

DOCUMENT MICROFILMING IDENTIFICATION

G.I.-30 SEPT. 1976

GEOCRES No. 30M13-44

DIST. 6 REGION CENTRAL

W.P. No. _____

CONT. No. _____

W. O. No. _____

STR. SITE No. _____

HWY. No. _____

LOCATION JANE ST. (NORTH OF
STEELES)

OVERSIZE DRAWINGS TO BE INCLUDED WITH THIS REPORT. _____

REMARKS: _____

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BK 1840

OUR FILE NO.	
OUR ORDER NO.	TD-3640
CLIENT'S ORDER NO.	

DONALD INSPECTION LIMITED

INSPECTING AND TESTING ENGINEERS

1189 GUY STREET
MONTREAL, QUE.340 RICHMOND ST. W.
TORONTO, ONT.

REPORT NO.	
	764-5010
SHEET	OF

RECEIVED

MAY 7 1964

A. D. D.

G. K. H.

J. H.

B. C.

R. C.

P. F.

J. G.

REPORT OF

April 30th, 1964.

SOIL TEST BORINGS**30m 13-44**

GEOCRE No.

For Duncan, Hopper & Associates,

Add-ess 1885 Wilson Avenue, Weston, Ontario

LOCATION: Bridge Site, Jane Street (North Site), Vaughan Township, North of Steeles

REPORTED TO: Duncan Hopper & Associates

We report herein results of two soil test borings made at the above site during the week of April 27th, 1964.

The bore holes were located by your representative at two points. Soil boring logs and bore hole locations are shown on the accompanying sketch which forms a part of this report.

DESCRIPTION OF FIELD WORK

The holes were made using wash-boring methods and using BX casing to maintain the holes clean above the sampling and testing levels.

At shallow intervals through the soil profile, standard penetration tests were made and soil samples were taken.

The standard penetration tests consisted in determining the number of blows required to drive a 2" O.D. split-spoon soil sampler for a depth of 1'-0" into the undisturbed soil by the impact of a 140 lb. hammer dropping freely through a height of 30 inches.

In our laboratory the unconfined compression strength of the cohesive sub-soil was established by conducting strength penetrometer tests on soil samples extracted from the split-spoon soil sampler. Results of strength and penetrometer tests are plotted under columns "Q" and "N" respectively in the boring logs.

DESCRIPTION OF SUB-SOIL

At the surface loose clay fill was present at the two locations and extended to depths of 4' (No. 1) and 7' (No. 2).

Original sub-soil followed and consisted of a highly preconsolidated cohesive glacial till (sandy silt with trace of clay and gravel) which extended to the final levels penetrated.

Below depths of 5' (No. 1) and 9'-6" (No. 2) the sub-soil was in a state

.....continued

DESCRIPTION OF SUB-SOIL.....continued

of very hard consistency having insitu unconfined compression strength values in excess of 4.5 tons/sq.ft.

CONCLUSION

Conventional spread-footings founded on the hard glacial till sub-soil can be satisfactorily utilized for supporting the proposed structure. An allowable soil bearing value of 4 tons/sq.ft. is indicated to be a safe figure for this stratum below depths of 5' (No. 1) and 9'-6" (No. 2).

DONALD INSPECTION LIMITED

S. Nowski

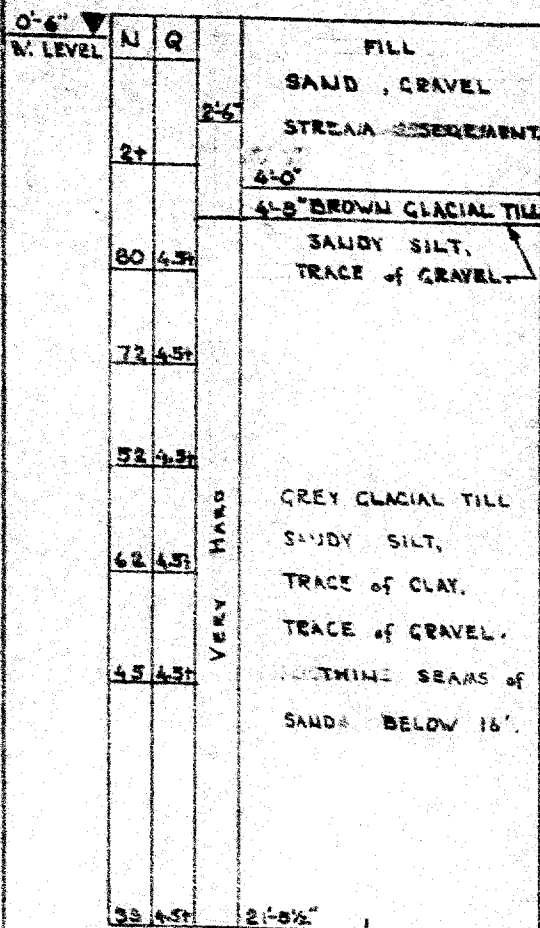
S. Nowski, P. Eng.

SN/bm

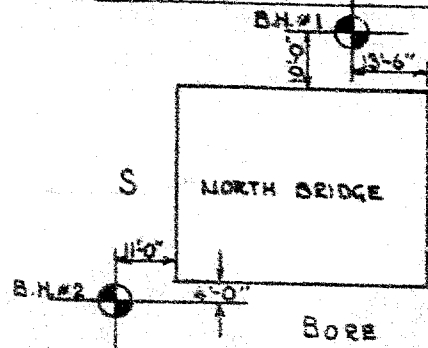
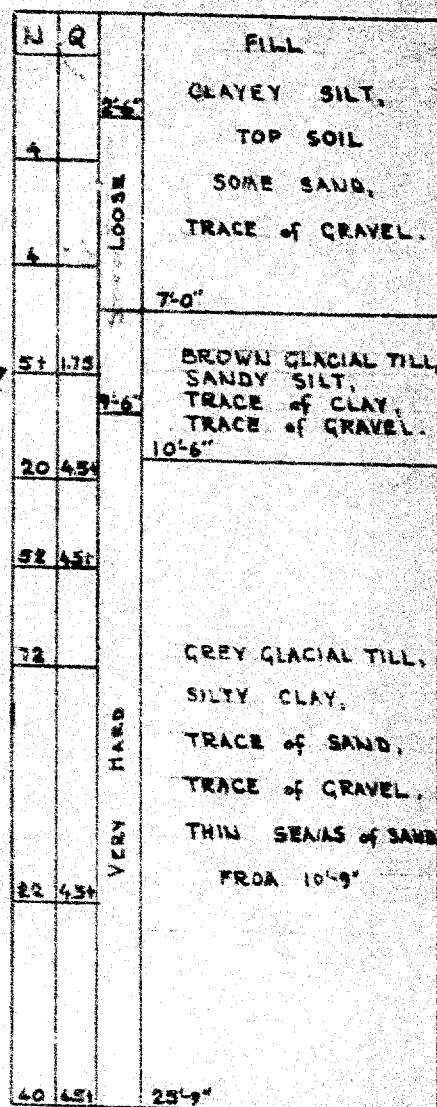
Prep. By

B.H.#1

B.H.#2



9'-0" W. LEVEL



BORE HOLE LOGS,
ON SITE FOR JANE ST. BRIDGE (NORTH).

SCALE, = 1" = 30'-0" & 1" = 4'-0" VERTICALLY.