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GEOCRES No. 30M12-200

DIST. 6 REGION 201
202
203

W.P. No. 20-87-01

CONT. No. _____

W. O. No. _____

STR. SITE No. N/A

HWY. No. 401

LOCATION HWY 401 CULVERTS &
REST AREA (2.0 KM WEST OF
MISSISSAUGA RD.)

OVERSIZE DRAWINGS TO BE INCLUDED WITH THIS REPORT. _____

REMARKS: _____



Ministry of
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FOUNDATION DESIGN SECTION

**foundation
investigation and
design report**

ENGINEERING MATERIALS OFFICE
FOUNDATION DESIGN SECTION

WP 20-87-01-2

DIST 6

HWY 401

STR SITE NA

High Mast Lighting
for
Travel Information Centre

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FOUNDATION INVESTIGATION REPORT

For

High Mast Lighting

for

Travel Information Centre

@ Hwy 401

WP 20-87-01

District 6, Toronto

INTRODUCTION

This report summarizes the foundation investigation results for the High Mast Lighting (HML) at the Travel Information Centre.

The field investigation was conducted between 87-10-05 and 87-10-09 utilizing a continuous flight auger machine equipped with 83 mm I.D. hollow stem augers and BX rock coring techniques.

This work consisted of advancing sampled boreholes at or near all proposed HML locations. Boreholes pertaining to this project are numbered BH 14 to 18, corresponding to the associated HML number of 1 to 5. Borehole locations are summarized in Table 1 and shown in plan on Dwg. No. 208701-D in the Appendix. However, Borehole 15 was moved about 6.5 m northerly from the HML location (HML 2), since the drill rig could not be approached the proposed location.

Site Description

The site is located in the physiographic region known as the "PEEL PLAIN" as described by the Physiography of Southern Ontario (Chapman and Putnam, 1984). The deposit characterizing this area consists of a glacial till containing large amounts of Palaeozoic Shale and limestone. The original ground surface in this region can be described as level to undulating in relief. Specifically, the site under investigation has an undulating topography.

This area is drained by the Credit River. At the site, water drains from the south ends of the culverts along a ditch and through a farm land. During the investigation, the drainage channel was completely dry.

SUBSURFACE CONDITIONS

General

The subsoil consists mainly of Gravelly Sand to Sandy Gravel with varying amounts of cobbles and boulders (Glacial till), overlying up to 0.6 m thick silty clay topsoil layer. The full extent of the glacial till stratum was not investigated, but was found to extend to a maximum depth of 12.7 m below the ground surface. Occasional shaly zones and cohesive zones were encountered in the lower portion of the glacial till stratum. Weathered Shaly slab was encountered in two locations, BH's #16 and #17, at approximate elevation of 185 m. A large size boulder was encountered in BH #18 as shown on borehole log. Groundwater was observed in the boreholes at approximate elevations between 191.8 m and 195.2 m as shown on Borehole logs in the Appendix.

The following is a detailed description of the soils encountered at the site.

Topsoil

The site is covered by a thin 0.6 m layer of silty clay topsoil.

Atterberg Limits tests and grain size distribution test was carried out on the sample and the results are listed below:

	<u>%</u>		<u>%</u>
Moisture Content (w)	15.5	Gravel	2
Liquid Limit (w _L)	28.5	Sand	40
Plastic Limit (w _p)	16.5	Silt	42
Plasticity Index (I _p)	12.0	Clay	16

The topsoil can be described as Silty Clay with random pockets or layers of sand and gravel. The fines are of low plasticity (CL Group).

Gravelly Sand to sandy gravel (Glacial till)

The topsoil is underlain by a glacial till deposit. This material was encountered in all boreholes immediately below the topsoil. Within the glacial till stratum, occasional cohesive zones and occasional cobbles were encountered at the lower portion of the glacial till. Large size boulders was encountered in Borehole #18 as shown on borehole log. Shaly slabs were encountered at an approximate elevation of 185 m in Boreholes #16 and #17. The full extent of the glacial till was not investigated.

Atterberg Limits tests were performed on this material as shown on Figure 1, and the results are summarized below:

	<u>Range %</u>
Moisture Content (w)	4.5 - 17
Liquid Limit (w_L)	16.5 - 33.5
Plastic Limit (w_p)	13 - 17.5
Plasticity Index (I_p)	3.5 - 17

Grain size distribution tests were carried out on the glacial till and the results are summarized below. Figure 2 in the Appendix shows the results in envelope form.

	<u>Range %</u>
Gravel	1 - 59
Sand	21 - 83
Silt	5 - 49
Clay	1 - 10

The glacial till can be described as gravelly sand to sandy gravel with some silt, trace of clay and occasional cobbles and boulders. The matrix is composed primarily of non-cohesive material with occasional cohesive zones that exhibit low plasticity (CL Group).

In this glacial till stratum, the 'N' values ranged from 2 to 112 blows/0.3 m, indicating the deposit may be described as very loose to very dense.

GROUNDWATER CONDITIONS

The groundwater level was measured in open boreholes few days after the boreholes were completed. The groundwater elevation was found to range between elev. 191.8 m and 195.2 m which corresponds to some 2.2 m to 6.4 m below the existing ground surface. The observed groundwater levels for each borehole are indicated on the Record of Borehole Sheets in the Appendix.

DISCUSSION AND RECOMMENDATIONS

A foundation investigation was carried out at the above-noted site in order to establish soil parameters for the design of the 5 HML foundations. The investigation consisted of advancing 1 borehole at each of the HML locations to a depth ranging between 9.4 and 12.7 m below the ground level.

Table 1 indicates the proposed location of each of the 30 m high HML and the appropriate locations of the boreholes. Since the proposed HML location is situated on the mid slope and further permission to enter the private property could not be obtained within the limited time, the borehole location for HML #2 (B.H. No. 15) was moved towards the Hwy. 401 some 6.5 m from the proposed HML location.

Design Consideration

The High Mast Lighting foundations will be supported on a single concrete caissons and the design should be in accordance with the method described by Broms as per the following papers:

Broms, B.B.

Later Resistance of Piles in Cohesive Soils,

Journal of the Soil Mechanics and Foundations Division, ASCE,

Vol. 90, No. SM2

Paper 3825, March 1964

and

Lateral Resistance of Piles in Cohesionless Soils,

Journal of the Soil Mechanics and Foundation Division, ASCE,

Vol. 90, No. SM3

Paper 3909, May 1964

The soil parameters provided in Table 2 are recommended for the design of the HML foundations. The following notation has been adopted:

ϕ = apparent angle of friction for cohesionless soils

q_u = unconfined compressive strength in kPa ($q_u = 2 \times C_u$) for cohesive soils

γ = bulk unit weight in kN/m^3

The material within the zone of frost penetration depth should not be included in calculations of lateral resistance. At this site, the depth of frost penetration is 1.2 m.

Construction Considerations

The investigation revealed that the subsurface conditions across the site are generally uniform and consist of gravelly sand to sandy gravel with varying amounts of cobbles and boulders (Glacial Till). At BH #18 boulders up to 0.5 m in size were encountered where HML #5 is situated as shown on borehole log.

Caissons will be located below the prevailing groundwater in granular subsoil. In view of this, the following special provision should be included in the contract:

"The contractor shall install concrete footings in earth for high mast poles. At the various pole locations, soil deposits consist of mixtures of silts, sands and gravels with occasional cobbles and boulders. Groundwater is likely to be encountered from two to six metres below the existing ground surface. The soil is highly susceptible to conditions of unbalanced hydrostatic head and seepage forces and is likely to 'boil' and become unstable under such conditions. The contractor shall maintain the stability of the soil in the sides and bases of the holes for the concrete footings at all times from commencement of their construction to the placing of concrete".

MISCELLANEOUS

The fieldwork for this investigation was carried out during the period of 87-10-05 to 87-10-09, under the supervision of John Petruzzello (Senior Technician). The equipment was owned and operated by Malone's Soil Samples Co. Ltd. of Etobicoke.

This report was written by T. C. Kim and reviewed by M. Devata, Chief Foundations Engineer (East).



Tae C. Kim
Tae C. Kim, P.Eng.
Project Foundations Engineer

Murty Devata
Murty Devata, P.Eng.
Chief Foundation's Engineer (East)

TABLE 1 - HML AND BOREHOLE LOCATIONS

<u>Proposed HML Locations</u>			<u>Actual Borehole Locations</u>		
<u>HML #</u>	<u>STATION NO.</u>	<u>O/S Fr. Property Line</u>	<u>BH No.</u>	<u>STATION NO.</u>	<u>O/S Fr. Property Line</u>
1	11 + 203.0	11.1 m Lt.	14	11 + 202.9	11.1 m Lt.
2	11 + 351.5	3.7 m Lt.	15	11 + 352.0	10.2 m Lt.
3	11 + 558.5	58.7 m Rt.	16	11 + 560.0	55.6 m Rt.
4	11 + 808.5	6.1 m Lt.	17	11 + 808.4	6.1 m Lt.
5	11 + 959.0	11.6 m Lt.	18	11 + 959.0	11.6 m Lt.

TABLE 2 - Soil Parameters

<u>Pole</u>	<u>Elev. (m)</u>	<u>Type of Soil</u>	<u>Denseness or</u> <u>Consistency</u>	ϕ <u>Degree</u>	<u>qu</u> <u>kPa</u>	γ <u>kN/m³</u>
	<u>From - to</u>					
1 (BH 14)	199.4 - 198.8	Cohesive (Top Soil)	-	-	-	19.3
	198.8 - 197.3	Non-Cohesive	Compact	31	-	20.1
	197.3 - 196.5	Non-Cohesive	Dense	35	-	20.7
	196.5 - 195.0	Non-Cohesive	Very Dense	43	-	21.2
	195.0 - 191.9	Non-Cohesive	Dense	35	-	21.0
	191.9 - 186.7	Non-Cohesive	Very Dense	43	-	21.2
2 (BH 15)	198.2 - 197.6	Cohesive (Top Soil)	-	-	-	19.3
	197.6 - 195.3	Non-Cohesive	Loose	26	-	19.3
	195.3 - 192.6	Non-Cohesive	Dense	36	-	21.0
	192.6 - 191.0	Non-Cohesive	Very Dense	43	-	21.2
	191.0 - 188.0	Non-Cohesive	Dense	34	-	20.7
	188.0 - 185.5	Non-Cohesive	Very Dense	39	-	21.2
3 (BH 16)	196.6 - 196.0	Cohesive (Top Soil)	-	-	-	19.3
	196.0 - 193.7	Non-Cohesive	Loose	27	-	19.3
	193.7 - 188.3	Non-Cohesive	Dense	34	-	20.4
	188.3 - 185.6	Non-Cohesive	Very Dense	39	-	21.2
	185.6 - 184.3	Weathered Shale	Hard	43	-	21.2
4 (BH 17)	194.5 - 193.9	Cohesive (Top Soil)	-	-	-	19.3
	193.9 - 192.3	Non-Cohesive	Dense	35	-	20.7
	192.3 - 190.1	Non-Cohesive	Compact	32	-	20.1
	190.1 - 188.6	Non-Cohesive	Dense	37	-	21.2
	188.6 - 186.3	Non-Cohesive	Very Dense	39	-	21.2
	186.3 - 185.4	Non-Cohesive	Dense	34	-	20.7
	185.4 - 184.4	Weathered Shale	Hard	43	-	21.2
5 (BH 18)	194.7 - 194.2	Cohesive (Top Soil)	-	-	-	19.3
	194.2 - 185.3	Non-Cohesive	Very Dense	43	-	21.2

APPENDIX

EXPLANATION OF TERMS USED IN REPORT

N VALUE: THE STANDARD PENETRATION TEST (SPT) N VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 51mm O.D. SPLIT BARREL SAMPLER TO PENETRATE 0.3m INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5kg, FALLING FREELY A DISTANCE OF 0.76m. FOR PENETRATIONS OF LESS THAN 0.3m N VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N VALUE IS DENOTED THUS \bar{N} .

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (51mm O.D. 60° CONE ANGLE) DRIVEN BY 475 J IMPACT ENERGY ON 'A' SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3m ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

c_u (kPa)	0 - 12	12 - 25	25 - 50	50 - 100	100 - 200	>200
	VERY SOFT	SOFT	FIRM	STIFF	VERY STIFF	HARD

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS/0.3m)	0 - 5	5 - 10	10 - 30	30 - 50	>50
	VERY LOOSE	LOOSE	COMPACT	DENSE	VERY DENSE

ROCKS ARE DESCRIBED BY THEIR COMPOSITION AND STRUCTURAL FEATURES AND /OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (RQD), FOR MODIFIED RECOVERY, IS:

RQD (%)	0 - 25	25 - 50	50 - 75	75 - 90	90 - 100
	VERY POOR	POOR	FAIR	GOOD	EXCELLENT

JOINTING AND BEDDING:

SPACING	50mm	50 - 300mm	0.3m - 1m	1m - 3m	>3m
JOINTING	VERY CLOSE	CLOSE	MOD. CLOSE	WIDE	VERY WIDE
BEDDING	VERY THIN	THIN	MEDIUM	THICK	VERY THICK

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

S S	SPLIT SPOON	T P	THINWALL PISTON
W S	WASH SAMPLE	O S	OSTERBERG SAMPLE
S T	SLOTTED TUBE SAMPLE	R C	ROCK CORE
B S	BLOCK SAMPLE	P H	T W ADVANCED HYDRAULICALLY
C S	CHUNK SAMPLE	P M	T W ADVANCED MANUALLY
T W	THINWALL OPEN	F S	FOIL SAMPLE

STRESS AND STRAIN

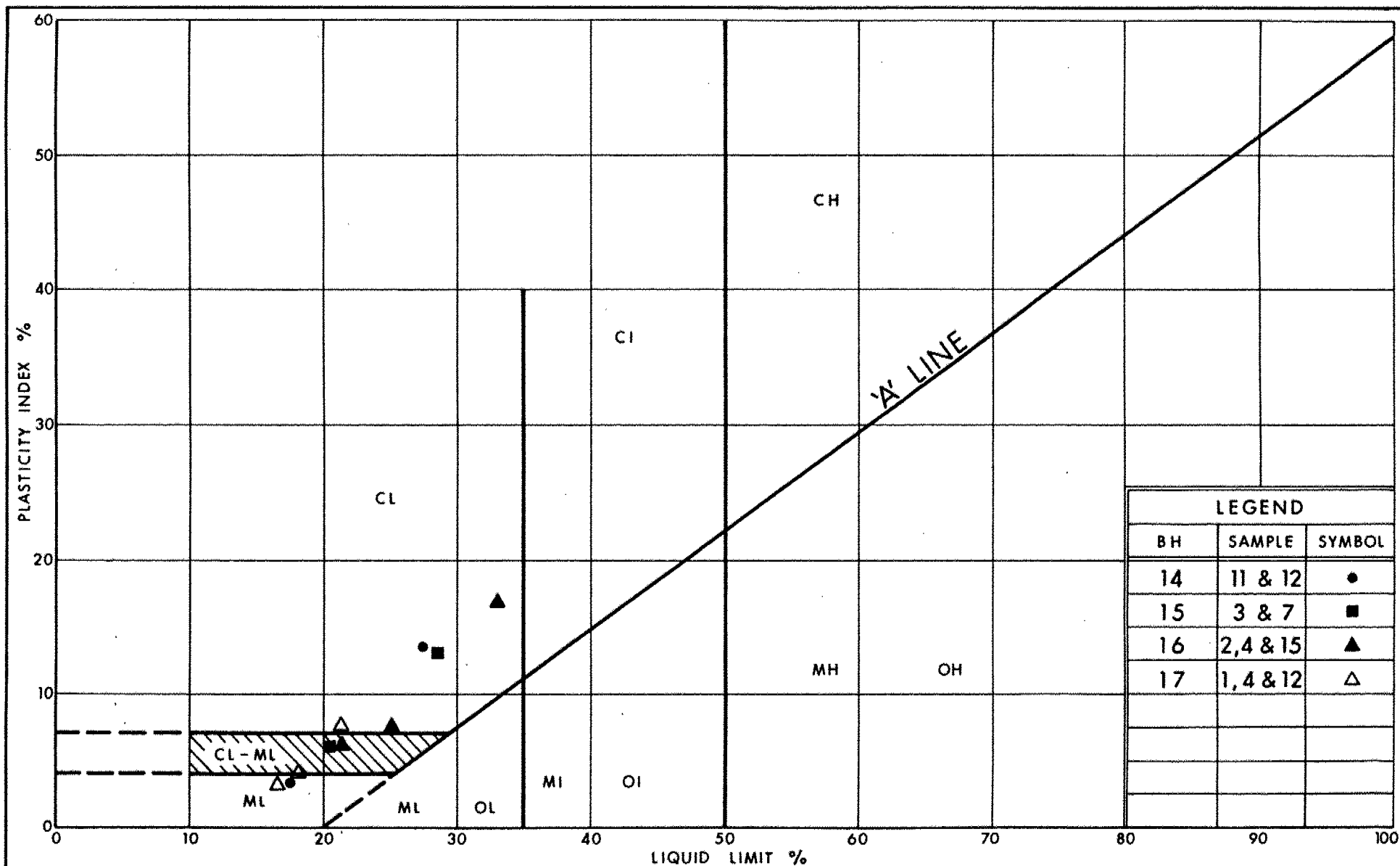
u_w	kPa	PORE WATER PRESSURE
r_u	1	PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ϵ	%	LINEAR STRAIN
$\epsilon_1, \epsilon_2, \epsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF SHEAR DEFORMATION
G	kPa	MODULUS OF LINEAR DEFORMATION
μ	1	COEFFICIENT OF FRICTION

MECHANICAL PROPERTIES OF SOIL

m_v	kPa ⁻¹	COEFFICIENT OF VOLUME CHANGE
C_c	1	COMPRESSION INDEX
C_s	1	SWELLING INDEX
α	1	RATE OF SECONDARY CONSOLIDATION
c_v	m ² /s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v	1	TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
ϕ'	-°	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
ϕ_u	-°	APPARENT ANGLE OF INTERNAL FRICTION
τ_R	kPa	RESIDUAL SHEAR STRENGTH
τ_r	kPa	REMOULDED SHEAR STRENGTH
S_r	1	SENSITIVITY = $\frac{c_u}{\tau_r}$

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m ³	DENSITY OF SOLID PARTICLES	e	1, %	VOID RATIO	e_{min}	1, %	VOID RATIO IN DENSEST STATE
γ_s	kn/m ³	UNIT WEIGHT OF SOLID PARTICLES	n	1, %	POROSITY	I_D	1	DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m ³	DENSITY OF WATER	w	1, %	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kn/m ³	UNIT WEIGHT OF WATER	S_r	%	DEGREE OF SATURATION	D_n	mm	n PERCENT - DIAMETER
ρ	kg/m ³	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u	1	UNIFORMITY COEFFICIENT
γ	kn/m ³	UNIT WEIGHT OF SOIL	w_p	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m ³	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m ³ /s	RATE OF DISCHARGE
γ_d	kn/m ³	UNIT WEIGHT OF DRY SOIL	I_p	%	PLASTICITY INDEX = $w_L - w_p$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m ³	DENSITY OF SATURATED SOIL	I_L	1	LIQUIDITY INDEX = $\frac{w - w_p}{I_p}$	i	1	HYDRAULIC GRADIENT
γ_{sat}	kn/m ³	UNIT WEIGHT OF SATURATED SOIL	I_C	1	CONSISTENCY INDEX = $\frac{w_L - w}{I_p}$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m ³	DENSITY OF SUBMERGED SOIL	e_{max}	1, %	VOID RATIO IN LOOSEST STATE	j	kn/m ³	SEEPAGE FORCE
γ'	kn/m ³	UNIT WEIGHT OF SUBMERGED SOIL						



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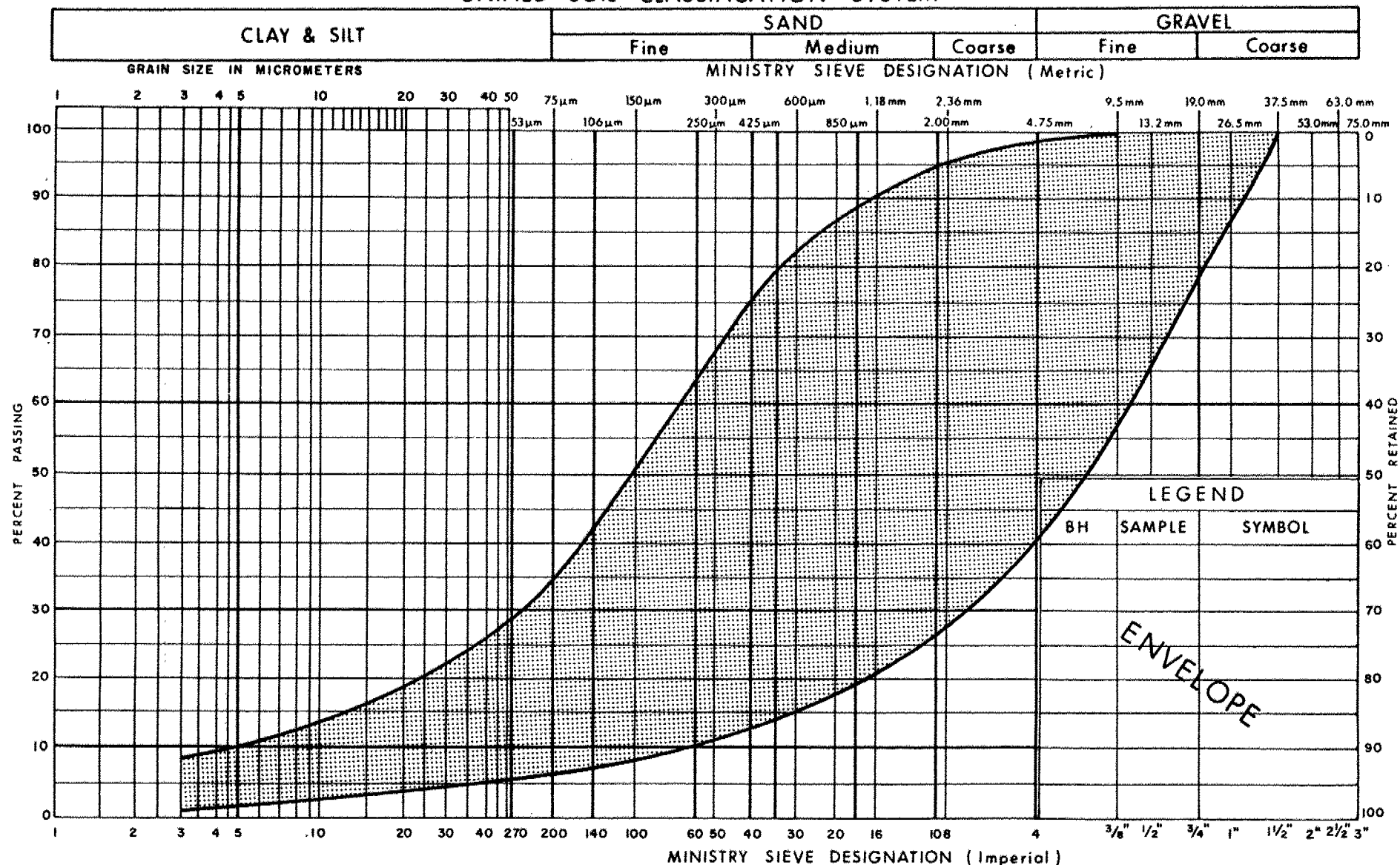
PLASTICITY CHART
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
WITH SOME SILT, TRACE OF CLAY, OCC COHESIVE ZONES

FIG No 1

W P 20-87-01

(H ML)

UNIFIED SOIL CLASSIFICATION SYSTEM



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GRAIN SIZE DISTRIBUTION
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
 WITH SOME SILT, TRACE OF CLAY, OCC COHESIVE ZONES

FIG No 2

W P 20 - 87 - 01

(H M L)

RECORD OF BOREHOLE No 14

METRIC

W P 20-87-01 LOCATION Sta. 11 + 202.9; O/S 11.1 m Lt. of Property Line ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 05 CHECKED BY

SOIL PROFILE		SAMPLES		GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100	SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L	WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER								
199.4	Ground Level										
0.0	Silty Clay with Sand		1	AS*							2 40 42 16
198.8	trace gravel (Top Soil)		2	SS	19						
0.6			3	SS	26						3 80 14 3
	Gravelly Sand to Sandy Gravel with some Silt		4	SS	39						
	trace of clay		5	SS	72						36 47 13 4
	Compact to Very Dense (Glacial Till)		6	SS	112						
			7	SS	31						14 63 17 6
	Occ. Cohesive Zones		8	SS	42						
	Occ. Cobbles		9	SS	42						2 80 15 3
			10	SS	43						4 82 12 2
			11	SS	103						
			12	SS	66						32 47 16 5
			13	SS	62						
			14	SS	43						
186.7	End of Borehole		15	SS	90						37 41 18 4
12.7	* AS: Auger Sample										

OFFICE REPORT ON SOIL EXPLORATION

RECORD OF BOREHOLE No 15

METRIC

W P 20-87-01 LOCATION Sta. 11 + 352.0; O/S 10.2 m Lt. of Property Line ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 09 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT	NATURAL MOISTURE CONTENT	LIQUID LIMIT	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	W _p	W	W _L			WATER CONTENT (%)
								SHEAR STRENGTH						
								○ UNCONFINED ● QUICK TRIAXIAL	+ FIELD VANE x LAB VANE					
198.2	Ground Level													
0.0 197.6 0.6	Silty Clay (Top Soil)						198							
	Gravelly Sand to Sandy Gravel with some Silt trace of clay	Very Loose	1	SS	11								16 58 17 9	
			2	SS	2		196						21 60 15 4	
	Compact to Very Dense (Glacial Till)		3	SS	3									
			4	SS	36		194						5 78 13 4	
			5	SS	48									
			6	SS	45									
191.8 6.4	Weathered Shaly Slab		7	SS	104		192						52 33 12 3	
			8	SS	32		190						13 73 14 0	
	Occ. Cohesive Zones		9	SS	37									
	Occ. Cobbles		10	SS	50		188						5 81 10 4	
185.5			11	SS	67		186							
12.7	End of Borehole													

OFFICE REPORT ON SOIL EXPLORATION



RECORD OF BOREHOLE No 16

METRIC

W P 20-87-01 LOCATION Sta. 11 + 560.0; O/S 55.6 m Rt. of Property Line ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 10 05 & 06 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES						
196.6	Ground Level										
0.0	Silty Clay (Top Soil)		1	AS*							
0.6			2	SS	18						
			3	SS	3						
	Gravelly Sand to Sandy Gravel with some Silt		4	SS	2						
			5	SS	22						
	trace of clay		6	SS	30						
	Compact to Very Dense		7	SS	31						
	(Glacial Till)		8	SS	32						
			9	SS	31						
	Occ. Cohesive Zones		10	SS	38						
	Occ. Cobbles		11	SS	44						
			12	SS	59						
			13	SS	57						
185.6			14	SS	120						
11.0	Weathered Shaly Slab		15	SS	125/20 cm						
184.3			16	SS	105/12 cm						
12.3	End of Borehole										
	* AS: Auger Sample										

OFFICE REPORT ON SOIL EXPLORATION

RECORD OF BOREHOLE No 17

METRIC

W P 20-87-01 LOCATION Sta. 11 + 808.4; O/S 6.1 m Lt. of Property Line ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 07 CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100						
194.5	Ground Level													
0.0 193.9	Silty Clay (Topsoil)													GR SA SI CL
0.6			1	SS	30		194							8 66 21 5
			2	SS	46									
	Gravelly Sand to Sandy Gravel with some Silt		3	SS	29	87 10 9	192							6 83 8 3
	trace of clay		4	SS	32									34 49 12 5
	Compact to Very Dense (Glacial Till)		5	SS	19		190							
			6	SS	45									
	Occ. Cohesive Zones		7	SS	49									28 64 4 4
	Occ. Cobbles		8	SS	50		188							
			9	SS	62									10 60 26 4
			10	SS	70									32 41 22 5
			11	SS	34		186							
185.2			12	SS	100	15 cm								16 49 30 5
9.3 184.4	Weathered Shaly Slab		13	SS	120	15 cm								
10.1	End of Borehole													

OFFICE REPORT ON SOIL EXPLORATION

RECORD OF BOREHOLE No 18

METRIC

W P 20-87-01 LOCATION Sta. 11 + 959.0; O/S 11.6 m Lt. of Property Line ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and BX Casing, Cone Tests COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 06, 07 & 08 CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT Y	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	SHEAR STRENGTH					
194.7	Ground Level													GR SA SI CL
194.7 0.5	Silty Clay (Top Soil)	4					194							45 38 12 5
		1	SS		98/23 cm									50 32 13 5
		2	SS		92/20 cm									59 33 7 1
		3	SS		70									51 34 11 4
	Limestone Boulder	4	SS		*									47 38 14 1
		5	RC		80% 87 10 9									
	Limestone Boulder	6	SS		60% 5 cm									
		7	RC		75%									
	Gravelly Sand to Sandy Gravel some Silt trace of clay Very Dense (Glacial Till) Occ. Cobbles and Boulder	8	SS		70% 10 cm		190							
		9	SS		93		188							
		10	SS		75% 15 cm									
	Limestone Boulders with Sand and Gravel	11	RC		REC 20%		186							10 71 14 5
185.3		12	SS		75% 10 cm									
9.4	End of Borehole													
	* Spoon Bouncing No Sample Recovered													

OFFICE REPORT ON SOIL EXPLORATION

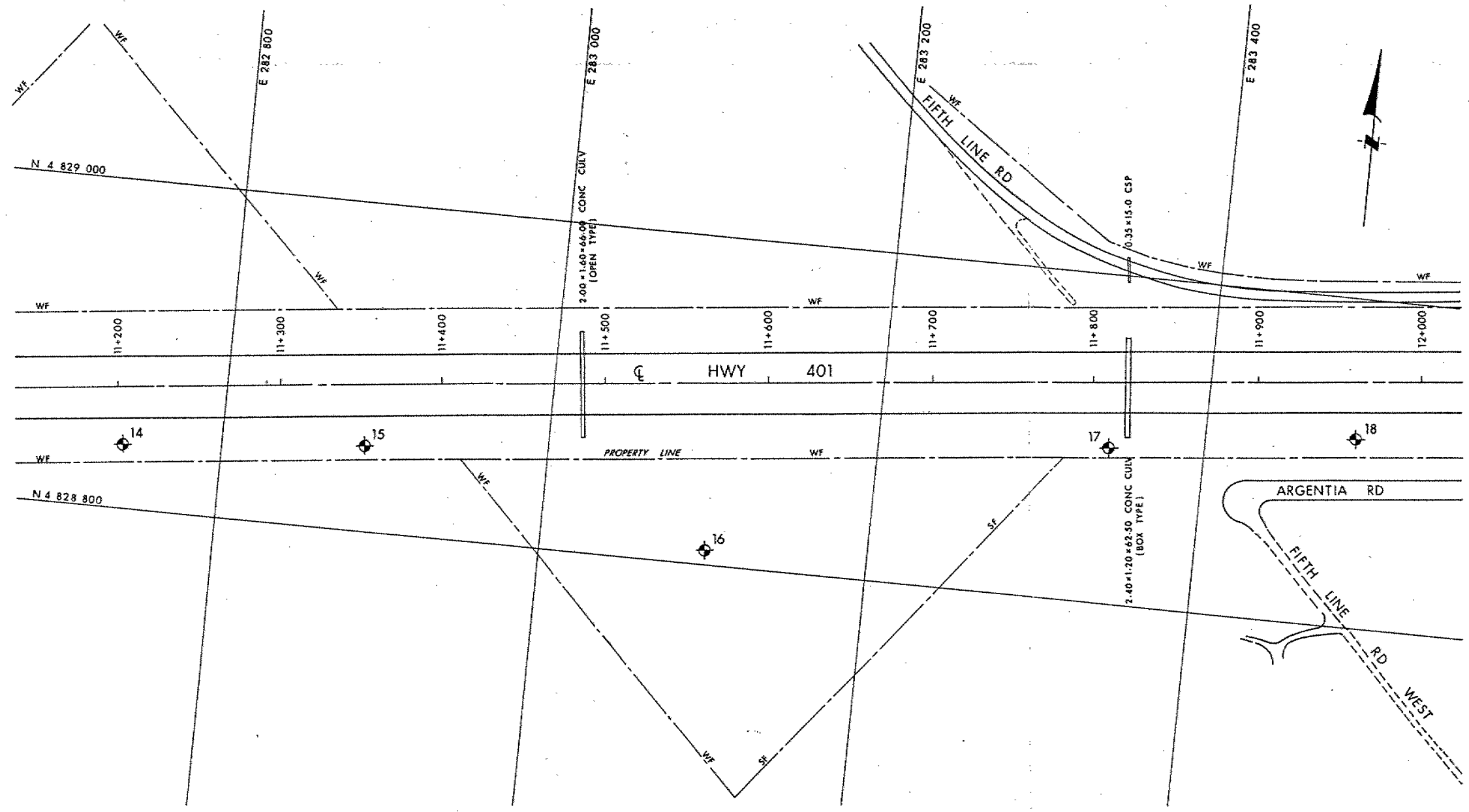
METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES.

CONT No
WP No 20-87-01



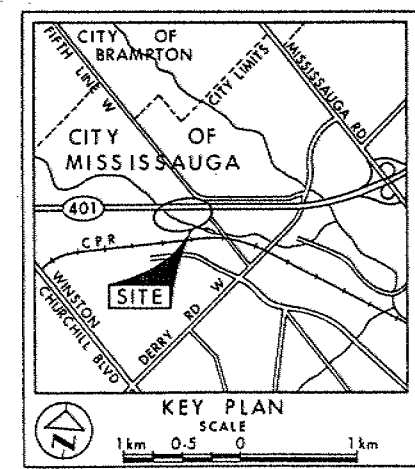
HIGH MAST LIGHTING
(FOR TRAVEL INFORMATION CENTRE)
BORE HOLE LOCATIONS & SOIL STRATA

SHEET



PLAN
SCALE
20m 10 0 20m

NOTE:
For Subsurface details refer to
Record of Borehole sheets.



- LEGEND**
- Bore Hole
 - ⊕ Dynamic Cone Penetration Test (Cone)
 - ⊕ Bore Hole & Cone
 - N Blows/0.3m (Std Pen Test, 475 J/blow)
 - CONE Blows/0.3m (60° Cone, 475 J/blow)
 - W L at time of investigation 82 10

No	ELEVATION	STATION	* OFFSET
14	199.4	11+202.9	11.1m Lt
15	198.2	11+352.0	10.2m Lt
16	196.6	11+560.0	55.6m Rt
17	194.5	11+808.4	6.1m Lt
18	194.7	11+959.0	11.6m Lt

* OFFSET MEASURED FROM (WF) SOUTH
PROPERTY LINE

NOTE
The boundaries between soil strata have been established
only at Bore Hole locations. Between Bore Holes the
boundaries are assumed from geological evidence.

NOTE: The complete foundation investigation and design report for
this project and other related documents may be examined at the
Engineering Materials Office, Downsview. Information contained in
this report and related documents is specifically excluded in
accordance with the conditions of Section 102-2 of Form 100.

REV	DATE	BY	DESCRIPTION
Geocres No 30M12-202			
HWY No 401			DIST 6
SUBMITTAL CHECKED			DATE 87 11 26 SITE
DRAWN DT			APPROVED DWG 208701-D

ENGINEERING MATERIALS OFFICE
FOUNDATION DESIGN SECTION

WP 20-87-01

DIST 6

HWY 401

STR SITE NA

Culvert Extensions
Roadside Rest/Travel
Information Centre

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FOUNDATION INVESTIGATION REPORT
For
Culvert Extensions @ Hwy. 401
WP 20-87-01
District 6, Toronto

INTRODUCTION

This report summarizes the information obtained from a foundation investigation carried out at the above mentioned site between 87-06-30 to 87-07-02 and on 87-07-10. It is proposed to extend two culverts at this site to facilitate the entrance and exit ramps of a Roadside Rest/Travel Information Centre. This report is applicable to the culvert extensions at Sta. 11+497.1 o/s 47.5 m RT from the centreline of Hwy. 401 and Sta. 11+824.8 o/s 37.6 m RT of the centreline of Hwy. 401. This report contains recommendations pertaining to the foundations of the proposed culvert extensions.

SITE DESCRIPTION AND GEOLOGY

This site is located on Hwy. 401, 2.0 km west of Mississauga Road. At present, there are two concrete culverts located at the site. The first culvert, 65 metres in length, is located at Sta. 11+486 and has an outlet elevation of approximately 195.8 metres. The second culvert, 61.5 metres in length, is located at Sta. 11+822 and has an outlet elevation of approximately 194.3 metres. The site is located in the City of Mississauga, Regional Municipality of Peel.

The land use in this area is predominantly industrial and agricultural. Highway 401 runs from east to west along the northern boundary of the site and a farmers field is located to the south of the site. Industrial buildings have been constructed to the east of the site and are under construction on the north side of the 401. To the west of the site, the land is undeveloped and is used for agricultural purposes.

The site is located in the physiographic region known as the "Peel Plain" as described by The Physiography of Southern Ontario (Chapman and Putnam, 1984). The deposit characterizing this area consists of a glacial till containing large amounts of Palaeozoic shale and limestone. The surface in this region can be described as level to undulating in relief. Specifically, the site under investigation has an undulating topography. This area is drained by the Credit River. At the site, water drains from

the south end of the culvert at Sta. 11+486 easterly along a ditch for approximately 70 metres and then southerly through a farm field. This drainage runs parallel to a hill, which is situated between the two culverts. Water on the east side of the hill drains into a ditch which flows easterly towards the culvert at Sta. 11+822. The runoff from the south end of this culvert flows into the ditch, which proceeds easterly for 30 metres before changing course and heading south. During the investigation, ponded water could be founded at a depth of approximately 50 mm in the ditch, at the outlet to the culvert at Sta. 11+822.

The two existing culverts are in good condition showing little or no deterioration. Very little erosion has occurred at the culvert outlets and along the drainage ditches.

Field Investigation

The field investigation was conducted between 87-06-30 and 87-07-02 and on 87-07-10. Two boreholes were advanced; one near the end of each proposed culvert extension. Borehole 1 at Sta. 11+497.1 was advanced to a depth of 14.2 metres, and borehole 2 at Sta. 11+824.8 was advanced to a depth of 9.6 metres below ground level. Dynamic cone penetration tests accompanied each of the boreholes. The borings were advanced by an 87 mm hollow stem augering machine. Soil sampling was performed at 0.76 metre intervals using a 50 mm OD split spoon sampler. Bedrock was not encountered in either borehole.

Groundwater elevations were measured in the boreholes and test pits were excavated to determine if a dewatering scheme was necessary for the excavations. The pits were located at Sta. 11+800, O/S 40 metres RT of Hwy. 401 centreline and Sta. 11+525, O/S 40 metres RT of Hwy. 401 centreline. The pits were 1.2 x 1.2 metres in area. At Sta. 11+800, the test pit reached a depth of 1.9 metres below ground level, and at Sta. 11+525, the test pit reached a depth of 1.8 metres below the ground level. The approximate elevations at the bottom of the excavations are 193.1 and 194.0 respectively.

Lab Work

The engineering properties of the soils were determined by performing grain size distribution and Atterberg Limits Tests on 10 of the samples

obtained. Natural moisture content and bulk density tests were also performed on 7 samples.

The results of laboratory tests are plotted on the record of Borehole Sheets and summarized in the Figures contained in the Appendix.

SUBSURFACE CONDITIONS

General

The stratigraphy across the site is fairly uniform and consists mainly of a sandy glacial till with varying amounts of gravel and cobbles. Underlying an up to 1.0 metre thick sandy topsoil layer, the predominant material across the site, the sandy glacial till stratum, is encountered. The full extent of the glacial till stratum was not investigated, but was found to extend to a maximum depth of 14.2 m below the ground level. Occasional shaly zones, seams of grey silt and grey silty fine sand were encountered between 6 and 14 metres below ground level, in the glacial till stratum. These seams range in thickness from 50 to 75 mm. Bedrock was not encountered in any of the boreholes.

The boundaries of the subsoil types, insitu and laboratory test results, as well as groundwater levels, are shown in the Record of Borehole sheets in the Appendix. The location of each borehole is shown in plan on Dwg. No. 208701-A together with a stratigraphical section.

The following is a detailed description of the soils encountered at the site.

Topsoil

The site is covered in a layer of sandy topsoil with the thickness varying across the site. This material was encountered in BH 1 at a depth of 1.0 metre below the ground level.

Atterberg Limits tests and a grain size distribution test were performed on the sample and the results are listed below.

		%		%
Moisture Content	W	9.5	Gravel	8.5
Liquid Limit	W _L	25.5	Sand	42.5
Plastic Limit	W _p	14.5	Silt	36.5
Plasticity Index	I _p	11	Clay	12.5

The topsoil can be described as a silty sand, some clay, trace gravel. The fines are of low plasticity (CL group). A Standard Penetration 'N' value of 14 indicates the topsoil is in a compact state.

Glacial Till

The topsoil deposit is underlain by a sandy glacial till deposit. The material was encountered in both boreholes; just below ground level in BH 2, and 1.5 metres below ground level in BH 1. Within the glacial till stratum, some silt and silty fine sand seams and occasional pink shaly zones were encountered from 13.7 to 14.2 metres below ground level in Borehole 1, and from 6.5 to 8.7 metres below ground level in Borehole 2. The full extent of the glacial till deposit was not investigated.

Grain size distribution tests were carried out on the glacial till and the results are summarized below. Figure 1 in the Appendix shows the results in envelope form.

	Range %
Gravel	0 - 49.5
Sand	34 - 68.5
Silt	8.5 - 38.5
Clay	1 - 10

Atterberg Limits tests were performed on this material, and the results are summarized below.

		Avg. %	% Range
Moisture Content	W	10	7 - 12
Liquid Limit	W _L	22.2	20 - 25.5
Plastic Limit	W _p	15.2	14.5 - 16.5
Plasticity Index	I _p	7.0	4.5 - 11

To the west, in the vicinity of BH 1, numerous cobbles were encountered during the excavation of the test pit. Easterly, at BH 2, the glacial till stratum contained occasional cobbles.

The glacial till can be described as sand, trace/with gravel, some/with silt, trace clay, with numerous cobbles in the west to occasional cobbles in the east. The matrix is composed primarily of non-cohesive sand with

occasional zones that exhibit low plasticity (CL-ML group or CL group). With depth, the glacial till changed from brown to pink in color.

The silt and silty sand seams and occasional shaly zones yielded 'N' values ranging between 70 and greater than 120 blows/0.3 m, indicating a hard consistency for the cohesive zones and very dense state for the non-cohesive zones.

In the glacial till stratum, the SPT 'N' values ranged from 7 to over 120 blows/0.3 m indicating a state of compaction described as loose to very dense.

GROUNDWATER CONDITIONS

Water levels were measured during the time of the investigation in the open boreholes. In BH 1, the water level was at an elevation of 196.1, and in BH 2, the water level was 190.30. Water was used to advance BH 1 and it is believed the established water level is not representative of the stabilized water level. Test pit #2 in the vicinity of BH 1, was completed to an elevation of 194.0 and the water table was not encountered. Test pit #1, in the vicinity of BH 2, was advanced to an elevation of 193.1 without encountering the water table.

DISCUSSION AND RECOMMENDATIONS

It is proposed to extend two culverts on the south side of the 401, 2.0 km west of Mississauga Road, to facilitate the construction of entrance and exit ramps at a proposed Roadside Rest/Travel Information Centre. The culvert at Sta. 11+486, a 1.8 x 2.24 x 66 m (HxWxL) open culvert, will be extended approximately 18 metres. The culvert at Sta. 11+822, a 1.6 x 2.84 x 61.5 m (HxWxL) box culvert, will be extended approximately 4 metres. The dimensions of the existing culverts have been determined in the field since site plans were not available.

It is proposed to construct the extensions as box culverts, cast against the existing culverts, with construction joints providing a non rigid connection.

The soil stratigraphy at the site consists of a sandy glacial till. This deposit extends to a depth of 14.2 m below ground level (the extent of this investigation). Seams of silty fine sand, shaly zones and grey silt are encountered in the till deposit at elevations 185.2 and 188.0 metres (in BH 1 and 2 respectively).

The glacial till is predominantly sand with numerous cobbles towards the west end of the site and occasional cobbles towards the east.

The natural groundwater elevation is believed to be at an elevation of 190.3 metres in BH 2. A test pit in the vicinity of BH 1 indicates the water table to be lower than the elevation measured in BH 1 after drilling was completed. The test pit was completed to an elevation of 194.0 metres without encountering the water table. It is believed, therefore, that the natural water table elevation is below 194 metres in the vicinity of BH 1.

The following are our foundation recommendations for the design and construction of the two culvert extensions located at Sta. 11+497.1 and Sta. 11+824.8 on Hwy. 401, 2.0 km east of Mississauga Road.

Culvert Foundations

The following table provides details for the design of the box culvert extensions.

Culvert Extension Location	Ref. BH	Approx. Base of Culvert Elev.	Factored Bearing Capacity at U.L.S. kPa	Bearing Capacity @ S.L.S II kPa
Sta. 11+497.1 O/S 47.5 m RT	1	195.7	490	350
Sta. 11+824.8 O/S 37.6 m RT	2	194.0	280	200

The founding material is a sandy glacial till with numerous to occasional cobbles. It is recommended to excavate an additional 150 mm below the approximate base of culvert elevation to facilitate a granular pad, in order to alleviate the problem of boulders or cobbles providing an uneven surface for the culvert foundation.

Frost Protection

Normally 1.2m of earth cover or equivalent, to the base of the foundation is required for frost protection. However, in this case, it has been proposed to match the culvert extensions to the existing culvert foundations.

Settlement - A construction joint between the culverts and culvert extensions is recommended to facilitate any differential settlement that occurs. Due to the nature of the material, settlement problems are not expected.

Earth Pressures

Backfill to the culverts should consist of granular material in accordance with MTC Standard Special Provision #121 (83-10) and placed in accordance with OPSD 803.02. For design purposes, the following physical properties can be assumed for backfill.

<u>Material</u>	<u>ϕ</u>	<u>γ</u>
Granular "A"	35°	22.0 kN/m ³
Granular "B"	30°	21.2 kN/m ³

Computation of earth pressures should be in accordance with Section 6-6.1.2 of the O.H.B.D.C.

Sliding Resistance

Sliding resistance between concrete and the granular pad should be calculated in accordance with Section 6-7.3.3.2 of the O.H.B.D.C. assuming and unfactored ϕ value of 35° for granular "A". Within the native material, sliding should also be calculated in accordance with Section 6-7.3.3.2.

Dewatering

The groundwater levels measured in BH 1 and 2 were 196.1 metres and 190.3 metres respectively. Two test pits were excavated at Sta. 11+800 O/S 40 m RT and 11+525 O/S 40 m RT. Since the water table was not encountered at either location, no major dewatering problems are anticipated. If the water table is low, no problems will occur. However, any minor seepage or surface runoffs into the excavation can be handled by oversized excavation with perimeter drainage ditches and sump pumps.

The upstream flow from the culverts should be diverted during construction to avoid runoff into the open excavation.

Stability of Slopes

For fill heights in the order of 3 metres, stability problems are not anticipated if slopes are constructed with 2H:1V slope.

Construction

During excavation, care should be exercised to prevent undermining the existing culvert foundations. Dewatering during the excavation should be provided if required, and no excavation should be done under the existing slab. Immediately after the excavation, backfill to the base of the existing slab with granular "A" is required for a distance of 2 metres.

MISCELLANEOUS

The fieldwork for this investigation was carried out during the period from 87-06-30 to 87-07-02 and also on 87-07-10, under the supervision of Mary Lou Pauly. The equipment was owned and operated by Eastern Soil of Scarborough.

This report was written by M. L. Pauly and reviewed by D. Dundas, Sr. Foundation Engineer and M. Devata, Chief Foundation Engineer (East).

Mary Lou Pauly

Mary Lou Pauly
Project Foundation Engineer



D. H. Dundas

D. H. Dundas, P. Eng.
Senior Foundations Engineer

A P P E N D I X

EXPLANATION OF TERMS USED IN REPORT

N VALUE: THE STANDARD PENETRATION TEST (SPT) N VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 51mm O.D. SPLIT BARREL SAMPLER TO PENETRATE 0.3m INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5kg, FALLING FREELY A DISTANCE OF 0.76m. FOR PENETRATIONS OF LESS THAN 0.3m N VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N VALUE IS DENOTED THUS \bar{N} .

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (51mm O.D. 60° CONE ANGLE) DRIVEN BY 475 J IMPACT ENERGY ON 'A' SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3m ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

c_u (kPa)	0 - 12	12 - 25	25 - 50	50 - 100	100 - 200	> 200
	VERY SOFT	SOFT	FIRM	STIFF	VERY STIFF	HARD

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS/0.3m)	0 - 5	5 - 10	10 - 30	30 - 50	> 50
	VERY LOOSE	LOOSE	COMPACT	DENSE	VERY DENSE

ROCKS ARE DESCRIBED BY THEIR COMPOSITION AND STRUCTURAL FEATURES AND /OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (RQD), FOR MODIFIED RECOVERY, IS:

RQD (%)	0 - 25	25 - 50	50 - 75	75 - 90	90 - 100
	VERY POOR	POOR	FAIR	GOOD	EXCELLENT

JOINTING AND BEDDING:

SPACING	50mm	50 - 300mm	0.3m - 1m	1m - 3m	> 3m
JOINTING	VERY CLOSE	CLOSE	MOD. CLOSE	WIDE	VERY WIDE
BEDDING	VERY THIN	THIN	MEDIUM	THICK	VERY THICK

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

SS	SPLIT SPOON	TP	THINWALL PISTON
WS	WASH SAMPLE	OS	OSTERBERG SAMPLE
ST	SLOTTED TUBE SAMPLE	RC	ROCK CORE
BS	BLOCK SAMPLE	PH	TW ADVANCED HYDRAULICALLY
CS	CHUNK SAMPLE	PM	TW ADVANCED MANUALLY
TW	THINWALL OPEN	FS	FOIL SAMPLE

STRESS AND STRAIN

u_w	kPa	PORE WATER PRESSURE
r_u	1	PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ϵ	%	LINEAR STRAIN
$\epsilon_1, \epsilon_2, \epsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF LINEAR DEFORMATION
G	kPa	MODULUS OF SHEAR DEFORMATION
μ	1	COEFFICIENT OF FRICTION

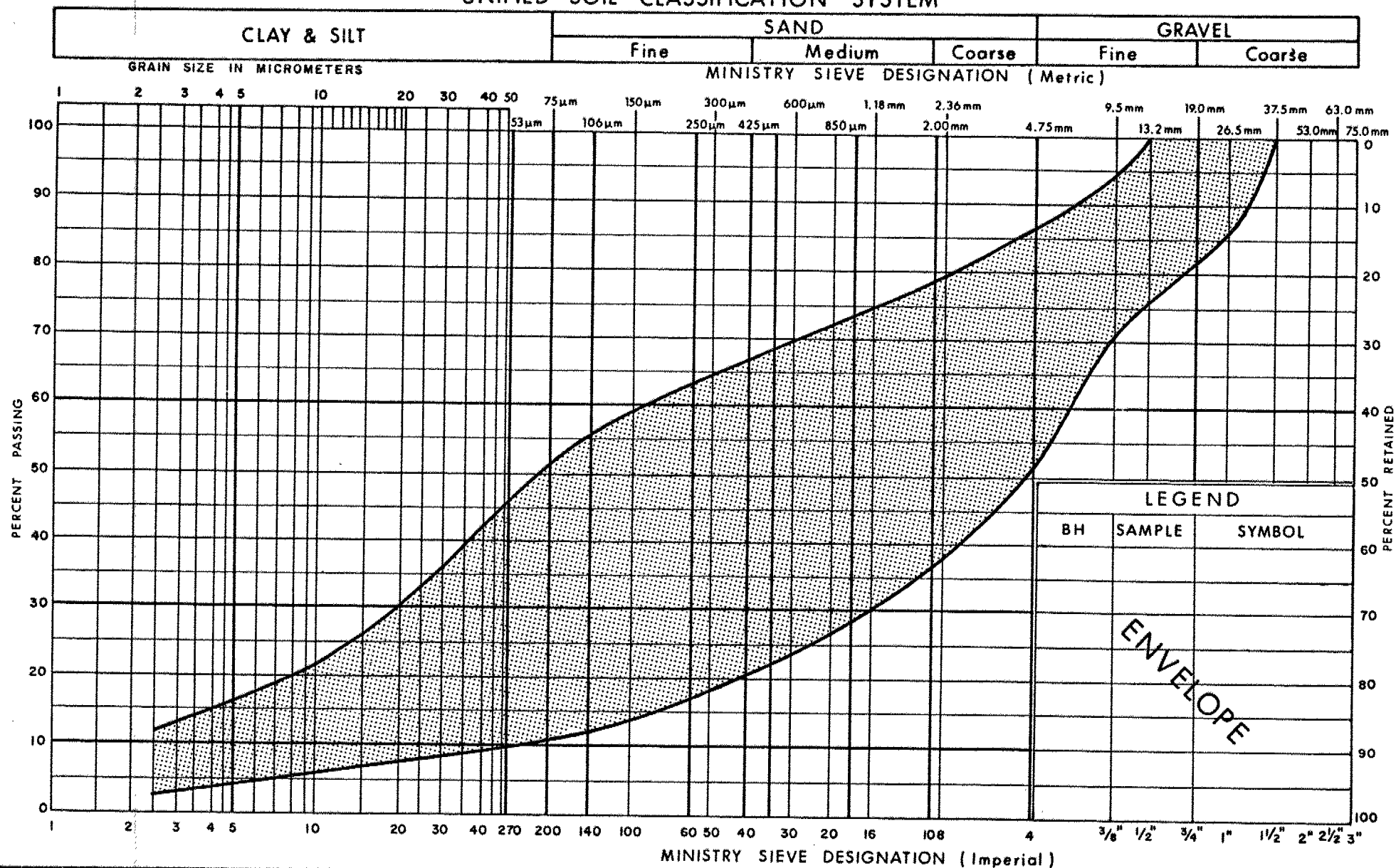
MECHANICAL PROPERTIES OF SOIL

m_v	kPa ⁻¹	COEFFICIENT OF VOLUME CHANGE
C_c	1	COMPRESSION INDEX
C_s	1	SWELLING INDEX
C_α	1	RATE OF SECONDARY CONSOLIDATION
c_v	m ² /s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v	1	TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
ϕ'	-°	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
ϕ_u	-°	APPARENT ANGLE OF INTERNAL FRICTION
τ_r	kPa	RESIDUAL SHEAR STRENGTH
τ_r	kPa	REMOULDED SHEAR STRENGTH
S_t	1	SENSITIVITY = $\frac{c_u}{\tau_r}$

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m ³	DENSITY OF SOLID PARTICLES	e	1, %	VOID RATIO	e_{min}	1, %	VOID RATIO IN DENSEST STATE
γ_s	kN/m ³	UNIT WEIGHT OF SOLID PARTICLES	n	1, %	POROSITY	I_D	1	DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m ³	DENSITY OF WATER	w	1, %	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kN/m ³	UNIT WEIGHT OF WATER	S_r	%	DEGREE OF SATURATION	D_n	mm	n PERCENT - DIAMETER
ρ	kg/m ³	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u	1	UNIFORMITY COEFFICIENT
γ	kN/m ³	UNIT WEIGHT OF SOIL	w_p	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m ³	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m ³ /s	RATE OF DISCHARGE
γ_d	kN/m ³	UNIT WEIGHT OF DRY SOIL	I_p	%	PLASTICITY INDEX = $w_L - w_p$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m ³	DENSITY OF SATURATED SOIL	I_L	1	LIQUIDITY INDEX = $\frac{w - w_p}{I_p}$	i	1	HYDRAULIC GRADIENT
γ_{sat}	kN/m ³	UNIT WEIGHT OF SATURATED SOIL	I_C	1	CONSISTENCY INDEX = $\frac{w_L - w}{I_p}$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m ³	DENSITY OF SUBMERGED SOIL	e_{max}	1, %	VOID RATIO IN LOOSEST STATE	j	kn/m ³	SEEPAGE FORCE
γ'	kN/m ³	UNIT WEIGHT OF SUBMERGED SOIL						

UNIFIED SOIL CLASSIFICATION SYSTEM



**Ministry of
Transportation and
Communications**

GRAIN SIZE DISTRIBUTION

SAND, TRACE/WITH GRAVEL, SOME/WITH SILT, TRACE/SOME CLAY
(Glacial Till)

FIG No 1

W P 20-87-01



RECORD OF BOREHOLE No 1

METRIC

W P 20-87-01 LOCATION Sta. 11 + 497.1; O/S 47.5 m Rt. 4 Hwy. 401 ORIGINATED BY MLP
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY MLP
DATUM Geodetic DATE 1987 06 30 & 1987 07 02 CHECKED BY DD

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	W _p					
199.0	Ground Level													
0.0	Sand Some Clay Trace													
198.1	Gravel (Topsoil)													
0.9	Loose to Compact 													

+³, x⁵; Numbers refer to
Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10

RECORD OF BOREHOLE No 2

METRIC

W P 20-87-01 LOCATION Sta. 11 + 824.8; O/S 37.6 m Rt. of E Hwy. 401 ORIGINATED BY MLP
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY MLP
 DATUM Geodetic DATE 1987 06 30 CHECKED BY DD

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20	40	60	80	100					
194.5	Ground Level																GR SA SI CL
0.0			1	SS	7		194							o np			13 69 14 4
	Loose to Compact		2	SS	13												0 67 29 4
	Sand		3	SS	30		192										37 52 9 2
	Trace/with Gravel		4	SS	43												
	Some/with Silt		5	SS	39												
	Trace/Some Clay		6	SS	57		190							o np			29 60 10 1
	Occasional Cobbles		7	SS	88												
	Dense to Very Dense (Glacial Till)		8	SS	70		188										
	Occ. slightly Cohesive Zones		9	SS	120/	29 cm											
	Seams of Silt and Shaly Zones		10	SS	69		186										
			11B	SS	120/	20 cm								o np			5 63 27 5
184.9			12	SS	120/	23 cm											
9.6	End of Borehole																



RECORD OF TEST PIT No 1

METRIC

W P 20-87-01 LOCATION Sta. 11 + 800; O/S 40.0 m Rt. of 4 Hwy. 401 ORIGINATED BY MLP
DIST 6 HWY 401 BOREHOLE TYPE Test Pit (1.2m x 1.2m) COMPILED BY MLP
DATUM Geodetic DATE 1987 07 10 CHECKED BY DD

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20	40	60	80	100					
195.0	Ground Level																
0.0	Sand with Silt Trace to Some Gravel Trace Clay Occ. Cobbles					*	194										
193.1																	
1.9	End of Test Pit * Water Table not encountered																

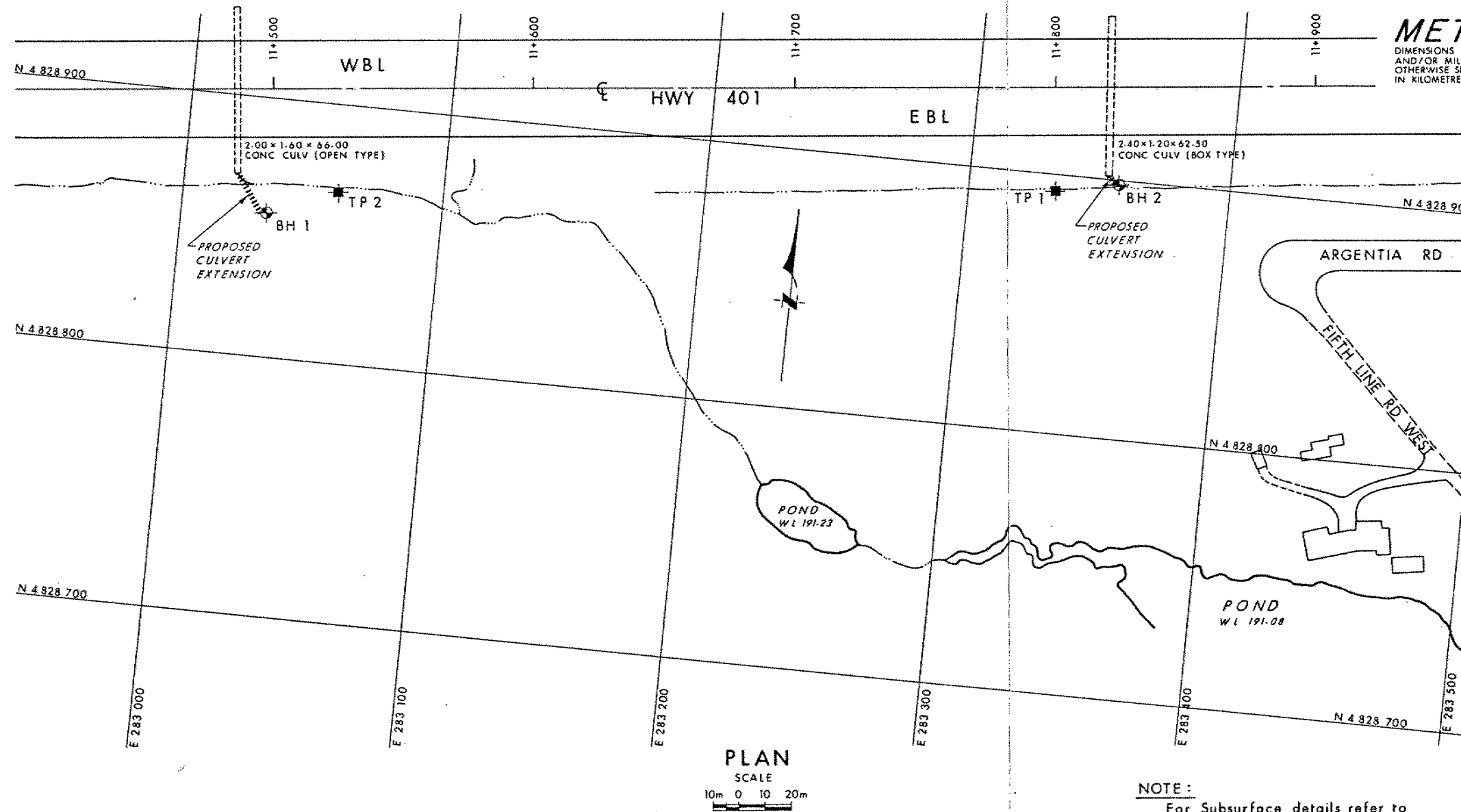


RECORD OF TEST PIT No 2

METRIC

W P 20-87-01 LOCATION Sta. 11 + 525; O/S 40.0 m Rt. of Hwy. 401 ORIGINATED BY MLP
DIST 6 HWY 401 BOREHOLE TYPE Test Pit (1.2m x 1.2m) COMPILED BY MLP
DATUM Geodetic DATE 1987 07 10 CHECKED BY DD

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20	40	60	80	100					
195.8	Ground Level																
0.0	Sand with Gravel Trace/Some Silt/Clay Numerous Cobbles					*	195										
194.0	End of Test Pit																
1.8	* Water Table not encountered																

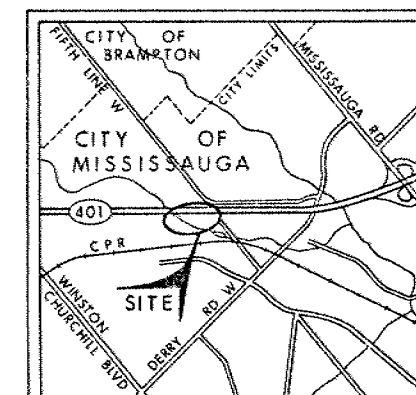


CONT No
WP No 20-87-01

CULVERT EXTENSIONS
BORE HOLE LOCATIONS & SOIL STRATA



SHEET



LEGEND

- ◆ Bore Hole
- ⊕ Dynamic Cone Penetration Test (Cone)
- ◆ Bore Hole & Cone
- N Blows/0.3m (Std Pen Test, 475 1/blow)
- CONE Blows/0.3m (60° Cone, 475 1/blow)
- W.L. at time of investigation 87 07
- ★ Test Pit

No	ELEVATION	STATION	OFFSET
BH 1	199.0	11+497.1	47.5m Rt
BH 2	194.5	11+824.8	37.6m Rt
TP 1	195.0	11+800.0	40.0m Rt
TP 2	195.8	11+525.0	40.0m Rt

NOTE

The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence.

NOTE: The complete foundation investigation and design report for this project and other related documents may be examined at the Engineering Materials Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with the conditions of Section 102-2 of Form 100.

REV	DATE	BY	DESCRIPTION

Geocres No 30M12-200

HWY No 401	CHECKED	DATE 87 07 14	DIST 6
SUBMITTAL	CHECKED	APPROVED	SITE
DRAWN	CHECKED	APPROVED	DWG 208701-A

ENGINEERING MATERIALS OFFICE
FOUNDATION DESIGN SECTION

WP 20-87-01

DIST 6

HWY 401

STR SITE NA

Pedestrian Bridge and
Concrete Culvert at
Travel Information Centre

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FOUNDATION INVESTIGATION REPORT

For

The Pedestrian Bridge

And

Concrete Culvert

At

The Travel Information Centre

@ Hwy 401

WP 20-87-01

District 6, Toronto

INTRODUCTION

This report summarizes the information obtained from a foundation investigation carried out at the above mentioned site between 87-10-14 and 87-10-15. This report contains factual information together with recommendations pertaining to the foundations of the proposed pedestrian bridge and concrete culvert at the Travel Information Centre as shown on Drawing No. 208701-E.

SITE DESCRIPTION

This site is located on Hwy 401, 2.0 km West of Mississauga Rd. in the city of Mississauga, Regional Municipality of Peel. At present, there are no buildings except two culverts are located north of the site immediately south of Hwy 401.

The site is located in the physiographic region known as the "PEEL PLAIN" as described by the Physiography of Southern Ontario (Chapman and Putnam, 1984). The deposit characterizing this area consists of a glacial till containing large amounts of Palaeozoic shale and limestone. The original ground surface in this region can be described as level to undulating in relief. Specifically, the site under investigation has an undulating topography. However, it should be noted that the existing ground surface for the proposed pedestrian bridge and concrete culvert at the Travel Information Centre consists of various fill material placed during the early construction of Hwy. 401.

This area is drained by the Credit River. At the site, water drains from the south end of the culvert at sta. 11 + 486 of Hwy. 401 easterly along a ditch for approximately 70 metres and then southerly through a farm land. this drainage runs parallel to the proposed building site. During the investigation, the drainage channel was completely dry.

Field Investigation

The field investigation was conducted between 87-10-14 and 87-10-15. Three boreholes were advanced within the foundation areas of the Pedestrian bridge and concrete culvert as shown on Drawing No. 208701-E. Borehole No's 19 to 21 were advanced to a depth of 8.1 m to 11.2 m as shown on borehole logs.

Dynamic cone penetration tests accompanied each of the boreholes. The borings were advanced by means of 83 mm hollow stem augering machine. Soil sampling was performed at 0.76 m intervals using a 50 mm OD split spoon sampler. Bedrock was not proven in any of the borehole by coring methods except weathered shaly slabs were encountered at a depth between 7.5 m and 10.1 m below the ground surface, approximate elevation of 186.0 m, in two boreholes as shown on borehole logs (Section E-E, Figure 1).

Laboratory Work

The engineering properties of the soils were determined by performing grain size distribution and Atterberg Limits Tests. In addition, natural moisture content was also performed.

The results of laboratory tests are plotted on the record of Borehole Sheets and summarized in the Figures contained in the Appendix.

SUBSURFACE CONDITIONS

General

The soil stratigraphy at the site consists of fill material which consists of a mixture of silt, sand and gravel with random pockets or layers of silty clay, underlain by a compact to very dense gravelly sand to sandy gravel with some silt and trace of clay.

The fill material contains various amounts of old concrete slabs, tree trunk and steel debris. The thickness of the fill varies from 1.4 metre at Borehole 19 up to 3.7 metre at Borehole 20. A tree trunk of about 0.6 m in thickness was encountered within the fill material at Borehole 20.

The glacial till is predominantly sand with various amounts of gravel and occasional cobbles. Seams of silty fine sand, Shaly slabs and occasional cohesive zones are encountered within this glacial till deposit. This deposit extends to a depth of at least 10.1 m below ground level, underlain by weathered Shaly slabs.

The boundary of the Subsoil types and laboratory tests results, as well as groundwater levels, are shown in the Record of Borehole Sheets in the Appendix. The location of each borehole is shown in plan on Dwg. No. 208701-E. A stratigraphical section is shown on Figure 1.

The following is a detailed description of the soils encountered at the site.

Fill Materials

The site is covered with fill material which also contains various amounts of old concrete slabs, tree trunks and steel debris, and the overall thickness varies across the site. This material was encountered in all boreholes ranging in depth from 1.4 metres up to 3.7 metres as shown on Section E-E (Figure 1). Two different types of fill material can be categorized as follows:

i) Mixture of silt, Sand and Gravel (Fill Material)

A mixture of silt, sand and gravel was encountered in Boreholes 19 and 21. This fill material is extended to a depth of approximately 2.1 m below the existing ground surface.

Atterberg Limits tests and grain size distribution tests were performed on the samples and the results are shown on Figures 2 and 4 and listed below:

	<u>Range %</u>
Moisture Content (w)	16 - 17.5
Liquid Limit (w_L)	24.5 - 30.0
Plastic Limit (w_p)	17 - 21.5
Plasticity Index (I_p)	7.5 - 8.5

	<u>Range %</u>
Gravel	24 - 64.5
Sand	19 - 47
Silt	10 - 21
Clay	5 - 8

This material can be described as a mixture of silt, sand and gravel with random pockets or layers of silty clay. Based on the results of Atterberg Limits tests, these pockets can generally be considered to be CL or CI material.

Based on the interpretation of Standard penetration Test 'N' values ranging between generally 6 and 11 blows/0.3 m, this fill material is generally subjected to poor compaction.

ii) Silty clay (Fill Material)

In Borehole #20 the fill material is cohesive consisting of Silty clay extending to a depth of approximately 3.7 m below the existing ground surface.

Atterberg Limits tests and grain size distribution tests were carried out on the samples and the results are shown on Figures 2 and 4 and listed below:

	<u>Range %</u>
Moisture Content (w)	16.5 - 24.5
Liquid Limit (w_L)	30 - 43.0
Plastic Limit (w_p)	15 - 25.0
Plasticity Index (I_p)	15 - 19.5

	<u>Range %</u>
Gravel	5 - 9
Sand	18 - 39
Silt	39 - 46
Clay	13 - 30

This fill material can be described as Silty clay with random pockets or layers of sand and gravel. The plasticity is generally low or medium (CL or CI). Standard Penetration 'N' values between 7 and 15 indicate the cohesive fill is in a firm to stiff state.

Gravelly Sand to Sandy Gravel (Glacial Till)

The fill material is underlain by a glacial till deposit. This material was encountered in all boreholes immediately below the fill material, about 1.4 m below ground level in BH 19, and about 3.7 m below ground level in BH 20. Within the glacial till stratum, occasional cohesive zones and occasional cobbles were encountered at the lower portion of the glacial till. Shaly slabs were encountered at an approximate elevation 186.0 m in Borehole Nos. 19 and 20 as shown on borehole logs. The full extent of the glacial till deposit was not investigated.

Atterberg Limits tests were performed on this material as shown on Figure 3, and the results are summarized below:

	<u>Range %</u>
Moisture Content (W)	8 - 21
Liquid Limit (w_L)	19.5 - 31
Plastic Limit (w_p)	3.5 - 17
Plasticity Index (I_p)	7 - 10

Grain size distribution tests were carried out on the glacial till and the results are summarized below. Figure 5 in the Appendix shows the result in envelope form.

	<u>Range %</u>
Gravel	3 - 84.5
Sand	14 - 85.5
Silt	2 - 20
Clay	1 - 10

The glacial till can be described as gravelly sand to sandy gravel with some silt, trace of clay and occasional cobbles. The matrix is composed primarily on non-cohesive sand with occasional cohesive zones that exhibit low plasticity (CL- ML Group). With depth, the glacial till changed from brown to pink in color.

In the glacial till stratum, the 'N' values ranged from 3 to 51 blows/0.3 m indicating that the deposit is loose to very dense.

GROUNDWATER CONDITIONS

The groundwater level was measured in open boreholes few days after the boreholes were completed. The groundwater elevation was found to range between Elev. 192.0 m and 192.5 m, which corresponds to some 1.4 to 4.4 m below the existing ground surface. The observed groundwater levels are indicated on the Record of Borehole Sheets in the Appendix and are shown on the stratigraphical section (Figure 1).

DISCUSSION AND RECOMMENDATIONS

As mentioned above, the soil stratigraphy at the site consists of a mixture of fill material, underlain by a sandy glacial till. The fill material contains various amounts of old concrete slabs, tree trunks and steel debris. The thickness of this fill varies from 1.4 m at Borehole 19 up to 3.7 m at Borehole 20. The glacial till is predominantly sand with various amounts of gravel and occasional cobbles. Occasional cohesive zones encountered within this glacial till deposit. This deposit extends to a depth of about 10 m below ground level, underlain by Weathered Shaly slab at an approximate elevation of 186.0 m.

The groundwater elevation is believed to be at an elevation of approximately 192.0 m at the location of the bridge, which is slightly higher than that of pond water of 191.2 m. The groundwater elevation at the proposed culvert location is approximately 192.5 m.

Our foundation recommendations for the design and construction of the Pedestrian Bridge and Concrete Culvert at the Travel Information Centre as follows:

1. Foundations for Pedestrian Bridge

Spread Footings

It is recommended that the new pedestrian bridge structure be supported on spread footing type of foundations placed within the natural subsoil of compact to very dense granular natural soil or on well compacted Granular 'A' material. The founding elevations and the parameters for the footing design are as follows:

Location	Footing Elev. (m)	Factored U.L.S. (kPa)	S.L.S. Type II (kPa)
East and West	191.5	700	300
Abutments	or below		

The abutments can alternatively be perched within a well-compacted core of Granular 'A' provided that the existing fill material is removed. The details of this engineered fill are shown on Figure 6. If this scheme is adopted, loadings of 900 kPa at U.L.S. and 350 kPa S.L.S. Type II can be used in the design.

Resistance to sliding of the footings can be calculated assuming a coefficient of friction of 0.6 between the underside of the concrete footing and granular type material and that of 0.7 between the underside of the concrete footing and the Granular 'A' core.

Piles

Alternatively, the bridge structure can be supported on end bearing steel 'H' piles (310 HP 110) driven into the weathered shaly slab using the following design values and tip elevations:

Location	Footing Tip Elev. (m)	Factored U.L.S. (kPa)	S.L.S. Type II (kPa)
East and West Abutments	185.0	1600	980

2. Foundation for Concrete Culvert

Based on the site investigation, the proposed concrete culvert may be constructed on a Granular 'A' pad and the following table provides details for the design of the concrete culvert.

Ref. BH	Approx. Base of Culvert Elev. m	Factored U.L.S. (kPa)	S.L.S. Type II (kPa)
21	193.3	600	250

All fill material should be removed to its full depth within the foundation area to approximate Elev. 192.0 m.

The excavated area should be backfilled with well compacted Granular 'A' material to the base of the culvert as shown on Figure 7.

Earth Pressures

The Granular 'A' or 'B' backfill to the abutments of the bridge as well as the culvert should be in accordance with Special Provision No. 121 (dated October, 1983).

The following parameters are recommended for the granular backfill:

	<u>Granular 'A'</u>	<u>Granular 'B'</u>
Angle of Internal Friction	$\phi = 35^\circ$	30°
Unit Weight (kN/m^3)	$\gamma = 22.8$	21.2
Coefficient of Active Earth Pressure (K_A)	0.27	0.33
Coefficient of Earth Pressure at Rest (K_0)	0.43	0.5

Computation of earth pressure should be in accordance with Section 6.6.1.2. of the O.H.B.D.C.

Frost Protection

The footings should be placed so as to have a minimum earth cover of 1.2 m to allow for frost protection.

Construction

During excavation, care shall be exercised by not leaving any old fill material underneath the proposed footings. Dewatering for the footing excavations may be a problem, since excavations will be carried out below the water level in granular subsoil. One method of achieving this is carrying out excavations by means of oversize perimeter ditches and constantly pumping out water from the ditches as shown on Figure 7.

Slope Stability

For fill heights in the order of 4 metres, stability problems are not anticipated if slopes are constructed at 2H:1V. However, erosion protection for the toe of the slope would be required along the relocated channel. Armor stones or Gabion stones would be appropriate for this purposes.

MISCELLANEOUS

The fieldwork for this investigation was carried out during the period of 87-10-14 to 87-10-15, under the supervision of Mike Shenarr (Student Engineer). The equipment was owned and operated by Master Soil Investigation Ltd. of Toronto.

This report was written by T.C. Kim and reviewed by M. Devata, Chief Foundation Engineer (East).



Tae C. Kim

Tae C. Kim, P.Eng.

Project Foundations Engineer

Murty Devata

Murty Devata, P.Eng.

Chief Foundations Engineer (East)

APPENDIX

EXPLANATION OF TERMS USED IN REPORT

N VALUE: THE STANDARD PENETRATION TEST (SPT) N VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 51mm O.D. SPLIT BARREL SAMPLER TO PENETRATE 0.3m INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5kg, FALLING FREELY A DISTANCE OF 0.76m. FOR PENETRATIONS OF LESS THAN 0.3m N VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N VALUE IS DENOTED THUS \bar{N} .

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (51mm O.D. 60° CONE ANGLE) DRIVEN BY 475 J IMPACT ENERGY ON 'A' SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3m ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

c_u (kPa)	0 - 12	12 - 25	25 - 50	50 - 100	100 - 200	> 200
	VERY SOFT	SOFT	FIRM	STIFF	VERY STIFF	HARD

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS / 0.3 m)	0 - 5	5 - 10	10 - 30	30 - 50	> 50
	VERY LOOSE	LOOSE	COMPACT	DENSE	VERY DENSE

ROCKS ARE DESCRIBED BY THEIR COMPOSITION AND STRUCTURAL FEATURES AND / OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (R Q D), FOR MODIFIED RECOVERY, IS:

RQD (%)	0 - 25	25 - 50	50 - 75	75 - 90	90 - 100
	VERY POOR	POOR	FAIR	GOOD	EXCELLENT

JOINTING AND BEDDING:

SPACING	50mm	50 - 300mm	0.3m - 1m	1m - 3m	> 3m
JOINTING	VERY CLOSE	CLOSE	MOD. CLOSE	WIDE	VERY WIDE
BEDDING	VERY THIN	THIN	MEDIUM	THICK	VERY THICK

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

S S	SPLIT SPOON	T P	THINWALL PISTON
W S	WASH SAMPLE	O S	OSTERBERG SAMPLE
S T	SLOTTED TUBE SAMPLE	R C	ROCK CORE
B S	BLOCK SAMPLE	P H	T W ADVANCED HYDRAULICALLY
C S	CHUNK SAMPLE	P M	T W ADVANCED MANUALLY
T W	THINWALL OPEN	F S	FOIL SAMPLE

STRESS AND STRAIN

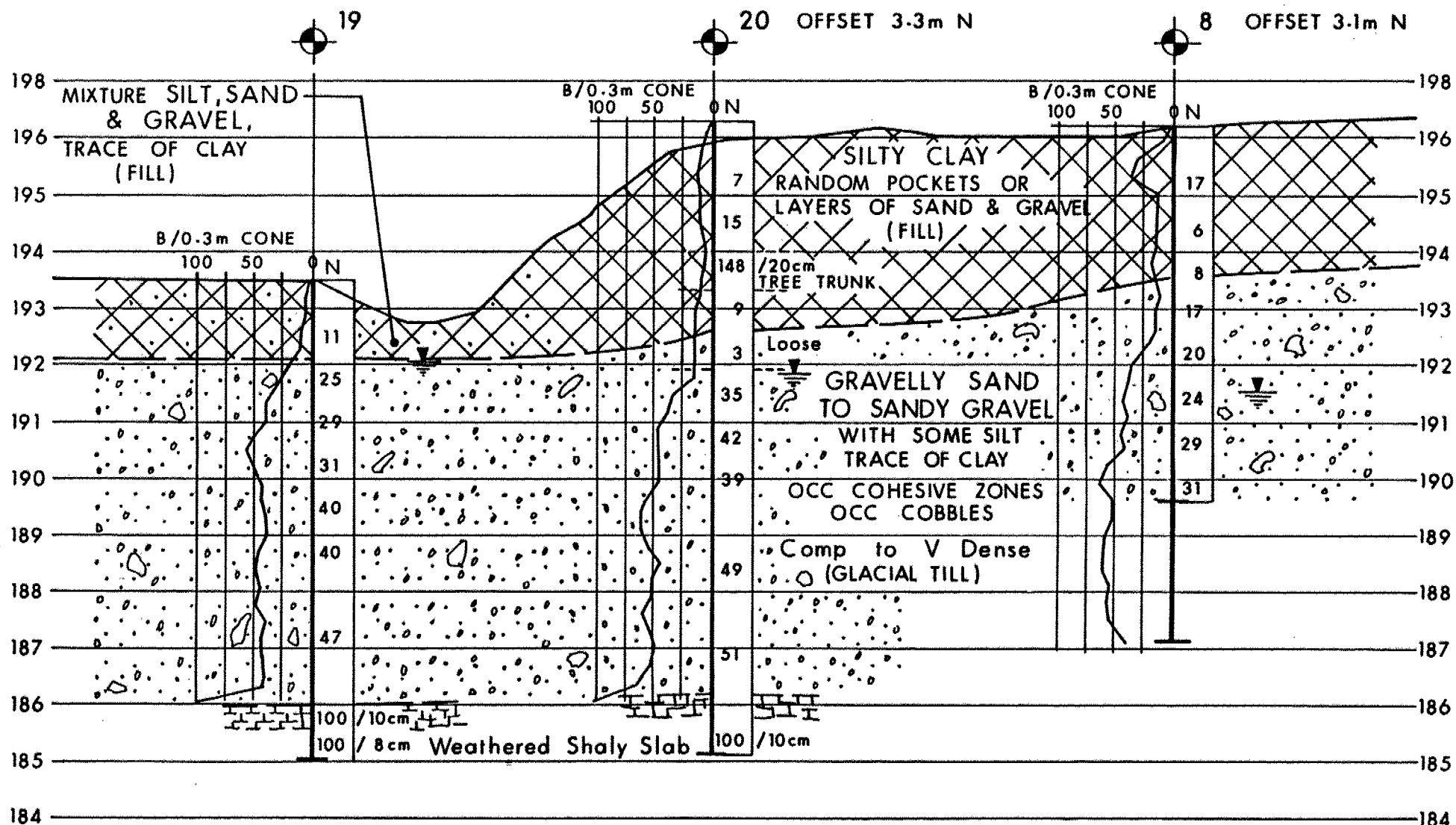
u_w	kPa	PORE WATER PRESSURE
r_u	1	PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ϵ	%	LINEAR STRAIN
$\epsilon_1, \epsilon_2, \epsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF LINEAR DEFORMATION
G	kPa	MODULUS OF SHEAR DEFORMATION
μ	1	COEFFICIENT OF FRICTION

MECHANICAL PROPERTIES OF SOIL

m_v	kPa ⁻¹	COEFFICIENT OF VOLUME CHANGE
C_c	1	COMPRESSION INDEX
C_s	1	SWELLING INDEX
C_α	1	RATE OF SECONDARY CONSOLIDATION
c_v	m ² /s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v	1	TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
ϕ'	-°	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
ϕ_u	-°	APPARENT ANGLE OF INTERNAL FRICTION
τ_R	kPa	RESIDUAL SHEAR STRENGTH
τ_r	kPa	REMOULDED SHEAR STRENGTH
S_t	1	SENSITIVITY = $\frac{c_u}{\tau_r}$

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m ³	DENSITY OF SOLID PARTICLES	e	1, %	VOID RATIO	e_{min}	1, %	VOID RATIO IN DENSEST STATE
γ_s	kn/m ³	UNIT WEIGHT OF SOLID PARTICLES	n	1, %	POROSITY	I_D	1	DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m ³	DENSITY OF WATER	w	1, %	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kn/m ³	UNIT WEIGHT OF WATER	S_r	%	DEGREE OF SATURATION	D_n	mm	n PERCENT - DIAMETER
ρ	kg/m ³	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u	1	UNIFORMITY COEFFICIENT
γ	kn/m ³	UNIT WEIGHT OF SOIL	w_p	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m ³	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m ³ /s	RATE OF DISCHARGE
γ_d	kn/m ³	UNIT WEIGHT OF DRY SOIL	I_p	%	PLASTICITY INDEX = $w_L - w_p$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m ³	DENSITY OF SATURATED SOIL	I_L	1	LIQUIDITY INDEX = $\frac{w - w_p}{I_p}$	i	1	HYDRAULIC GRADIENT
γ_{sat}	kn/m ³	UNIT WEIGHT OF SATURATED SOIL	I_C	1	CONSISTENCY INDEX = $\frac{w_L - w}{I_p}$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m ³	DENSITY OF SUBMERGED SOIL	e_{max}	1, %	VOID RATIO IN LOOSEST STATE	j	kn/m ³	SEEPAGE FORCE
γ'	kn/m ³	UNIT WEIGHT OF SUBMERGED SOIL						



SECTION E-E (PEDESTRIAN BRIDGE LOCATION)

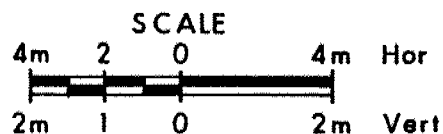
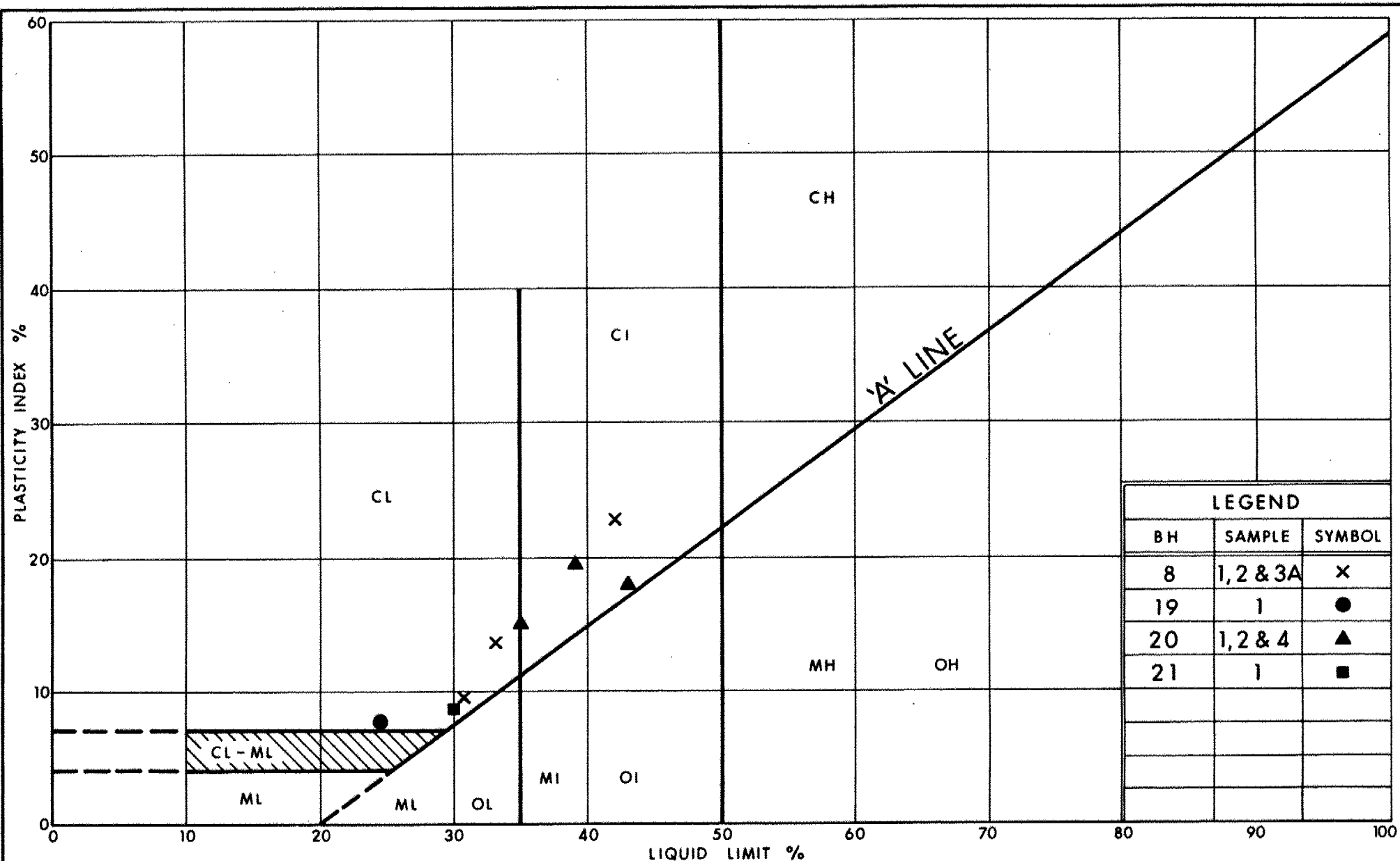


Fig 1
WP 20-87-01



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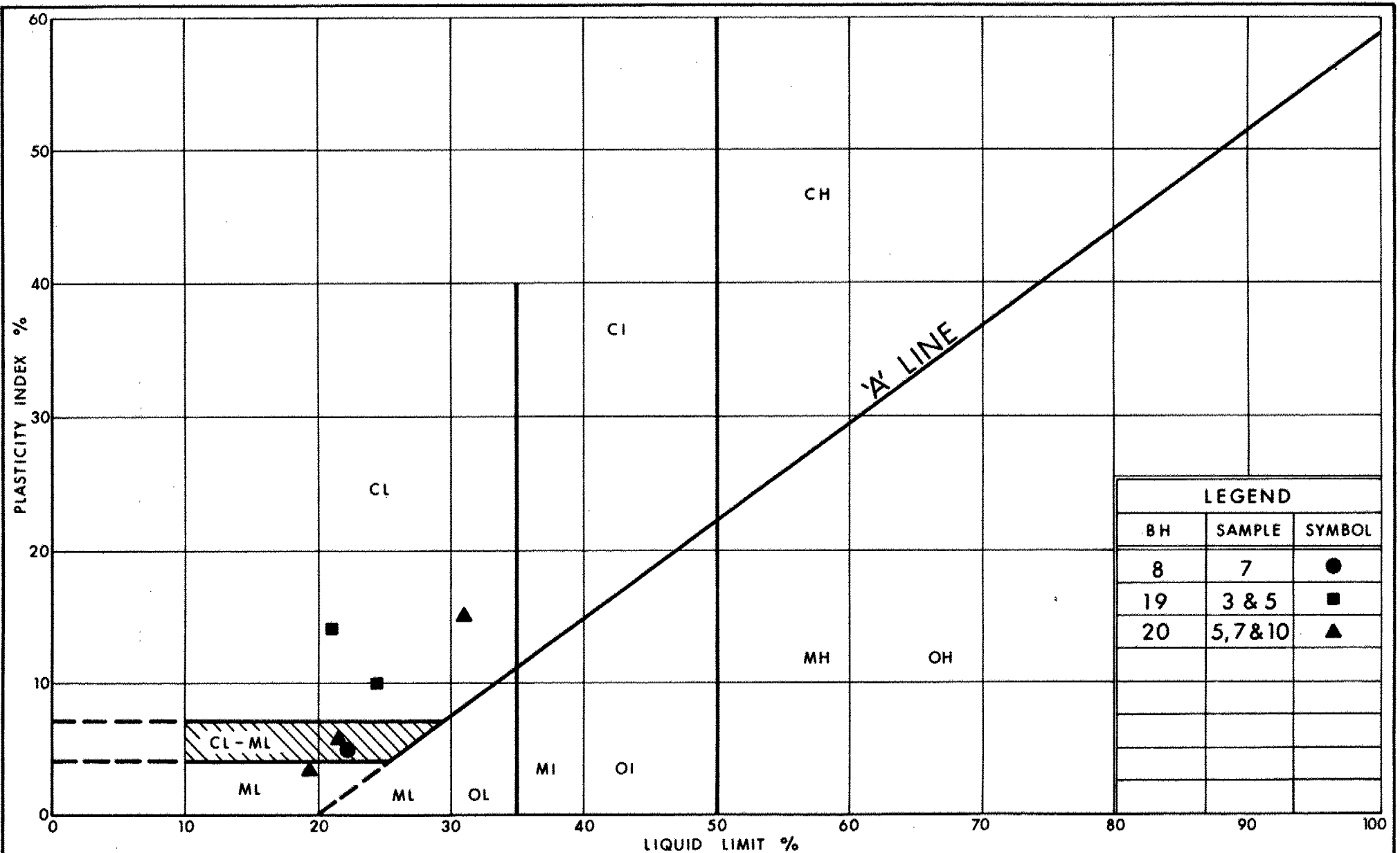
Ontario

PLASTICITY CHART FILL MATERIAL

FIG No 2

W P 20 - 87 - 01

(BRIDGE & CULVERT)



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Ontario

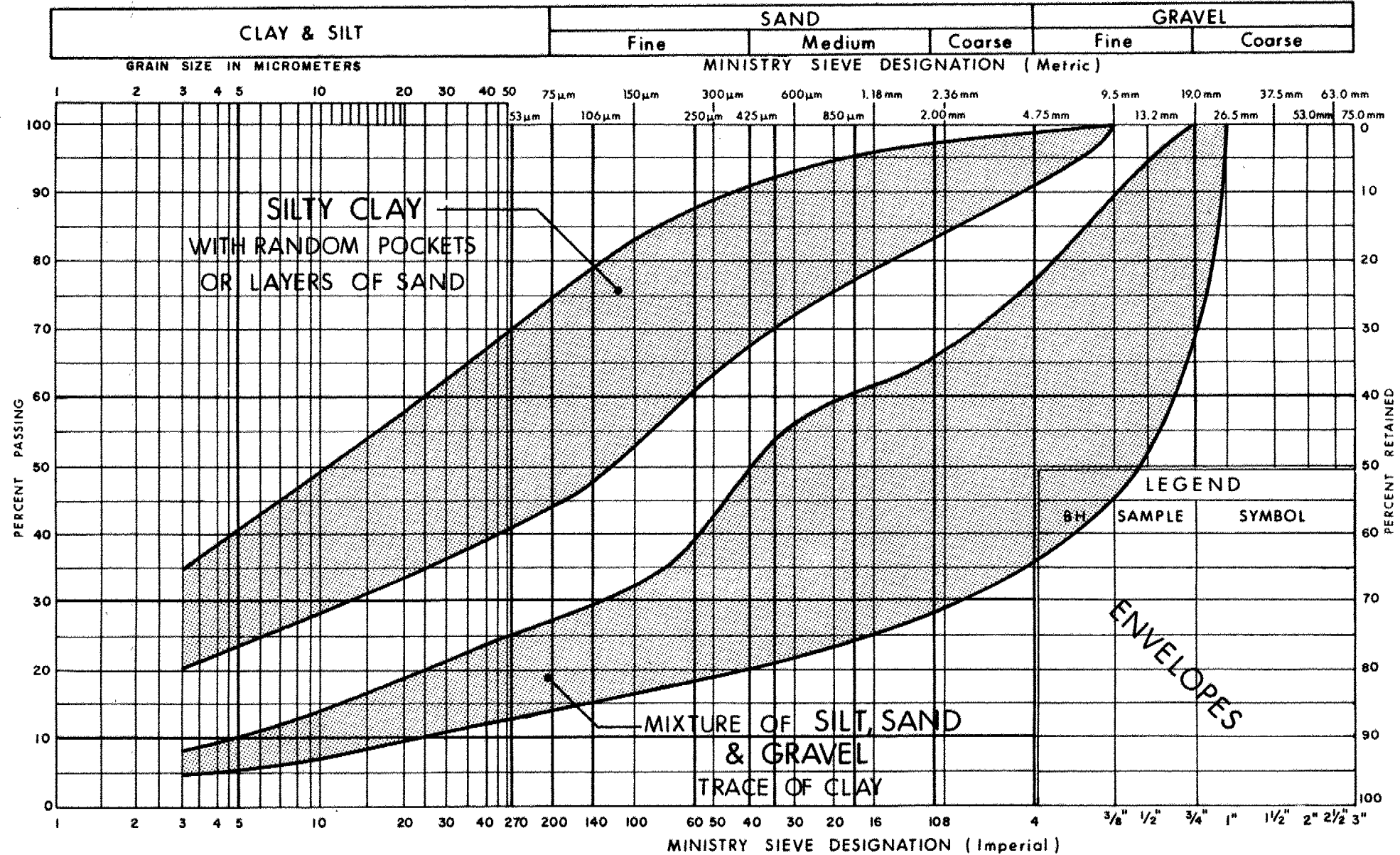
PLASTICITY CHART
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
 WITH SOME SILT, TRACE OF CLAY, OCC COHESIVE ZONES

FIG No 3

W P 20-87-01

(BRIDGE & CULVERT)

UNIFIED SOIL CLASSIFICATION SYSTEM



Ministry of
Transportation

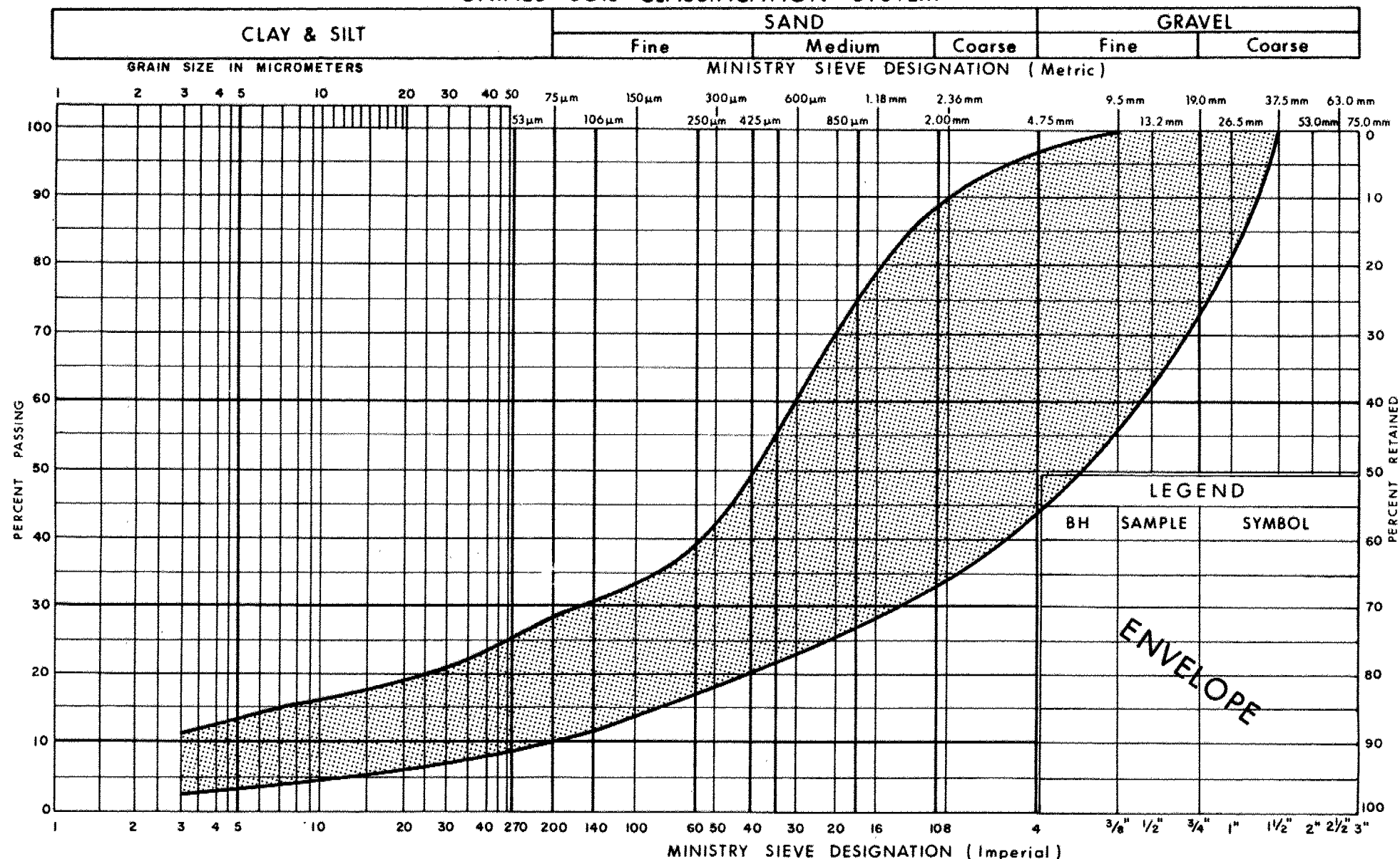
GRAIN SIZE DISTRIBUTION FILL MATERIAL

FIG No 4

W P 20-87-01

(BRIDGE & CULVERT)

UNIFIED SOIL CLASSIFICATION SYSTEM



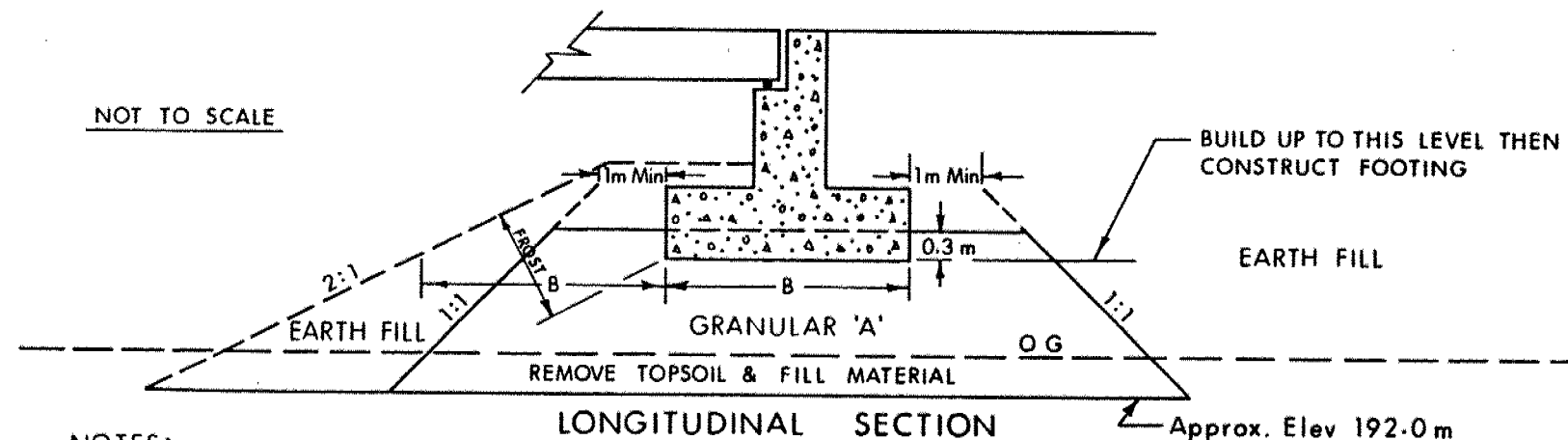
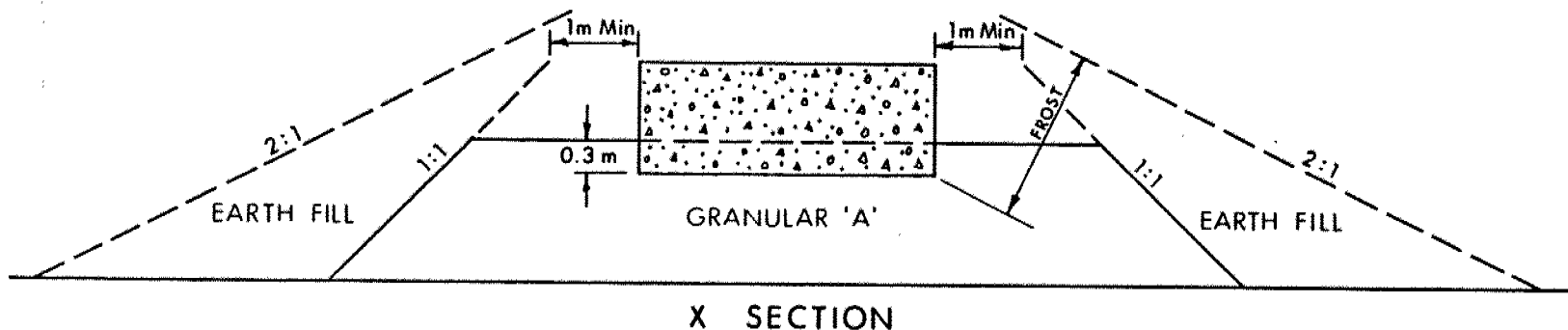
Ministry of
Transportation

GRAIN SIZE DISTRIBUTION
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
 WITH SOME SILT, TRACE OF CLAY, OCC COHESIVE ZONES

FIG No 5

W P 20 - 87 - 01

(BRIDGE & CULVERT)



NOTES:

- 1 - REMOVE TOPSOIL &/OR SOFT SUBSOIL UNDER AREA OF COMPACTED GRANULAR 'A' & EARTH FILL.
- 2 - PLACE GRANULAR 'A' & EARTH FILL TO BOTTOM OF FOOTING LEVEL, COMPACTED ACCORDING TO CURRENT M T C STANDARDS.
- 3 - CONSTRUCT CONCRETE FOOTING.
- 4 - PLACE REMAINDER OF GRANULAR 'A' & EARTH FILL AS REQUIRED.

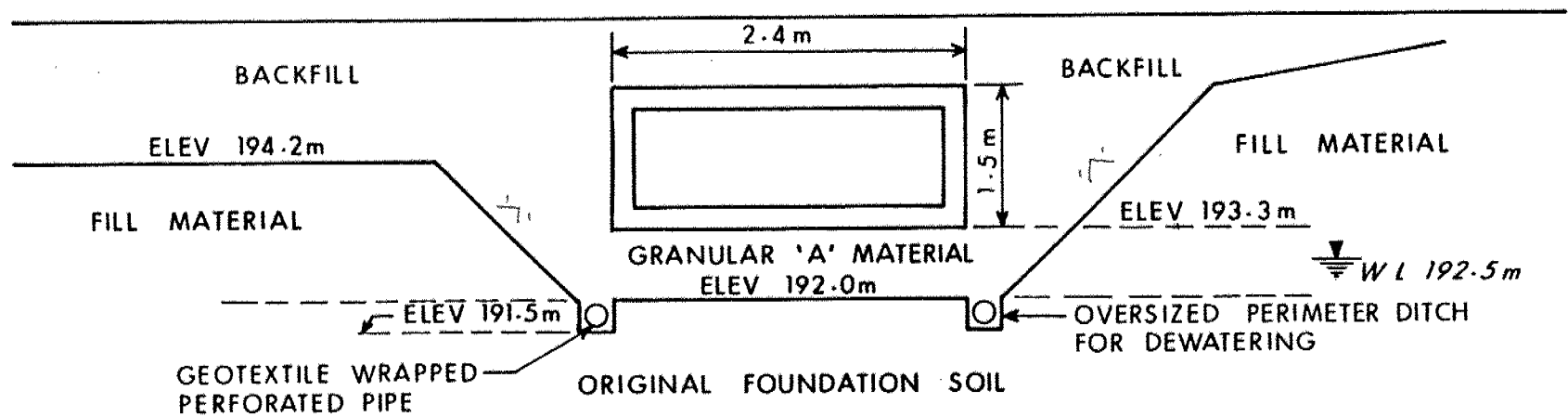


Ministry of
Transportation

ABUTMENT ON COMPACTED FILL SHOWING GRANULAR 'A' CORE

FIG No 6

W P 20-87-01



SKETCH OF CONCRETE CULVERT
(NOT TO SCALE)



RECORD OF BOREHOLE No 8

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 053.0; 0/S 16.8 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 09 10 CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL * LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES						
196.2	Ground Level										
0.0	Silty Clay with random pockets or layers of Sand and Gravel (Fill)		1	AS*			196				8 25 52 15
			1	SS	17						5 31 49 15
			2	SS	6						2 54 24 20
193.6			3	SS	8		194				9 73 12 6
2.6	Gravelly Sand to Sandy Gravel with some Silt trace of clay		4	SS	17						
	Compact to Dense (Glacial Till)		5	SS	20						
	Occ. Cohesive Zones		6	SS	24		192				
	Occ. Cobbles		7	SS	29						32 52 14 2
189.6	End of Borehole		8	SS	31		190				
6.6											
187.1							188				
9.1	End of Cone Test										
	* AS: Auger Sample										

+3, x5: Numbers refer to
Sensitivity

20
15 5 5 (%) STRAIN AT FAILURE
10



RECORD OF BOREHOLE No 19

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 057.0; O/S 46.8 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 10 15 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES								
193.5	Ground Level												
0.0	Mixture Silt, Sand and Gravel, trace of clay (Fill)		1	SS	11								24 47 21 8
192.1			2	SS	25								50 34 11 5
1.4	Gravelly Sand to Sandy Gravel with some Silt trace of clay		3	SS	29								
	Compact to Dense (Glacial Till)		4	SS	31								
	Occ. Cohesive Zones		5	SS	40								
	Occ. Cobbles		6	SS	40								
			7	AS*									84 13 2 1
			8	SS	47								12 72 14 2
186.0			9	SS	100/	10 cm							
7.5	Weathered Shaly Slab		10	SS	100/	8 cm							
185.0			11	AS*									0 13 57 30
8.5	End of Borehole												

* AS: Sample taken from the Tip of Auger

+3, x5: Numbers refer to Sensitivity

20
15
10
5 (%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 20

METRIC

W P 20-87-01

LOCATION NS Baseline Sta. 0 + 053.5; O/S 32.7 m Rt.

ORIGINATED BY TCK

DIST 6 HWY 401

BOREHOLE TYPE HS Auger and Cone Test

COMPILED BY TCK

DATUM Geodetic

DATE 1987 10 15

CHECKED BY _____

[illegible]

OFFICE REPORT ON SOIL EXPLORATION

+³, x⁵ : Numbers refer to Sensitivity

20
15 ϕ 5 (%) STRAIN AT FAILURE
10

RECORD OF BOREHOLE No 21

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 020.3; O/S 50.5 m Rt. ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 14 CHECKED BY _____

SOIL PROFILE			SAMPLES		GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT Y	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE								
194.2	Ground Level											GR SA SI CL
0.0	Mixture Silt, Sand and Gravel with random pockets or layers of Silty Clay (Fill)		1	SS	10							62 19 13 6
192.1			2	SS	6							64 21 10 5
2.1	Sandy Gravel to Gravelly Sand with some Silt trace of clay Dense (Glacial Till) Occ. Cohesive Zones Occ. Cobbles		3	SS	35							56 31 10 3
			4	SS	31							4 85 10 1
			5	SS	35							3 76 17 4
			6	SS	44							
			7	SS	46							
			8	SS	44							22 63 13 2
186.1			9	SS	37							
8.1	End of Borehole											
184.5												
9.7	End of Cone Test											

OFFICE REPORT ON SOIL EXPLORATION

METRIC

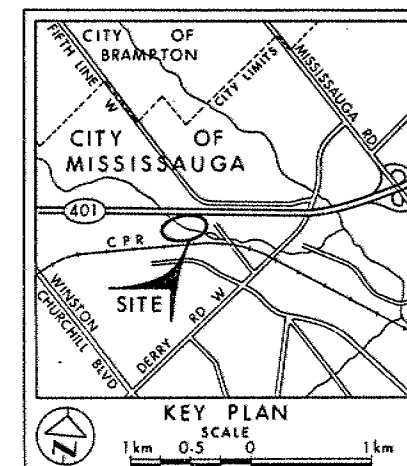
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES.

CONT No
WP No 20-87-01

PEDESTRIAN BRIDGE & CONC CULVERT
AT TRAVEL INFORMATION CENTRE
BORE HOLE LOCATIONS & SOIL STRATA



SHEET



KEY PLAN
SCALE
1 km 0.5 km 0 1 km

LEGEND

- Bore Hole
- ⊕ Dynamic Cone Penetration Test (Cone)
- ⊙ Bore Hole & Cone
- N Blows/0.3m (Std Pen Test, 475 J/blow)
- CONE Blows/0.3m (60° Cone, 475 J/blow)
- W L at time of investigation

No	ELEVATION	N S BASELINE STATION	OFFSET
3	196.7	0+039.5	13.5m Rt
4 & 4B	196.2	0+044.5	28.8m Rt
4A	196.3	0+045.4	27.2m Rt
5	196.2	0+051.9	5.9m Rt
6	196.4	0+051.9	18.8m Lt
7	195.8	0+066.7	18.1m Rt
7A	195.9	0+065.8	19.5m Rt
8	196.2	0+053.0	16.8m Rt
9	196.6	0+041.3	18.2m Rt
10	195.7	0+071.4	9.8m Rt
11	195.8	0+071.2	1.4m Lt
12	196.3	0+058.3	4.7m Lt
13	196.6	0+040.1	20.7m Lt
19	193.5	0+057.0	46.8m Rt
20	196.3	0+053.5	32.7m Rt
21	194.2	0+020.3	50.5m Rt

NOTE

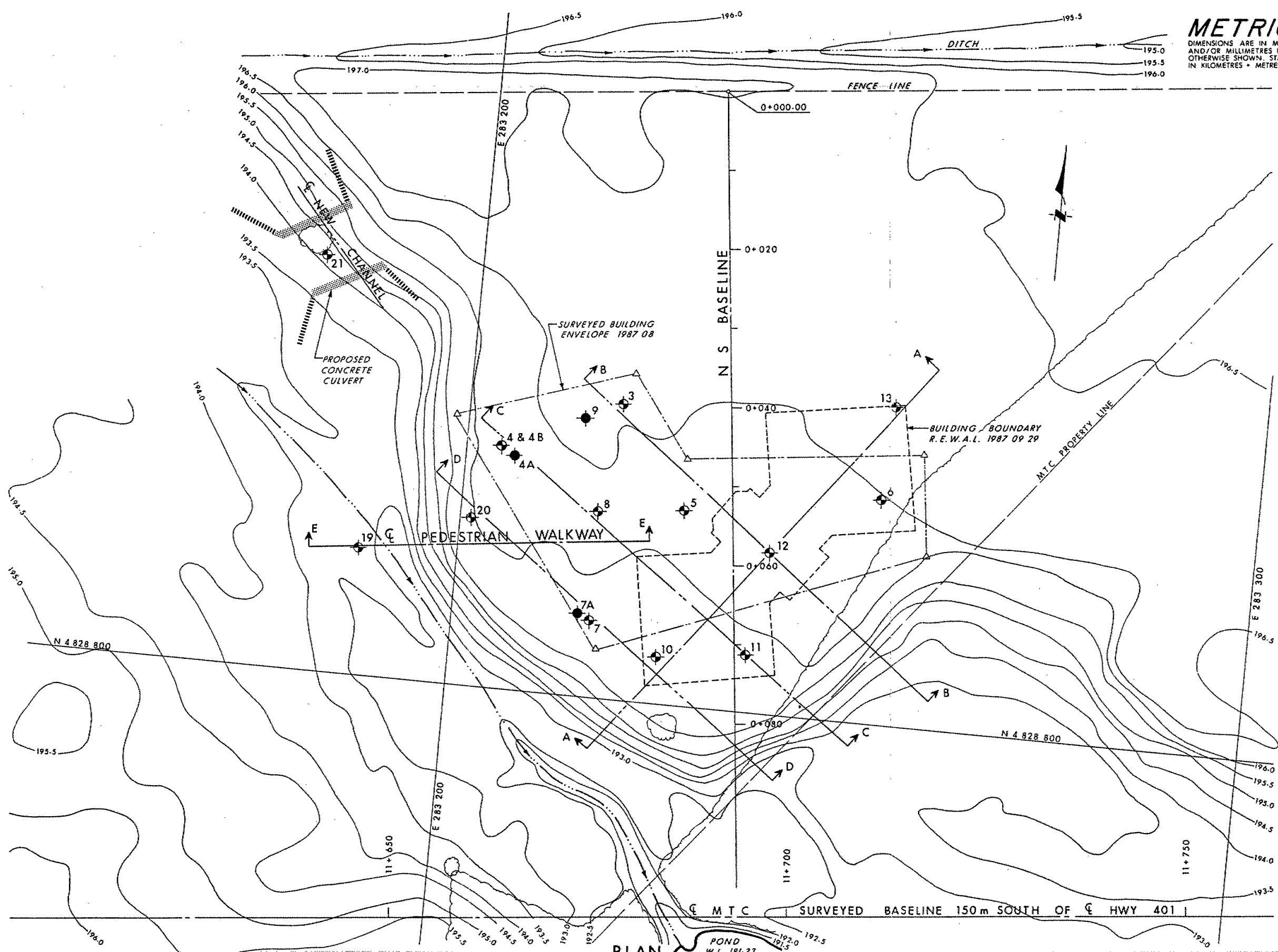
The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence.

NOTE: The complete foundation investigation and design report for this project and other related documents may be examined at the Engineering Materials Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with the conditions of Section 102-2 of Form 100.

REV	DATE	BY	DESCRIPTION

Geocres No 30M12-203

HWY No 401	CHECKED	DATE 1987 11 18	DIST 6
SUBMITTAL	CHECKED	APPROVED	SITE
DRAWN	CHECKED	APPROVED	DWG 208701-E



PLAN

SCALE



NOTE:

Sections A-A, B-B, C-C and D-D are not shown in this report they are included in the Building Foundation Report.

For Section E-E refer to Fig 1 in the Appendix of this report.

For Borehole 21 refer to Record of Borehole Sheet.

ENGINEERING MATERIALS OFFICE
FOUNDATION DESIGN SECTION

WP 20-87-01

DIST 6

HWY 401

STR SITE

Travel Information Centre

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FOUNDATION INVESTIGATION REPORT

For
Building Foundations
for
Travel Information Centre
@ Hwy 401
WP 20-87-01
District 6, Toronto

INTRODUCTION

This report summarizes the information obtained from a foundation investigation carried out at the above mentioned site between 87-09-02 and 87-10-02. This report contains factual information together with recommendations pertaining to the foundations of the proposed Travel Information Centre shown on Drawing No. 208701-B.

SITE DESCRIPTION AND GEOLOGY

This site is located on Hwy 401, 2.0 km West of Mississauga Rd. in the city of Mississauga, Regional Municipality of Peel. At present, there are no buildings except two culverts are located north of the site immediately south of Hwy 401.

The land use in this area is predominantly industrial and agricultural. Hwy 401 runs from east to west along the north boundary of the site and a farmland is located to the west of the site. A pond and wooded area are located to the south of the site. Industrial buildings have been constructed to the east of the site and several are under construction on the northeast side of the Hwy 401.

The site is located in the physiographic region known as the "PEEL PLAIN" as described by the Physiography of Southern Ontario (Chapman and Putnam, 1984). The deposit characterizing this area consists of a glacial till containing large amounts of Palaeozoic shale and limestone. The original ground surface in this region can be described as level to undulating in relief. Specifically, the

site under investigation has an undulating topography. However, it should be noted that the existing ground surface for the proposed Travel Information Centre consists of various fill material placed during the early construction of Hwy. 401.

This area is drained by the Credit River. At the site, water drains from the south end of the culvert at sta. 11 + 486 of Hwy. 401 easterly along a ditch for approximately 70 metres and then southerly through a farm field. This drainage runs parallel to the proposed building site. During the investigation, the drainage channel was completely dry.

FIELD INVESTIGATION

The field investigation was conducted between 87-09-02 and 87-10-02. Seven boreholes were advanced within the initial building envelope between 87-09-02 and 87-09-10 as shown on Drawing B. Boreholes No's 3 to 9 were advanced to a depth of 3.5 m to 10.4 m as shown in borehole logs.

It should be noted that there was some discrepancy with the regards to borehole locations and the initial staking out of the building in the field by R.E. Winters and Associates. These problems were finally resolved and the pertinent information was provided to this office by R.W. Winters and Associates in memo dated 87-10-01.

In addition to the survey errors, the information centre building was relocated and the revised one is rotated by about 90 degrees parallel to east property line as shown on Drawing B. Consequently, only one borehole (Borehole 6) is located within the newly revised building boundary. As a result of this additional investigation consisting of four more boreholes were advanced within the boundary of new building between 87-10-01 and 87-10-02 as shown on Drawing B. Boreholes #10 to #13 were advance to a depth of 9.6m below the ground surface.

Dynamic cone penetration tests accompanied each of the boreholes. The borings were advanced by means of a 83mm hollow stem augering machine. Soil sampling was performed at 0.76 metre intervals using a 50mm OD split spoon sampler. Shelby tube samples were taken from two boreholes (BH's #4a and #7a).

In-situ vane tests were performed at Boreholes #4b and #7a in order to obtain undrained shear strength of soils. Bedrock was not proven in any of the borehole by coring methods except weathered shaly slab was encountered at a depth between 7.8m and 10.2m below the ground surface in eight boreholes as shown on borehole logs.

Laboratory Work

The engineering properties of the soils were determined by performing grain size distribution and Atterberg Limits Tests. In addition natural moisture content was also performed. Two unconfined Compression Tests were performed on organic silty clay taken from Borehole 4a.

The results of laboratory tests are plotted on the record of Borehole Sheets and summarized in the Figures contained in the Appendix.

SUBSURFACE CONDITIONS

General

The soil stratigraphy at the site consists of fill material which consists of a mixture of silt, sand and gravel with random pockets or layers of silty clay, underlain by a compact to very dense gravelly sand to sandy gravel with some silt and trace of clay.

The fill material contains various amounts of old concrete slabs, tree trunk and steel debris. The thickness of this fill varies from 1.1 metre easterly at Borehole 6 up to 4 metre westerly towards the creek at Borehole 11.

The glacial till is predominantly sand with various amounts of gravel and occasional cobbles. Seams of silty fine sand, Shaly slabs and occasional cohesive zones are encountered within this glacial till deposit. This deposit extends to a depth of at least 10.2m below ground level, underlain by weathered Shaly slab.

The boundary of the Subsoil types, insitu and laboratory test results, as well as groundwater levels, are shown in the Record of Borehole Sheets in the Appendix. The location of each borehole is shown in plan on Dwg. No. 208701-B. Four stratigraphical sections are shown on Dwg. 208701-C.

The following is a detailed description of the soils encountered at the site.

Fill Materials

The site is covered with fill material which also contains various amounts of old concrete slabs, tree trunks and steel debris, and the overall thickness varies across the site. This material was encountered in all boreholes ranging in depth from 1 metre easterly up to 4 metres westerly toward the Creek as shown on Section A-A (Drawing 208701-C). The various types of fill material can be categorized as follows:

i) Mixture silt, Sand and Gravel (Fill Material)

A mixture of silt, sand and gravel was encountered in Boreholes 3, 4, 5, 6, 9, 10 and 12. This fill material is extended to a depth of approximately 3.7m below the existing ground surface.

Atterberg Limits tests and grain size distribution tests were performed on the samples and the results are shown on Figures 1 and 5 and listed below:

	<u>Range %</u>
Moisture Content (W)	6.5 - 16.5
Liquid Limit (W_L)	25 - 38.5
Plastic Limit (W_p)	15.5 - 22
Plasticity Index (I_p)	9.5 - 17.5

	<u>Range %</u>
Gravel	3 - 41
Sand	18 - 57
Silt	18 - 51
Clay	7 - 26

iii) Organic silty clay (Surficial Fill Material)

A localized surficial deposit of grey organic silty clay was encountered in Boreholes 4, 4A, 7, 7A, 10, 11 with the thickness varying from 0.7m to 1.8m overlying the original ground surface.

Atterberg Limits and grain size distribution tests were performed on the samples as shown on Figures 3 and 7.

The results are listed as follows:

	<u>Range %</u>
Moisture Content (W)	15 - 30.5
Liquid Limit (W_L)	28.5 - 42.0
Plastic Limit (W_p)	15.5 - 29.0
Plasticity Index (I_p)	11.0 - 21.5

	<u>Range %</u>
Gravel	1 - 14
Sand	16 - 50
Silt	24 - 55
Clay	11 - 29

Organic content tests were also carried out on two samples from Boreholes 4A and 7A. The results shows that the organic content ranges between 2.2 and 6.4% by weight.

This fill material can be described as an organic silty clay with random pockets or layers of sand and gravel. The organic silty clay can be considered to be low or medium plasticity (CL or CI Group). Standard penetration 'N' values between 2 and 13 indicated that the organic silty clay is in a soft to stiff state.

Glacial Till (Gravelly Sand to Sandy Gravel)

The fill material is underlain by a glacial till deposit. This material was encountered in all boreholes immediately below the fill material, about 1m below ground level in BH5, and about 4m below ground level in BH11. Within the glacial till stratum, occasional cohesive zones and occasional cobbles were encountered at the lower portion of the glacial till. Shaly slabs were encountered between elevation 188.5 and 187.7 in BH12, and below the elevations between 187.1 and 185.6m in the other boreholes as shown on boreholes logs. It appears that these shaly slabs have been disturbed by glacial action. Sand and gravel were forced between these slabs and its bedrock. The full extent of the glacial till deposit was not investigated.

Atterberg Limits tests were performed on this material as shown on Figure 4, and the results are summarized below:

	<u>Range %</u>
Moisture Content (W)	3.5 - 18.5
Liquid Limit (W _L)	15.5 - 34.5
Plastic Limit (W _p)	13.0 - 18.5
Plasticity Index (I _p)	1.5 - 16.0

Grain size distribution tests were carried out on the glacial till and the results are summarized below. Figure 8 in the Appendix shows the result in envelope form.

	<u>Range %</u>
Gravel	2 - 47
Sand	22 - 87
Silt	3 - 18
Clay	1 - 9

The glacial till can be described as gravelly sand to sandy gravel with some silt, trace of clay and occasional cobbles. The matrix is composed primarily on non-cohesive sand with occasional zones that exhibit low plasticity (CL- ML Group). With depth, the glacial till changed from brown to pink in color.

Occasional weathered shaly slabs yielded 'N' values ranging from 36 and 220 blows/0.3m, indicating a hard consistency.

In the glacial till stratum, the 'N' values ranged from 4 to 100 blows/0.1m indicating a state of compaction described as loose to very dense.

GROUNDWATER CONDITIONS

The groundwater level was measured in open boreholes few days after the boreholes were completed. The groundwater elevation was found to range between Elev. 191.5 and 192.0m, which corresponds to some 4 to 5m below the existing ground surface. The observed groundwater levels are indicated on the Record of Borehole Sheets in the Appendix and are shown on the stratigraphical sections (Dwg. No. 208701-C).

DISCUSSION AND RECOMMENDATIONS

As mentioned above, the soil stratigraphy at the site consists of a mixture of fill material, underlain by a sandy glacial till. The fill material contains various amounts of old concrete slabs, tree trunks and steel debris. The thickness of this fill varies from 1 metre easterly up to 4 metres westerly towards the creek. The glacial till is predominantly sand with various amounts of gravel and occasional cobbles. Seams of silty fine sand, shaly slabs and occasional cohesive zones are also present within this glacial till deposit. This deposit extends to a depth of at least 10.2m below ground level, underlain by weathered shale.

The groundwater elevation is believed to be at an elevation of approximately 191.5 metres in most boreholes, which is slightly higher than that of pond water of 191.2 m.

It is proposed to construct a new building for the Travel Information Centre. The following are our foundation recommendations for the design and construction of the new building and the related patio roof columns.

Foundation for Building

It is recommended that the new building structure be supported on a spread footing type of foundations placed on well compacted granular 'A' material or within natural subsoil of compact to very dense natural foundation soil. It is necessary that all the fill material be removed at the building location prior to the construction of new foundations.

The bearing capacity requirements for these foundation soils using working stress method are as follows:

Allowable bearing pressure up to 250 kPa may be used for building foundations at or below the proposed footing elevation on the granular subsoil provided the sub-excavation of the existing loose fill being removed and replaced with well compacted Granular 'A' at certain locations. The proposed footing elevation with the basement to about

elev. 193.5 m, while that without basement is proposed to be at elev. 196.0 m. The pertinent details including the limits of sub-excavation are shown on the enclosed figures 10 and 11. The above bearing capacity was based on the assumption that the footing width of the wall be 0.61 m (2 ft.) and the differential settlement will not exceed 25 mm.

For sliding resistance between the footing and granular subsoil, a coefficient of friction of 0.6 may be used. The footing should have a minimum of earth cover of 1.2 m to the underside of the footing to provide adequate frost protection.

In the event that the structure is designed as per O.H.B.D.C., the following parameters shall be used for the footing design:

	<u>Factored Capacity</u> <u>at U.L.S. (kPa)</u>	<u>Allowable Capacity at</u> <u>S.L.S. Type II (kPa)</u>
Spread Footings	600	250

Resistance to sliding of the footings can be calculated assuming a coefficient of friction of 0.6 between the underside of the concrete footing and the granular type material.

In designing the basement walls, the following parameters should be used in computing the lateral earth pressures for the granular type of backfill:

	<u>Granular Type of Material</u>
Angle of Internal Friction (ϕ)	30°
Unit Weight (KN/m ³) (γ)	19.0
Coefficient of Earth Pressure at Rest (K_0)	0.5

Foundation for Patio Columns

Columns for the patio outside of the building can be also designed on square or rectangular units using the above-mentioned design bearing capacities with appropriate dimensions to satisfy the design.

Sub-Excavation Details

As discussed above, all mixed fill material should be sub-excavated and replaced with well compacted granular 'A' material.

Two options can be considered as follows:

With Basement

If a basement is included in building scheme, the fill material in the vicinity of Boreholes 10 and 11 should be excavated down to elevation of 191.5m. The excavation boundary should be back sloping towards Boreholes 12, and then extending to the basement floor elevation of 194.5m near Boreholes 6 and 13 as shown on Section A-A (Figure 9).

Immediately after the excavation in deep excavated area, backfill to the base of the proposed footing elevation 193.5m, with well compacted Granular 'A' is required for the foundations.

In the Shallow excavated areas near Boreholes 6 and 13, the footings for the wall may be placed on the natural subsoil (granular material) after the trench for the footing is excavated to elevation 193.5 m as shown on the attached sketch (Figure 10).

Without Basement

If the basement is eliminated for the proposed buildings, the fill material into area near Boreholes 10 and 11 should be excavated down to elevation of 192.0 m along the boundary between fill material and original ground surface. The excavation boundary should be back sloping along the boundary between the fill material and original ground toward Borehole 12 with 3 (H) to 1 (V) slope, and then extending to the boundary elevation of 195.0 m near Borehole 6 and 13 as shown on Section A-A.

In the excavated areas, granular 'A' material should be backfilled to elevation 196.0 m as shown on Figure 11.

It should be emphasized that all fill material be excavated within the base width of the foundations prior to the construction of the footings.

Stability of slopes

Excavation depth is in the order of 4.5 m and consequently stability problems are not anticipated provided slopes are maintained with 2H:1V.

Construction

During excavation, care shall be exercised by not leaving any old fill material underneath the proposed footings. Dewatering during the excavation should not be a major problem since excavation will be carried above the prevailing water level. However, any minor seepage or surface run-off into the excavations for foundations could be readily handled by pumping sumps.

Miscellaneous

The fieldwork for this investigation was carried out during the period of 87-09-02 to 87-10-02, under the supervision of Tae C. Kim and Mike Shenarr (Student Engineer). The equipment was owned and operated by Malone's Soil Samples Co. Ltd. of Etobicoke.

This report was written by T.C. Kim and reviewed by M. Devata, Chief Foundations Engineer (East).



Tae C. Kim
Tae C. Kim, P.Eng.
Projects Foundations Engineer

M. Devata
Murty Devata, P.Eng.
Chief Foundations Engineer (East)

APPENDIX

EXPLANATION OF TERMS USED IN REPORT

N VALUE: THE STANDARD PENETRATION TEST (SPT) N VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 51mm O.D. SPLIT BARREL SAMPLER TO PENETRATE 0.3m INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5kg, FALLING FREELY A DISTANCE OF 0.76m. FOR PENETRATIONS OF LESS THAN 0.3m N VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N VALUE IS DENOTED THUS \bar{N} .

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (51mm O.D. 60° CONE ANGLE) DRIVEN BY 475 J IMPACT ENERGY ON 'A' SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3m ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

c_u (kPa)	0 - 12	12 - 25	25 - 50	50 - 100	100 - 200	> 200
	VERY SOFT	SOFT	FIRM	STIFF	VERY STIFF	HARD

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS/0.3m)	0 - 5	5 - 10	10 - 30	30 - 50	> 50
	VERY LOOSE	LOOSE	COMPACT	DENSE	VERY DENSE

ROCKS ARE DESCRIBED BY THEIR COMPOSITION AND STRUCTURAL FEATURES AND / OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (R Q D), FOR MODIFIED RECOVERY, IS:

R Q D (%)	0 - 25	25 - 50	50 - 75	75 - 90	90 - 100
	VERY POOR	POOR	FAIR	GOOD	EXCELLENT

JOINTING AND BEDDING:

SPACING	50mm	50 - 300mm	0.3m - 1m	1m - 3m	> 3m
JOINTING	VERY CLOSE	CLOSE	MOD. CLOSE	WIDE	VERY WIDE
BEDDING	VERY THIN	THIN	MEDIUM	THICK	VERY THICK

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

S S	SPLIT SPOON	T P	THINWALL PISTON
W S	WASH SAMPLE	O S	OSTERBERG SAMPLE
S T	SLOTTED TUBE SAMPLE	R C	ROCK CORE
B S	BLOCK SAMPLE	P H	T W ADVANCED HYDRAULICALLY
C S	CHUNK SAMPLE	P M	T W ADVANCED MANUALLY
T W	THINWALL OPEN	F S	FOIL SAMPLE

STRESS AND STRAIN

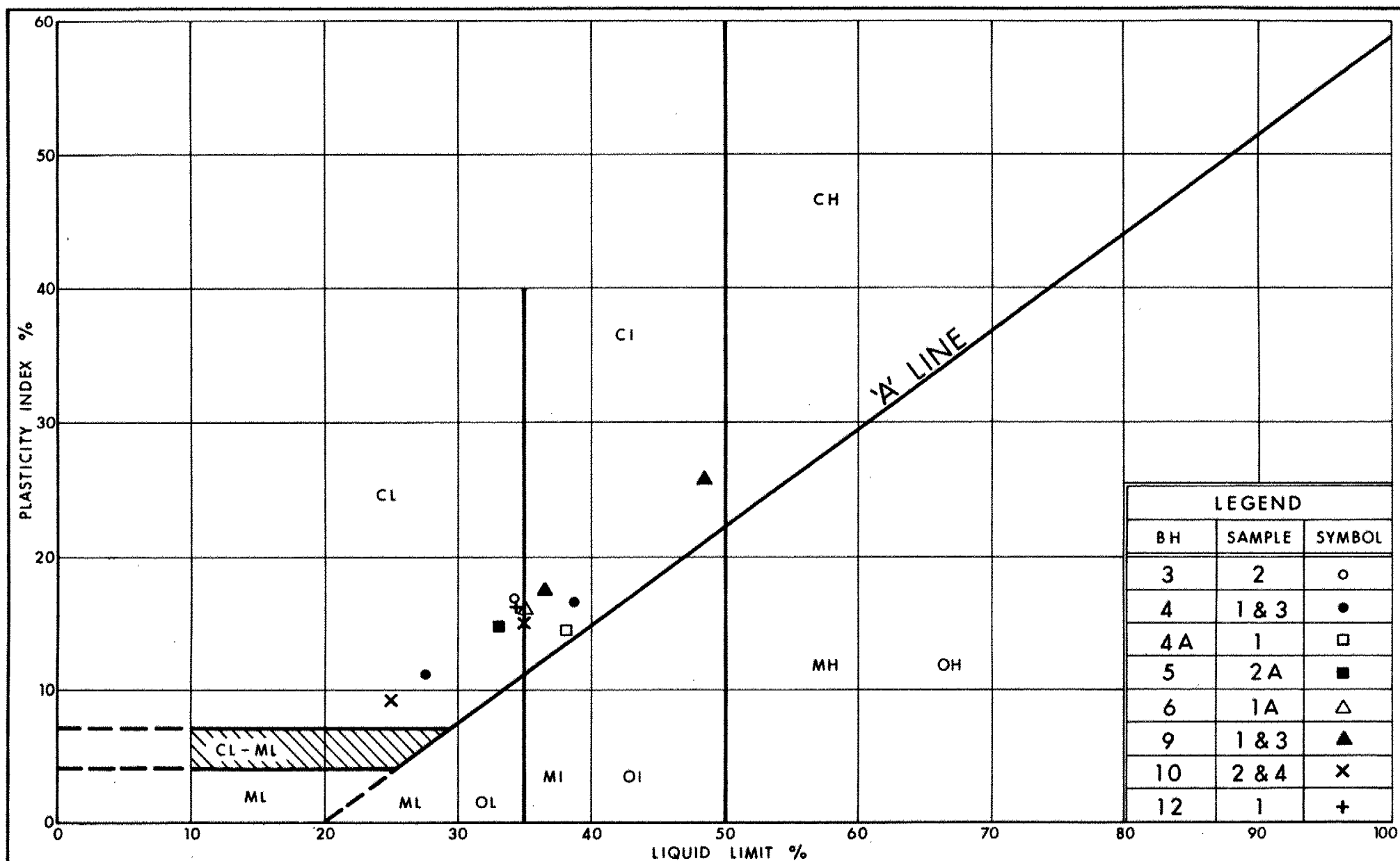
u_w	kPa	PORE WATER PRESSURE
r_u	1	PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ϵ	%	LINEAR STRAIN
$\epsilon_1, \epsilon_2, \epsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF LINEAR DEFORMATION
G	kPa	MODULUS OF SHEAR DEFORMATION
μ	1	COEFFICIENT OF FRICTION

MECHANICAL PROPERTIES OF SOIL

m_v	kPa^{-1}	COEFFICIENT OF VOLUME CHANGE
C_c	1	COMPRESSION INDEX
C_s	1	SWELLING INDEX
C_α	1	RATE OF SECONDARY CONSOLIDATION
c_v	m^2/s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v	1	TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
ϕ'	-°	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
ϕ_u	-°	APPARENT ANGLE OF INTERNAL FRICTION
τ_r	kPa	RESIDUAL SHEAR STRENGTH
τ_r	kPa	REMOULDED SHEAR STRENGTH
S_t	1	SENSITIVITY = $\frac{c_u}{\tau_r}$

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m^3	DENSITY OF SOLID PARTICLES	e	1, %	VOID RATIO	e_{min}	1, %	VOID RATIO IN DENSEST STATE
γ_s	kN/m^3	UNIT WEIGHT OF SOLID PARTICLES	n	1, %	POROSITY	I_D	1	DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m^3	DENSITY OF WATER	w	1, %	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kN/m^3	UNIT WEIGHT OF WATER	S_r	%	DEGREE OF SATURATION	D_n	mm	n PERCENT - DIAMETER
ρ	kg/m^3	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u	1	UNIFORMITY COEFFICIENT
γ	kN/m^3	UNIT WEIGHT OF SOIL	w_p	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m^3	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m^3/s	RATE OF DISCHARGE
γ_d	kN/m^3	UNIT WEIGHT OF DRY SOIL	I_p	%	PLASTICITY INDEX = $w_L - w_p$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m^3	DENSITY OF SATURATED SOIL	I_L	1	LIQUIDITY INDEX = $\frac{w - w_p}{I_p}$	i	1	HYDRAULIC GRADIENT
γ_{sat}	kN/m^3	UNIT WEIGHT OF SATURATED SOIL	I_C	1	CONSISTENCY INDEX = $\frac{w_L - w}{I_p}$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m^3	DENSITY OF SUBMERGED SOIL	e_{max}	1, %	VOID RATIO IN LOOSEST STATE	j	kN/m^3	SEEPAGE FORCE
γ'	kN/m^3	UNIT WEIGHT OF SUBMERGED SOIL						



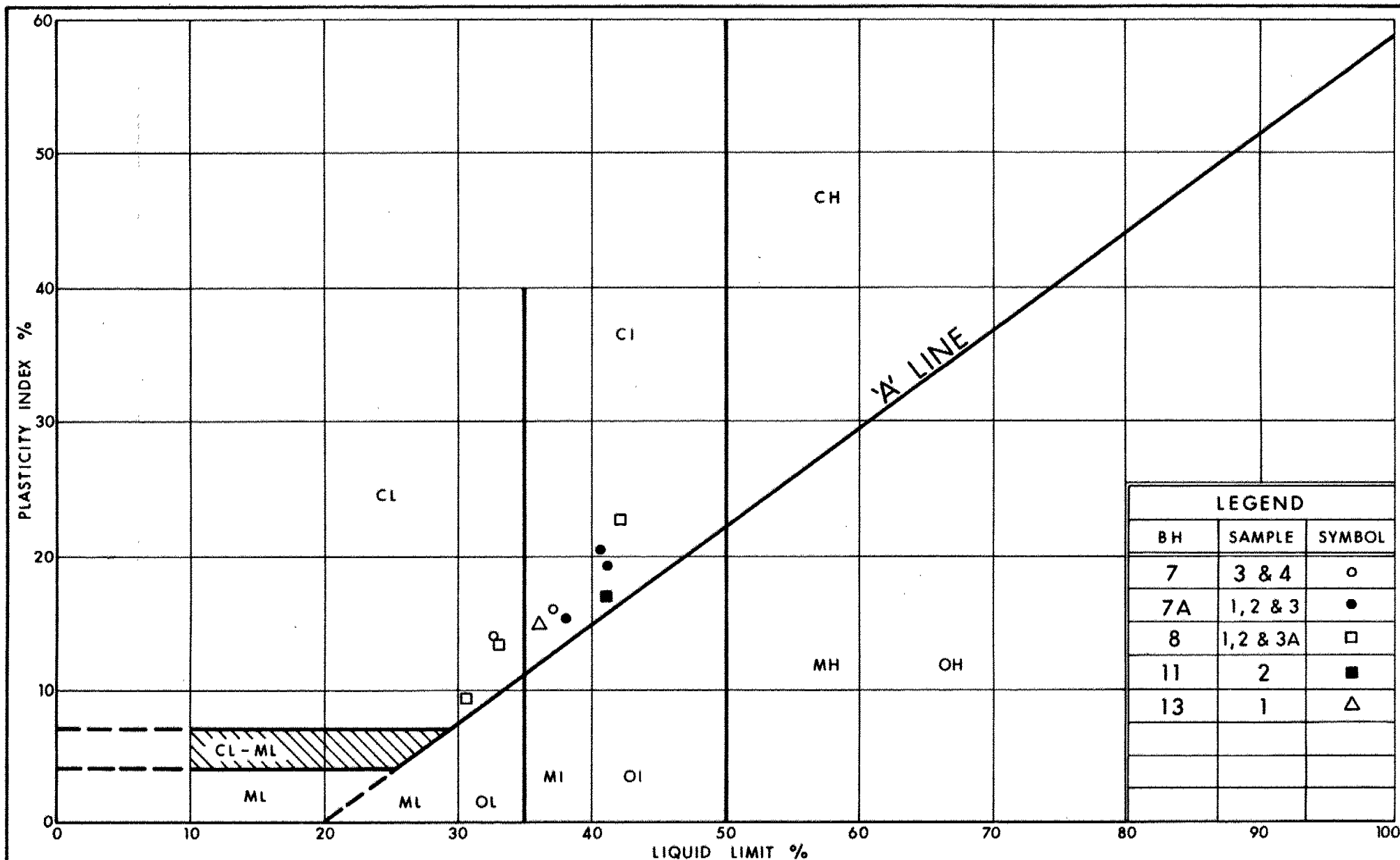
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Ontario

PLASTICITY CHART
MIXTURE OF SILT, SAND & GRAVEL (FILL)
WITH RANDOM POCKETS & LAYERS OF SILTY CLAY

FIG No 1

W P 20-87-01



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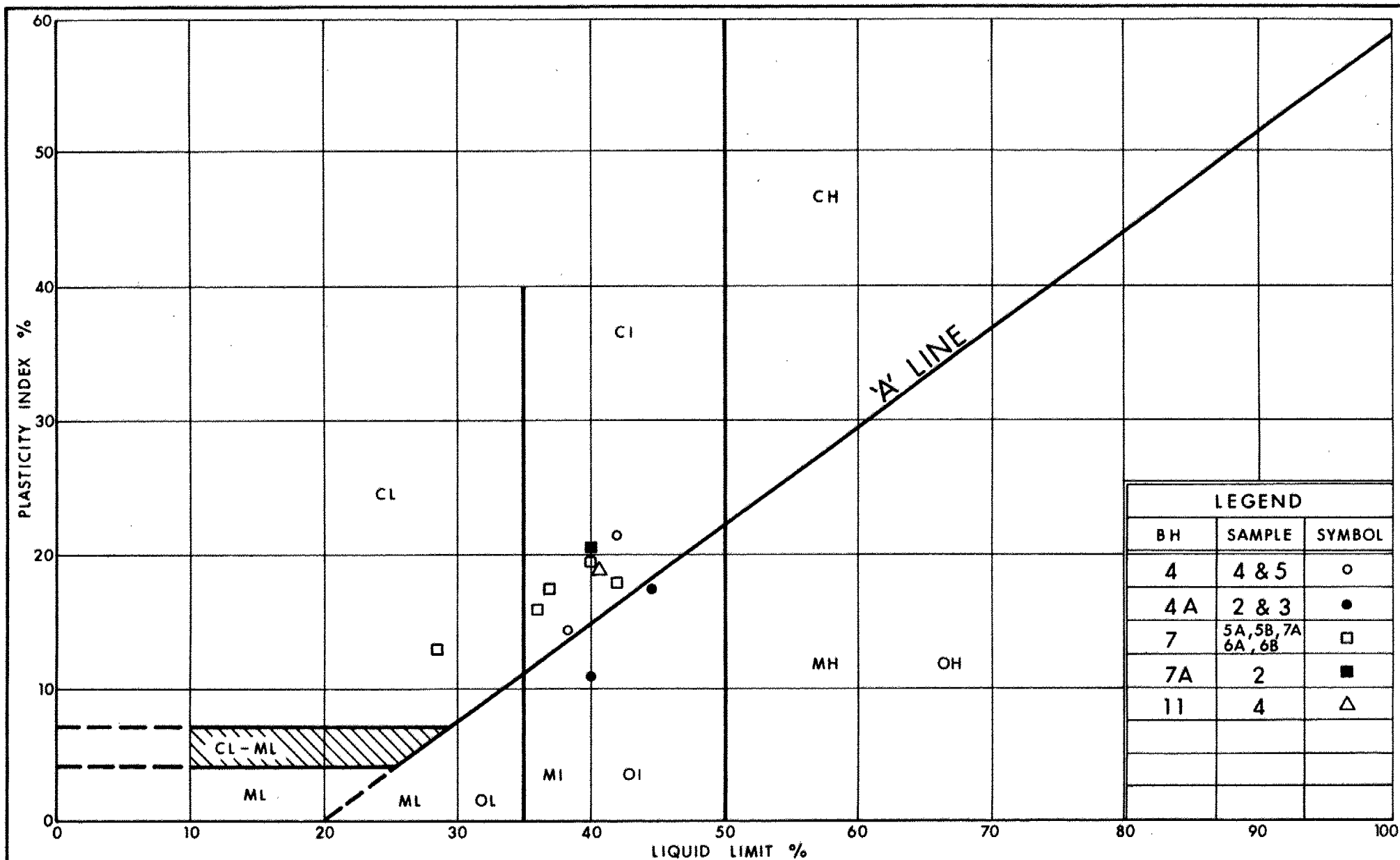
Ontario

PLASTICITY CHART SILTY CLAY (FILL)

WITH RANDOM POCKETS OR LAYERS OF SAND & GRAVEL

FIG No 2

W P 20-87-01

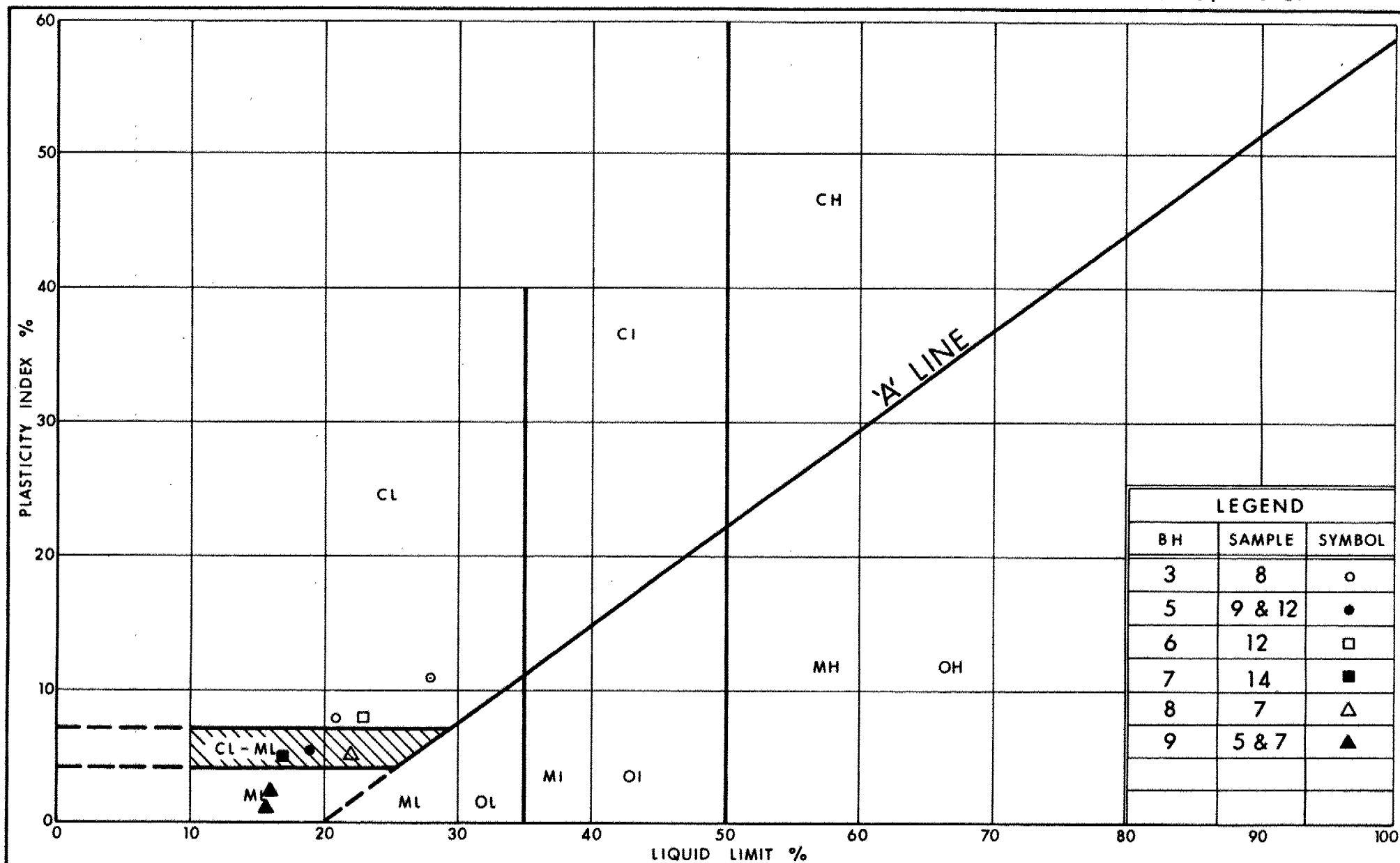


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PLASTICITY CHART ORGANIC SILTY CLAY (FILL)

FIG No 3

W P 20-87-01



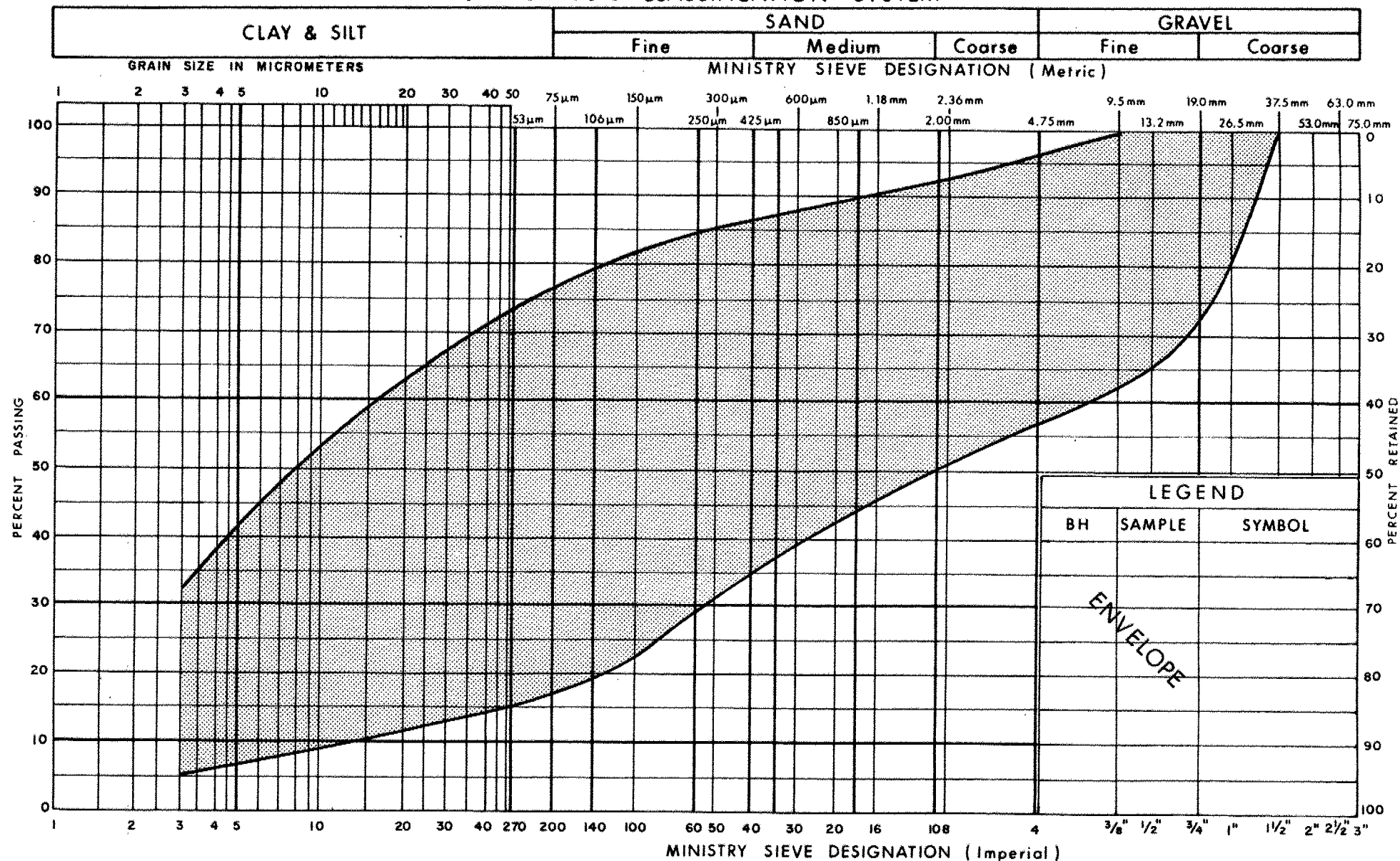
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PLASTICITY CHART
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
WITH SOME SILT, TRACE OF CLAY, OCC. COHESIVE ZONES

FIG No 4

W P 20-87-01

UNIFIED SOIL CLASSIFICATION SYSTEM



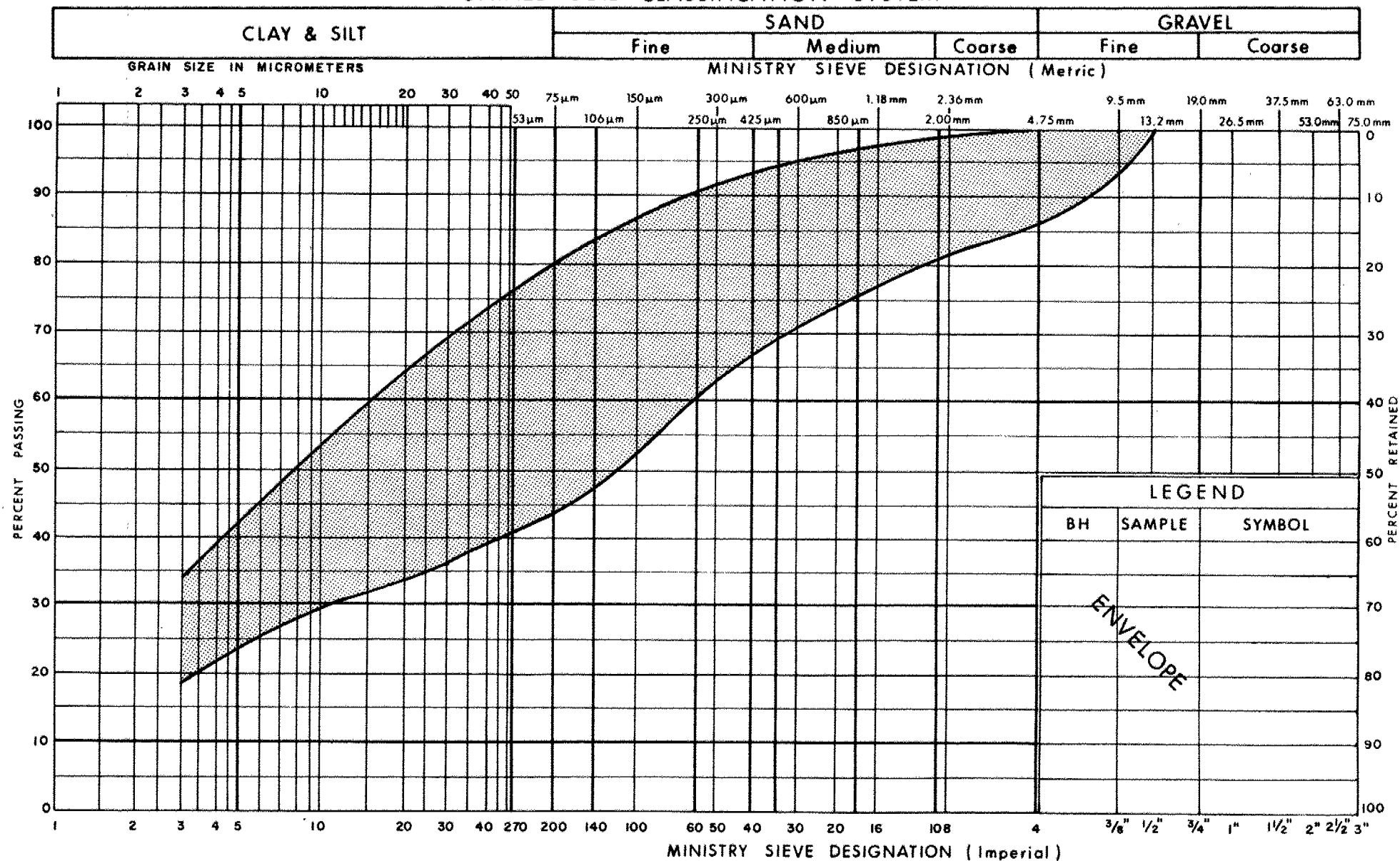
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GRAIN SIZE DISTRIBUTION
MIXTURE OF SILT, SAND & GRAVEL (FILL)
 WITH RANDOM POCKETS & LAYERS OF SILTY CLAY

FIG No 5

W P 20-87-01

UNIFIED SOIL CLASSIFICATION SYSTEM



Ministry of
Transportation

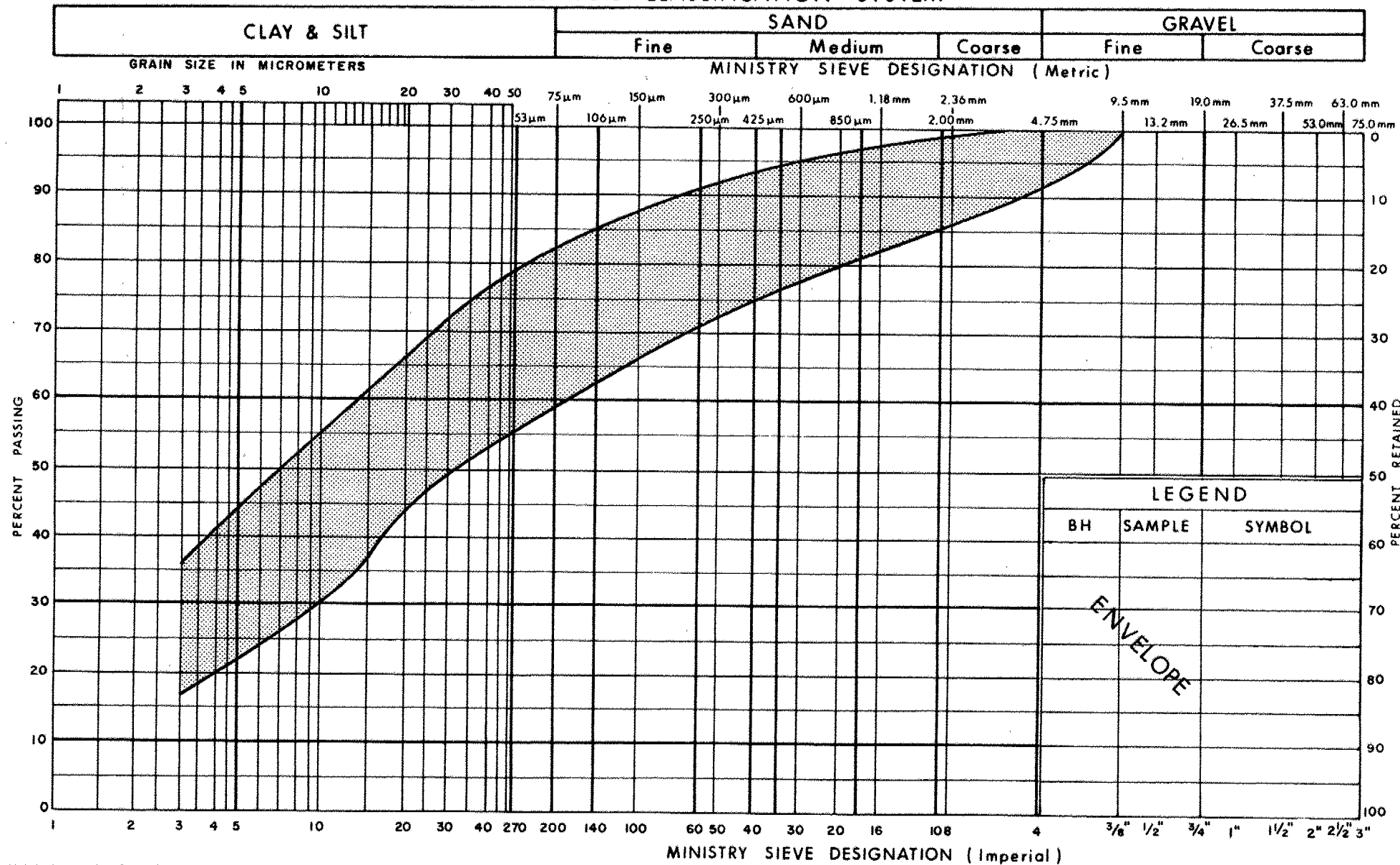
GRAIN SIZE DISTRIBUTION SILTY CLAY (FILL)

WITH RANDOM POCKETS OR LAYERS OF SAND & GRAVEL

FIG No 6

W P 20 - 87 - 01

UNIFIED SOIL CLASSIFICATION SYSTEM



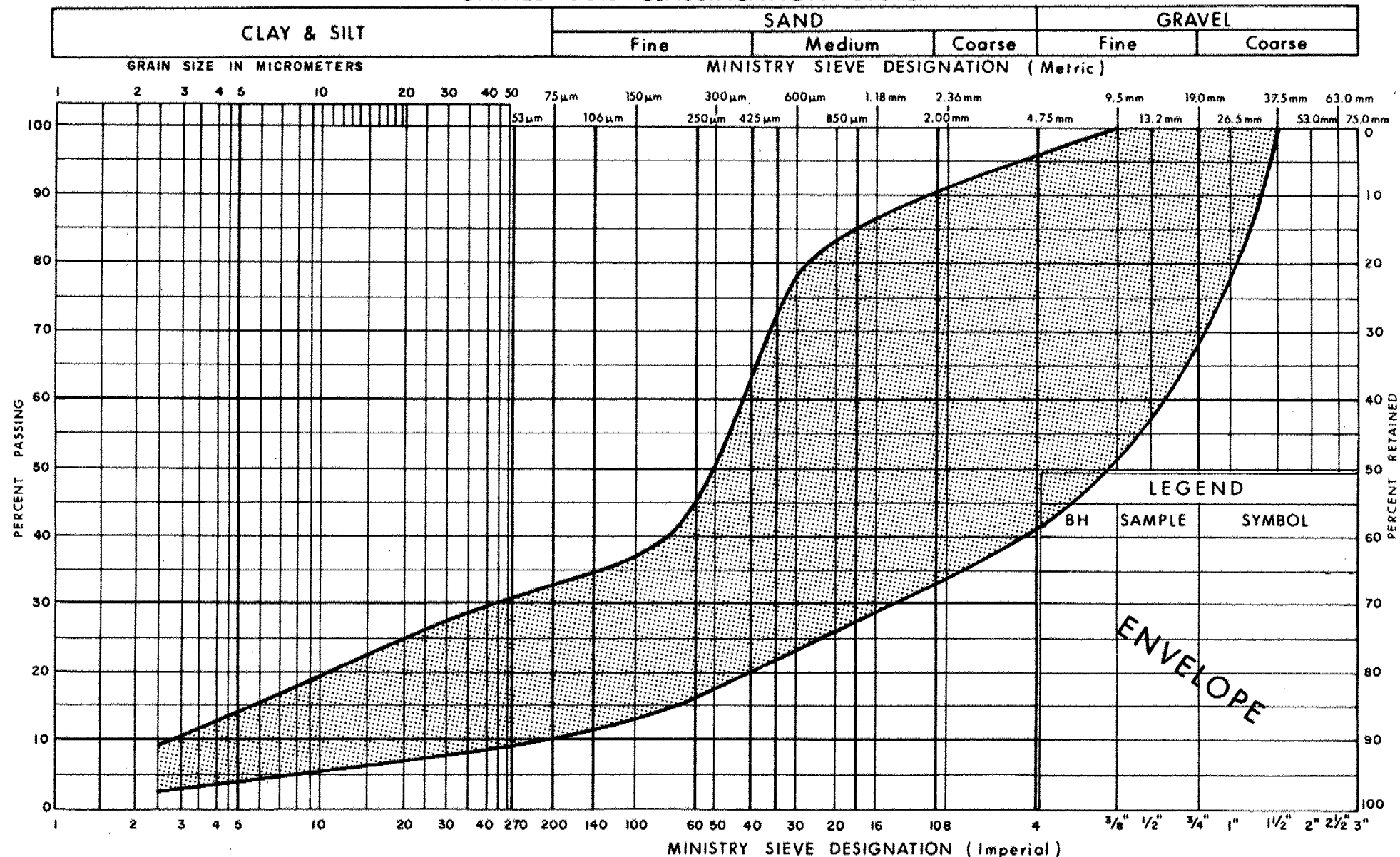
Ministry of
Transportation

GRAIN SIZE DISTRIBUTION
ORGANIC SILTY CLAY
(FILL)

FIG No 7

W P 20-87-01

UNIFIED SOIL CLASSIFICATION SYSTEM

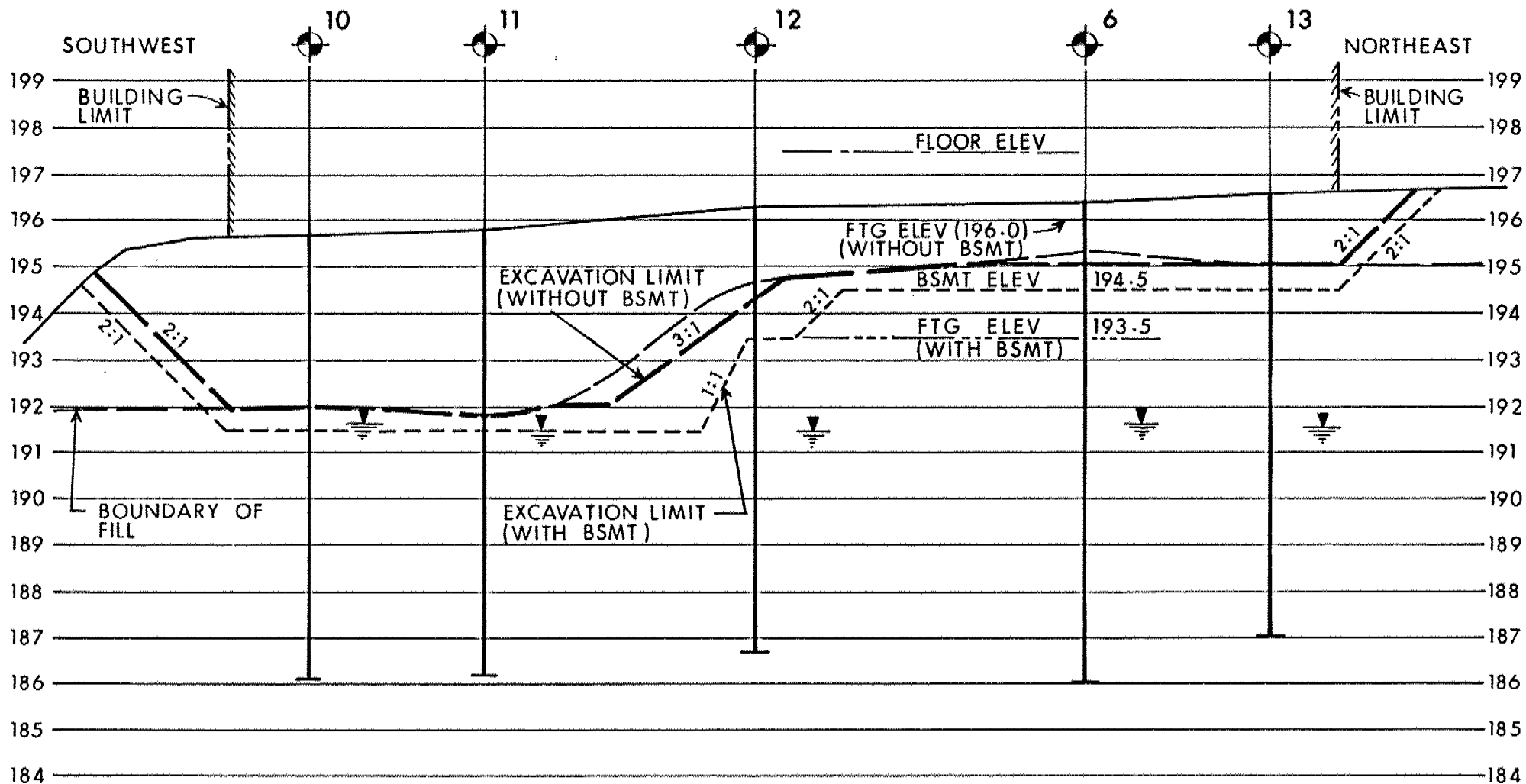


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Transportation

GRAIN SIZE DISTRIBUTION
GRAVELLY SAND TO SANDY GRAVEL (Glacial Till)
 WITH SOME SILT, TRACE OF CLAY, OCC. COHESIVE ZONES

FIG No 8

W P 20-87-01



PROPOSED EXCAVATION RECOMMENDATIONS ALONG SECTION A-A

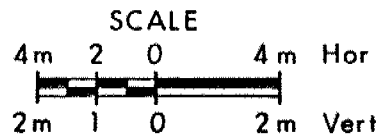
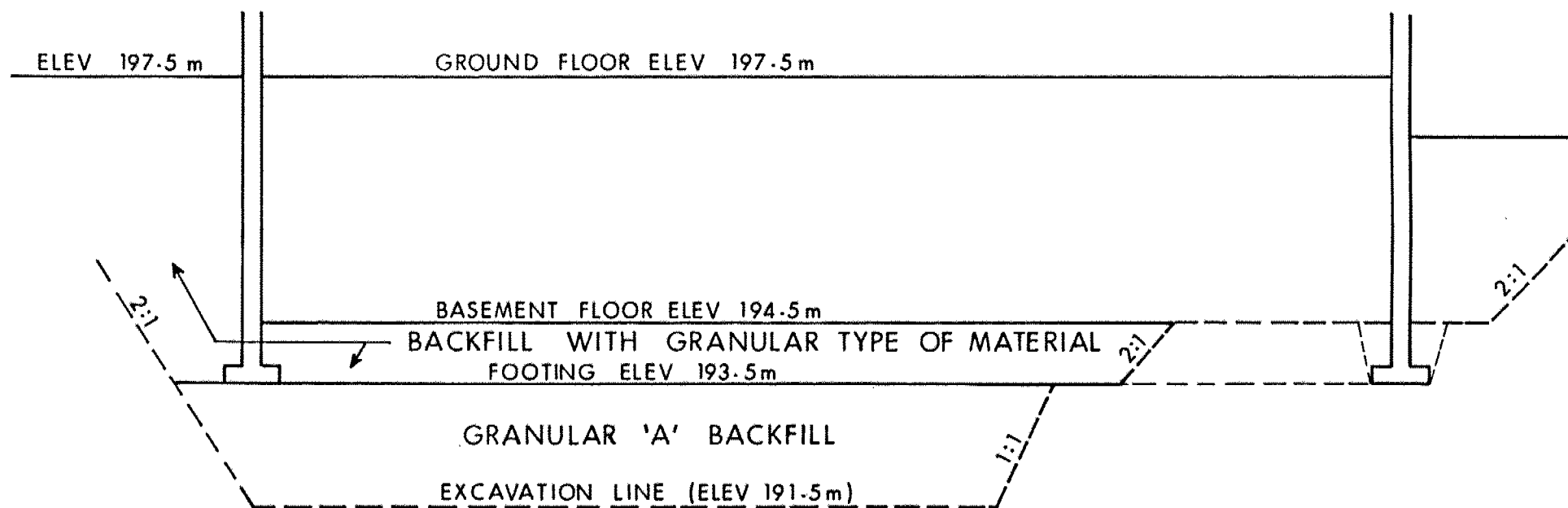
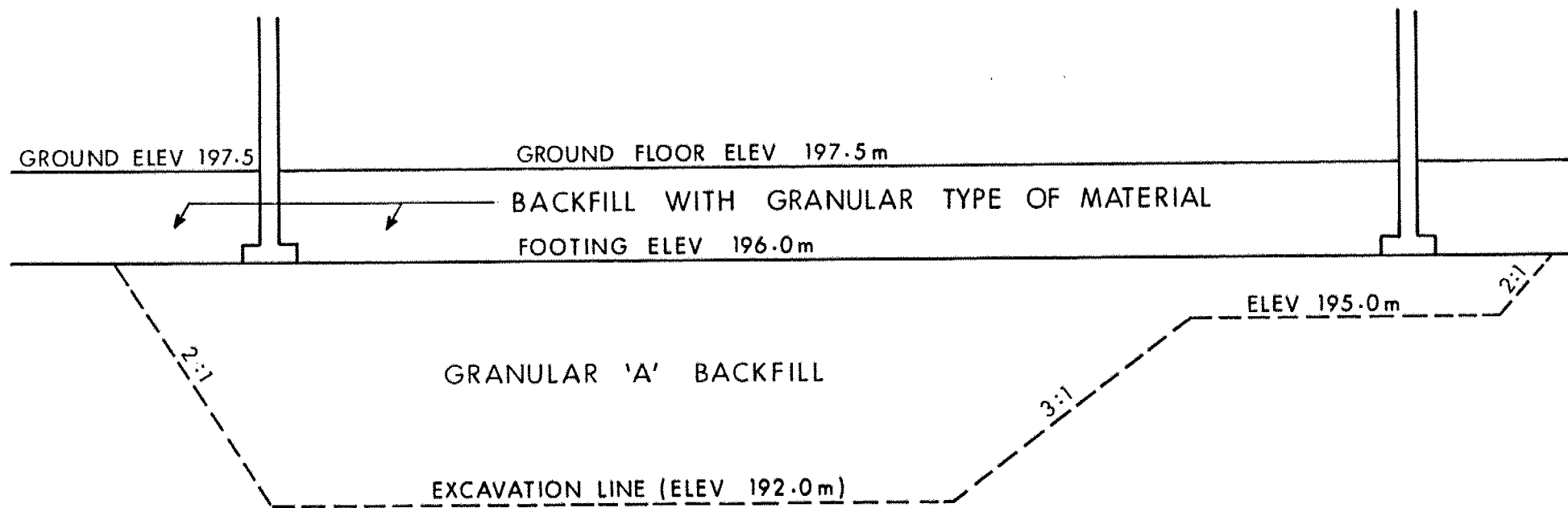


Fig 9
WP 20-87-01



SKETCH FOR EXCAVATION & BACKFILL
(WITH BASEMENT)



SKETCH FOR EXCAVATION & BACKFILL
(WITHOUT BASEMENT)



Ministry of
Transportation and
Communications
Ontario

RECORD OF BOREHOLE No 3

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 039.5; O/S 13.5 m RT ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 09 02 & 03 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100	SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L	WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES								
196.7	Ground Level												
0.0	Mixture Silt, Sand and Gravel with random pockets or layers of Silty Clay (Fill)		1	SS	38	8 cm	196						41 18 34 7
195.3			2	SS	12								24 20 40 16
1.4			3	SS	32								
			4	SS	21								
	Gravelly Sand to Sandy Gravel with some silt trace of clay		5	SS	41		194						13 80 4 3
			6	SS	36								
			7	SS	35		192						
	Compact to Very Dense (Glacial Till) Occ. Cohesive Zones Occ. Cobbles		8	SS	44								41 38 17 4
			9	SS	31		190						
			10	SS	36								
			11	SS	42								
187.2	Weathered Shaly Slab		12	SS	220		188						28 50 18 4
9.5													
187.1	End of Borehole												
9.6													

+3, x5: Numbers refer to Sensitivity

20
15
10
5 (%) STRAIN AT FAILURE

OFFICE REPORT ON SOIL EXPLORATION

RECORD OF BOREHOLE No 4 & 4B

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 044.5; O/S 28.8 m Rt. ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test (Vane Test at BH 4B) COMPILED BY TCK
 DATUM Geodetic DATE 1987 09 02 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT Y	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	20 40 60 80 100					
196.2	Ground Level													
0.0	Mixture Silt, Sand and Gravel with random pockets or layers of Silty Clay (Fill)		1	AS*	110	15 cm	196							40 20 29 11
194.3			2	SS	21									40 32 18 10
1.9			3	SS	28									20 46 25 9
	Organic Silty Clay		4	SS	8		194	+1.5	1.4					1 23 53 23
192.8			5	SS	7									14 44 24 18
3.4														
	Gravelly Sand to Sandy Gravel with some Silt		6	SS	20	87 09 16	192							40 48 9 3
	trace of clay		7	SS	27									
	Compact to Very Dense (Glacial Till)		8	SS	15		190							
	Occ. Cohesive Zones		9	SS	35									
	Occ. Cobbles		10	SS	41		188							19 63 15 3
186.1			11	SS	31	18 cm								
10.1	Weathered Shaly Slab		12	SS	59		186							55 15 25 5
185.8	End of Borehole													
10.4														

* AS: Auger Sample

OFFICE REPORT ON SOIL EXPLORATION

+3, x5: Numbers refer to
Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10

RECORD OF BOREHOLE No 4A

METRIC

W P 20-87-01 LOCATION NS Baseline Sta.0 + 045.4; O/S 27.2 m Rt ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger COMPILED BY TCK
 DATUM Geodetic DATE 1987 09 10 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT Y KN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL				
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			SHEAR STRENGTH kPa							WATER CONTENT (%)			
								20 40 60 80 100										
196.3	Ground Level																	
0.0	Mixture of Silt, Sand and Gravel, with random pockets or layers of Silty Clay (Fill)					*	196											
194.5																		
1.8	Organic Silty Clay		1	TW	PH	**	194						18.5	8 22 49 21				
192.9			2	TW	PH									0 23 53 24				
3.4	Sand with Gravel		3	TW	PH								18.9	5 36 48 11				
192.5			4	TW	PH									32 55 8 5				
3.8	End of Borehole																	
	* Note: Borehole Dry																	
	** Organic Content by Weight 6.4%																	

OFFICE REPORT ON SOIL EXPLORATION

+³, x⁵: Numbers refer to
Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10



RECORD OF BOREHOLE No 5

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 051.9; O/S 5.9 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 09 04 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES						
196.2	Ground Level										
0.0	Mixture Silt, Sand and Gravel with random pockets and layers of Silty Clay (Fill)		1	SS	28		196				3 28 44 25
195.2			2	SS	23						6 76 15 3
1.0			3	SS	17						
			4	SS	17						
	Gravelly Sand to Sandy Gravel with some Silt trace of clay		5	SS	32						
			6	SS	22						
	Compact to Dense (Glacial Till)		7	SS	26		192				21 64 13 2
	Occ. Cohesive Zones		8	SS	23						
	Occ. Cobbles		9	SS	20						
			10	SS	21						38 48 11 3
			11	SS	29						
186.8	Weathered Shaly Slab		12	SS	75		188	160/23 cm			41 25 27 7
9.4	End of Borehole										
186.6											
9.6	* AS: Auger Sample										

+3, x5: Numbers refer to Sensitivity
20
15
10
5 (%) STRAIN AT FAILURE

OFFICE REPORT ON SOIL EXPLORATION



RECORD OF BOREHOLE No 6

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 051.9; O/S 18.8 m Lt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 09 04 and 09 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES								
196.4	Ground Level												
0.0	Mixture Silt, Sand and Gravel with random pockets and layers of Silty Clay (Fill)		1	AS*			196						
195.3			1	SS	36								
1.1			2	SS	41								
	Gravelly Sand to Sandy Gravel with some Silt		3	SS	31								
	trace of clay		4	SS	32								
	Compact to Dense (Glacial Till)		5	SS	33								
	Occ. Cohesive Zones		6	SS	23								
	Occ. Cobbles		7	SS	32								
			8	SS	34								
			9	SS	49								
			10	SS	35								
			11	SS	26								
187.3			12	SS	62								
9.1	Weathered Shaly Slab		13	SS	40								
186.4													
10.0	Sand and Gravel												
186.0	End of Borehole												
10.4													

* AS: Auger Sample



RECORD OF BOREHOLE No 7

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 066.7; O/S 18.1 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 09 09 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	20 40 60 80 100					
195.8	Ground Level		1	AS*										
0.0	Silty Clay with random pockets or layers of Sand and Gravel (Fill)		2	SS	17									14 31 41 14
193.7			3	SS	12									2 18 57 23
2.1	Organic Silty Clay		4	SS	6									2 18 56 24
			5	SS	5									1 16 54 29
191.9			6	SS	10									1 28 47 24
3.9	Loose		7	SS	7									2 50 35 13
	Gravelly Sand to Sandy Gravel with some Silt trace of clay		8	SS	26									5 30 44 21
	Compact to Dense (Glacial Till)		9	SS	27									17 55 18 10
	Occ. Cohesive Zones		10	SS	21									
	Occ. Cobbles		11	SS	25									4 79 14 3
			12	SS	31									
185.6	Weathered Shaly Slab		13	SS	24									
10.2	End of Borehole		14	SS	40									22 33 38 7
185.4														
10.4														

* AS: Auger Sample

+³, x⁵: Numbers refer to
Sensitivity

20
15
10

5 (%) STRAIN AT FAILURE



RECORD OF BOREHOLE No 7A

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 065.8; O/S 19.5 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger COMPILED BY TCK
DATUM Geodetic DATE 1987 09 09 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT Y	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			SHEAR STRENGTH kPa									
195.9	Ground Level							20	40	60	80	100					
0.0	Silty Clay with random pockets or layers of Sand and Gravel (Fill)		1	TW	PH	*	194										2 18 55 25 5 15 51 29
193.5			2	TW	PH	**											1 18 54 27 14 56 17 13
2.4	Organic Silty Clay		3	TW	PH												5 41 39 15
192.8																	
3.1	End of Borehole at Sand Layer																
192.4																	
3.5																	
	* Note: Borehole Dry ** Organic Content by Weight 2.2%																

OFFICE REPORT ON SOIL EXPLORATION

+³, x⁵: Numbers refer to
Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10



RECORD OF BOREHOLE No 8

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 053.0; O/S 16.8 m Rt.

ORIGINATED BY TCK

DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test

COMPILED BY TCK

DATUM Geodetic DATE 1987 09 10

CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES						
196.2	Ground Level										
0.0	Silty Clay with random pockets or layers of Sand and Gravel (Fill)		1	AS*			196				8 25 52 15
			1	SS	17						5 31 49 15
193.6			2	SS	6		194				2 54 24 20
2.6	Gravelly Sand to Sandy Gravel with some Silt trace of clay		3	SS	8						9 73 12 6
			4	SS	17						
			5	SS	20						
	Compact to Dense (Glacial Till)		6	SS	24		192				
	Occ. Cohesive Zones		7	SS	29						
189.6	Occ. Cobbles		8	SS	31		190				32 52 14 2
6.6	End of Borehole										
							188				
187.1	End of Cone Test										
9.1	* AS: Auger Sample										

+3, x5: Numbers refer to 20
Sensitivity 15 5 (%) STRAIN AT FAILURE
10

OFFICE REPORT ON SOIL EXPLORATION

RECORD OF BOREHOLE No 9

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 041.3; O/S 18.2 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger COMPILED BY TCK
DATUM Geodetic DATE 1987 09 10 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20	40					
196.6	Ground Level													
0.0	Mixture Silt, Sand and Gravel with random pockets or layers of Silty Clay (Fill)		1	AS*			196							16 26 45 13
			1	SS	36									
			2	SS	8									
193.4			3	SS	7		194							11 46 17 26
3.2	Gravelly Sand to Sandy Gravel with some silt trace of clay		4	SS	18									
	Compact to Dense (Glacial Till)		5	SS	28									
			6	SS	27		192							9 72 14 5
	Occ. Cohesive Zones		7	SS	30									
190.0	Occ. Cobbles		8	SS	26									45 44 8 3
6.6	End of Borehole													
	* AS: Auger Sample													

OFFICE REPORT ON SOIL EXPLORATION

+3, x5: Numbers refer to Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10

RECORD OF BOREHOLE No 10

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 071.4; O/S 9.8 m Rt. ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 02 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100	SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES							
195.7	Ground Level											
0.0	Mixture Silt, Sand and Gravel with random pockets or layers of Silty Clay (Fill)		1	SS	12							
193.6	Organic Silty Clay		2	SS	14							
2.1			3	SS	13							
192.0			4	SS	11							
3.7	Gravelly Sand with some silt trace of clay		5	SS	17							
	Compact to Dense (Glacial Till)		6	SS	25							
	Occ. Cohesive Zones		7	SS	31							
	Occ. Cobbles		8	SS	20							
			9	SS	33							
186.1	End of Borehole		10	SS	36							
9.6												

+3, x5: Numbers refer to Sensitivity 20 15 10 5 (%) STRAIN AT FAILURE

OFFICE REPORT ON SOIL EXPLORATION



RECORD OF BOREHOLE No 11

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 071.2; O/S 1.4 m Lt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 10 01 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	SHEAR STRENGTH						WATER CONTENT (%)	
195.8	Ground Level							○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	10 20 30					GR SA SI CL		
0.0	Silty Clay with random pockets or layers of sand and Gravel (Fill)	[Symbol]	1	SS	21	87 10 5										
			2	SS	9											3 21 59 17
192.6			3	SS	3											
3.2	Organic Silty Clay	[Symbol]	4	SS	2										0 23 47 30	
191.8			5	SS	14										10 66 17 7	
4.0	Gravelly Sand to Sandy Gravel with some silt trace of clay Compact to Dense (Glacial Till) Occ. Cohesive Zones Occ. Cobbles	[Symbol]	6	SS	23										44 42 12 2	
			7	SS	43											
			8	SS	34											
			9	SS	36											44 41 12 3
186.3			Weathered Shaly Slab	[Symbol]	10			SS	106							
9.5	End of Borehole															
186.2																
9.6																

+3, x5: Numbers refer to
Sensitivity

20
15 ± 5 (%) STRAIN AT FAILURE
10



RECORD OF BOREHOLE No 12

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 058.3; O/S 4.7 m Lt.
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Tests
DATUM Geodetic DATE 1987 10 02

ORIGINATED BY TCK
COMPILED BY TCK
CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p NATURAL MOISTURE CONTENT W LIQUID LIMIT W _L WATER CONTENT (%) 10 20 30	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES						
196.3	Ground Level										
0.0	Mixture Silt, Sand and Gravel with random pockets and layers of Silty Clay (Fill)		1	SS	18		196				
194.7			2	SS	4		194				15 66 11 8
1.6	Gravelly Sand to Sandy Gravel with some Silt trace of clay		3	SS	8		194				2 87 6 5
	Compact to Dense (Glacial Till)		4	SS	24		192				5 83 11 1
	Occ. Cohesive Zones		5	SS	22		192				12 72 10 6
	Occ. Cobbles		6	SS	20		190				
			7	SS	27		190				
			8	SS	40		188				
188.5			9	SS	36		188				39 44 14 3
7.8	Weathered Shaly Slab										
187.7											
8.6	Gravelly Sand with some Silt, trace of clay		10	SS	*						
186.7											
9.6	End of Borehole										
	Note: * Blow Counts were not attainable										

OFFICE REPORT ON SOIL EXPLORATION

+3, x5: Numbers refer to
Sensitivity

20
15 5 (%) STRAIN AT FAILURE
10

RECORD OF BOREHOLE No 13

METRIC

W P 20-87-01 LOCATION NS Baseline Sta. 0 + 040.1; O/S 20.7 m Lt. ORIGINATED BY TCK
 DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Tests COMPILED BY TCK
 DATUM Geodetic DATE 1987 10 01 CHECKED BY _____

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES			20 40 60 80 100	20 40 60 80 100					
196.6	Ground Level													
0.0	Silty Clay with Sand trace of gravel (Fill)		1	SS	21		196							6 32 41 21
195.0			2	SS	29									20 63 12 5
1.6	Gravelly Sand to Sandy Gravel with some Silt trace of clay Compact to Very Dense (Glacial Till) Occ. Cohesive Zones Occ. Cobbles		3	SS	35		194							45 38 15 2
			4	SS	100	10 cm								
			5	SS	42									
			6	SS	27		192							47 35 13 5
			7	SS	49	87 105								
			8	SS	38		190							31 51 13 5
188.7			9	SS	42									
7.9	Weathered Shaly Slab						188							
187.1	Weathered Shaly Slab		10	SS	55									59 22 15 4
9.5	End of Borehole													
187.0														
9.6														

+3, x5: Numbers refer to
Sensitivity

20
15 5 (5%) STRAIN AT FAILURE
10

OFFICE REPORT ON SOIL EXPLORATION



RECORD OF BOREHOLE No 20

METRIC

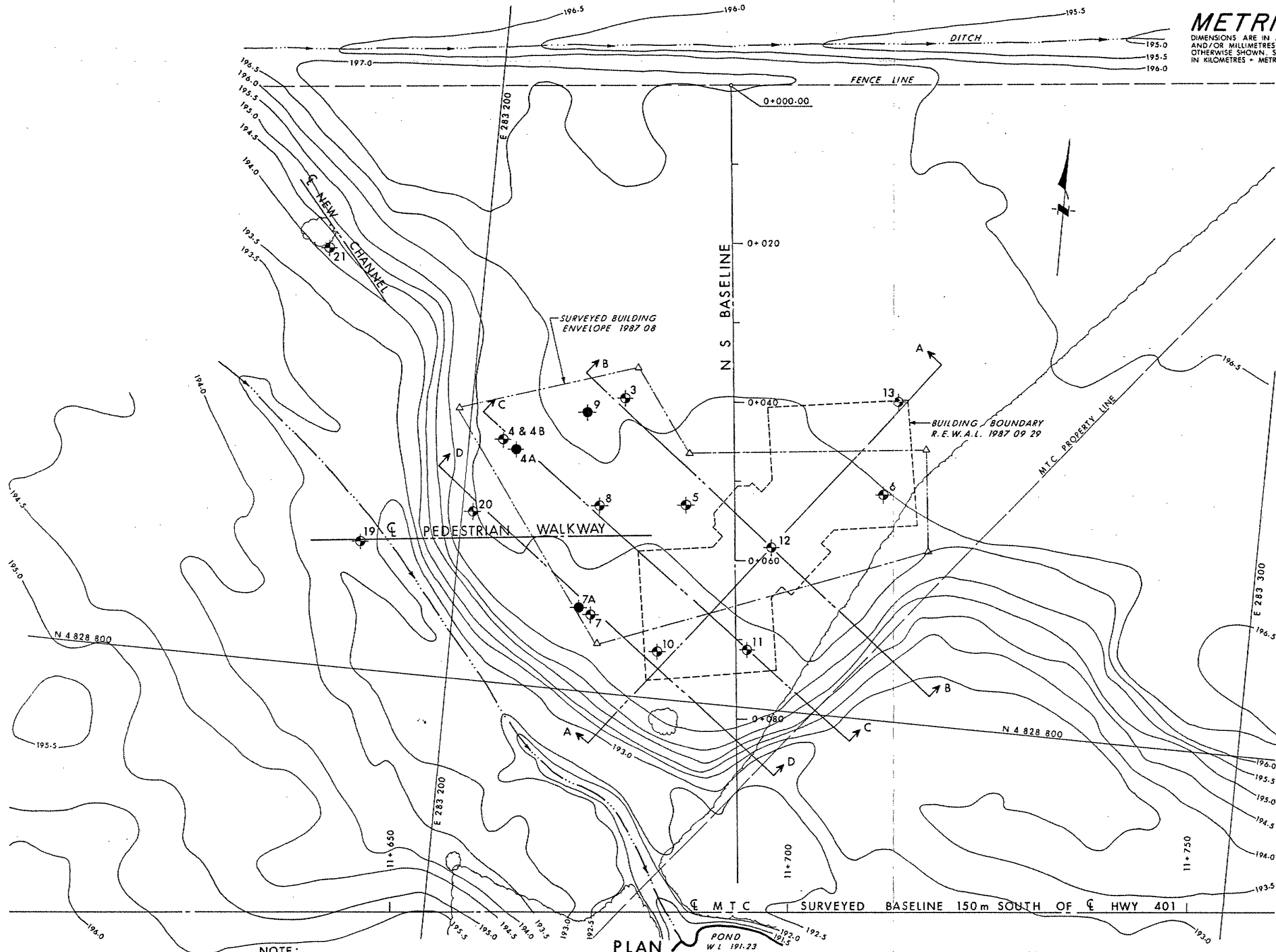
W P 20-87-01 LOCATION NS Baseline Sta. 0 + 053.5; O/S 32.7 m Rt. ORIGINATED BY TCK
DIST 6 HWY 401 BOREHOLE TYPE HS Auger and Cone Test COMPILED BY TCK
DATUM Geodetic DATE 1987 10 15 CHECKED BY

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT 20 40 60 80 100 SHEAR STRENGTH ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	'N' VALUES								
196.3	Ground Level												
0.0	Silty Clay with random pockets or layers of Sand and Gravel (Fill)		1	SS	7		196						5 37 39 19
			2	SS	15								7 18 46 29
	Tree Trunk		3	SS	148	20 cm	194						
192.6			4	SS	9								9 39 39 13
3.7	Loose		5	SS	3		192						14 57 20 9
	Gravelly Sand to Sandy Gravel with some Silt trace of clay		6	SS	35	87 10 26							24 56 15 5
	Dense to Very Dense (Glacial Till)		7	SS	42		190						
	Occ. Cohesive Zones		8	SS	39								
	Occ. Cobbles		9	SS	49		188						
186.2			10	SS	51								34 48 14 4
10.1	Weathered Shaly Slab		11	SS	100	10 cm	186						0 7 76 17
185.1													
11.2	End of Borehole												

+³, x⁵: Numbers refer to
Sensitivity

20
15
10
5 (%) STRAIN AT FAILURE

OFFICE REPORT ON SOIL EXPLORATION

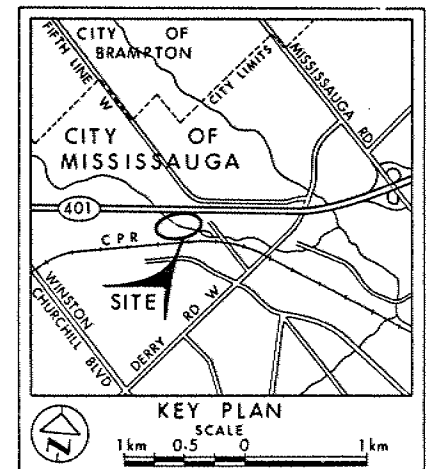


METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES.

CONT No
WP No 20-87-01

BUILDING FOUNDATIONS FOR
TRAVEL INFORMATION CENTRE
BORE HOLE LOCATIONS & SOIL STRATA

SHEET



- LEGEND**
- Bore Hole
 - Dynamic Cone Penetration Test (Cone)
 - Bore Hole & Cone
 - N Blows/0.3m (Std Pen Test, 475 J/blow)
 - CONE Blows/0.3m (60° Cone, 475 J/blow)
 - WL at time of investigation

No	ELEVATION	N S BASELINE STATION	OFFSET
3	196.7	0+039.5	13.5m Rt
4 & 4B	196.2	0+044.5	28.8m Rt
4A	196.3	0+045.4	27.2m Rt
5	196.2	0+051.9	5.9m Rt
6	196.4	0+051.9	18.8m Lt
7	195.8	0+066.7	18.1m Rt
7A	195.9	0+065.8	19.5m Rt
8	196.2	0+053.0	16.8m Rt
9	196.6	0+041.3	18.2m Rt
10	195.7	0+071.4	9.8m Lt
11	195.8	0+071.2	1.4m Lt
12	196.3	0+058.3	4.7m Lt
13	196.6	0+040.1	20.7m Lt
19	193.5	0+057.0	46.8m Rt
20	196.3	0+053.5	32.7m Rt
21	194.2	0+020.3	50.5m Rt

NOTE
The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence.

NOTE: The complete foundation investigation and design report for this project and other related documents may be examined at the Engineering Materials Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with the conditions of Section 102-2 of Form 100.

NOTE:
For Borehole No 9 refer to Record of Borehole Sheet.
Boreholes No 19 and 21 will be included in the report for the Pedestrian Bridge and Concrete Culvert.

NOTE:
For Sections A-A, B-B, C-C and D-D Refer to Dwg No 208701-C

REV.	DATE	BY	DESCRIPTION

Geocres No 30M12-201

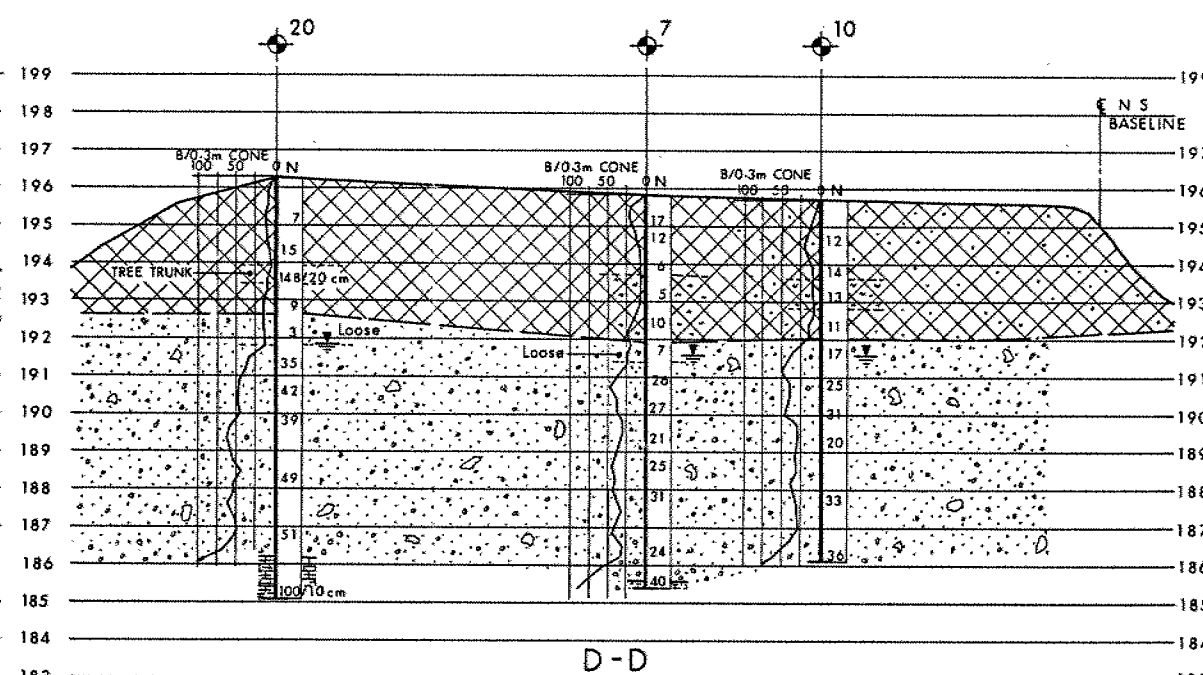
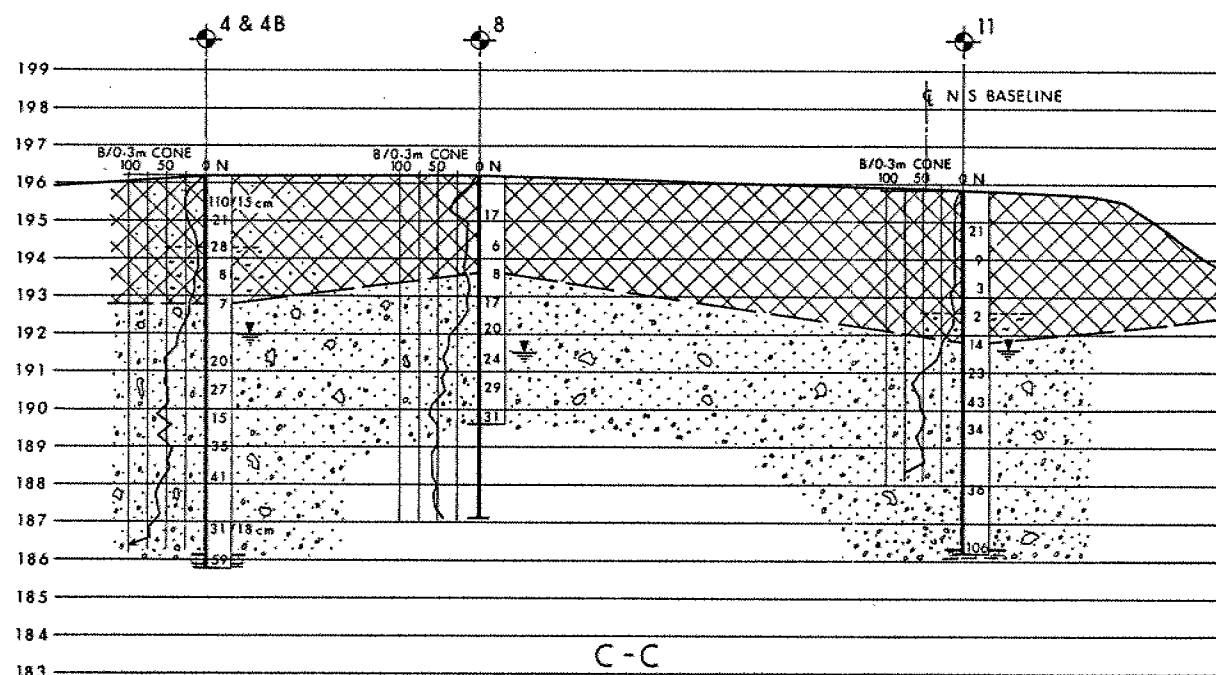
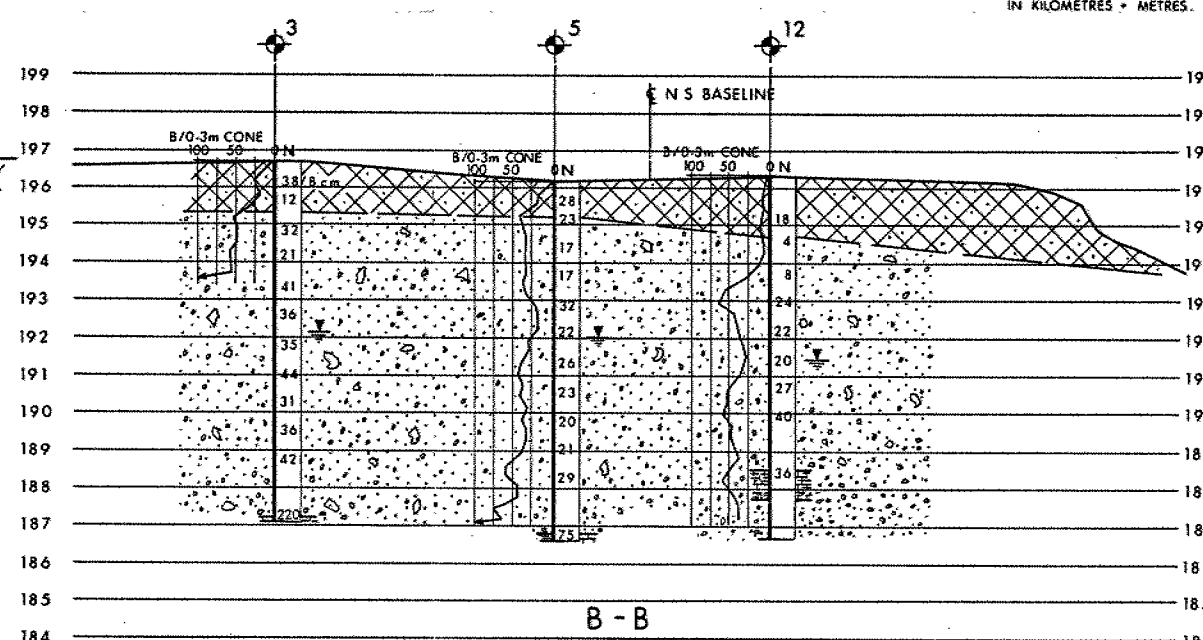
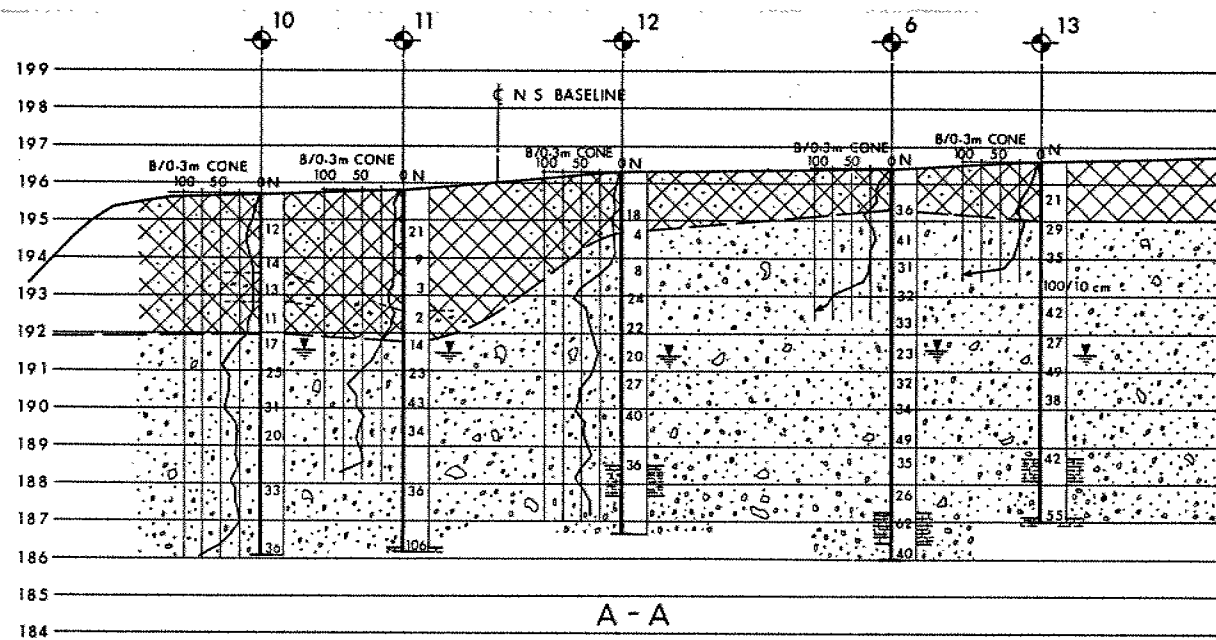
HWY No 401	DIST 6
SUBMITTAL CHECKED	DATE 1987 11 18
DRAWN BY	APPROVED
	DWG 208701-B

METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES.

CONT No
WP No 20-87-01

BUILDING FOUNDATION FOR
TRAVEL INFORMATION CENTRE
BORE HOLE LOCATIONS & SOIL STRATA

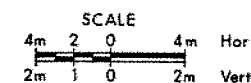
SHEET



SOIL STRATIGRAPHY LEGEND

	MIXTURE SILT, SAND & GRAVEL WITH RANDOM POCKETS AND/OR LAYERS OF SILTY CLAY (FILL)		GRAVELLY SAND TO SANDY GRAVEL WITH SOME SILT, TR OF CLAY, OCC ZONES, OCC COBBLES Comp to V Dense (GLACIAL TILL)
	SILTY CLAY WITH RANDOM POCKETS OR LAYERS OF SAND & GRAVEL (WITH SAND, TR GRAVEL) (FILL)		WEATHERED SHALY SLAB
	ORGANIC SILTY CLAY (FILL)		

SECTIONS



LEGEND

- Bore Hole
- ⊕ Dynamic Cone Penetration Test (Cone)
- ⊕ Bore Hole & Cone
- N Blows/0.3m (Std Pen Test, 475 J/blow)
- CONE Blows/0.3m (60° Cone, 475 J/blow)
- W L at time of investigation
87 09 and 87 10

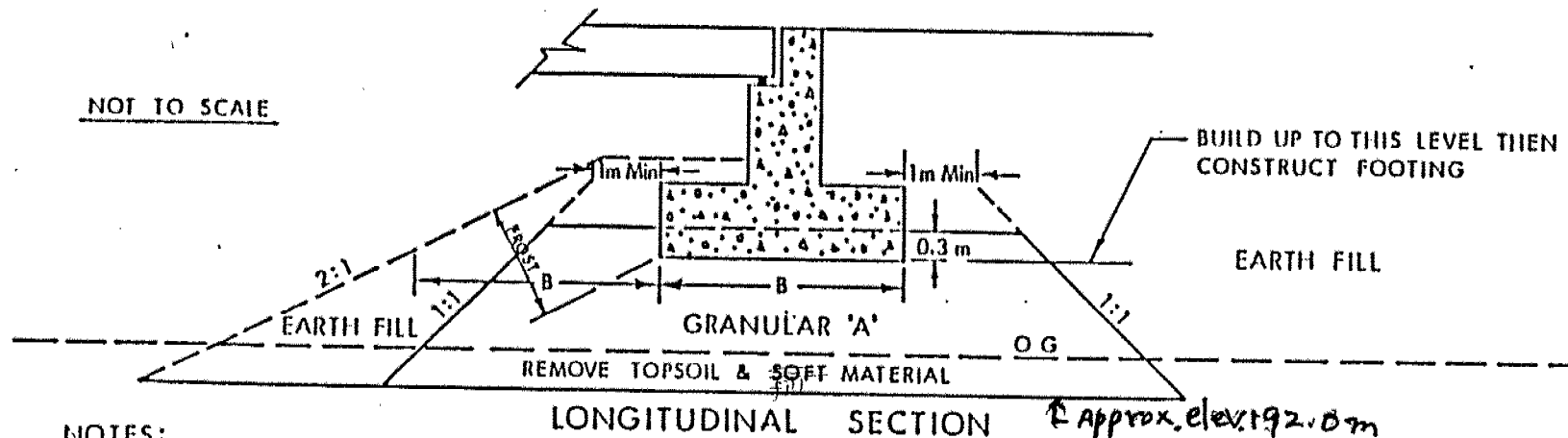
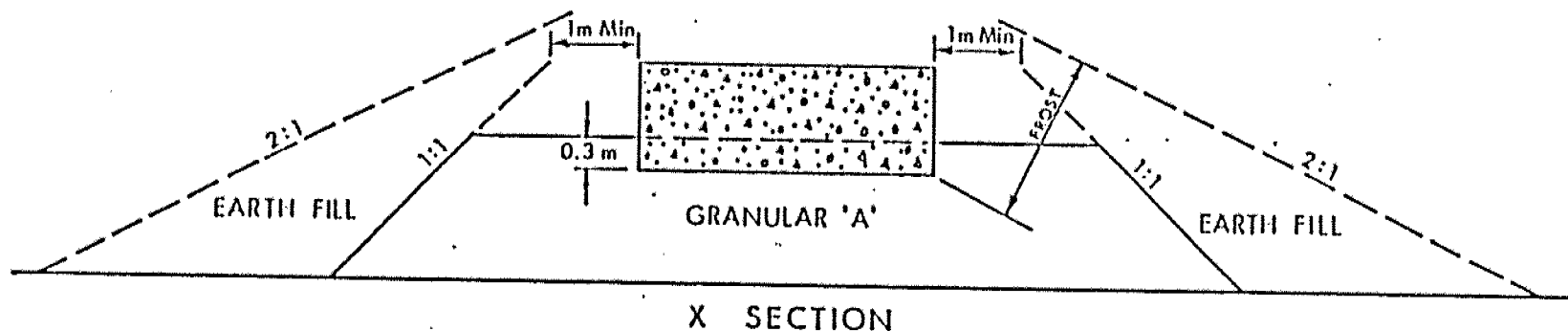
No	ELEVATION		
3	196.7		
4 & 4B	196.2		
5	196.2		
6	196.4		
7	195.8		
8	196.2		
10	195.7		
11	195.8		
12	196.3		
13	196.6		
20	196.3		

NOTE
The boundaries between soil strata have been established only at Bore Hole locations. Between Bore Holes the boundaries are assumed from geological evidence.

NOTE: The complete foundation investigation and design report for this project and other related documents may be examined at the Engineering Materials Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with the conditions of Section 102-2 of Form 100.

Note:
For Plan Refer to
Dwg No 208701-B

REV.	DATE	BY	DESCRIPTION
Geocres No 30M12-201			
HWY No 401		DIST 6	
SUBMITTAL CHECKED		DATE 87 11 16	
DRAWN BY		SITE	
		DWG 208701-C	



NOTES:

- 1- REMOVE TOPSOIL &/OR SOFT SUBSOIL UNDER AREA OF COMPACTED GRANULAR 'A' & EARTH FILL.
- 2- PLACE GRANULAR 'A' & EARTH FILL TO BOTTOM OF FOOTING LEVEL, COMPACTED ACCORDING TO CURRENT M T C STANDARDS.
- 3- CONSTRUCT CONCRETE FOOTING.
- 4- PLACE REMAINDER OF GRANULAR 'A' & EARTH FILL AS REQUIRED.



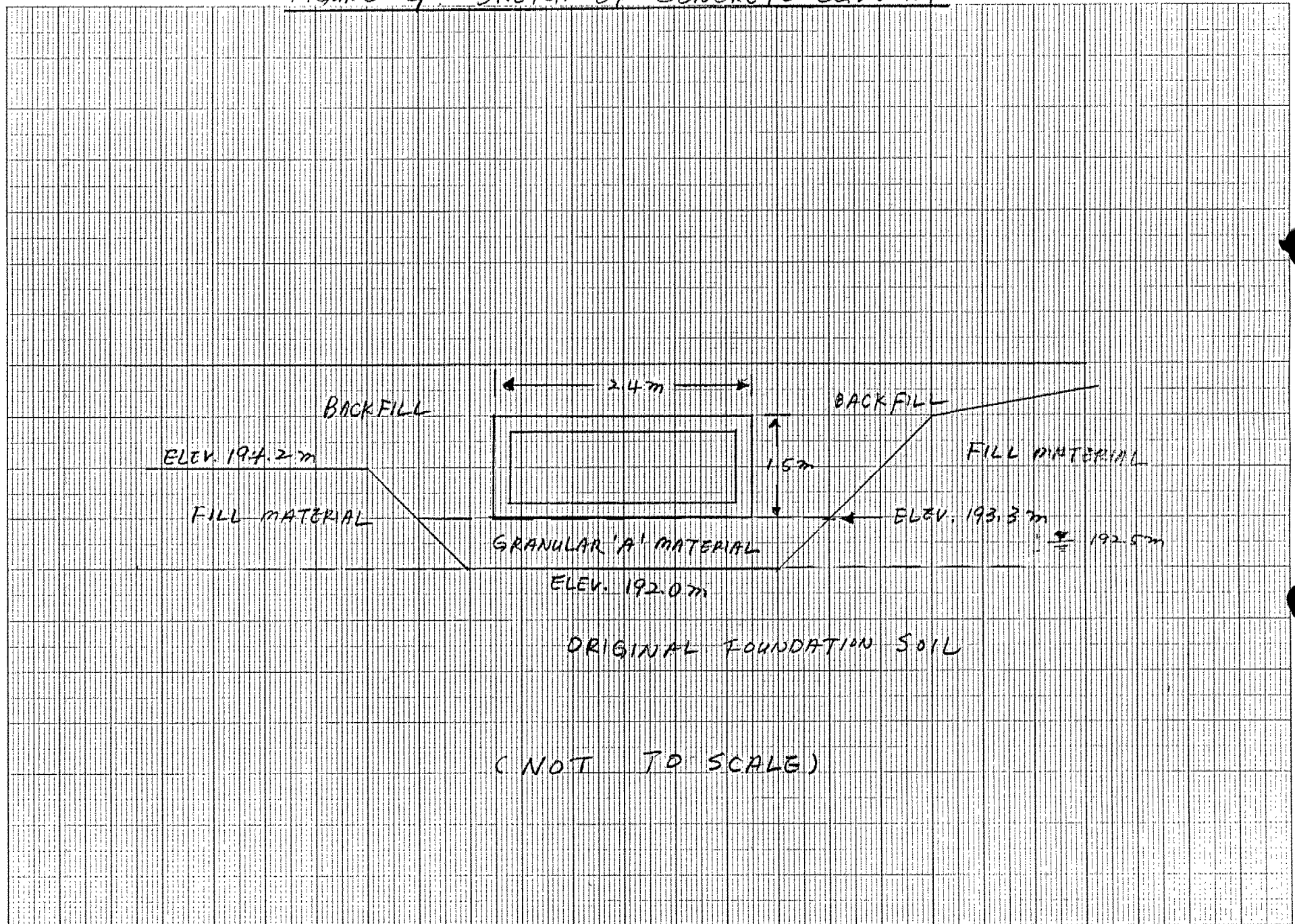
Ministry of
Transportation and
Communications

ABUTMENT ON COMPACTED FILL
SHOWING GRANULAR 'A' CORE

FIG No 3

WP 20-87-01

FIGURE 4. SKETCH OF CONCRETE CULVERT



CAR PARKING

1961

MINISTRY OF TRANSPORTATION AND COMMUNICATIONS

BUILDING BOUNDARY
RE.WAL. SEPT 1971

**SURVEYED
ENVELOPE**

SURVEY
ENVELOPE

BASELINE SURVEY
BOREHOLES SHEET

NEW CHANNEL

SKETCH FOR EXCAVATION AND BACKFILL

Parking Lot
ELEV 196.0

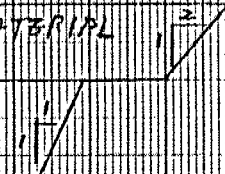


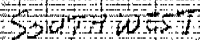
BASEMENT FLOOR ELEV. 194.5'

BACKFILL WITH GRANULAR TYPE OF MATERIAL
FOOTING ELEV. 193.5

GRANULAR 'A' BACKFILL

ELEV. 191.5 (EXCAVATION LINE)





memorandum



To: File

Date: 1987 08 12

From: D. Dundas

RE: Roadside Rest/Picnic/Information Centre
W.P. 20-87-01, Site N/A
Hwy. 401, District 6, Toronto

On 1987 07 29, Mary Lou Pauly of this Section attended a meeting at which R.E. Winter Associates Ltd. (REWAL) identified a need for a hydrogeological study for the above-noted project. Ms. Pauly advised that if the Foundation Design Section was to be requested to carry out the study, the request, with a well-defined problem statement, should be submitted through the Central Region Planning and Design Section.

On 1987 08 05, I returned a call to Ron Kawaguchi of REWAL. During our conversation Mr. Kawaguchi advised me that a hydrogeological study for the above-noted project was required as an extremely high priority, and that he had arranged for a hydrogeological study proposal to be presented at a meeting scheduled for 1987 08 06. I advised Mr. Kawaguchi that we would consult Central Region Planning and Design for direction.

At this time both Bill Lankinen and Len Dutchak of P & D were away. However, I had a discussion with Margot Sheppard who invited us to the meeting on behalf of P & D.

At the meeting Mr. Kawaguchi stated that Mr. Dutchak had authorized him to proceed with obtaining a proposal for a hydrogeological study, a copy of which was provided to me.

After the meeting we immediately attempted to contact P & D, which subsequently led to my telephone conversation of 1987 08 10 with Bill Lankinen in which Murty Devata of this office participated. During the conversation we advised Mr. Lankinen that geotechnical investigations of this nature are normally carried out through this office. We also agreed that any further requests for our involvement in this project would be from the P & D Section, as we are not aware of REWAL's authority to request our services.

However, on 1987 08 11 we were invited to another meeting by Mr. Kawaguchi. When I contacted Mr. Lankinen for clarification he advised me that REWAL was acting as project manager for this project and was calling on his behalf. He also advised that the purpose of the meeting was to discuss a proposal for a hydrogeological study prepared by REWAL's geotechnical consultant.

I responded that we could not contribute to a meeting involving a review of the REWAL geotechnical consultant's proposal for a hydrogeological study.

.....2

In order to avoid further inconsistencies, I again clarified our position as follows.

- A hydrogeological study is a geotechnical investigation. Normal MTC policy is to arrange such studies through this office. The study would either be conducted in house or assigned to a geotechnical consultant under our technical supervision. In this way we can ensure both the technical integrity and cost effectiveness of the study.
- If we can be of assistance with any aspect of this project related to geotechnical engineering, P & D should contact this office.

DHD/pb

c.c.: M. Devata
D.R. Brohm

D.H. Dundas

D.H. Dundas, P. Eng.
Sr. Foundations Engineer

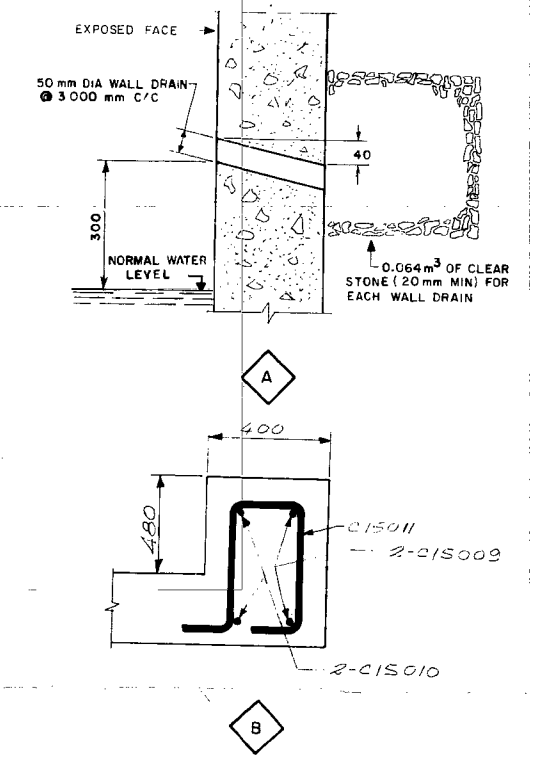
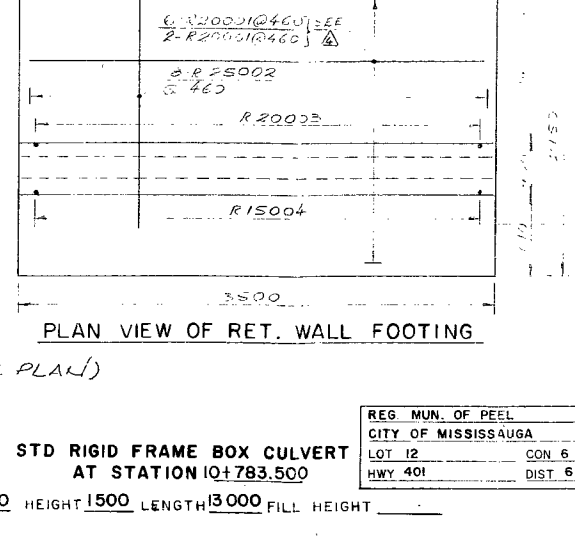
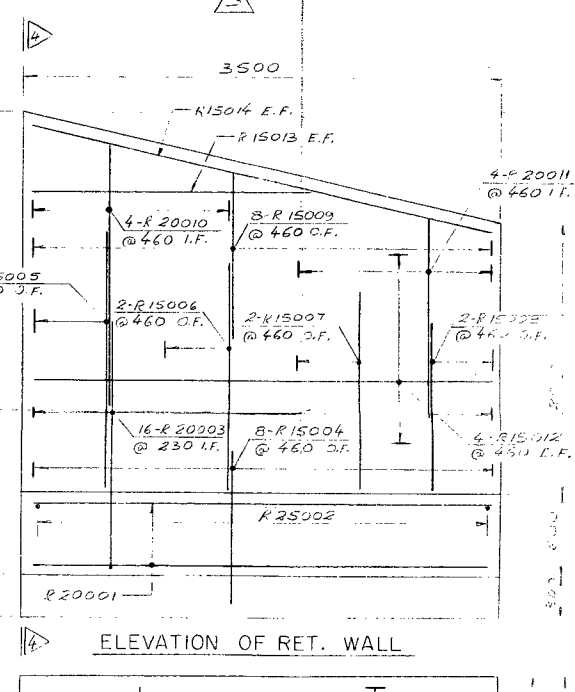
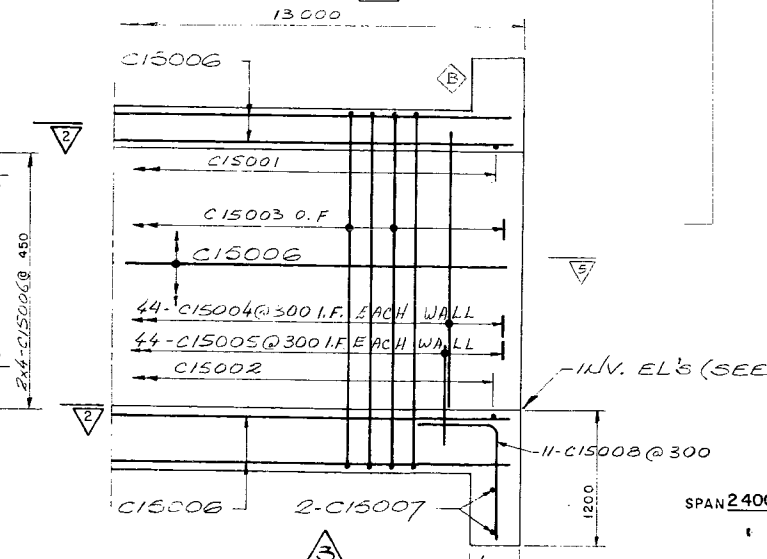
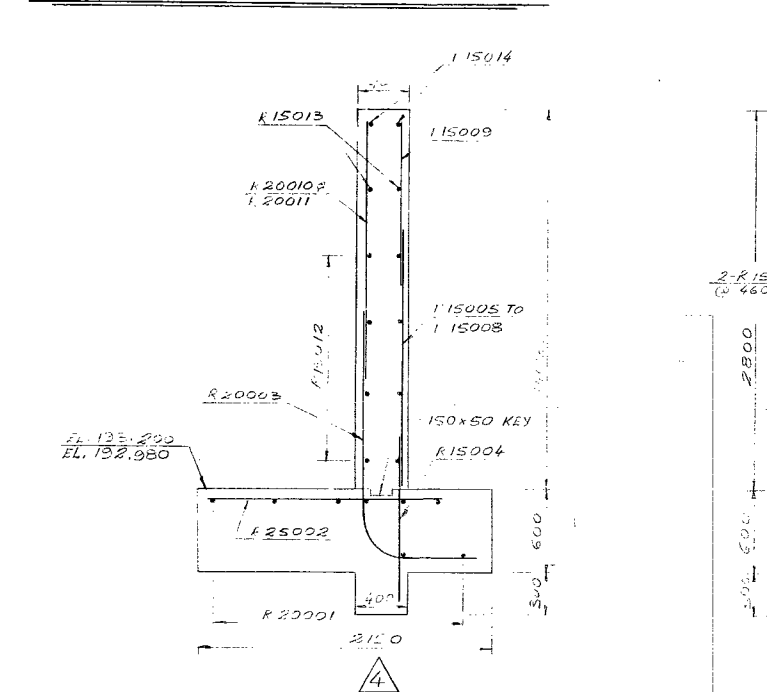
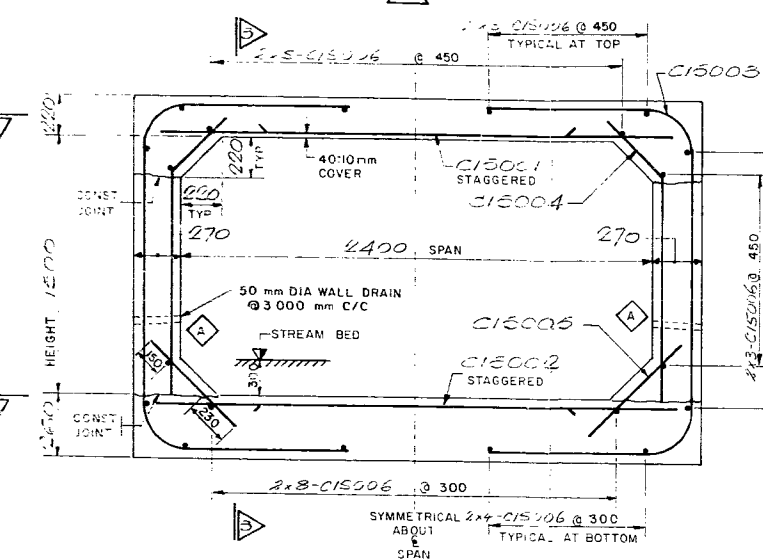
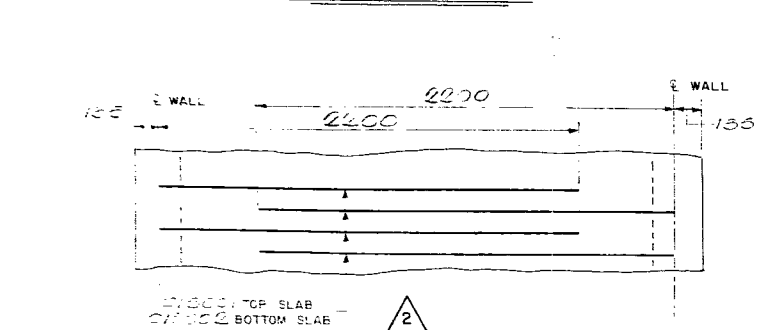
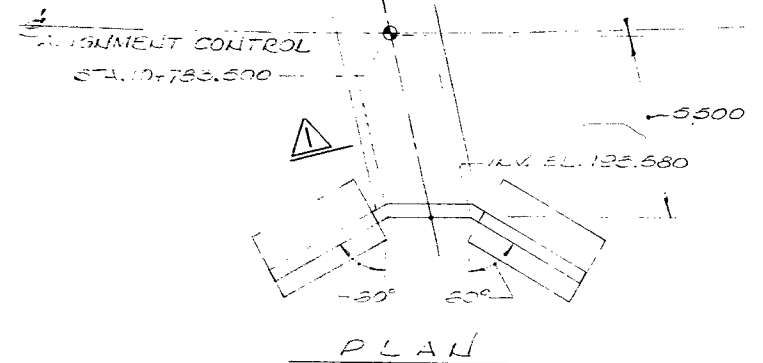
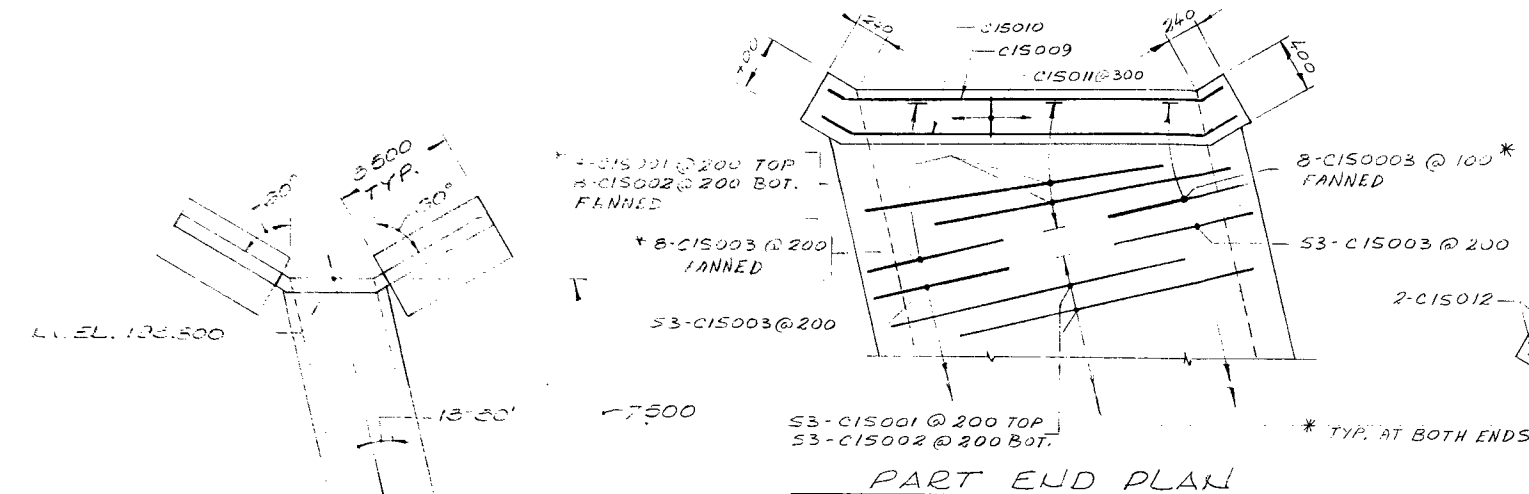
METRIC

DIMENSIONS ARE IN MILLIMETRES
UNLESS OTHERWISE SHOWN.
ELEVATIONS, COORDINATES, CURVE
AND ALIGNMENT DATA ARE IN METRES.
STATIONS ARE IN KILOMETRES + METRES.

CONT No
WP No 21-87-01

CULVERT AT HWY. 401
TRAVEL INFORMATION CENTRE
STATION 10+783.500

SHEET



- GENERAL NOTES**
- CLASS OF CONCRETE 20 MPa
 - CLEAR COVER TO REINFORCING STEEL 70/20mm EXCEPT AS NOTED
 - ALL EXPOSED CORNERS TO BE CHAMFERED 20mm
 - NO CONCRETE SHALL BE PLACED IN BOTTOM SLABS, UNTIL THE DEPTH OF THE EXCAVATION AND CHARACTER OF THE FOUNDATION MATERIAL HAVE BEEN APPROVED BY THE ENGINEER
 - FILL SHALL BE PLACED AT BOTH SIDES OF CULVERT SIMULTANEOUSLY
 - CULVERTS AND RETAINING WALLS (WHERE APPLICABLE) SHALL BE BUILT IN ACCORDANCE WITH O.P.S.S. 904.
 - REINFORCING STEEL SHALL BE HARD GRADE
 - STEEL FOR EACH CULVERT (INCLUDING RETAINING WALLS WHERE APPLICABLE) SHALL BE BUNDLED SEPARATELY AND MARKED WITH STATION NUMBER
 - WALL DRAIN OPENINGS TO BE FORMED USING NON-METALLIC MATERIAL. VERTICAL LOCATION OF WALL DRAINS SHALL BE DETERMINED IN FIELD BY THE ENGINEER
 - IF DENOTES INSIDE FACE
 - OF DENOTES OUTSIDE FACE
 - EF DENOTES EACH FACE

QUANTITIES			
ITEM	WALLS & SLABS	RETAINING WALL	TOTAL
MASS OF REINF STL tonnes			
VOL OF CONCRETE cubic metres			

STD RIGID FRAME BOX CULVERT
AT STATION 10+783.500

SPAN 2400 HEIGHT 1500 LENGTH 13000 FILL HEIGHT

REG. MUN. OF PEEL
CITY OF MISSISSAUGA
LOT 12 CON 6 W.H.S.
HWY 401 DIST. 6

STANDARD DRAWING
MARCH 1982

SS 114-5

REVISIONS	DATE	BY	DESCRIPTION
DESIGN	CHECK B.S.	LOADING	DATE 88-01
DRAWING 85/48	CHECK B.S.	SITE No 24-43/C	DWG 1

DRAWING NOT TO BE SCALED
100 mm ON ORIGINAL DRAWING