



ONTARIO
DEPARTMENT OF HIGHWAYS

Memo to Mr. A. M. Toye, Date February 5, 1960.
Bridge Engineer. Subject FOUNDATION INVESTIGATION - by
From Materials & Research Section. E.M. Peto Associates, Ltd.

Attention: Mr. S. McCombie.

Re: Chedoke Expressway
at
C.N.R. & C.P.R. Tracks
District 4

This memo accompanies a report on the foundation conditions at the above proposed subway location submitted by E. M. Peto Associates, Ltd. The information contained in this report is primarily factual, and if the Design Group have any comments with respect to the information contained therein, or the interpretation of data presented, we would be pleased to discuss this with you.

As a result of a meeting held in C.C. Parker's Office, Hamilton, Thursday, February 4th, the alignment at the location of these two structures, as given in this report, was confirmed. Also, a result of this committee meeting was that the median width at the subway location would be reduced to 30 feet.

C.C. Parker have been handed their copy of this report and it will not be necessary for you to forward one to them.

LGS/MdeF
Attach.

L. G. Soderman,
PRINCIPAL SOILS & FOUNDATIONS ENGINEER

cc: Messrs. A. M. Toye (2)
H. A. Tregaskes
D. G. Ramsay
I. Campbell
R. E. Richardson
P. F. Weber
Foundation Section
Gen. Files.

e. m. peto associates ltd.

YOUR REFERENCE:-

OUR REFERENCE:- 6002 and 6003

1287 caledonia road,
TORONTO 19, ONTARIO.
RUss-11 9-1126

26 January 1960

The Soil and Foundation Engineering Dept.,
Department of Highways of Ontario,
c/o Parliament Buildings,
Toronto 2, Ontario.

60-5-3080

Attention: Mr. L. Soderman, P. Eng.

Re: Soil Investigation for Chedoke Expressway
at C.N.R. and C.P.R. tracks.

Gentlemen:

In accordance with a verbal request by Mr. L. Soderman, a soil investigation was carried out at the above site. We have pleasure in forwarding herewith ten copies of our final borehole logs and a covering letter. The covering letter includes:

1. Programme of Work
2. General information
3. Soil and water conditions
4. Observations and conclusions

PROGRAMME OF WORK

Jan. 5th 1960	The equipment and the drilling units were moved onto the site.
Jan. 6th 1960	The equipment and the machines were set up. Unit no. 4 drove a dutch cone adjacent to borehole 9.
Jan. 7th 1960	Unit no. 1 started borehole 10. Unit no. 4 started borehole 9. Unit no. 6 drove a dutch cone adjacent to borehole 7 and started borehole 7.
Jan. 8th 1960	Unit no. 1, advancing borehole 10, delayed by breakdown. Unit no. 4 advanced borehole 9. Unit no. 6 completed borehole 7.

PROGRAMME OF WORK (continued)

- January 9 1960 Machine no. 1 under repair; unit no. 4 completed work at borehole 9 and started at borehole 11. Unit no. 8 started work at borehole 6.
- January 11 1960 Unit no. 1 completed work at borehole 10. Unit no. 4 advanced borehole 11. Unit no. 6 completed borehole 6.
- January 12 1960 Unit no. 1 started work at borehole 2. Unit no. 4 advanced borehole 11. Unit no. 6 started borehole 8.
- January 13 1960 Unit no. 1 advanced borehole 2. Units 4 and 6 completed work at boreholes 11 and 8 respectively. Following instructions issued by Mr. K. Peaker all the work at this location was stopped and the equipment and the drilling units were moved to Mercer Glen site.

GENERAL INFORMATION

(a) The boreholes were put down in accordance with our standard procedure. The attached borehole logs contain detailed descriptions of the recovered samples along with all the other pertinent data.

(b) The boreholes were put down at locations staked out by representatives of C. C. Parker and Parsons, Brinckerhoff Ltd. A 60°-2 ins. diameter Standard Dutch cone was driven at four locations. These were:

- P2 - 2 ft. north of borehole 2
- P7 - 4 ft. east of borehole 7
- P8 - 6 ft. east of borehole 9
- P11 - 7 ft. west of borehole 11.

The results of these tests have been included in the Observations and Conclusions as Section (e).

(c) Details of the holes put down were:

<u>Hole no.</u>	<u>Ground level elevation</u>	<u>Depth</u>	<u>Casing</u>
P 2	339.35	18' +	BX (and AXT non-coring bit)
	339.61	12'	
	325.70	44'	BX (and AXT coring bit)
P 7	324.14	44'	BX (and AXT coring bit)
	324.40	2'5"	
	322.16	41'5"	BX (and AXT coring bit)
P 9	325.46	47'3"	BX (and BX coring bit)
	325.02	5'3"	
	322.17	38'9"	BX (and AXT coring bit)
P 11	343.39	55'	BX (and BX coring bit)
	343.39	8'	

GENERAL INFORMATION (continued)

(d) The elevations given above are related to Bench Marks supplied by C. C. Parker and Parsons, Brinckerhoff Ltd. These were BM 338+98 (Elevation 341.63) top of north-east bolt on concrete base of C. P. R. light signal 100 ft. RT Sta. 338+98 E. B. L. and B. M. 337+60 (Elev. 325.39) top of north-east bolt on concrete base of C. N. R. light signal 62 ft. LT Sta. 337+60 E. B. L.

(e) Holes were put down without the use of wash water to virtual refusal, from this point downwards water was used to advance the holes with either a chopping bit, a non coring diamond bit or a coring diamond bit.

(f) The site is located at the C. N. R. and C. P. R. tracks about 1/4 mile west of Highway 2 and Highway 5 and just north-west of Mercer Glen in Hamilton.

Geologically the site is located on the east edge of the Niagara Escarpment just west of Hamilton. At this point the escarpment is made up of the Queenston shale which is Ordovician in age.

SOIL CONDITIONS

In general the material revealed by this investigation can be divided into two strata, an upper stratum of overburden and a lower stratum of soft to medium hard shale, reddish-brown in colour.

(a) Overburden

This stratum extended to the following depths in the borehole put down.

Borehole no.

2	0' - 3'
6	0' - 2'
7	0' - 2'
8	0' - 1'
9	0' - 5'
10	0' - 3'
11	0' - 2'

The material in this stratum consisted of topsoil or fill underlain by silty clay with sand and gravel. Detailed investigation of the material in this stratum can be seen on the borehole logs.

(b) Soft to medium hard shale

The colour of this stratum was reddish-brown with green-grey bands and specks. The upper part of this stratum is soft turning medium hard at the following depths in the boreholes put down.

- 4 -

SOIL CONDITIONS (continued)

Borehole No.

2	11 ft. 11 ins.
6	5 ft. 5 ins.
7	8 ft. 0 ins. approximately
8	3 ft. 0 ins.
9	7 ft. 0 ins.
10	7 ft. 1 in.
11	10 ft. 0 ins.

Lithologically the stratum is made up of a thinly bedded shale with sandy and possibly calcareous layers.

In addition to the upper soft part, the stratum contains several other soft layers of a minor nature. These were observed at:

<u>Borehole no.</u>	<u>Depth</u>	<u>Thickness of layers</u>
6	35 ft.	1 in.
	36 ft. 9 ins.	1 in.
	38 ft.	1 in.
	41 ft. 6 ins.	1 in.
7	38 ft. 10 ins.	2 ins.
8	16 ft. 6 ins.	3 ins.
	38 ft. 8 ins.	1/2 in.
	40 ft.	2 ins.
	41 ft. 10 ins.	3 ins.

Other soft layers may be present in the sections where the core recovery was poorer.

WATER CONDITIONS

The following is a table of water level readings taken at the time of the investigation:

WATER CONDITIONS (continued)

<u>Borehole 6</u>					
<u>Date</u> <u>(1960)</u>	<u>Time</u>	<u>Depth of</u> <u>Casing</u>	<u>Hole</u>	<u>Depth to water</u>	<u>Remarks</u>
Jan. 9		nil	2'	1'1"	Seepage from ditch
11	2.52pm	5' 5"	44'	0	Hole filled up
	2.54pm	5' 5"	44'	7'8"	
	2.56pm	5' 5"	44'	15'1"	
	3.00pm	5' 5"	44'	28'8"	
	3.05pm	5' 5"	44'	32'6"	
	3.56pm	5' 5"	44'	36'2"	
	6.35pm	5' 5"	44'	36'5"	
12	7.43am	5' 5"	44'	36'6"	

<u>Borehole 7</u>					
Jan. 7	6.00pm	9'	10'	9'9"	Hole bailed out to 9'9"
Jan. 8	7.40am	9'	10'	9'9"	
	7.00pm	9'	44'		Water level in hole dropped after pump shut off.
	7.18pm	9'	44'	35'8"	
Jan. 9	8.00am	9'	44'	36'3"	

After pulling casing water from ditch filled up hole.

<u>Borehole 8</u>					
Jan. 12		nil	2'	9"	Seepage from ditch

At 37'5" depth only about half the wash water, pumped down returned to the surface; the other half was lost through permeable layers.

Jan. 8		6'	37'		Hole filled up with water
9	7.30am	6'	37'	6'6"	
	1.00pm	nil	47'6"	6'6"	
11	7.30am	nil	47'6"	5'6"	
12	9.15am	nil	44'	4'10"	

<u>Borehole 10</u>					
11	4.05pm	7'	38'10"	0	Hole filled up
	4.07pm	7'	38'10"	11'2"	
	4.09pm	7'	38'10"	15'11"	
	4.14pm	7'	38'10"	19'5"	
	4.24pm	7'	38'10"	21'1"	
	5.00pm	7'	38'10"	23'2"	
	5.20pm	7'	38'10"	23'2"	
12	8.00am	nil	38'10"	6'	Surface water entered hole.

WATER CONDITIONS (continued)

As a result of the above water level readings, the following conclusions can be drawn: water can be expected to arise from two sources (a) surface (b) permeable layers in the shale.

1. A considerable amount of water was flowing in the north ditch along the C.N.R. tracks during the period of our investigation. This water may not exist during the dry summer months.

2. Although at the time of our investigation wash-water was being lost through permeable layers in the shale, these permeable layers can be expected to be water-bearing during the wetter climatic periods. These layers were noted at the following approximate depths in the boreholes shown.

<u>Borehole no.</u>	<u>Depth</u>
6	36 ft. 6 ins. (290.2)
7	36 ft. 3 ins. (287.9)
8	37 ft. 5 ins. (284.7)
9	5-6 ft. (325.0 ⁺) (very minor seepage)
10	23 ft. 2 ins. (299.0)
11	10 - 13 ft (333.4 - 330.4)
	16 ft (327.4)
	28 ft. (315.4)

OBSERVATIONS AND CONCLUSIONS

- (a) The soil at this site consists of a stratum of overburden underlain by a stratum of shale bedrock. The upper portion of the shale is soft, changing to a medium hard shale with depth. Surface water seepage and possible water from permeable layers in the shale may have to be controlled.
- (b) We are of the opinion that the overburden and the softer upper portion of the shale could be excavated by normal excavating equipment; whereas the medium hard shale may have to be drilled and blasted.
- (c) Apart from the softer upper 10 ft. or so, the shale core was of relatively good quality. Shale is, of course, generally unsuitable as a bulk fill since it deteriorates and reverts to a clay on exposure to air.

In the absence of an adequate supply of superior fills, it may be used under very carefully controlled circumstances as a bulk fill, either as "rock" ballast under water, or as "core" embankment fill where the shale is to be well covered and protected from the atmosphere by superior fill material. Recent experience in Hamilton has confirmed that the shale must be used in the dry summer months, since it will

OBSERVATIONS AND CONCLUSIONS (continued)

take up moisture readily and will roll and heave during compaction effort if it becomes too wet.

(d)

Since we understand that this site is to be abandoned as the crossing for the expressway, we have not carried out any strength tests on the rock core. In the absence of such tests we would suggest an allowable bearing value for this shale of 6.0 tons per square foot. This figure is, in our opinion, quite conservative.

(e)

As stated previously, Dutch cones were driven at boreholes 2, 7, 9 and 11. The following table gives the results:

P-2

<u>Depth</u>	<u>Blows per foot</u>
0 - 1'	12
1 - 2'	6
2 - 3'	17
3 - 4'	26
4 - 5'	42
5 - 6'	60
6 - 7'	67
7 - 8'	120
8 - 9'	100
9 - 10'	133
10 - 11'	160
11' - 11'11"	240
11'11" - 12'	108

P-7

0 - 1'	1
1 - 2'	79
2' - 2'8"	100
2'8" - 2'5"	100

P-9

0 - 1'	1
1 - 2'	1
2 - 3'	1
3 - 4'	23
4 - 5'	140
5' - 5'2"	100
5'2" - 5'3"	200

OBSERVATIONS AND CONCLUSIONS (continued)

P - 11

<u>Depth</u>	<u>Blows per foot</u>
0 - 1'	3
1 - 2'	1
2 - 3'	21
3 - 4'	41
4 - 5'	47
5 - 6'	50
6 - 7'	80
7 - 8'	300

We trust that the information supplied briefly herewith may be useful in review of conditions existing at any adjacent site now under examination. Should you require any supplementary information concerning this investigation, we shall be pleased to be of further service.

Yours very truly,

E. M. PETO ASSOCIATES LTD.

C. F. Freeman

C. F. Freeman, P. Eng.
Chief Engineer

UJV/jn

BOREHOLE LOG

Checked By U. J. V.





D.T.P.L. DRIER THAN PLASTIC LIMIT

[illegible]

e. m. petro associates ltd.
SOIL ENGINEERING SERVICE - TORONTO, ONTARIO
BOREHOLE LOG

Job Name Chedoke Expressway, C.N.R Job No. 6003 Borehole No. 6
 Client Dept. of Highways, Ontario Casing BX (AX core) Boring Date Jan. 9 & 11 1960
 Datum Client's Compiled By P.M. & A.M. Checked By E.M.P.

SAMPLE CONDITION

 **UNDISTURBED**
 **FAIR**
 **DISTURBED**
 **LOST**

SAMPLE TYPE

A.S. AUGER SAMPLE
 C.S. CASING SAMPLE
 S.S. 2" STANDARD SPLIT TUBE SAMPLE
 S.L. SPLIT BARREL WITH LINERS
 S.T. THIN-WALLED SHELBY TUBE SAMPLE
 W.S. WASH SAMPLE
 R.C. ROCK CORE

ABBREVIATIONS

V.T. IN SITU VANE SHEAR TEST
 C. SOIL SHEAR STRENGTH LBS/SQ.FT.
 W.L. WATER LEVEL IN CASING
 W.T. GROUND WATER TABLE IN SOIL
 W.T.P.L. WETTER THAN PLASTIC LIMIT
 D.T.P.L. DRIER THAN PLASTIC LIMIT

SOIL DESCRIPTION	COLOUR	Density or Consistency	Depth Elevation	Legend	Sample No. and Condition	Sample Type	No. of Blows per Ft.	Natural Moisture Content	WATER LEVELS & REMARKS
Ground surface			0'0"						
Gravelly silty clay, roots	Dk red-brown		320.70			C.S.			Sat.
Clay shale	Red-brn & grn-grey		2'0"			C.S.			Dry
Shale	Reddish-brn.					S.S.	127/1"	5.4	Dry
Transitional shale	Reddish-brn & band of grn-grey		5'5"			S.S.	100/8"	8.2	
Reamed hole from 5'5" to									
25'0" with concave bit									
(No attempt to recover core)									
Att. core recovered below 25'0"			25'0"						
Red -brown shale with 3 bands of grey shale 1/4" to 3" thick						R.C.			Longest piece 6-3/4" 38 fractures
			30'0"						4'8 1/2" recovery 94%
Red-brown shale with 6 bands of grey shale 1/4" to 1 1/2" thick						R.C.			Longest piece 3-1/4" 36 fractures 100% recovery
			34'0"						(Core v. moist to wet from 34' to 35'8")
Red-brown shale with 3 bands of grey shale 1/4" to 7" thick						R.C.			Longest piece 2-3/4" Very badly fractured 100% recovery
			39'0"						
Red-brown shale with 3 bands of grey shale 1/4" to 15" thick									(Wet seam 40'9" to 41'3" approx.) Longest piece 3-3/4". 47 fractures
			44'0"						4'9" Recovery 95%
Borehole terminated at 44'0"									Wash water returned during diamond drilling but W.L. dropped to 34' on completion.

SOIL ENGINEERING SERVICE - TORONTO, ONTARIO

BOREHOLE LOG

Job Name Chedoke Expressway C.N.R. Job No. 6003

Client Dept. of Highways, Ontario Casing BX & AY

Datum Client's Compiled By J.N.

Borehole No. 7


Boring Date Jan 7th and 8th, 1960

Checked By E.M.P.

SAMPLE CONDITION

SAMPLE TYPE

ABBREVIATIONS



UNDISTURBED

 FAIR

☒ DISTURBED

LOST

A.S. AUGER SAMPLE

C.S. CASING SAMPLE

S.S. 2" STANDARD SPLIT TUBE SAMPLE

S.L. SPLIT BARREL WITH LINERS

S.T. THIN-WALLED SHELBY TUBE SAMPLE

W.S. WASH SAMPLE

R.C. ROCK CORE

V.T. IN SITU VANE SHEAR TEST

C. SOIL SHEAR STRENGTH LBS/SQ.FT.

W.L. WATER LEVEL IN CASING

W.T. GROUND WATER TABLE IN SOIL

W.T.P.L. WETTER THAN PLASTIC LIMIT

D.T.P.L. DRIER THAN PLASTIC LIMIT

1. *Journal of Management Studies*, 1996, 33, 1, 1-14.

[illegible]

SOIL ENGINEERING SERVICE - TORONTO, ONTARIO





BOREHOLE LOG

Job Name Chedoke Expressway C.N.R. Job No. 6003 Borehole No. 8
Client D.H.O. Casing BX Boring Date Jan. 12 & 13th, 1960
Datum CLIENTS Compiled By S. J. B. Checked By U. J. V.

SAMPLE CONDITION

SAMPLE TYPE

ABBREVIATIONS

	UNDISTURBED
	FAIR
	DISTURBED
	LOST

A.S. AUGER SAMPLE
C.S. CASING SAMPLE
S.S. 2" STANDARD SPLIT TUBE SAMPLE
L.L. SPLIT BARREL WITH LINERS
S.T. THIN-WALLED SHELBY TUBE SAMPLE
W.S. WASH SAMPLE
R.C. ROCK CORE

V.T.	IN SITU VANE SHEAR TEST
C.	SOIL SHEAR STRENGTH LBS/SQ.FT.
W.L.	WATER LEVEL IN CASING
W.T.	GROUND WATER TABLE IN SOIL
W.T.P.L.	WETTER THAN PLASTIC LIMIT
D.T.P.L.	DRIER THAN PLASTIC LIMIT

[illegible]

e. m. peto associates ltd.
SOIL ENGINEERING SERVICE - TORONTO, ONTARIO
BOREHOLE LOG

Job Name Chedoke Expressway, C.N.R. Job No. 6003

Borehole No. 9

Client Dept. of Highways, Ontario Casing BK

Boring Date Jan. 7, - 9th, 1960





Date Client's Compiled By P.M. & A.M.

Checked By E.M.P.

SAMPLE CONDITION

SAMPLE TYPE

ABBREVIATIONS

	UNDISTURBED
	FAIR
	DISTURBED
	LOST

A.S. AUGER SAMPLE
C.S. CASING SAMPLE
S.S. 2" STANDARD SPLIT TUBE SAMPLE
S.L. SPLIT BARREL WITH LINERS
S.T. THIN-WALLED SHELBY TUBE SAMPLE
W.S. WASH SAMPLE
R.C. ROCK CORE

V.T. IN SITU VANE SHEAR TEST
C. SOIL SHEAR STRENGTH LBS/SQ.FT.
W.L. WATER LEVEL IN CASING
W.T. GROUND WATER TABLE IN SOIL
W.T.P.L. WETTER THAN PLASTIC LIMIT
D.T.P.L. DRIER THAN PLASTIC LIMIT

[illegible]

BOREHOLE LOG

Borehole No. 10

Boring Date **January 7, 8th, 1960**

Checked By E.M.P.

ABBREVIATIONS

Y.T. IN SITU VANE SHEAR TEST

C. SOIL SHEAR STRENGTH LBS/SQ.FT.

W.T. GROUND WATER TABLE IN SOIL

P.T.P.L. PRIOR THAN PLASTIC LIMIT

R.C. ROCK CORE

Borehole terminated at 38' 9"

BOREHOLE LOG

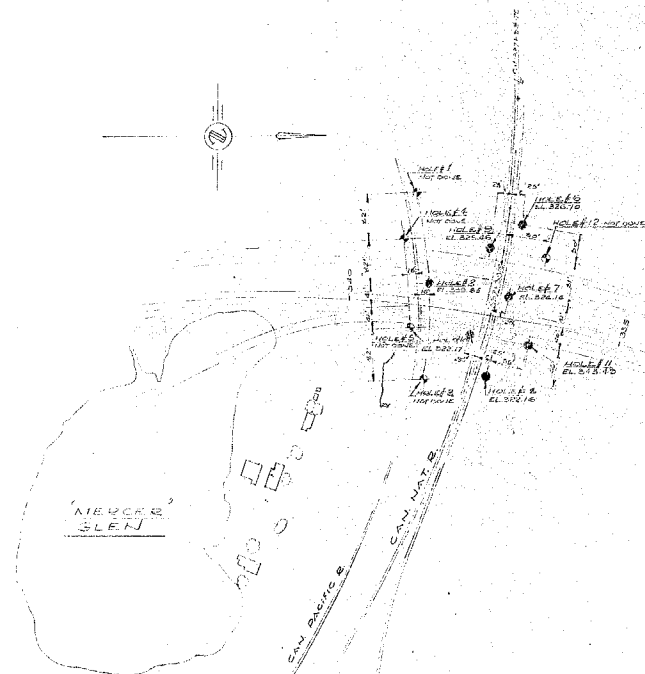
Borehole No. 11
Boring Date Jan. 9, 11, 12 & 13 1960
Checked By U. J. V.

ABBREVIATIONS

V.T.	IN SITU VANE SHEAR TEST
C.	SOIL SHEAR STRENGTH LBS/SQ.FT.
W.L.	WATER LEVEL IN CASING
W.T.	GROUND WATER TABLE IN SOIL
W.T.P.L.	WETTER THAN PLASTIC LIMIT
D.T.P.L.	DRIER THAN PLASTIC LIMIT

SOIL DESCRIPTION	COLOR	Density or Consistency	Depth Elevation	Legend	Sample No. and Condition	Sample Type	No. of Blows per Ft.	Moisture Content	WATER LEVELS & REMARKS
Ground surface			0'0"						
Topsoil			343.39						
Clay shale, organic matter reddish-brn	Dk.brown		2'0"		X	C.S.			3" frost Sat. W.T.P.L.
Clay shale, frag. of gr shale Reddish brn	Reddish brn		2'0"		X	C.S.			D.T.P.L.
Soft shale	Lt.olive-grey				X	S.S.	24	4.6	S.moist
Soft shale	As above				X	C.S.			Dry
Soft shale	As above		5'0"		X	C.S.			Dry
Soft shale, frag. of gr. shale	Red-brown				X	S.S.	47	11.2	A.dry
As above	As above				X	S.S.	130	11.4	A.dry
As above	As above				X	R.C.	cone from 85-90'		drilled & recovered 9" bx (47% recovery)
Soft shale	As above		10'0"		X	S.S.	130/5"	7.7	Longest unbroken pc. 5" A. dry
Med. hard shale with layers of sandy shale (with holes in it 10-13')	As above with gr-green layers								Turns denser at 10' 10-15' 4'4" recovery (87%) 2 gr-green layers 2" thick longest unbroken pc. 11"
			15'0"			S.S.	100/4"		No sample in S.S. 15-20' 4'4" recovery (87%) Loosing wash water at 16'
As above	Reddish-brown					R.C.			Longest unbroken pc. 12"
			20'0"			S.S.	100/1"		No sample in S.S. 20-25' 4'7" recovery (92%) 2 gr-green layers 2" thick Longest unbroken pce 9"
As above	As above with gr-green layers					R.C.			Longest unbroken pce 9"
			25'0"			S.S.	100/1"		No sample in S.S. 25-26' 7'4" recovery (67%) Longest unbroken pce .3" 26'-28' 2" 2'2" recovery 100%
As above	reddish-brown					R.C.			Longest unbroken pc. 4" 28' 2" 33' 5" 51' 3" recov Poss. perm. layer at 100%
As above	As above with grey-green layers		30'0"			R.C.			1 gr-green layer 1' thick 30-31'
						S.S.	100/1"		Longest unbroken pce. 6" No sample in S.S. 33' 5"-39' recovered 4'11" (88%)
As above	As above		35'0"			R.C.	3 gr		Longest unbroken pc. 5" 3 gr-green layers 2"-6" thick
			40'0"						34'-44' 3" Recovered 5' 3" 100%
As above	As above					R.C.			Longest unbroken pc. 4" 3 gr-green layers 2"-4" thick
			45'0"						44' 5"-49' 6" recov. 5' 3" (100%)
As above	As above					R.C.			Longest unbroken pce. 6" 2 grey-green layers 2" thick
			50'0"						49' 6" - 54' 0"
As above	As above					R.C.			Recovered 5' 3" (100%) Longest unbroken pc. 5" 1 grey-green layer 2" thick
			55'0"						49' 6" - 59' recovered 2' 4" (55%)
As above	As above 56'-59' grey green					R.C.			55' 2" 2" layer grey green shale Longest unbroken piece is 3"
			59'0"						Hole terminated at 59'0"

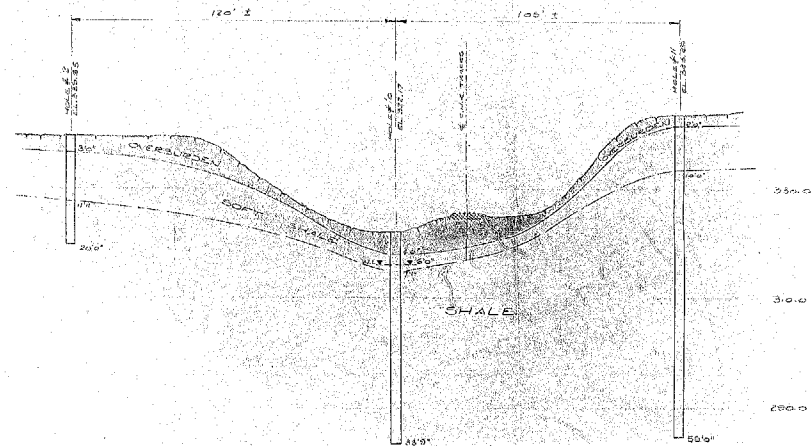
60-F-308C
CHEDOKE
EXPRESSWAY
C.N.R. & C.P.R. TRACKS



S.M. 355.32.00 (TOP OF NORTH EAST BOLT ON CONCRETE BASE OF LIGHT SIGNAL 100 FT. AT STA 338+00 E.C.L.)

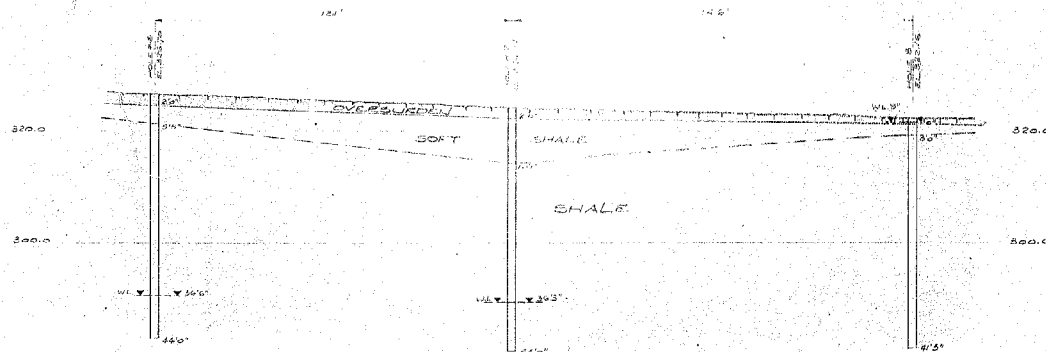
S.M. 355.32.00 (TOP OF NORTH EAST BOLT ON CONCRETE BASE OF C.N.R. LIGHT SIGNAL 60 FT. LT STA 337+20 E.C.L.)

NOTE:
BENCH MARKS SURVEYED BY
C.E. PARKER & ASSOCIATES, BRIDGEPORT, CT.



SECTION ON HOLES 2, 10 & 11.

SCALE: HORIZ. 1" = 20' 0"
VERT. 1" = 10' 0"



SECTION ON HOLES 6, 7 & 8.

SCALE: HORIZ. 1" = 20' 0"
VERT. 1" = 10' 0"

LEGEND

BOREHOLE
NATURAL LEVEL

NOTE:
SEE ATTACHED LOGS FOR COMPLETE DATA



e.m. peto & associates Inc.

SOIL SITE INVESTIGATION

AT
CHODOKE EXPRESSWAY
C.N.R. & C.P. TRACKS
FOR
DEPT. OF HIGHWAYS OF C.N.R.

DUR. 13th, 2002 & 2003
COUNTY PLAN No. 100 (4/20/03) IN C.N.R.