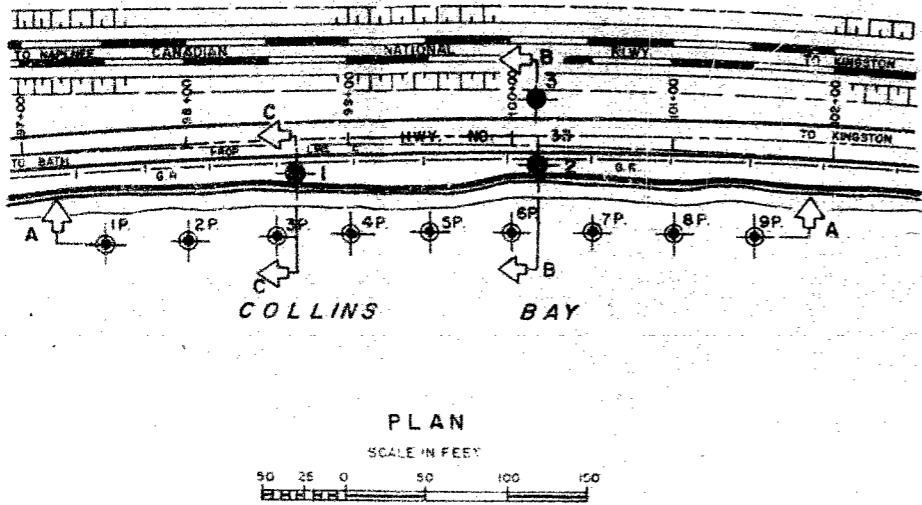
NOTE

The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence and may be subject to considerable error.

DEPARTMENT OF HIGHWAYS - ONTARIO MATERIALS & RESEARCH SECTION

COLLINS BAY FAILURE AREA HWY. NO. 33

ORIGINATED BY	SERIALIZED BY	DISPATCHED BY	DATE
DRAWN BY	CLARK	ARM	JUN 1966
CHECKED BY		CARL W.	
APPROVED BY	W.H. HANNA	W.J. GILL	60-129

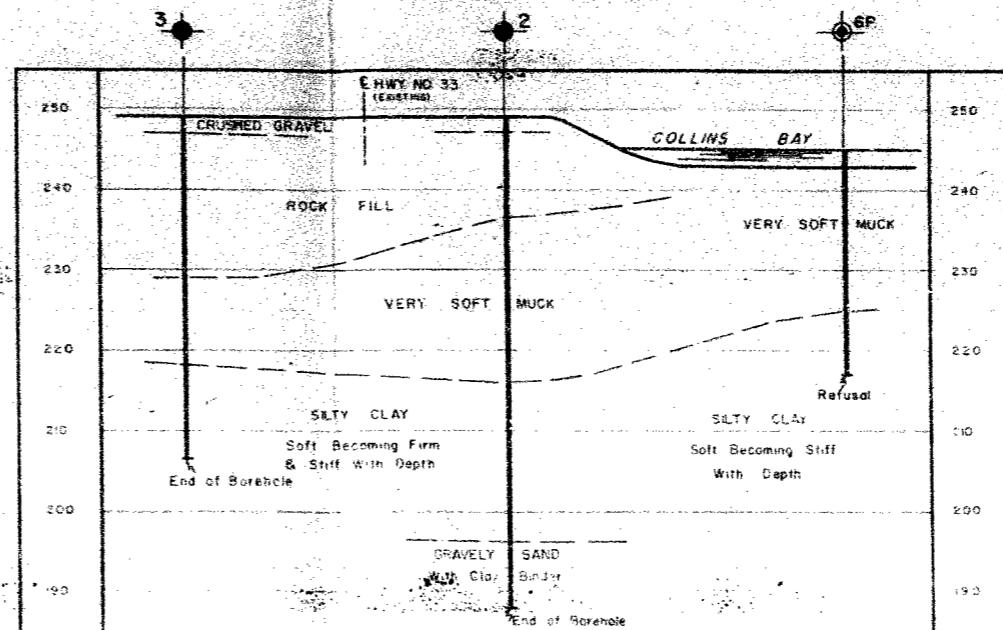
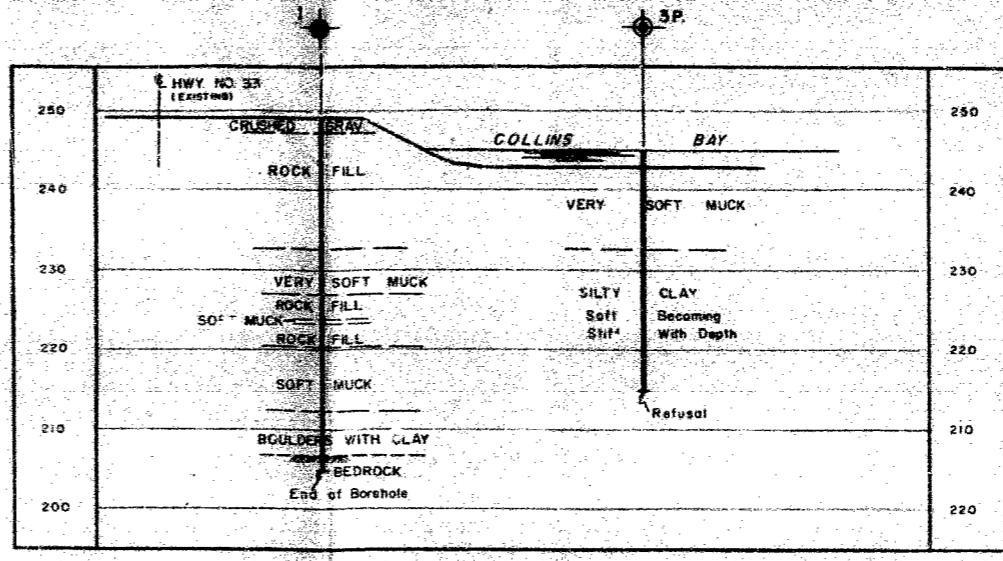


A - A

SCALE IN FEET

50 25 0 50 100 150

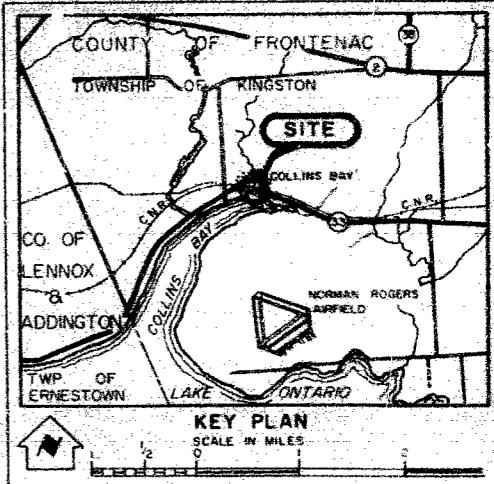
HORIZONTAL SCALE



B - B

SCALE IN FEET

10 5 0 10 20 30



LEGEND

- Bore Hole
- Cone Penetration Hole
- Bore & Cone Penetration Hole
- ▼ Water Levels established at time of field investigation
- ◆ Power Auger Hole

NO.	ELEVATION	STATION	OFFSET
1	249.0	98+00	23' RT
2	249.0	100+15	18' RT
3	249.0	100+15	22' LT
IP	245.0	97+50	60' LT
2P	"	98+00	"
3P	"	98+55	"
4P	"	99+70	"
5P	"	99+50	"
6P	"	100+00	"
7P	"	103+50	"
8P	"	104+00	"
9P	"	104+50	"

NOTE

The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence and may be subject to considerable error.

DEPARTMENT OF HIGHWAYS - ONTARIO
MATERIALS & RESEARCH SECTION

COLLINS BAY FAILURE AREA

HWY. NO. 33

UNNUMBERED OR IDENTIFIED	CONTRACT NO. E	DATE JUNE 14, 1952
DRAWN BY E. L. SPALDING	W.P. NO.	FILE NO.
CHECKED BY	DATE NO.	DRAWING NO.
APPROVED BY E. L. SPALDING	DATE NO.	60-129