



THURBER ENGINEERING LTD.

**FINAL
FOUNDATION INVESTIGATION REPORT**
Culvert 24, Highway 69 – 5.8 km South of Highway 7182
Township of Shawanaga
Rehabilitation of Highway 69 and Shawanaga River Bridge
G.W.P. 5246-18-00
AGREEMENT NO: 5246-18-00
GEOCRES NO.: 41H09-001

Location: Lat: 45.510856°, Long: -80.228054°

Client Name: Egis Group

Date: June 4, 2024

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**FOUNDATION INVESTIGATION REPORT
CULVERT 24, HIGHWAY 69 – 5.8 KM SOUTH OF HIGHWAY 7182
TOWNSHIP OF SHAWANAGA, ONTARIO
G.W.P. 5246-18-00
AGREEMENT NO.: 5020-E-0003**

PART 1. FACTUAL INFORMATION

1. INTRODUCTION

This section of the report presents the factual findings obtained from a foundation investigation conducted by Thurber Engineering Ltd. (Thurber) for the rehabilitation of Culvert 24 which crosses Highway 69 approximately 5.8 km south of Highway 7182 (Shebeshekong Road) in the Township of Shawanaga, Ontario. Thurber carried out the assignment as a sub-consultant to Egis Group (Egis) under Assignment No. 5020-E-0003.

The purpose of the investigation was to explore the subsurface conditions at the site and based on this data obtained, provide a borehole location plan, record of boreholes, stratigraphic profile, laboratory test results and a written description of the subsurface conditions. A stratigraphic profile of the subsurface conditions was developed during the current investigation.

It is a condition of this report that Thurber's performance of its professional services is subject to the attached Statement of Limitations and Conditions.

2. SITE DESCRIPTION

2.1 General

The deep fill embankment culvert (CL-24) is located on Highway 69, approximately 5.8 km south of the intersection of Highway 69 and Shebeshekong Road at approximately Station 11+449 in the Township of Shawanaga, Ontario. For project orientation and reporting purposes, Highway 69 is herein described as orientated north-south and the culvert is described as oriented east-west. The existing culvert allows surface water to flow in an east to west direction under the highway.

At the location of the culvert, Highway 69 is a two-lane, undivided highway with a posted speed limit of 90 km/hr. The culvert crosses under the north and southbound lanes.

The base plan provided by Egis indicates that the existing culvert is an 1800 mm diameter corrugated steel pipe (CSP). Field observations confirmed that the culvert is a CSP. The plan indicates the length of the culvert is approximately 32. The culvert invert is at approximate Elev. 210.0 m at the inlet (east end) and at approximate Elev. 209.6 m at the outlet (west end). Cover above the invert of the culvert is approximately 4.5 m under the north and southbound lanes. The existing highway embankment at the culvert site is approximately 6 m to 6.5 m high and has side slopes inclined at approximately 1.4H:1V. Rock fill was noted on the embankment slopes around the culvert inlet and outlet.

The lands surrounding the site are generally flat to gently undulating, following the shallow bedrock surface. Bedrock is observed at ground surface both north and south of the culvert site and appears to dip towards the culvert and creek channel. The lands east and west of the culvert consist of swamp and some open water (observed seasonally). Photographs in Appendix E show the general nature of the site and the existing culvert.

2.2 Site Geology

Based on surficial geology mapping¹ prepared by the Ontario Geological Survey the culvert is located in an area mapped as Precambrian bedrock. Bedrock mapping² also prepared by the Ontario Geological Survey maps the local bedrock as migmatic, tonalitic to granodioritic gneiss.

3. INVESTIGATION PROCEDURE

The foundation investigation and field-testing program for CL-24 was carried out in conjunction with several other culvert investigations reported under separate covers. The boreholes for this culvert investigation were advanced on August 10th and August 30th, 2023. The investigation consisted of two (2) boreholes, designated as 23-01 and 23-02, advanced to depths of 12.0 m and 6.3 m below ground surface (Elev. 198.0 m and 203.9 m), respectively. Boreholes 23-01 and 23-02 were drilled near the culvert inlet and outlet, respectively.

The Record of Borehole sheets are included in Appendix B. The approximate borehole locations are shown on the attached Borehole Locations and Soil Strata Drawing in Appendix A.

A summary of the borehole coordinates, elevations, and termination depths is provided in Table 3.1. The as-drilled borehole elevations and coordinates for Borehole 23-02 were provided by

¹ Ontario Geological Survey 2010. *Surficial geology of Southern Ontario*; Ontario Geological Survey, Miscellaneous Release – Data 128_REV.

² Culshaw, N.G., Corrigan, D., Ketchum, J.W.F., Wallace, P. and Wodicka, N. 2004. *Precambrian geology, Naiscoot area*; Ontario Geological Survey, Preliminary Map P.3549, scale 1:50 000.

Callon Dietz, Egis' survey subconsultant. The survey was completed in the horizontal datum MTM Zone 10 CSRS CBNv6-2010.0 and the vertical datum CGVD 1928:1978 with a horizontal and vertical accuracy +/- 5 cm. Borehole 23-01 could not be located at the time of the survey. The coordinates and elevation of Borehole 23-01 are approximate and were determined by correlating field sketches and photographs with the topographic survey. The borehole coordinates and elevations are also shown on the Borehole Location and Soil Strata drawing included in Appendix A and on the individual Record of Borehole sheet included in Appendix B.

Table 3-1 Borehole Summary

Borehole	Northing (m)	Easting (m)	Ground Surface Elevation (m)	Termination Depth Below Ground Surface (m)
23-01	5,041,481.7	247,923.4	210.0	12.0
23-02	5,041,455.2	247 904.4	210.1	6.3

Utility clearances were obtained prior to the start of drilling.

Boreholes 23-01 and 23-02 were drilled at the ends of the culvert using a track mounted CME-55 provided by Downing Drilling and a Diedrich D-50 drill rig provided by Walker Drilling Ltd, respectively. The boreholes were advanced through the overburden using hollow stem augers and wash boring methodologies. Soil samples were obtained at selected intervals using a split spoon sampler in conjunction with Standard Penetration Testing (SPT). HQ coring methods were used to advance the two boreholes into bedrock.

The drilling and sampling operations were supervised on a full-time basis by a member of Thurber's technical staff. The supervisor logged the boreholes and processed the recovered soil and rock core samples for transport to Thurber's laboratory for further examination and testing.

The rock cores were logged, and the Total Core Recovery (TCR), Solid Core Recovery (SCR), Rock Quality Designation (RQD) and Fracture Index (FI) were determined.

Due to water being introduced during drilling and coring operations groundwater conditions were not observed in the open boreholes. A standpipe piezometer consisting of a 25 mm diameter PVC pipe with a 3.05 m long slotted screen, enclosed in a column of filter sand was installed in Borehole 23-01 to permit groundwater level monitoring. A monitoring well consisting of 50 mm diameter Schedule 40 PVC pipe with a 1.5 m long slotted screen, enclosed in a column of filter sand was installed in Borehole 23-02. The monitoring well was installed as per Egis' request to allow for well testing to be carried out by Egis to support a potential Permit to Take Water application or registration on the Environmental Activity and Sector Registry. Well installation details, groundwater level observations and water level readings are shown on the Record of

Borehole sheets in Appendix B. A surface water sample at the culvert inlet was obtained during the field investigation and submitted to a specialist analytical laboratory under chain of custody procedures for testing for a corrosivity related parameters. The laboratory testing results are shown in Appendix C.

Details of the drilling program, including drilling depths, monitoring well/ piezometer installation and completion details are summarized in Table 3.2 below.

Table 3-2: Borehole Completion Details

Borehole Number	Top of Borehole Elevation (m)	Borehole Depth / Base Elevation (m)	Monitoring Well/ Piezometer Depth / Elevation (m)	Completion Details
23-01	210.0	12.0 / 198.0	4.6 / 205.4	Borehole backfilled with bentonite holeplug from surface to 1.2 m, filter sand from 1.2 m to 4.9 m, bentonite holeplug from 4.9 m to 12.0 m.
23-02	210.1	6.3/203.8	3.7 / 206.4	Borehole backfilled with bentonite holeplug from ground surface to 1.2 m, peltonite from 1.2 m to 1.6 m, sand from 1.6 m to 4.2 m, and bentonite from 4.2 m to 6.3 m.

The standpipe piezometer was decommissioned in general accordance with O.Reg. 903 upon collection of the final water level reading. The monitoring well was left in place and will be decommissioned as part of the construction contract under Egis' direction.

4. LABORATORY TESTING

Laboratory testing was selected in general accordance with the current MTO Guideline for Foundation Engineering Services, Section 5. Geotechnical laboratory testing consisted of natural moisture content determination and visual identification of all retained soil samples and grain size distribution analysis. Atterberg Limits testing was not carried out as plastic soils were not encountered in either of the boreholes. The rock cores were photographed, and the total core recovery (TCR), solid core recovery (SCR), and rock quality designation (RQD) were measured. Unconfined compressive strength (UCS) and point load testing was carried out on select intact bedrock cores to assess the unconfined compressive strength (UCS) of the bedrock. The results

of this testing program are summarized on the Record of Borehole sheets in Appendix B and are shown on the figures included in Appendix C.

In order to assess the potential for sulphate attack on buried concrete structures, as well as the potential for corrosion associated with buried steel elements of the structures, a sample of the native soil from the boreholes, as well as a surface water sample from the upstream end of the culvert were collected during the investigation. The samples were submitted to SGS, a CALA accredited analytical laboratory in Mississauga, Ontario, for analytical testing of corrosivity parameters and sulphate content. The results of the analytical testing are summarized in this report and presented in Appendix C.

5. DESCRIPTION OF SUBSURFACE CONDITIONS

Details of the encountered soil stratigraphy are presented on the Record of Borehole sheets included in Appendix B and on the Borehole Locations and Soil Strata drawing in Appendix A. A general description of the stratigraphy, based on the conditions encountered in the boreholes, is given in the following paragraphs. However, the factual data presented in the Record of Borehole sheets governs any interpretation of the site conditions. It must be recognized that soil conditions may vary between and beyond the borehole locations.

In general, the encountered stratigraphy consists of topsoil overlying native silt or sand overlying bedrock.

5.1 Topsoil

75 mm and 100 mm of topsoil were encountered at the ground surface in Boreholes 23-01 and 23-02, respectively. Natural moisture contents measured in the topsoil ranged from 40% to 130%. The topsoil thickness may vary between and beyond the boreholes.

5.2 Silt and Sand to Silt

A layer of silt and sand to silt was encountered in Borehole 23-02 at 0.1 m (Elev. 210.0 m) and extended to bedrock at 0.7 m depth (Elev. 209.4 m). The silt to sand and silt ranged in colour from brown to grey, and trace amounts of gravel and clay were observed within the layer.

One SPT N-value of 12 blows per 0.3 m of penetration was observed, indicating a compact relative density. A natural moisture content of 19% was recorded in the silt and sand to silt material.

5.3 Sand

A deposit of sand to silty sand was encountered below the topsoil in Borehole 23-01 at a depth of 75mm below ground surface. The base of the layer was encountered overlying bedrock at 8.2 m below ground surface (Elev. 201.8 m). The sand layer contained trace to some gravel and was generally described as containing trace silt and ranging in colour from brown to grey. Trace organics and a silty composition were noted below the topsoil and extending to 2.2 m (Elev. 207.8 m).

SPT “N” values ranging from 0 to 16 blows per 0.3 m of penetration indicating a relative density of very loose to compact were recorded. The natural moisture contents of the deposit varied from 14% to 36%.

Grain size analyses were performed on two selected samples of this deposit and the results are presented on the relevant Record of Borehole sheets and in Figure C1 in Appendix C. The test results are summarized below in Table 5-1.

Table 5-1: Grain size distribution of Sand

Soil Particle	Percentage (%)
Gravel	6 to 16
Sand	79 to 89
Silt and Clay	5

5.4 Bedrock

The overburden soils described above are underlain by gneiss bedrock. Bedrock was proven by coring in both boreholes. The bedrock is described as grey to pink in colour, highly weathered to fresh and weak to very strong. Photographs of the bedrock core are provided in Appendix D.

Table 5-2 summarizes the depths and elevations of the top of bedrock at the borehole locations.

Table 5-2: Depths and Elevations of Top of Bedrock

Borehole	Top of Bedrock	
	Depth (m)	Elevation (m)
23-01	8.2	201.8
23-02	0.7	209.4

The rock core recovery measurements, rock quality designation and rock core laboratory testing results are summarized in Table 5-3 below.

Table 5-3 Bedrock Details

Parameter	Range
Total Core Recovery (TCR), %	87 to 100
Solid Core Recovery (SCR), %	62 to 100
Rock Quality Designation (RQD), %	41 to 100
Fracture Index (fractures per 0.3 m)	0 to 6
Unconfined Compressive Strength Testing (MPa)	91 to 117
Unconfined Compressive Strength Testing (MPa) from Point Load Tests	12 to 132

Based on the RQD, the bedrock quality ranged from poor to excellent, typically excellent (CFEM 5th Edition, 2023). The results of UCS and point load testing indicate that the tested samples of the bedrock are weak to very strong, typically strong to very strong (CFEM 5th Edition, 2023).

Locally in Run 1 of Borehole 23-01, the gneissic bedrock was described as moderately to highly weathered. UCS values as low as 12 MPa were measured from point load tests, indicating weak rock classification. RQD values of 41 to 47% were measured in 23-01 Runs 1 and 2, indicating the rock was poor quality.

The results of the UCS and point load testing are included in Appendix C.

5.5 Groundwater Level

Water levels were not observed upon completion of the boreholes as water was introduced into the borehole for bedrock coring and levels may not be representative. The measured groundwater levels observed in piezometer / monitoring well installations are summarized in the table below.

Table 5-4: Groundwater Measurements

Borehole	Date	Water Level (m)		Remark
		Depth	Elevation	
23-01	September 21, 2023	0.0	210.0	In piezometer
23-02	August 11, 2023	0.2	209.9	In monitoring well
	September 21, 2023	0.5	209.7	In monitoring well
	October 4, 2023	0.6	209.5	In monitoring well

It should be noted that the above value is considered a short-term reading and may not reflect the groundwater level at the time of construction. Seasonal fluctuations of the groundwater level are to be expected. In particular, the groundwater level may be at a higher elevation after periods of significant and/or prolonged precipitation events.

At the time of the investigation, no water was observed flowing through the culvert. Small pools/puddles of stagnant water were noted near each end of the culvert and the ground was generally wet.

6. CORROSIVITY AND SULPHATE TEST RESULTS

A sample of native silt from Borehole 23-02 was submitted for analytical testing of corrosivity parameters and sulphate. A sample of surface water taken from the surface water at the inlet of the culvert was also submitted for analytical testing of pH, sulphate, chloride, resistivity, and conductivity. The laboratory certificates of analysis for the current investigation are presented in Appendix C. The results of the analytical tests are summarized below in Table 6-1.

Table 6-1: Analytical Corrosivity Test Results

Parameter	Units (Soil)	Units (Water)	Test Results	
			23-02 SS#1 CORR (0.0m – 0.61m)	Culvert Inlet Culvert 24 STA 11+449
			(Silt and Sand to Silt)	(Surface Water)
Redox Potential	mV	mV	287	143
Sulphide	%	µg/L	<0.04	< 6
pH	-	-	7.12	5.80
Chloride	µg/g	mg/L	130	210
Sulphate	µg/g	mg/L	17	12
Conductivity	uS/cm	uS/cm	267	657
Resistivity	Ohms.cm	-	3750	---

7. MISCELLANEOUS

Thurber obtained utility clearances for the borehole locations prior to drilling. Borehole locations were selected and established in the field by Thurber.

George Downing Estate Drilling (Downing) of Hawkesbury, Ontario and Walker Drilling of Utopia, Ontario supplied the drill rigs and conducted the drilling, sampling and in-situ testing operations.



Traffic control services were provided by Beacon Lite of Ottawa, Ontario for access to the boreholes.

All geotechnical laboratory testing of soil samples and point load testing of bedrock core samples were carried out in Thurber's geotechnical laboratory. Uniaxial compressive strength tests were carried out by Geomechanica Inc. Analytical testing of soil and water samples was carried out by SGS Canada Inc.

The field investigation was supervised on a full-time basis by Mr. Jakob Flood of Thurber. The overall supervision of the field program was conducted by Ms. Madisan Chiarotto, EIT and Mr. Matthew Boucher, P.Eng. of Thurber.

Interpretation of the field data and preparation of this report was carried out by Ms. Rachel Bourassa, EIT and Mr. Matthew Boucher, P.Eng. The report was reviewed by Mr. Jason Lee, P.Eng., a Designated Principal Contact for MTO Foundations Projects.

Thurber Engineering Ltd.

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This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

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All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

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- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

6. RELEASE OF POLLUTANTS OR HAZARDOUS SUBSTANCES

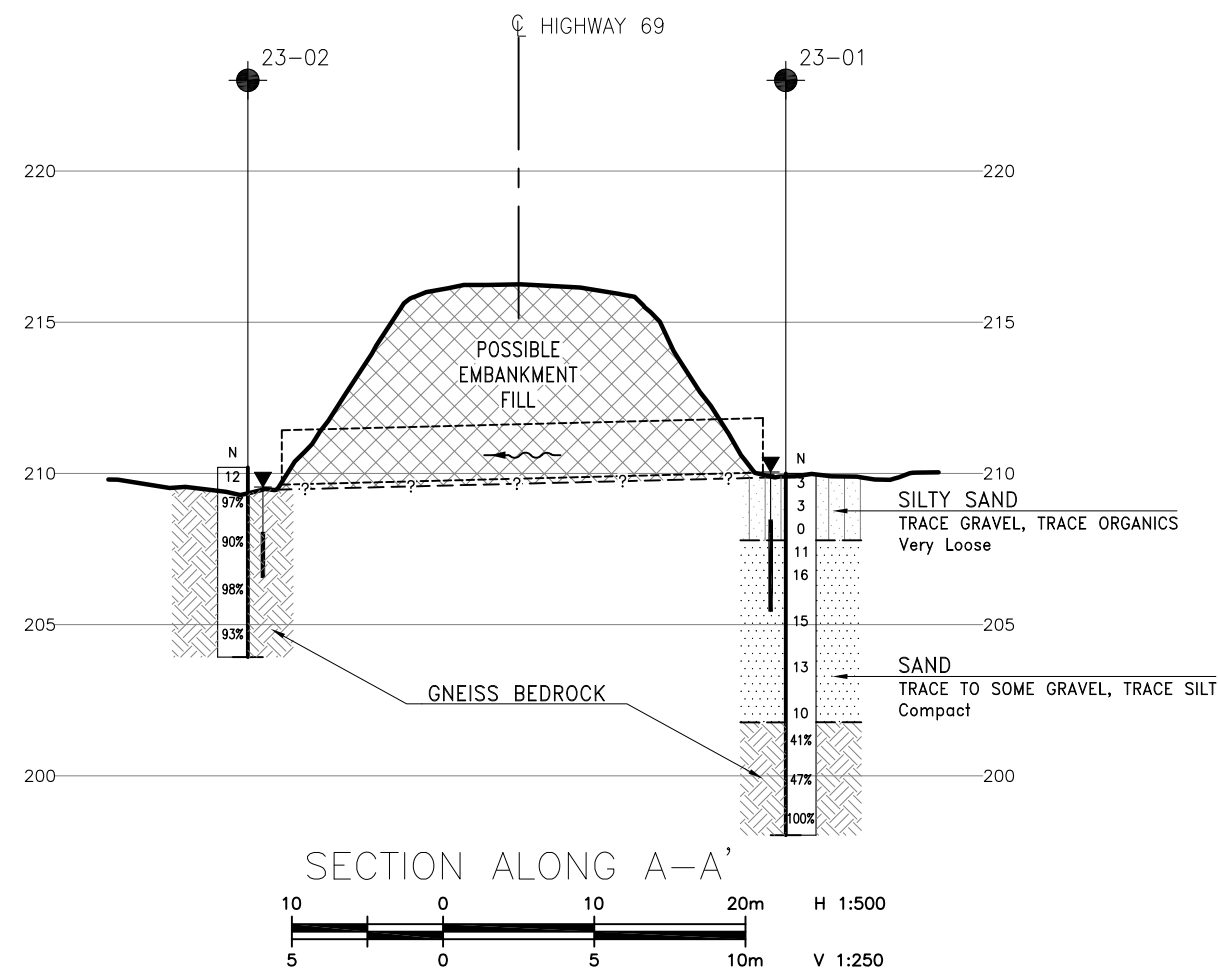
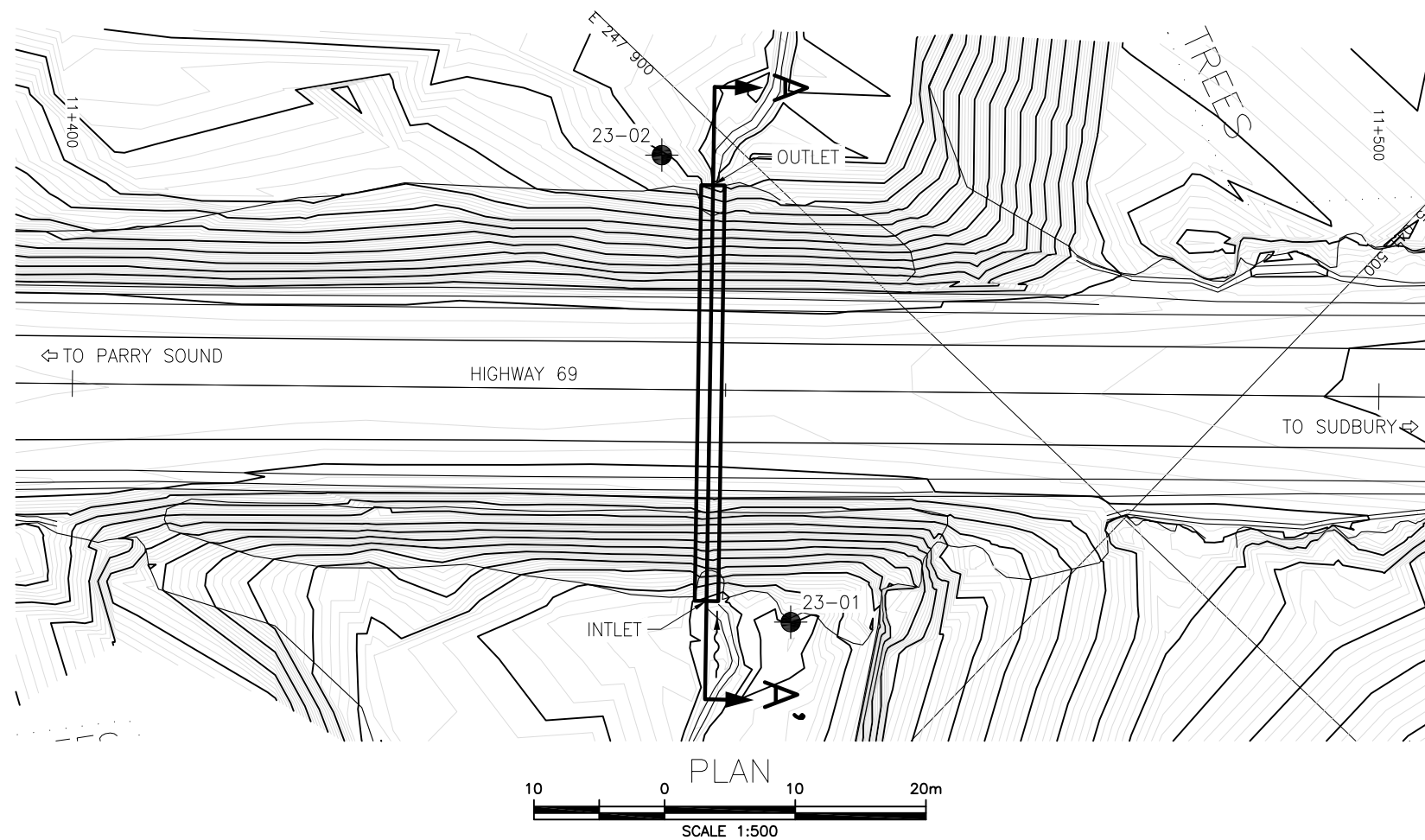
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APPENDIX A

Borehole Locations and Strata Drawing



METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES
UNLESS OTHERWISE SHOWN



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GWP No 5246-18-00)

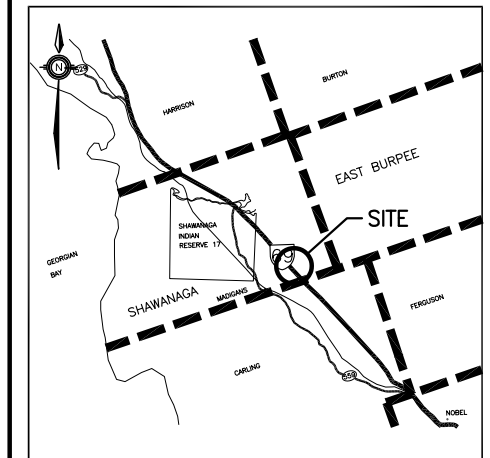


SHEET

HIGHWAY 69
SHAWANAGA RIVER BRIDGE
CULVERT 24
BOREHOLE LOCATIONS AND SOIL STRATA








THURBER ENGINEERING LTD.



KEYPLAN

LEGEND

	Borehole
	Borehole and Cone
N	Blows /0.3m (Std Pen Test, 475J/blow)
CONE	Blows /0.3m (60° Cone, 475J/blow)
PH	Pressure, Hydraulic
	Water Level Upon Completion of Drilling
	Water Level in Monitoring Well/Piezometer
	Monitoring Well/Piezometer Screen
90%	Rock Quality Designation (RQD)
A/R	Auger Refusal

NO	ELEVATION	NORTHING	EASTING
23-01	210.0	5 041 487.1	247 923.4
23-02	210.1	5 041 455.2	247 904.4

-NOTES-

- 1) The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.
- 2) This drawing is for subsurface information only. Surface details and features are for conceptual illustration.
- 3) Coordinate system is MTM NAD 83 Zone 10.

GEOCRES No. 41H09-001

REVISIONS								
	DATE	BY			DESCRIPTION			
DESIGN	RB		CHK	MTB		LOAD	DATE	JUN 2024
DRAWN	AN		CHK	RB	SITE	STRUCT	DWG	1

APPENDIX B

Record of Boreholes Sheet

SYMBOLS, ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES

1. TEXTURAL CLASSIFICATION OF SOILS

CLASSIFICATION	PARTICLE SIZE	VISUAL IDENTIFICATION
Boulders	Greater than 200mm	same
Cobbles	75 to 200mm	same
Gravel	4.75 to 75mm	5 to 75mm
Sand	0.075 to 4.75mm	Not visible particles to 5mm
Silt	0.002 to 0.075mm	Non-plastic particles, not visible to the naked eye
Clay	Less than 0.002mm	Plastic particles, not visible to the naked eye

2. COARSE GRAIN SOIL DESCRIPTION (50% greater than 0.075mm)

TERMINOLOGY	PROPORTION
Trace or Occasional	Less than 10%
Some	10 to 20%
Adjective (e.g. silty or sandy)	20 to 35%
And (e.g. sand and gravel)	35 to 50%

3. TERMS DESCRIBING CONSISTENCY (COHESIVE SOILS ONLY)

DESCRIPTIVE TERM	UNDRAINED SHEAR STRENGTH (kPa)	APPROXIMATE SPT ⁽¹⁾ 'N' VALUE
Very Soft	12 or less	Less than 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 200	15 to 30
Hard	Greater than 200	Greater than 30

NOTE: Hierarchy of Soil Strength Prediction

- 1) Laboratory Triaxial Testing
- 2) Field Insitu Vane Testing
- 3) Laboratory Vane Testing
- 4) SPT value
- 5) Pocket Penetrometer



4. TERMS DESCRIBING DENSITY (COHESIONLESS SOILS ONLY)

DESCRIPTIVE TERM	SPT "N" VALUE
Very Loose	Less than 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	Greater than 50

5. LEGEND FOR RECORDS OF BOREHOLES


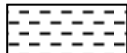



SYMBOLS AND ABBREVIATIONS FOR SAMPLE TYPE	SS Split Spoon Sample	WS Wash Sample	AS Auger (Grab) Sample
	TW Thin Wall Shelby Tube Sample	TP Thin Wall Piston Sample	
	PH Sampler Advanced by Hydraulic Pressure	PM Sampler Advanced by Manual Pressure	
	WH Sampler Advanced by Self Static Weight	RC Rock Core	SC Soil Core

$$\text{Sensitivity} = \frac{\text{Undisturbed Shear Strength}}{\text{Remoulded Shear Strength}}$$

 Water Level
 Shear Strength Determination by Pocket Penetrometer

- (1) SPT 'N' Value Standard Penetration Test 'N' Value – refers to the number of blows from a 63.5kg hammer free falling a height of 0.76m to advance a standard 50 mm outside diameter split spoon sampler for 0.3 m depth into undisturbed ground.
- (2) DCPT Dynamic Cone Penetration Test – Continuous penetration of a 50 mm outside diameter, 60° conical steel point attached to "A" size rods driven by a 63.5 kg hammer free falling a height of 0.76 m. The resistance to cone penetration is the number of hammer blows required for each 0.3 m advance of the conical point into undisturbed ground.

EXPLANATION OF ROCK LOGGING TERMS

<u>ROCK WEATHERING CLASSIFICATION</u>		<u>SYMBOLS</u>	
Fresh (FR)	No visible signs of weathering.		
Fresh Jointed (FJ)	Weathering limited to the surface of major discontinuities.		CLAYSTONE
Slightly Weathered (SW)	Penetrative weathering developed on open discontinuity surfaces, but only slight weathering of rock material.		SILTSTONE
Moderately Weathered (MW)	Weathering extends throughout the rock mass, but the rock material is not friable.		SANDSTONE
Highly Weathered (HW)	Weathering extends throughout the rock mass and the rock is partly friable.		COAL
Completely Weathered (CW)	Rock is wholly decomposed and in a friable condition, but the rock texture and structure are preserved.		Bedrock (general)

<u>DISCONTINUITY SPACING</u>		<u>STRENGTH CLASSIFICATION</u>			
Bedding	Bedding Plane Spacing	Rock Strength	Approximate Uniaxial Compressive Strength		Field Estimation of Hardness*
			(MPa)	(psi)	
Very thickly bedded	Greater than 2m	Extremely Strong	Greater than 250	Greater than 36,000	Specimen can only be chipped with a geological hammer
Thickly bedded	0.6 to 2m				
Medium bedded	0.2 to 0.6m	Very Strong	100-250	15,000 to 36,000	Requires many blows of geological hammer to break
Thinly bedded	60mm to 0.2m				
Very thinly bedded	20 to 60mm	Strong	50-100	7,500 to 15,000	Requires more than one blow of geological hammer to break
Laminated	6 to 20mm				
Thinly Laminated	Less than 6mm	Medium Strong	25.0 to 50.0	3,500 to 7,500	Breaks under single blow of geological hammer.
		Weak	5.0 to 25.0	750 to 3,500	Can be peeled by a pocket knife with difficulty
		Very Weak	1.0 to 5.0	150 to 750	Can be peeled by a pocket knife, crumbles under firm blows of geological pick.
		Extremely Weak (Rock)	0.25 to 1.0	35 to 150	Indented by thumbnail

<u>TERMS</u>	
Total Core Recovery: (TCR)	Core recovered as a percentage of total core run length.
Solid Core Recovery: (SCR)	Percent Ratio of solid core of full cylindrical shape recovered. Expressed with respect to the total length of core run.
Rock Quality Designation: (RQD)	Total length of sound core recovered in pieces 0.1m in length or larger as a percentage of total core run length.
Uniaxial Compressive Strength (UCS)	Axial stress required to break the specimen
Fracture Index: (FI)	Frequency of natural fractures per 0.3m of core run.

RECORD OF BOREHOLE No 23-01

1 OF 2

METRIC

GWP# 5246-18-00 LOCATION HWY 69 & Shawanaga River Bridge N 5 041 487.1 E 247 923.4 ORIGINATED BY JF
DIST Parry Sound HWY 69 BOREHOLE TYPE Hollow Stem Auger and HQ Coring COMPILED BY AS
DATUM Geodetic DATE 2023.08.30 - 2023.08.30 LATITUDE 45.510984 LONGITUDE -80.227927 CHECKED BY MB

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa						
210.0	GROUND SURFACE							20 40 60 80 100		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L		
0.0	TOPSOIL± (75mm)							20 40 60 80 100		WATER CONTENT (%)				
0.1	Silty, SAND , trace gravel, trace organics Very Loose Brown Wet		1	SS	3									
			2	SS	3		209							
	No recovery in SS3		3	SS	0		208							
207.8														
2.2	SAND , trace to some gravel, trace silt Compact Grey Moist		4	SS	11		207							6 89 5 (SI+CL)
			5	SS	16									
							206							
			6	SS	15		205							
			7	SS	13		204							
							203							
			8	SS	10		202							16 79 5 (SI+CL)
201.8														
8.2	BEDROCK (GNEISS) , moderately to highly weathered, weak to very strong, grey and pink		1	RUN			201							RUN #1 TCR=97% SCR=76% RQD=41% UCS=15.1MPa (PLT)
														RUN #2 TCR=87% SCR=62% RQD=47% UCS=90.5MPa UCS=120.3MPa (PLT)

Continued Next Page

+³, ×³: Numbers refer to Sensitivity
20
15
10
(%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 23-01

2 OF 2

METRIC

GWP# 5246-18-00 LOCATION HWY 69 & Shawanaga River Bridge N 5 041 487.1 E 247 923.4 ORIGINATED BY JF
DIST Parry Sound HWY 69 BOREHOLE TYPE Hollow Stem Auger and HQ Coring COMPILED BY AS
DATUM Geodetic DATE 2023.08.30 - 2023.08.30 LATITUDE 45.510984 LONGITUDE -80.227927 CHECKED BY MB

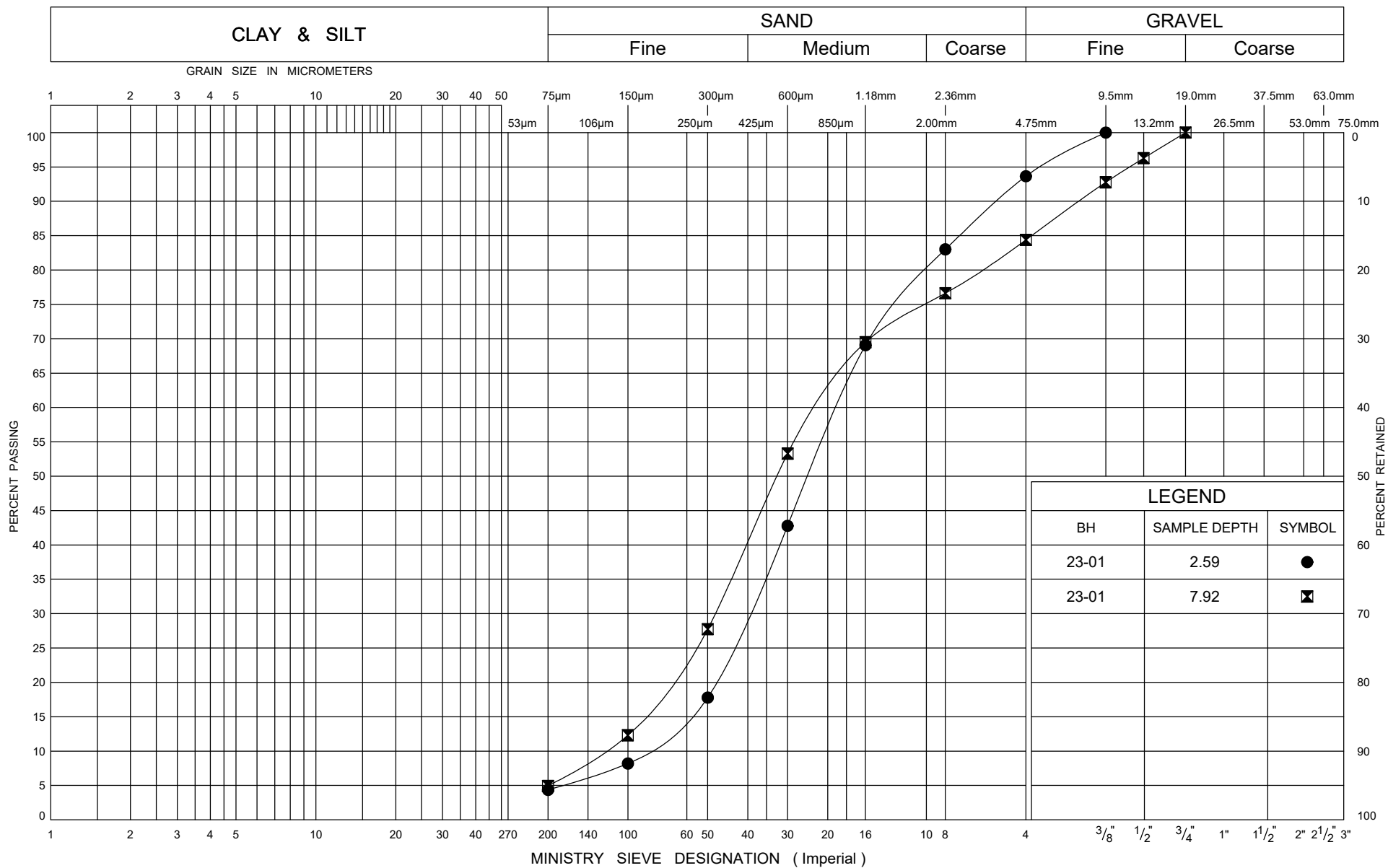
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa										
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE										
	Continued From Previous Page		2	RUN			199	20	40	60	80	100	20	40	60	5 0 0 1 2 1	RUN #3 TCR=100% SCR=100% RQD=100% UCS=115.0MPa (PLT)	
	Becoming slightly weathered to fresh, strong to very strong																	
198.0			3	RUN														
12.0	END OF BOREHOLE AT 12.0m. Well installation consists of 25mm diameter Schedule 40 PVC pipe with a 3.05m slotted screen. WATER LEVEL READINGS DATE DEPTH(m) ELEV.(m) 2023.09.21 0.0 210.0																	

METRIC

+³, ×³: Numbers refer to Sensitivity

APPENDIX C

Laboratory Test Results



**THURBER ENGINEERING LTD.****POINT LOAD TEST SHEET****ASTM D5731-08**

Job No: 30351
 Client: Egis
 Project Name: Highway 69
 Core Size: HQ BH No : 23-01

Date Drilled: 30-Aug-23
 Date Tested: 14-Sep-23
 Tester: AK
 Reviewed by: RB

Test No.	Run No.	Depth (m)	Axial or Diametral	Gauge (MPa)	Diameter (mm)	Length (mm)	$I_s(50)$ (MPa)	UCS (MPa)	Rock Type	Rock Strength (after Hoek & Brown, 1997)
1	1	8.9	D	2.6	60.9	91.3	0.7	17.6	Gneiss	Weak
2	1	9.1	D	1.9	60.9	96.6	0.5	12.5	Gneiss	Weak
3	2	9.7	D	17.4	60.9	103.5	4.9	116.6	Gneiss	Very Strong
4	2	10.1	D	18.5	60.9	96.6	5.2	124.0	Gneiss	Very Strong
5	3	11.5	D	16.1	60.9	85.4	4.5	107.8	Gneiss	Very Strong
6	3	11.7	D	18.2	60.9	107.4	5.1	122.1	Gneiss	Very Strong
7										
8										
9					Average Run 1			15.0		Weak
10					Average Run 2			120.3		Very Strong
11					Average Run 3			115.0		Very Strong
12										
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										
23										
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34										
35										

* It is ideal to perform axial test on core specimens with D/L ratio of 1.1 ± 0.1

* Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

* Diametral Test should have $0.7 \times D$ on either side of test point.

* Correlation factor to obtain UCS values is 24.



THURBER ENGINEERING LTD.

POINT LOAD TEST SHEET

ASTM D5731-08

Job No: 30351
 Client: Egis
 Project Name: Highway 69
 Core Size: HQ BH No : 23-02

Date Drilled: 10-Aug-23
 Date Tested: 06-Sep-23
 Tester: AK
 Reviewed by: RB

Test No.	Run No.	Depth (m)	Axial or Diametral	Gauge (MPa)	Diameter (mm)	Length (mm)	$I_s(50)$ (MPa)	UCS (MPa)	Rock Type	Rock Strength (after Hoek & Brown, 1997)
1	1	1.1	D	7.2	63.0	96.4	1.9	45.8	Gneiss	Medium Strong
2	1	1.2	D	20.2	63.0	121.4	5.3	128.2	Gneiss	Very Strong
3	2	1.6	D	10.0	63.0	122.9	2.7	63.7	Gneiss	Strong
4	2	2.5	D	19.4	63.0	115.3	5.2	123.7	Gneiss	Very Strong
5	3	3.3	D	20.6	63.0	101.1	5.5	130.9	Gneiss	Very Strong
6	3	3.9	D	20.7	63.0	89.3	5.5	131.7	Gneiss	Very Strong
7										
8										
9					Average Run 1			87.0		Strong
10					Average Run 2			93.7		Strong
11					Average Run 3			131.3		Very Strong
12										
13										
14										
15										
16										
17										
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35										

* It is ideal to perform axial test on core specimens with D/L ratio of 1.1 ± 0.1

* Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

* Diametral Test should have $0.7 \times D$ on either side of test point.

* Correlation factor to obtain UCS values is 24.

November 9, 2023

Madisan Chiarotto
Thurber Engineering Ltd.
103, 2010 Winston Park Drive
Oakville ON
L6H 5R7

Re: UCS testing (Thurber Project No. 30551)

Dear Madisan:

On October 20th, 2023, fourteen (14) rock core samples (HQ and NQ sized) were received by Geomechanica Inc. via drop-off by Thurber personnel. These samples were identified as being from Thurber project 30551 (Highway 69 and Shawanaga River Bridge Rehabilitation). From these samples, fourteen (14) UCS tests were completed.

Details regarding the steps of specimen preparation and testing are presented in the accompanying laboratory report and summary spreadsheet.

Sincerely,



Bryan Tatone Ph.D., P. Eng.

Geomechanica Inc.
Tel: (647) 478-9767
Email: bryan.tatone@geomechanica.com

Rock Laboratory Testing Results

A report submitted to:

Madisan Chiarotto
Thurber Engineering Ltd.
103, 2010 Winston Park Drive
Oakville, Ontario
Canada L6H 5R7

Prepared by:

Bryan Tatone, PhD, PEng
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Geomechanica Inc.
#14-1240 Speers Rd.
Oakville ON
L6L 2X4 Canada
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lab@geomechanica.com

November 9, 2023
Project number: 30351

Abstract

This document summarizes the results of laboratory testing, including 14 Uniaxial Compressive Strength (UCS) tests. The UCS values along with photographs of specimens before and after testing are presented herein.

In this document:

1 Uniaxial Compressive Strength Tests	1
Appendices	3

1 Uniaxial Compressive Strength Tests

1.1 Overview

This section summarizes the results of uniaxial compressive strength (UCS) testing. The testing was performed in Geomechanica's rock testing laboratory using a 150 ton (1.3 MN) Forney loading frame equipped with pressure-compensated control valve to maintain an axial displacement rate of approximately 0.05 mm/min (Figure 1). The preparation and testing procedure for each specimen included the following:

1. Unwrapping the core sample and inspecting it for damage.
2. Diamond cutting the core sample to obtain a cylindrical specimen with an appropriate length (length:diameter = 2:1) and nearly parallel end faces.
3. Diamond grinding the specimen to obtain flat (within ± 0.025 mm) and parallel end faces (within 0.25°).
4. Placing the specimen into the loading frame and axially loading the specimen to rupture while continuously recording axial force and axial deformation to determine the peak strength (UCS).



Figure 1: Forney loading frame setup for UCS testing.

Using a precision V-block mounted on the magnetic chuck of the surface grinder, test specimens met the end flatness, end parallelism, and perpendicularity criteria set out in ASTM D4543-19. The side straightness criteria, as checked with a feeler gauge, and the minimum length:diameter criteria were met for all specimens unless noted otherwise in Table 1. Testing of the specimens followed ASTM D7012-14 Method C.

1.2 Results

The results of UCS testing are summarized in Table 1. Additional specimens and testing details are included in the summary spreadsheet that accompanies this report.

Table 1: Summary of Uniaxial Compression test results.

Sample	Depth (m)	Bulk density ρ (g/cm ³)	UCS (MPa)	Lithology	Failure description
BH23-01	32'10" - 33'10"	2.769	90.5	Granite gneiss	1, 2
BH23-02	2'3" - 3'2"	2.770	117.4	Granite gneiss	1
BH23-03	3'10" - 4'8"	2.792	118.4	Granite gneiss	3
BH23-04	8'0" - 8'10"	2.753	101.2	Granite gneiss	1
BH23-05	21'0" - 22'11"	2.646	117.3	Pegmatite	4
BH23-07	13'5" - 14'4"	2.746	90.2	Granite gneiss	3
BH23-09	7'10" - 8'5"	2.753	113.8	Granite gneiss	3, 5
BH23-10	27'6" - 28'5"	2.661	129.2	Granite gneiss	3
BH23-12	21'5" - 22'4"	2.775	86.7	Granite gneiss	1
BH23-13	6'6" - 7'2"	2.674	129.4	Granite gneiss	6
BH23-15	17'2" - 17'10"	2.416	98.6	Granite gneiss	3, 5
BH23-16	7'3" - 7'11"	2.648	147.0	Granite gneiss	3, 2
BH23-17	12'6" - 13'5"	2.751	75.9	Granite gneiss	1, 5
BH23-18	28'7" - 29'7"	2.707	68.2	Granite gneiss	3, 5

¹ Inclined shear failure

² Partial hourglass failure

³ Inclined shear fracture and axial splitting failure

⁴ Axial splitting failure

⁵ Failure partly along pre-existing structure

⁶ Hourglass failure

1.3 Specimen photographs

Photographs of the specimens before and after testing are presented in the Appendix of this report.



Appendices

Specimen sheets



- BH23-01
- BH23-02

- BH23-03
- BH23-04
- BH23-05
- BH23-07
- BH23-09
- BH23-10
- BH23-12
- BH23-13
- BH23-15
- BH23-16
- BH23-17
- BH23-18



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-01	Depth	32' 10" - 33' 10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	60.80		
Length (mm) ^a	129.90		
Bulk density ρ (g/cm ³)	2.769		
UCS (MPa)	90.5		
Lithology	Granite gneiss		
Failure description ^b	1, 2		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ¹ Inclined shear failure; ² Partial hourglass failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-02	Depth	2'3" - 3'2"
Specimen parameters		Prior to testing	After testing
Diameter (mm) ^a	62.92		
Length (mm) ^a	129.18		
Bulk density ρ (g/cm ³)	2.770		
UCS (MPa)	117.4		
Lithology	Granite gneiss		
Failure description ^b	1		
^a Additional specimen measurement/details provided in accompanying summary spreadsheet.			
^b Failure description: ¹ Inclined shear failure;			
Remarks: Loading Rate: 0.05mm/min. Specimen experienced pre-peak localized failure(s).			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-03	Depth	3' 10" - 4' 8"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	60.75		
Length (mm) ^a	129.54		
Bulk density ρ (g/cm ³)	2.792		
UCS (MPa)	118.4		
Lithology	Granite gneiss		
Failure description ^b	3		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-04	Depth	8'0" - 8'10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	61.85		
Length (mm) ^a	129.19		
Bulk density ρ (g/cm ³)	2.753		
UCS (MPa)	101.2		
Lithology	Granite gneiss		
Failure description ^b	1		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ¹ Inclined shear failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-05	Depth	21'0" - 22'11"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	47.52		
Length (mm) ^a	102.95		
Bulk density ρ (g/cm ³)	2.646		
UCS (MPa)	117.3		
Lithology	Pegmatite		
Failure description ^b	4		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ⁴ Axial splitting failure;</div>			
Remarks: Loading Rate: 0.05mm/min. Specimen experienced pre-peak localized failure(s).			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-07	Depth	13'5" - 14'4"
<div>Specimen parameters</div> <div><div>Diameter (mm)^a</div><div>Length (mm)^a</div><div>Bulk density ρ (g/cm³)</div><div>UCS (MPa)</div><div>Lithology</div><div>Failure description^b</div><div></div></div>		<div>Prior to testing</div> <div></div>	<div>After testing</div> <div></div>
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-09	Depth	7' 10" - 8' 5"
Specimen parameters		Prior to testing	After testing
Diameter (mm) ^a	48.54		
Length (mm) ^a	103.59		
Bulk density ρ (g/cm ³)	2.753		
UCS (MPa)	113.8		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
^a Additional specimen measurement/details provided in accompanying summary spreadsheet.			
^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-10	Depth	27'6" - 28'5"
<div>Specimen parameters</div> <div><div>Diameter (mm)^a</div><div>Length (mm)^a</div><div>Bulk density ρ (g/cm³)</div><div>UCS (MPa)</div><div>Lithology</div><div>Failure description^b</div></div>		<div>Prior to testing</div> <div></div>	<div>After testing</div> <div></div>
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-12	Depth	21'5" - 22'4"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	46.99		
Length (mm) ^a	103.61		
Bulk density ρ (g/cm³)	2.775		
UCS (MPa)	86.7		
Lithology	Granite gneiss		
Failure description ^b	1		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ¹ Inclined shear failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-13	Depth	6'6" - 7'2"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	48.89		
Length (mm) ^a	103.89		
Bulk density ρ (g/cm ³)	2.674		
UCS (MPa)	129.4		
Lithology	Granite gneiss		
Failure description ^b	6		
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ⁶ Hourglass failure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

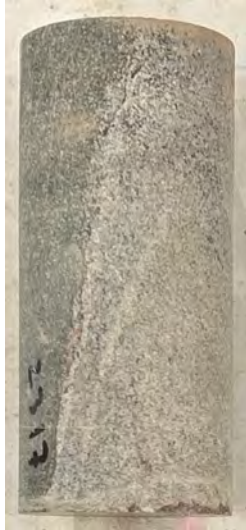

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-15	Depth	17'2" - 17'10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	49.61		
Length (mm) ^a	103.77		
Bulk density ρ (g/cm ³)	2.416		
UCS (MPa)	98.6		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-16	Depth	7'3" - 7'11"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	47.11		
Length (mm) ^a	103.57		
Bulk density ρ (g/cm ³)	2.648		
UCS (MPa)	147.0		
Lithology	Granite gneiss		
Failure description ^b	3, 2		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ³ Inclined shear fracture and axial splitting failure; ² Partial hourglass failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-17	Depth	12'6" - 13'5"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	46.83		
Length (mm) ^a	102.54		
Bulk density ρ (g/cm ³)	2.751		
UCS (MPa)	75.9		
Lithology	Granite gneiss		
Failure description ^b	1, 5		
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ¹ Inclined shear failure; ⁵ Failure partly along pre-existing structure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-18	Depth	28'7" - 29'7"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	62.81		
Length (mm) ^a	129.38		
Bulk density ρ (g/cm ³)	2.707		
UCS (MPa)	68.2		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



FINAL REPORT

CA40126-AUG23 R

30351, Parry Sound (North)

Prepared for

Thurber Engineering Ltd.



FINAL REPORT

CA40126-AUG23 R

First Page

CLIENT DETAILS		LABORATORY DETAILS	
Client	Thurber Engineering Ltd.	Project Specialist	Jill Campbell, B.Sc.,GISAS
Address	103, 2010 Winston Park Drive	Laboratory	SGS Canada Inc.
	Oakville, ON	Address	185 Concession St., Lakefield ON, K0L 2H0
	L6H 5R7, Canada		
Contact	Madisan Chiarotto	Telephone	2165
Telephone	647-548-8390	Facsimile	705-652-6365
Facsimile		Email	jill.campbell@sgs.com
Email	mchiarotto@thurber.ca	SGS Reference	CA40126-AUG23
Project	30351, Parry Sound (North)	Received	08/11/2023
Order Number		Approved	08/17/2023
Samples	Solution (7)	Report Number	CA40126-AUG23 R
		Date Reported	08/17/2023

COMMENTS
Temperature of Sample upon Receipt: 9 degrees C
Cooling Agent Present: Yes
Custody Seal Present: Yes
Chain of Custody Number: 036865


SIGNATORIES
Jill Campbell, B.Sc.,GISAS




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FINAL REPORT

CA40126-AUG23 R

Client: Thurber Engineering Ltd.

Project: 30351, Parry Sound (North)

Project Manager: Madisan Chiarotto

Samplers: Jakob Flood

MATRIX: WATER

Sample Number	6	7	8	9	10	11	12
Sample Name	Culvert 24 STA 11+449	Culvert 25 STA 11+680	Culvert 35 STA 14+631	Culvert 38 STA 15+250	Culvert 43 STA 16+534	Culvert 44 STA 16+764	Culvert 47 STA 17+808
Sample Matrix	Solution	Solution	Solution	Solution	Solution	Solution	Solution
Sample Date	09/08/2023	09/08/2023	09/08/2023	11/08/2023	11/08/2023	11/08/2023	08/08/2023

Parameter	Units	RL	Result	Result	Result	Result	Result	Result	Result
General Chemistry									
Conductivity	uS/cm	2	657	171	76	1120	80	66	1180
Redox Potential	mV	no	143	152	117	127	136	240	152
Sulphide	µg/L	6	< 6	< 6	16	7.0	23	41	< 6
Metals and Inorganics									
Sulphate	mg/L	0.04	12	8.1	3.0	28	0.74	5.8	16
Other (ORP)									
pH	No unit	0.05	5.80	6.28	5.68	6.32	5.98	5.77	6.41
Chloride	mg/L	0.04	210	38	17	380	14	10	380



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

Anions by IC
Method: EPA300/MA300-Ions1.3 | Internal ref.: ME-CA-IENVIIC-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Chloride	DIO0332-AUG23	mg/L	0.04	<0.04	6	20	102	90	110	108	75	125
Sulphate	DIO0332-AUG23	mg/L	0.04	<0.04	ND	20	97	90	110	92	75	125
Sulphate	DIO0393-AUG23	mg/L	0.04	<0.04	3	20	97	90	110	91	75	125

Conductivity
Method: SM 2510 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Conductivity	EWL0270-AUG23	uS/cm	2	< 2	0	20	100	90	110	NA		



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

pH
Method: SM 4500 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
pH	EWL0265-AUG23	No unit	0.05	NA	0		100			NA		
pH	EWL0277-AUG23	No unit	0.05	NA	0		100			NA		

Redox Potential
Method: SM 2580 |

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Redox Potential	EWL0243-AUG23	mV	no	NA	2	20	104	80	120	NA		

Sulphide by SFA
Method: SM 4500 | Internal ref.: ME-CA-IENVISFA-LAK-AN-008

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Sulphide	SKA0140-AUG23	ug/L	6	<0.006	ND	20	110	80	120	NA	75	125



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

Method Blank: a blank matrix that is carried through the entire analytical procedure. Used to assess laboratory contamination.

Duplicate: Paired analysis of a separate portion of the same sample that is carried through the entire analytical procedure. Used to evaluate measurement precision.

LCS/Spike Blank: Laboratory control sample or spike blank refer to a blank matrix to which a known amount of analyte has been added. Used to evaluate analyte recovery and laboratory accuracy without sample matrix effects.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate laboratory accuracy with sample matrix effects.

Reference Material: a material or substance matrix matched to the samples that contains a known amount of the analyte of interest. A reference material may be used in place of a matrix spike.

RL: Reporting limit

RPD: Relative percent difference

AC: Acceptance criteria

Multielement Scan Qualifier: as the number of analytes in a scan increases, so does the chance of a limit exceedance by random chance as opposed to a real method problem. Thus, in multielement scans, for the LCS and matrix spike, up to 10% of the analytes may exceed the quoted limits by up to 10% absolute and the spike is considered acceptable.

Duplicate Qualifier: for duplicates as the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Matrix Spike Qualifier: for matrix spikes, as the concentration of the native analyte increases, the uncertainty of the matrix spike recovery increases. Thus, the matrix spike acceptance limits apply only when the concentration of the matrix spike is greater than or equal to the concentration of the native analyte.



LEGEND

FOOTNOTES

NSS Insufficient sample for analysis.

RL Reporting Limit.

↑ Reporting limit raised.

↓ Reporting limit lowered.

NA The sample was not analysed for this analyte

ND Non Detect

Results relate only to the sample tested.

Data reported represent the sample as submitted to SGS. Solid samples expressed on a dry weight basis.

"Temperature Upon Receipt" is representative of the whole shipment and may not reflect the temperature of individual samples.

Analysis conducted on samples submitted pursuant to or as part of Reg. 153/04, are in accordance to the "Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act and Excess Soil Quality" published by the Ministry and dated March 9, 2004 as amended.

SGS provides criteria information (such as regulatory or guideline limits and summary of limit exceedances) as a service. Every attempt is made to ensure the criteria information in this report is accurate and current, however, it is not guaranteed. Comparison to the most current criteria is the responsibility of the client and SGS assumes no responsibility for the accuracy of the criteria levels indicated.

SGS Canada Inc. statement of conformity decision rule does not consider uncertainty when analytical results are compared to a specified standard or regulation.

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This report supersedes all previous versions.

-- End of Analytical Report --

REPORT INFORMATION				INVOICE INFORMATION				ANALYSIS REQUESTED												P.O. #:	
Company: THUNDER ENGINEERING LTD				Received By (signature): <u>[Signature]</u>				Project #: 30351												Site Location/ID: PARSY SOUND(NORTH)	
Contact: MADISON CHARTERED MATHEMATICS				Custody Seal Present: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>				TURNAROUND TIME (TAT) REQUIRED												TAT's are quoted in business days (exclude statutory holidays & weekends). Samples received after 9pm or on weekends: TAT begins next business day	
Address: 2010 WINSTON PARK DR				Custody Seal Intact: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>				RUSH TAT (Additional Charges May Apply): <input type="checkbox"/> 1 Day <input type="checkbox"/> 2 Days <input type="checkbox"/> 3 Days <input type="checkbox"/> 4 Days												PLEASE CONFIRM RUSH FEASIBILITY WITH SGS REPRESENTATIVE PRIOR TO SUBMISSION	
Phone: 905-829-8666				Temperature Upon Receipt (°C): 90x3				Specify Due Date:												NOTE: DRINKING (POTABLE) WATER SAMPLES FOR HUMAN CONSUMPTION MUST BE SUBMITTED WITH SGS DRINKING WATER CHAIN OF CUSTODY	
Fax: 905-829-8666				Email: ACCOUNTING@THUNDER.CA				M & I												Sewer Use:	
Email: MCHARTERED@THUNDER.CA				Other Regulations:				M & I												Sewer Use:	
REGULATIONS				Other Regulations:				M & I												Sewer Use:	
REGULATIONS				Other Regulations:				M & I												Sewer Use:	
O.Reg 153/04 <input type="checkbox"/> O.Reg 406/19 <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
Table 1 <input type="checkbox"/> Res/Park <input type="checkbox"/> Soil Texture: <input type="checkbox"/> Reg 347/558 (3 Day min TAT)				Other Regulations:				M & I												Sewer Use:	
Table 2 <input type="checkbox"/> Ind/Com <input type="checkbox"/> Coarse <input type="checkbox"/> PWQO <input type="checkbox"/> MMER <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
Table 3 <input type="checkbox"/> Agri/Other <input type="checkbox"/> Medium/Fine <input type="checkbox"/> CCME <input type="checkbox"/> Other: <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
Table <input type="checkbox"/> Appx. <input type="checkbox"/> MISA <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
Soil Volume <input type="checkbox"/> <350m3 <input type="checkbox"/> >350m3 <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
RECORD OF SITE CONDITION (RSC) <input type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/>				Other Regulations:				M & I												Sewer Use:	
SAMPLE IDENTIFICATION				DATE SAMPLED				TIME SAMPLED				# OF BOTTLES				MATRIX					
1 CULVERT 24 STA 11+449				08/09/23				17:00				3				WATER					
2 CULVERT 25 STA 11+680				08/09/23				16:45				3				WATER					
3 CULVERT 35 STA 14+631				08/09/23				9:30				3				WATER					
4 CULVERT 38 STA 15+250				08/11/23				9:10				3				WATER					
5 CULVERT 43 STA 16+534				08/11/23				9:05				3				WATER					
6 CULVERT 44 STA 16+764				08/11/23				9:00				3				WATER					
7 CULVERT 47 STA 17+808				08/08/23				10:00				3				WATER					
8																					
9																					
10																					
11																					
12																					
Observations/Comments/Special Instructions																					
Sampled By (NAME): JAYOB FLOOD																					
Relinquished by (NAME): JAYOB FLOOD																					
Signature: <u>[Signature]</u>																					
Signature: <u>[Signature]</u>																					
Date: 08/11/23 (mm/dd/yy)																					
Date: 08/11/23 (mm/dd/yy)																					
Pink Copy - Client																					
Yellow & White Copy - SGS																					



FINAL REPORT

CA40212-AUG23 R1

30351

Prepared for

Thurber Engineering Ltd.



FINAL REPORT

CA40212-AUG23 R1

First Page

CLIENT DETAILS		LABORATORY DETAILS	
Client	Thurber Engineering Ltd.	Project Specialist	Jill Campbell, B.Sc.,GISAS
Address	103, 2010 Winston Park Drive	Laboratory	SGS Canada Inc.
	Oakville, ON	Address	185 Concession St., Lakefield ON, K0L 2H0
	L6H 5R7, Canada		
Contact	Madisan Chiarotto	Telephone	2165
Telephone	647-548-8390	Facsimile	705-652-6365
Facsimile		Email	jill.campbell@sgs.com
Email	mchiarotto@thurber.ca	SGS Reference	CA40212-AUG23
Project	30351	Received	08/22/2023
Order Number		Approved	08/29/2023
Samples	Soil (6)	Report Number	CA40212-AUG23 R1
		Date Reported	08/29/2023

COMMENTS
Temperature of Sample upon Receipt: 8 degrees C
Cooling Agent Present: Yes
Custody Seal Present: Yes
Chain of Custody Number: 036623
Corrosivity Index is based on the American Water Works Corrosivity Scale according to AWWA C-105. An index greater than 10 indicates the soil matrix may be corrosive to cast iron alloys.


SIGNATORIES
Jill Campbell, B.Sc.,GISAS




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FINAL REPORT

CA40212-AUG23 R1

Client: Thurber Engineering Ltd.

Project: 30351

Project Manager: Madisan Chiarotto

Samplers: Madisan Chiarotto

MATRIX: SOIL

Sample Number	5	6	7	8	9	10
Sample Name	23-02 SS#1	23-04 SS#2	23-05 SS#5B	23-12 SS#7	23-16 SS#2B	23-17 SS#3
Sample Matrix	CORR	CORR	CORR	CORR	CORR	CORR
Sample Date	Soil	Soil	Soil	Soil	Soil	Soil
Sample Date	10/08/2023	09/08/2023	26/07/2023	02/08/2023	02/08/2023	04/08/2023

Parameter	Units	RL	Result	Result	Result	Result	Result	Result
-----------	-------	----	--------	--------	--------	--------	--------	--------

Corrosivity Index

Corrosivity Index	none	1	1	1	3	1	1	1
Soil Redox Potential	mV	no	287	304	295	249	453	368
Sulphide (Na ₂ CO ₃)	%	0.04	< 0.04	< 0.04	< 0.04	< 0.04	< 0.04	< 0.04
pH	pH Units	0.05	7.12	6.09	6.47	6.54	5.90	7.02
Resistivity (calculated)	ohms.cm	-9999	3750	3890	2260	5920	26300	22200

General Chemistry

Conductivity	uS/cm	2	267	257	443	169	38	45
--------------	-------	---	-----	-----	-----	-----	----	----

Metals and Inorganics

Moisture Content	%	0.1	13.7	18.4	15.4	19.8	11.6	22.4
Sulphate	µg/g	0.4	17	16	15	10	7.9	9.0

Other (ORP)

Chloride	µg/g	0.4	130	170	170	77	7.1	25
----------	------	-----	-----	-----	-----	----	-----	----



FINAL REPORT

CA40212-AUG23 R1

QC SUMMARY

Anions by IC
Method: EPA300/MA300-Ions1.3 | Internal ref.: ME-CA-IENVIIC-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Chloride	DIO0693-AUG23	µg/g	0.4	<0.4	3	35	101	80	120	100	75	125
Sulphate	DIO0693-AUG23	µg/g	0.4	<0.4	22	35	99	80	120	96	75	125

Carbon/Sulphur
Method: ASTM E1915-07A | Internal ref.: ME-CA-IENVIARD-LAK-AN-020

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Sulphide (Na2CO3)	ECS0120-AUG23	%	0.04	< 0.04	5	20	106	80	120			

Conductivity
Method: SM 2510 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Conductivity	EWL0460-AUG23	uS/cm	2	< 2	0	20	100	90	110	NA		



FINAL REPORT

CA40212-AUG23 R1

QC SUMMARY

pH
Method: SM 4500 | Internal ref.: ME-CA-|ENVIEWL-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
pH	EWL0460-AUG23	pH Units	0.05	NA	0		100			NA		

Method Blank: a blank matrix that is carried through the entire analytical procedure. Used to assess laboratory contamination.

Duplicate: Paired analysis of a separate portion of the same sample that is carried through the entire analytical procedure. Used to evaluate measurement precision.

LCS/Spike Blank: Laboratory control sample or spike blank refer to a blank matrix to which a known amount of analyte has been added. Used to evaluate analyte recovery and laboratory accuracy without sample matrix effects.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate laboratory accuracy with sample matrix effects.

Reference Material: a material or substance matrix matched to the samples that contains a known amount of the analyte of interest. A reference material may be used in place of a matrix spike.

RL: Reporting limit

RPD: Relative percent difference

AC: Acceptance criteria

Multielement Scan Qualifier: as the number of analytes in a scan increases, so does the chance of a limit exceedance by random chance as opposed to a real method problem. Thus, in multielement scans, for the LCS and matrix spike, up to 10% of the analytes may exceed the quoted limits by up to 10% absolute and the spike is considered acceptable.

Duplicate Qualifier: for duplicates as the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Matrix Spike Qualifier: for matrix spikes, as the concentration of the native analyte increases, the uncertainty of the matrix spike recovery increases. Thus, the matrix spike acceptance limits apply only when the concentration of the matrix spike is greater than or equal to the concentration of the native analyte.



LEGEND

FOOTNOTES

NSS Insufficient sample for analysis.

RL Reporting Limit.

↑ Reporting limit raised.

↓ Reporting limit lowered.

NA The sample was not analysed for this analyte

ND Non Detect

Results relate only to the sample tested.

Data reported represent the sample as submitted to SGS. Solid samples expressed on a dry weight basis.

"Temperature Upon Receipt" is representative of the whole shipment and may not reflect the temperature of individual samples.

Analysis conducted on samples submitted pursuant to or as part of Reg. 153/04, are in accordance to the "Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act and Excess Soil Quality" published by the Ministry and dated March 9, 2004 as amended.

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This report supersedes all previous versions.

-- End of Analytical Report --

Request for Laboratory Services and CHAIN OF CUSTODY



Industries & Environment - Lakefield: 185 Concession St., Lakefield, ON K0L 2H0 Phone: 705-652-2000 Fax: 705-652-6365 Web: www.sgs.com/environement
- London: 657 Concession Court, London, ON, N6E 2S8 Phone: 519-672-4500 Toll Free: 877-848-8060 Fax: 519-672-0361

Laboratory Information Section - Lab use only

Received By: BO
Received Date: 8 Dec 08 (mm/dd/yy)
Received Time: 11:00 (hr : min)

Received By (signature): [Signature]
Custody Seal Present: Yes ☒ No ☐
Custody Seal Intact: Yes ☒ No ☐
Cooling Agent Present: Yes ☒ No ☐
Temperature Upon Receipt (°C): 8.3

Quotation #: 30351 P.O. #: 30351
Project #: 30351 Site Location/ID: 30351

TURNAROUND TIME (TAT) REQUIRED
Regular TAT (5-7 days) ☒
RUSH TAT (Additional Charges May Apply): ☐ 1 Day ☐ 2 Days ☐ 3 Days ☐ 4 Days
PLEASE CONFIRM RUSH FEASIBILITY WITH SGS REPRESENTATIVE PRIOR TO SUBMISSION

Specify Due Date: 08/12/23
*NOTE: DRINKING (POTABLE) WATER SAMPLES FOR HUMAN CONSUMPTION MUST BE SUBMITTED WITH SGS DRINKING WATER CHAIN OF CUSTODY

INVOICE INFORMATION
Company: Thurber Engineering Ltd
Contact: Madison Chenev
Address: 2010 Winston Dr. Br.
Unit 103, Oakville, ON, L6H 5R7
Phone: (905) 829-8666
Fax: -
Email: mchenev@thurber.ca
mboucher@thurber.ca

REPORT INFORMATION
Company: Thurber Engineering Ltd
Contact: Madison Chenev
Address: 2010 Winston Dr. Br.
Unit 103, Oakville, ON, L6H 5R7
Phone: (905) 829-8666
Fax: -
Email: mchenev@thurber.ca
mboucher@thurber.ca

REGULATIONS
O.Reg 153/04 ☐ O.Reg 406/19 ☐
Table 1 ☐ Res/Park ☐ Soil Texture: ☐ Coarse ☐ Fine
Table 2 ☐ Ind/Com ☐ Agri/Other ☐ Medium/Fine
Table 3 ☐ Appx. ☐ >350m3
Soil Volume ☐ <350m3 ☐ >350m3

Other Regulations:
☐ Reg 347/558 (3 Day min TAT)
☐ PWQO ☐ MMER ☐ Other:
☐ CCME ☐ MISA ☐ ODWS Not Reportable *See note

Sewer By-Law:
☐ Sanitary ☐ Storm
☐ Municipality:

RECORD OF SITE CONDITION (RSC) ☐ YES ☒ NO

SAMPLE IDENTIFICATION	DATE SAMPLED	TIME SAMPLED	# OF BOTTLES	MATRIX
1 23-02 55#1 CORR	08/10/23		1	Soil
2 23-04 55#2 CORR	08/10/23		1	Soil
3 23-05 55#3 CORR	07/26/23		1	Soil
4 23-12 55#7 CORR	08/10/23		1	Soil
5 23-16 55#8 CORR	08/10/23		1	Soil
6 23-17 55#3 CORR	08/10/23		1	Soil
7				
8				
9				
10				
11				
12				

ANALYSIS REQUESTED

M & I	SVOC	PCB	PHC	VOC	Pest	Other (please specify)	SPLP	TCLP
Field Filtered (Y/N)	PAHs only	PCBs	F1-F4 + BTEX	BTEX only	Pesticides	Water Characterization Pkg	Specify tests	Specify tests
Metals & Inorganics (Cd, Cr, Cu, Hg, Pb, Se, V, Zn, As, Ba, Be, B, Ca, Co, Ni, Mn, Mo, Ni, Sb, Ag, Tl, U, V, Zn)	SVOCs all incl PAHs, ABNs, CPs	Total <input type="checkbox"/> Aroclor <input type="checkbox"/>	F1-F4 only	all incl BTEX	Organochlorine or specify other	General <input type="checkbox"/> Extended <input type="checkbox"/>	Metals <input type="checkbox"/> VOC <input type="checkbox"/> PCB <input type="checkbox"/> BTEX <input type="checkbox"/> ABN <input type="checkbox"/> Light <input type="checkbox"/>	Metals <input type="checkbox"/> VOC <input type="checkbox"/> PCB <input type="checkbox"/> BTEX <input type="checkbox"/> ABN <input type="checkbox"/> Light <input type="checkbox"/>

COMMENTS:

Observations/Comments/Special Instructions

Sampled By (NAME): Madison Chenev Signature: [Signature] Date: 08/12/23 (mm/dd/yy) Pink Copy - Client

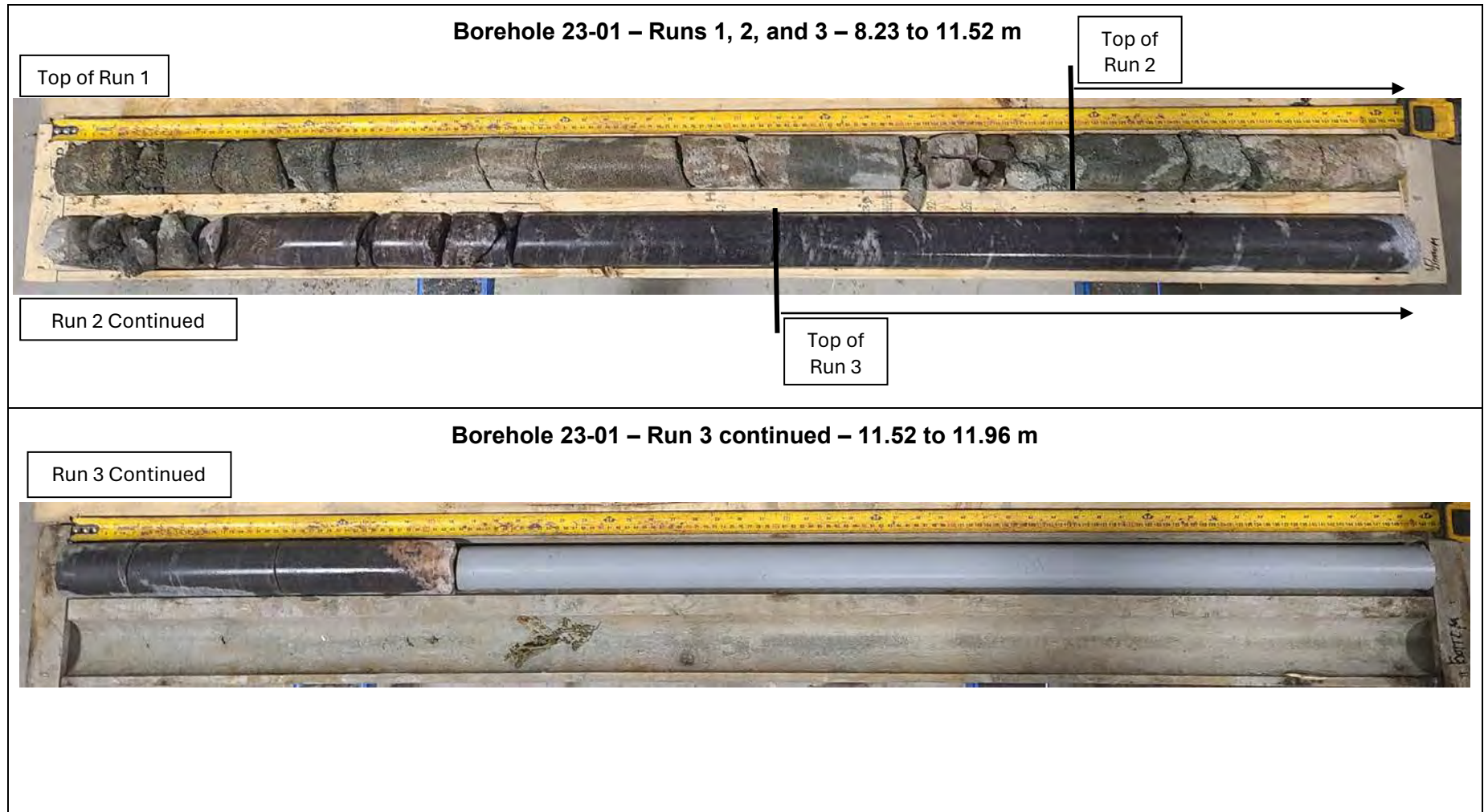
Relinquished by (NAME): Madison Chenev Signature: [Signature] Date: 08/12/23 (mm/dd/yy) Yellow & White Copy - SGS

Note: Submission of samples to SGS is acknowledgement that you have been provided direction on sample collection/handling and transportation of samples. (2) Submission of samples to SGS is considered authorization for completion of work. Signatures may appear on this form or be retained on file in the contract, or in an alternative format (e.g. shipping documents). (3) Results may be sent by email to an unlimited number of addresses for no additional cost. Fax is available upon request. This document is issued by the Company under its General Conditions of Service accessible at http://www.sgs.com/terms_and_conditions.htm. (Printed copies are available upon request.) Attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein.

APPENDIX D

Bedrock Core Photographs

HIGHWAY 69 AND REHABILITATION OF SHAWANAGA RIVER BRIDGE
Photographs of Rock Core



HIGHWAY 69 AND REHABILITATION OF SHAWANAGA RIVER BRIDGE
Photographs of Rock Core

Borehole 23-02 – Runs 1 and 2 – 0.69 to 3.28 m



Run 2 Continued

Borehole 23-02 – Runs 3 and 4 – 3.28 to 6.27 m



Top of Run 4

APPENDIX E

Site Photographs



Photograph 1: Drilling Borehole 23-01 Looking Southeast



Photograph 2: Borehole 23-01 after piezometer installation, looking southwest towards Culvert Inlet



Photograph 3: Culvert Inlet with Piezometer at Borehole 23-01



Photograph 4: Looking north towards the Borehole 23-02 site



Photograph 5: Borehole 23-02 after well installation, looking north towards Culvert outlet