



THURBER ENGINEERING LTD.

FINAL FOUNDATION INVESTIGATION AND DESIGN REPORT

Culvert 25, Highway 69 - 5.5 km South of Highway 7182

Township of Shawanaga

Rehabilitation of Highway 69 and Shawanaga River Bridge

G.W.P. 5246-18-00

ASSIGNMENT NO.: 5020-E-0003

GEOCRES NO.: 41H09-002

Location: Lat: 45.512357°, Long: -80.230144°

Client Name: Egis Group

Date: June 4, 2024

File: 30351

TABLE OF CONTENTS

PART 1. FACTUAL INFORMATION

1.	INTRODUCTION.....	1
2.	SITE DESCRIPTION.....	1
2.1	General.....	1
2.2	Site Geology.....	2
3.	INVESTIGATION PROCEDURE	2
4.	LABORATORY TESTING	4
5.	DESCRIPTION OF SUBSURFACE CONDITIONS.....	5
5.1	Topsoil.....	5
5.2	Silt to Sand and Silt	5
5.3	Silty Clay	6
5.4	Bedrock	6
5.5	Groundwater Level	7
6.	CORROSIVITY AND SULPHATE TEST RESULTS	8
7.	MISCELLANEOUS.....	9

PART 2. ENGINEERING DISCUSSION AND RECOMMENDATIONS

8.	GENERAL	11
9.	PROPOSED CULVERT REHABILITATION.....	12
9.1	Culvert Lining	12
10.	COFFERDAM DESIGN.....	12
11.	CORROSION AND SULPHATE ATTACK POTENTIAL	13
12.	CONSTRUCTION CONCERNS.....	13
13.	CLOSURE.....	14

STATEMENT OF LIMITATIONS AND CONDITIONS

IN-TEXT TABLES

Table 3-1 Borehole Summary	3
Table 3-2: Borehole Completion Details	4
Table 5-1: Grain Size Distribution on Silt to Sand and Silt.....	6
Table 5-2: Grain Size Distribution on Silty Clay	6
Table 5-3: Depths and Elevations of Top of Bedrock	7
Table 5-4: Bedrock Details.....	7
Table 5-5: Groundwater Measurements	8
Table 6-1: Analytical Corrosivity Test Results	8

APPENDICES

APPENDIX A

Borehole Locations and Strata Drawing

APPENDIX B

Record of Boreholes Sheet

APPENDIX C

Laboratory Test Results

APPENDIX D

Bedrock Core Photos

APPENDIX E

Site Photographs

**FOUNDATION INVESTIGATION AND DESIGN REPORT
CULVERT 25, HIGHWAY 69 – 5.5 KM SOUTH OF HIGHWAY 7182
TOWNSHIP OF SHAWANAGA, ONTARIO
G.W.P. 5246-18-00
ASSIGNMENT NO.: 5020-E-0003**

GEOCRES NO.: 41H09-002

PART 1. FACTUAL INFORMATION

1. INTRODUCTION

This section of the report presents the factual findings obtained from a foundation investigation conducted by Thurber Engineering Ltd. (Thurber) for the rehabilitation of Culvert 25 which crosses Highway 69 approximately 5.5 km south of Highway 7182 (Shebeshekong Road) in the Township of Shawanaga, Ontario. Thurber carried out the assignment as a sub-consultant to Egis Group (Egis) under Assignment No. 5020-E-0003.

The purpose of the investigation was to explore the subsurface conditions at the site and based on this data obtained, provide a borehole location plan, record of boreholes, stratigraphic profile, laboratory test results and a written description of the subsurface conditions. A stratigraphic profile of the subsurface conditions was developed during the current investigation.

It is a condition of this report that Thurber's performance of its professional services is subject to the attached Statement of Limitations and Conditions.

2. SITE DESCRIPTION

2.1 General

The deep fill embankment culvert (CL-25) is located on Highway 69, approximately 5.5 km south of the intersection of Highway 69 and Shebeshekong Road in the Township of Shawanaga, Ontario. For project orientation and reporting purposes, Highway 69 is herein described as orientated north-south and the culvert is described as oriented east-west. The existing culvert allows surface water to flow in an east to west direction under the highway.

At the location of the culvert, Highway 69 is a two-lane, undivided highway with a posted speed limit of 90 km/hr. The culvert crosses under the north and southbound lanes.

The base plan provided by Egis indicates that the existing culvert is a 1200 mm diameter corrugated steel pipe (CSP). Field observations confirmed that the culvert is a CSP. The plan indicates the length of the culvert is approximately 28 m. The culvert invert is at approximate Elev. 213.2 m at the inlet (east end) and at approximate Elev. 211.8 m at the outlet (west end). Cover above the invert of the culvert ranges from approximately 3.5 m under the northbound lanes to 4.5 m under the southbound lanes. The existing highway embankment at the culvert site is up to approximately 6 m high and has side slopes inclined at approximately 1.4V:1V at the inlet and outlet. Rock fill was observed on the embankment slopes around the culvert inlet and outlet.

The lands surrounding the site are generally low-lying and flat with bedrock outcrops noted nearby. The lands on the west and east side of the highway near the culvert are also low lying with some wet swampy terrain immediately surrounding the inlet / outlet. Photographs in Appendix E show the general nature of the site and the existing culvert.

2.2 Site Geology

Based on surficial geology mapping prepared by the Ontario Geological Survey¹ the culvert is located in an area mapped as Precambrian bedrock. Bedrock mapping² also prepared by the Ontario Geological Survey maps the local bedrock as migmatitic, tonalitic to granodioritic gneiss.

3. INVESTIGATION PROCEDURE

The foundation investigation and field-testing program for CL-25 was carried out in conjunction with several other culvert investigations reported under separate covers. The boreholes for this culvert investigation were advanced on August 9 to 10, 2023 and on August 29, 2023. The investigation consisted of two (2) boreholes, designated as 23-03 and 23-04, advanced to depths of 4.3 m and 5.2 m below ground surface (Elev. 209.5 m and 207.3 m), respectively. Boreholes 23-03 and 23-04 were drilled near the culvert inlet and outlet, respectively.

The Record of Borehole sheets are included in Appendix B. The approximate borehole locations are shown on the attached Borehole Locations and Soil Strata Drawing in Appendix A.

A summary of the borehole coordinates, elevations, and termination depths is provided in Table 3-1. The as-drilled borehole elevations and coordinates were provided by Callon Dietz, Egis' survey subconsultant. The survey was completed in the horizontal datum MTM Zone 10 CSRS

¹ Ontario Geological Survey 2010. *Surficial geology of Southern Ontario*; Ontario Geological Survey, Miscellaneous Release – Data 128_REV.

² Culshaw, N.G., Corrigan, D., Ketchum, J.W.F., Wallace, P. and Wodicka, N. 2004. *Precambrian geology, Naiscoot area*; Ontario Geological Survey, Preliminary Map P.3549, scale 1:50 000.

CBNv6-2010.0 and vertical datum CGVD 1928:1978 with a vertical and horizontal accuracy of +/- 5 cm. The borehole coordinates and elevations are shown on the Borehole Location and Soil Strata drawing included in Appendix A and on the individual Record of Borehole sheets included in Appendix B.

Table 3-1 Borehole Summary

Borehole	Northing (m)	Easting (m)	Ground Surface Elevation (m)	Termination Depth Below Ground Surface (m)
23-03	5,041,646.4	247,769.3	213.8	4.3
23-04	5,041,629.1	247,742.3	212.5	5.2

Utility clearances were obtained prior to the start of drilling.

Boreholes 23-03 and 23-04 were drilled at the ends of the culvert using a track mounted CME-55 drill rig and Diedrich D-50 drill rig, respectively. Borehole 23-03 was advanced through the overburden using hollow stem augers. Borehole 23-04 was advanced through the overburden using wash boring methodologies. Soil samples were obtained at selected intervals using a split spoon sampler in conjunction with Standard Penetration Testing (SPT). HQ coring methods were used to advance the two boreholes into bedrock.

The drilling and sampling operations were supervised on a full-time basis by a member of Thurber's technical staff. The supervisor logged the boreholes and processed the recovered soil and rock core samples for transport to Thurber's laboratory for further examination and testing.

The rock cores were logged, and the Total Core Recovery (TCR), Solid Core Recovery (SCR), Rock Quality Designation (RQD) and Fracture Index (FI) were determined.

Due to water being introduced during drilling and coring operations groundwater conditions were not observed in the open boreholes. A standpipe piezometer consisting of a 25 mm diameter PVC pipe with a 1.5 m long slotted screen, enclosed in a column of filter sand was installed in Borehole 23-03 to permit groundwater level monitoring. A monitoring well consisting of 50 mm diameter Schedule 40 PVC pipe with a 1.5 m long slotted screen, enclosed in a column of filter sand was installed in Borehole 23-04. The monitoring well was installed as per Egis' request to allow for well testing to be carried by Egis to support a potential Permit to Take Water application or registration on the Environmental Activity and Sector Registry. Well installation details, groundwater level observations and water level readings are shown on the Record of Borehole sheets in Appendix B. A surface water sample at the culvert inlet was obtained during the field investigation and submitted to a specialist analytical laboratory under chain of custody procedures for testing for a corrosivity related parameters.

Details of the drilling program, including drilling depths, monitoring well/ piezometer installation and completion details are summarized in Table 3-2 below.

Table 3-2: Borehole Completion Details

Borehole Number	Top of Borehole Elevation (m)	Borehole Depth / Base Elevation (m)	Monitoring Well/ Piezometer Depth / Elevation (m)	Completion Details
23-03	213.8	4.3 / 209.5	3.0 / 210.8	Borehole was backfilled with bentonite holeplug from 4.3 m to 3.4 m, filter sand from 3.4 m to 1.0 m, bentonite holeplug from 1.0 m to ground surface.
23-04	212.5	5.2 / 207.3	4.6 / 207.9	Borehole was backfilled with bentonite holeplug from 5.2 m to 4.8 m, filter sand from 4.8 m to 2.4 m, peltonite holplug from 2.4 m to 1.5 m, bentonite holeplug from 1.5 m to ground surface

The standpipe piezometer was decommissioned in general accordance with O.Reg. 903 upon collection of the final water level reading. The monitoring well was left in place and will be decommissioned as part of the construction contract under Egis' direction.

4. LABORATORY TESTING

Laboratory testing was selected in general accordance with the current MTO Guideline for Foundation Engineering Services, Section 5. Geotechnical laboratory testing consisted of natural moisture content determination and visual identification of all retained soil samples and grain size distribution analysis on selected soil samples. The rock cores were photographed, and the total core recovery (TCR), solid core recovery (SCR), and rock quality designation (RQD) were measured. Unconfined compressive strength (UCS) and point load testing was carried out on select intact bedrock cores to assess the unconfined compressive strength (UCS) of the bedrock. The results of this testing program are summarized on the Record of Borehole sheets in Appendix B and are shown on the figures included in Appendix C.

In order to assess the potential for sulphate attack on buried concrete structures, as well as the potential for corrosion associated with buried steel elements of the structures, a sample of the native soil from the boreholes, as well as a surface water sample from the upstream end of the culvert were collected during the investigation. The samples were submitted to SGS, a CALA

accredited analytical laboratory in Mississauga, Ontario, for analytical testing of corrosivity parameters and sulphate content. The results of the analytical testing are summarized in this report and presented in Appendix C.

5. DESCRIPTION OF SUBSURFACE CONDITIONS

Details of the encountered soil stratigraphy are presented on the Record of Borehole sheets included in Appendix B and on the Borehole Locations and Soil Strata drawing in Appendix A. A general description of the stratigraphy, based on the conditions encountered in the boreholes, is given in the following paragraphs. However, the factual data presented in the Record of Borehole sheets governs any interpretation of the site conditions. It must be recognized that soil conditions may vary between and beyond the borehole locations.

In general, the boreholes drilled at the ends of the culvert encountered a layer of organic soil overlying native soils consisting of both non-plastic silts and sands and plastic silty clay overlying bedrock.

5.1 Topsoil

75 mm and 50 mm of topsoil were encountered at the ground surface in Boreholes 23-03 and 23-04, respectively. Moisture contents of 56% and 120% were measured in the topsoil samples from Boreholes 23-03 and 23-04, respectively. The topsoil thickness may vary between and beyond the borehole locations.

5.2 Silt to Sand and Silt

A layer of silt to sand and silt was encountered in both boreholes, below the topsoil layer. This material was encountered at a depth of 0.1 m in both boreholes (Elev. 212.4 m and 213.7 m). The layer was 1.0 m and 0.6 m thick with underside depths of 1.1 m and 0.7 m (base Elev. 212.7 m and 211.8 m), in Boreholes 23-03 and 23-04, respectively. This material was described as brown in colour and also contained trace clay, trace gravel, and trace organics and rootlets.

SPT N-values of 2 to 66 blows per 0.3 m of penetration were recorded in the silt to sand and silt, indicating a very loose to very dense relative density, typically very loose. The higher blow count was likely due to the sampler contacting the top of bedrock.

Moisture contents measured in this material ranged from 15% to 31%. The results of grain size distribution analysis carried out on a sample of this material are provided on Figure C1 in Appendix C. The results are also summarized in Table 5-1 and on the Record of Borehole sheets in Appendix B.

Table 5-1: Grain Size Distribution on Silt to Sand and Silt

Soil Particle	Percentage (%)
Gravel	0
Sand	56
Silt	36
Clay	8

5.3 Silty Clay

A layer of silty clay was encountered below the silt in Borehole 23-04. The silty clay was described to contain some sand to sandy and was brown to grey in colour. The thickness of the silt clay layer was 0.9 m with an underside depth of 1.6 m (base Elev. 210.9 m).

SPT N-values of 21 blows per 0.3 m of penetration to 50 blows per approximately 0.1 m of penetration were recorded in the silty clay, indicating a very stiff to hard consistency.

A moisture content of 16% was recorded in the silty clay. The results of gradation analysis completed on a selected sample of the silty clay are illustrated on Figure C2 of Appendix C. The results of the tests are also summarized in the table below and on the Record of Borehole sheet in Appendix B.

Table 5-2: Grain Size Distribution on Silty Clay

Soil Particle	Percentage (%)
Gravel	0
Sand	38
Silt	42
Clay	20

Atterberg Limits testing was not carried out on the silty clay as insufficient sample was available to complete the laboratory test.

5.4 Bedrock

The soils described above are underlain by gneiss bedrock. Bedrock was proven by coring in both boreholes. The bedrock encountered consisted of slightly weathered to fresh, grey and pink,

medium strong to very strong gneissic rock. Photographs of the bedrock core are provided in Appendix D.

Table 5-3 summarizes the depths and elevations of the top of bedrock at the borehole locations.

Table 5-3: Depths and Elevations of Top of Bedrock

Borehole	Top of Bedrock	
	Depth (m)	Elevation (m)
23-03	1.1	212.7
23-04	1.6	210.9

The rock core recovery measurements, rock quality designation and rock core laboratory testing results are summarized in Table 5-4 below.

Table 5-4: Bedrock Details

Parameter	Range
Total Core Recovery (TCR), %	100
Solid Core Recovery (SCR), %	18 – 100
Rock Quality Designation (RQD), %	0 – 100
Fracture Index (fractures per 0.3 m)	0 – 5
Unconfined Compressive Strength from UCS Tests (MPa)	101 – 118
Unconfined Compressive Strength Estimated from Point Load Tests (MPa)	40 – 157

Based on the RQD, the bedrock quality is described as very poor to excellent, typically good (CFEM 5th Edition, 2023). The results of UCS and point load testing indicate that the tested samples of the bedrock are medium strong to very strong, typically very strong (CFEM 5th Edition, 2023). The results of the UCS and point load testing are included in Appendix C.

5.5 Groundwater Level

Upon completion of the boreholes water levels were not observed as water was introduced into the borehole for bedrock coring and the levels may not be representative of insitu conditions. The measured groundwater levels observed in piezometer / monitoring well installations are summarized in the table below.

Table 5-5: Groundwater Measurements

Borehole	Date	Water Level (m)		Remark
		Depth	Elevation	
23-03	September 21, 2023	0.4	213.4	In piezometer
23-04	August 11, 2023	0.6	211.9	In monitoring well
	September 21, 2023	0.8	211.7	
	October 4, 2023	0.8	211.7	

It should be noted that the above values are considered to be short-term readings and they may not reflect the groundwater level at the time of construction. Seasonal fluctuations of the groundwater level are to be expected. In particular, the groundwater level may be at a higher elevation after periods of significant and/or prolonged precipitation events.

At the time of the investigation, no water was observed flowing through the culvert. Small pools/puddles of stagnant water were noted near each end of the culvert and the ground was generally wet.

6. CORROSIVITY AND SULPHATE TEST RESULTS

A sample of native silty clay from Borehole 23-04 was submitted for analytical testing of corrosivity parameters and sulphate. A sample of surface water taken from the surface water at the inlet of the culvert was also submitted for analytical testing of pH, sulphate, chloride, resistivity, and conductivity. The laboratory certificates of analysis for the current investigation are presented in Appendix C. The results of the analytical tests are summarized below in Table 6-1.

Table 6-1: Analytical Corrosivity Test Results

Parameter	Units (Soil)	Units (Water)	Test Results	
			23-04 SS#2 CORR (0.8m – 1.4m)	Culvert Inlet Culvert 25 STA 11+680
			(Silty Clay)	(Surface Water)
Redox Potential	mV	mV	304	152
Sulphide	%	µg/L	<0.04	<6
pH	-	-	6.09	6.28
Chloride	µg/g	mg/L	170	38
Sulphate	µg/g	mg/L	16	8.1
Conductivity	uS/cm	uS/cm	257	171
Resistivity	Ohms.cm	-	3890	---

7. MISCELLANEOUS

Thurber obtained utility clearances for the borehole locations prior to drilling. Borehole locations were selected and established in the field by Thurber.

Walker Drilling Limited (Walker) of Utopia, Ontario supplied the drill rig and conducted the drilling, sampling and in-situ testing operations for Borehole 23-03. George Downing Estate Drilling (Downing) of Hawkesbury, Ontario supplied the drill rig and conducted the drilling, sampling and in-situ testing operations for Borehole 23-04. Traffic control services were provided by Thurber for Borehole 23-03. Traffic control services were provided by Beacon Lite of Ottawa, Ontario for Borehole 23-04.

All geotechnical laboratory testing of soil samples and point load testing of bedrock core samples were carried out in Thurber's geotechnical laboratory. Uniaxial compressive strength tests were carried out by Geomechanica Inc. Analytical testing of soil and water samples was carried out by SGS Canada Inc.

The field investigation was supervised on a full-time basis by Mr. Jakob Flood of Thurber. The overall supervision of the field program was conducted by Ms. Madisan Chiarotto, P.Eng. and Mr. Matthew Boucher, P.Eng. of Thurber.

Interpretation of the field data and preparation of this report was carried out by Ms. Madisan Chiarotto, P.Eng. and Mr. Matthew Boucher, P.Eng. The report was reviewed by Mr. Jason Lee, P.Eng., a Designated Principal Contact for MTO Foundations Projects.

Thurber Engineering Ltd.

Report Prepared By:



Madisan Chiarotto, P.Eng.
Geotechnical Engineer



Matthew Boucher, P.Eng.
Senior Associate / Senior Geotechnical Engineer



Jason Lee, P.Eng.
Designated MTO Principal Contact /
Senior Geotechnical Engineer

**FOUNDATION INVESTIGATION AND DESIGN REPORT
CULVERT 25, HIGHWAY 69 – 5.5 KM SOUTH OF HIGHWAY 7182
TOWNSHIP OF SHAWANAGA, ONTARIO
G.W.P. 5246-18-00
ASSIGNMENT NO.: 5020-E-0003**

GEOCRES NO.: 41H09-002

PART 2. ENGINEERING DISCUSSION AND RECOMMENDATIONS

8. GENERAL

This section of the report provides an interpretation of the factual data from Part 1 of the report and presents foundation design recommendations for the proposed rehabilitation of Culvert 25 which crosses Highway 69 approximately 5.5 km south of Highway 7182 (Shebeshekong Road) in the Township of Shawanaga, Ontario. The culvert crosses the highway at Station 11+680 of Highway 69.

This foundation investigation and design report with the interpretation and recommendations are intended for the use of Egis and the Ministry of Transportation and shall not be used or relied upon for any other purposes or by any other parties including the construction or design-build contractor. The Contractors must make their own interpretation based on the factual data in Part 1 of the report. Where comments are made on construction, they are provided only to highlight those aspects which could affect the design of the project. Contractors must make their own interpretation of the information provided in Part 1 of this report as it may affect equipment selection, proposed construction methods and scheduling.

The base plan provided by Egis indicates that the existing culvert is a 1200 mm diameter corrugated steel pipe (CSP). Field observations confirmed that the culvert is a CSP. The plan indicates the length of the culvert is 27.9 m. The culvert invert is at approximate Elev. 213.2 m at the inlet (east end) and at approximate Elev. 211.8 m at the outlet (west end). Cover above the obvert of the culvert ranges from approximately 3.5 m under the northbound lanes to 4.5 m under the southbound lanes. The existing highway embankment at the culvert site is up to approximately 6 m high and has side slopes inclined at approximately 1.4V:1V at the inlet and outlet. Rock fill was observed on the embankment slopes around the culvert inlet and outlet.

Based on preliminary discussions with Egis, it is understood that the culvert will be rehabilitated

by lining the inside of the existing culvert using trenchless techniques. It is understood that no changes to the existing Highway 69 embankment and no excavations are planned during the rehabilitation work. No wingwalls / headwalls are present at the existing culvert or planned as part of the culvert design.

It is a condition of this report that Thurber's performance of its professional services is subject to the attached Statement of Limitations and Conditions.

9. PROPOSED CULVERT REHABILITATION

9.1 Culvert Lining

We understand that the existing culvert is expected to be rehabilitated by adding a lining to the inside of the existing culvert using trenchless techniques. From a foundations perspective, this method is suitable and poses very little risk to the highway pavement or to the stability of the embankment. However, we recommend that the existing culvert be inspected and surveyed before rehabilitation work begins to ensure it has not deformed or sagged and is able to accommodate the lining.

10. COFFERDAM DESIGN

Construction of cofferdams may be required to divert surface water flow and to facilitate rehabilitation of the culvert in the dry.

The boreholes advanced indicate there is 1.1 m and to 1.6 m of soil overlying bedrock at the culvert inlet and outlet, respectively. This soil consists of very loose to compact sand to silt with trace organics near the ground surface in both boreholes and a very stiff silty clay below the sand to silt in Borehole 23-04. It is important to note that the ground conditions may vary beyond the borehole locations, and it is expected that the top of bedrock elevation will vary.

Typical options for cofferdams include interlocking sheet piles or sandbags. Installation of sheet piles is not feasible in the ground conditions encountered in the boreholes near the outlet and inlet of the culvert due to shallow bedrock conditions. The use of a sandbag cofferdam is recommended from a foundation's perspective at both the inlet and outlet of the culvert.

It is recommended that the work be carried out when the conditions around the culvert are relatively dry as they were at the time of the investigation. This may eliminate the need for the cofferdams or reduce the amount of surface water that needs to be diverted.

The Contractor should be aware that regardless of the cofferdam type selected, pumping from sumps installed within the cofferdams will be required to maintain a dry working area. The Contractor should plan to have the appropriate equipment including properly sized pumps available on site.

The design and selection of the cofferdam system is the responsibility of the Contractor. The Contractor should consider the ground conditions in selecting the type of cofferdam for this site.

11. CORROSION AND SULPHATE ATTACK POTENTIAL

Analytical tests on soil and surface water were completed to determine the potential for degradation of concrete in the presence of soluble sulphates and the potential for corrosion of exposed steel used in buried infrastructure. The results of the analytical tests are summarized in Section 6.

The water-soluble sulphate concentration measured in the soil sample and the sulphate concentration in the water provide an indication of the degree of sulphate attack that is expected for concrete in contact with soil and water. The water-soluble sulphate concentration measured in the soil sample and the sulphate concentration in the water sample indicate that a negligible degree of sulphate attack is expected for concrete in contact with soil and water at this site.

The potential for soil corrosion on buried steel or other metal objects is considered to be mild to moderate based on the testing completed on a surface water sample and sample of silty clay, respectively. The corrosive effects of road de-icing salts should also be considered.

12. CONSTRUCTION CONCERNS

Potential construction concerns include, but are not necessarily limited to:

- The bedrock surface elevation varied in the boreholes drilled as part of the investigation. The bedrock surface may fluctuate above and below that shown on the drawing in Appendix A

The successful performance of the project will depend largely upon good workmanship and quality control during construction.

13. CLOSURE

Interpretation of the preparation of this report was carried out by Ms. Madisan Chiarotto, P.Eng. and Mr. Matthew Boucher, P.Eng. The report was reviewed by Mr. Jason Lee, P.Eng., a Designated Principal Contact for MTO Foundations Projects.

Thurber Engineering Ltd.

Report Prepared By:



Madisan Chiarotto, P.Eng.
Geotechnical Engineer



Matthew Boucher, P.Eng.
Senior Associate / Senior Geotechnical Engineer,



Jason Lee, P.Eng.
Designated MTO Principal Contact /
Senior Geotechnical Engineer

STATEMENT OF LIMITATIONS AND CONDITIONS

1. STANDARD OF CARE

This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

2. COMPLETE REPORT

All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

IN ORDER TO PROPERLY UNDERSTAND THE SUGGESTIONS, RECOMMENDATIONS AND OPINIONS EXPRESSED HEREIN, REFERENCE MUST BE MADE TO THE WHOLE OF THE REPORT. THURBER IS NOT RESPONSIBLE FOR USE BY ANY PARTY OF PORTIONS OF THE REPORT WITHOUT REFERENCE TO THE WHOLE REPORT.

3. BASIS OF REPORT

The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

4. USE OF THE REPORT

The information and opinions expressed in the Report, or any document forming part of the Report, are for the sole benefit of the Client. NO OTHER PARTY MAY USE OR RELY UPON THE REPORT OR ANY PORTION THEREOF WITHOUT THURBER'S WRITTEN CONSENT AND SUCH USE SHALL BE ON SUCH TERMS AND CONDITIONS AS THURBER MAY EXPRESSLY APPROVE. Ownership in and copyright for the contents of the Report belong to Thurber. Any use which a third party makes of the Report, is the sole responsibility of such third party. Thurber accepts no responsibility whatsoever for damages suffered by any third party resulting from use of the Report without Thurber's express written permission.

5. INTERPRETATION OF THE REPORT

- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

6. RELEASE OF POLLUTANTS OR HAZARDOUS SUBSTANCES

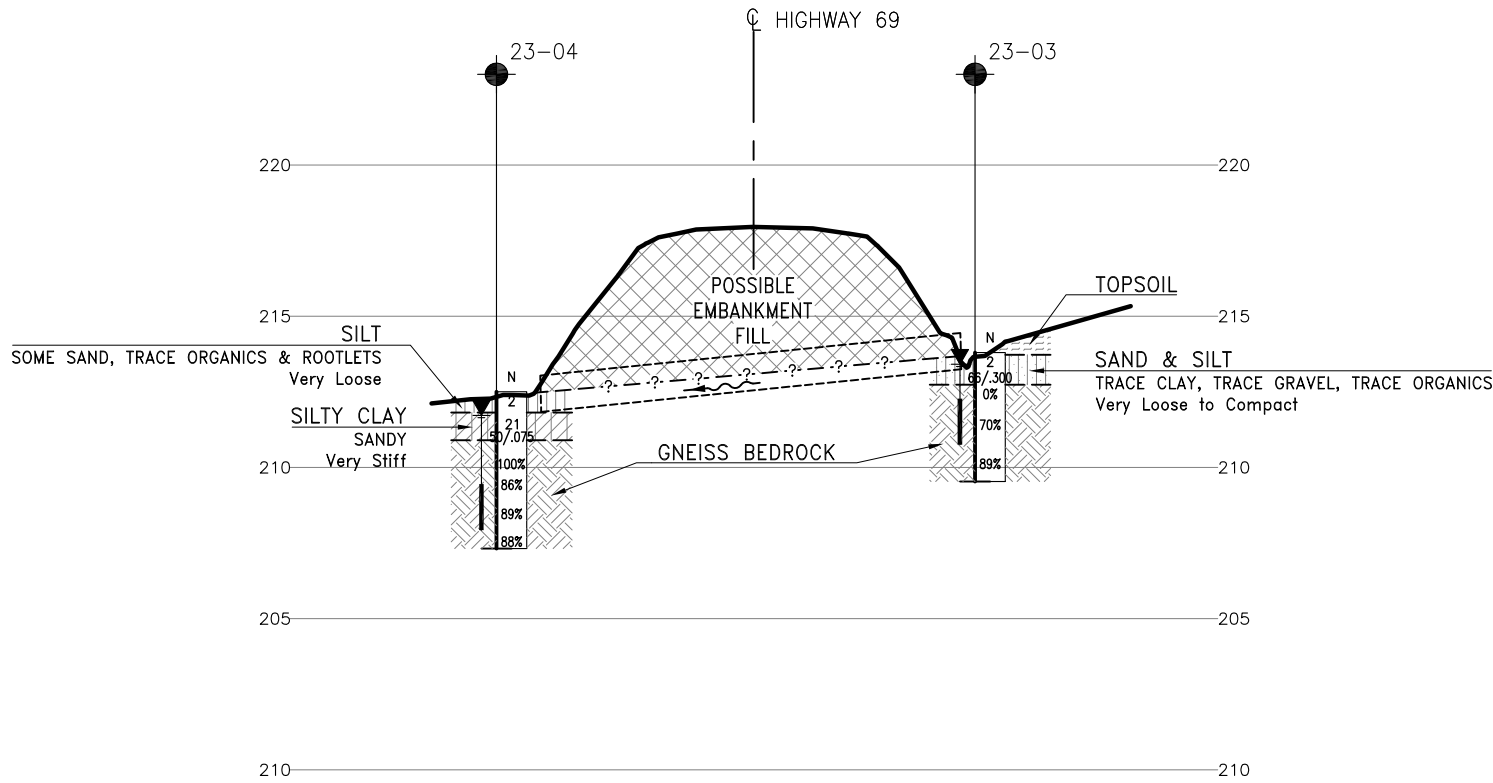
Geotechnical engineering and environmental consulting projects often have the potential to encounter pollutants or hazardous substances and the potential to cause the escape, release or dispersal of those substances. Thurber shall have no liability to the Client under any circumstances, for the escape, release or dispersal of pollutants or hazardous substances, unless such pollutants or hazardous substances have been specifically and accurately identified to Thurber by the Client prior to the commencement of Thurber's professional services.

7. INDEPENDENT JUDGEMENTS OF CLIENT

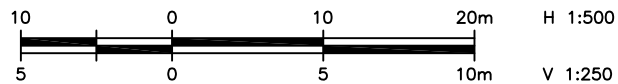
The information, interpretations and conclusions in the Report are based on Thurber's interpretation of conditions revealed through limited investigation conducted within a defined scope of services. Thurber does not accept responsibility for independent conclusions, interpretations, interpolations and/or decisions of the Client, or others who may come into possession of the Report, or any part thereof, which may be based on information contained in the Report. This restriction of liability includes but is not limited to decisions made to develop, purchase or sell land.

APPENDIX A

Borehole Locations and Strata Drawing



SECTION ALONG A-A'



METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES
UNLESS OTHERWISE SHOWN



CONT No
GWP No 5246-18-00

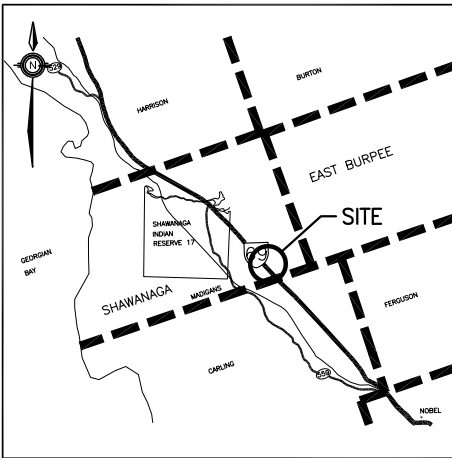
HIGHWAY 69
SHAWANAGA RIVER BRIDGE
CULVERT 25
BOREHOLE LOCATIONS AND SOIL STRATA



SHEET



THURBER ENGINEERING LTD.



KEYPLAN

LEGEND

	Borehole
	Borehole and Cone
N	Blows /0.3m (Std Pen Test, 475J/blow)
CONE	Blows /0.3m (60° Cone, 475J/blow)
PH	Pressure, Hydraulic
	Water Level Upon Completion of Drilling
	Water Level in Monitoring Well/Piezometer
	Monitoring Well/Piezometer Screen
90%	Rock Quality Designation (RQD)
A/R	Auger Refusal

NO	ELEVATION	NORTHING	EASTING
23-03	213.8	5 041 646.4	247 769.3
23-04	212.5	5 041 629.1	247 742.3

-NOTES-

- The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.
- This drawing is for subsurface information only. Surface details and features are for conceptual illustration.
- Coordinate system is MTM NAD 83 Zone 10.

GEOCRES No. 41H09-002

REVISIONS	DATE		BY		DESCRIPTION	
	DATE	BY	CHK	MTB	CODE	LOAD
DESIGN	MC	CHK	MTB	CODE	LOAD	DATE JUN 2024
DRAWN	AN	CHK	MC	SITE	STRUCT	DWG 1

APPENDIX B

Record of Boreholes Sheet

SYMBOLS, ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES

1. TEXTURAL CLASSIFICATION OF SOILS

CLASSIFICATION	PARTICLE SIZE	VISUAL IDENTIFICATION
Boulders	Greater than 200mm	same
Cobbles	75 to 200mm	same
Gravel	4.75 to 75mm	5 to 75mm
Sand	0.075 to 4.75mm	Not visible particles to 5mm
Silt	0.002 to 0.075mm	Non-plastic particles, not visible to the naked eye
Clay	Less than 0.002mm	Plastic particles, not visible to the naked eye

2. COARSE GRAIN SOIL DESCRIPTION (50% greater than 0.075mm)

TERMINOLOGY	PROPORTION
Trace or Occasional	Less than 10%
Some	10 to 20%
Adjective (e.g. silty or sandy)	20 to 35%
And (e.g. sand and gravel)	35 to 50%

3. TERMS DESCRIBING CONSISTENCY (COHESIVE SOILS ONLY)

DESCRIPTIVE TERM	UNDRAINED SHEAR STRENGTH (kPa)	APPROXIMATE SPT ⁽¹⁾ 'N' VALUE
Very Soft	12 or less	Less than 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 200	15 to 30
Hard	Greater than 200	Greater than 30

NOTE: Hierarchy of Soil Strength Prediction

- 1) Laboratory Triaxial Testing
- 2) Field Insitu Vane Testing
- 3) Laboratory Vane Testing
- 4) SPT value
- 5) Pocket Penetrometer



4. TERMS DESCRIBING DENSITY (COHESIONLESS SOILS ONLY)

DESCRIPTIVE TERM	SPT "N" VALUE
Very Loose	Less than 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	Greater than 50

5. LEGEND FOR RECORDS OF BOREHOLES


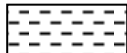



SYMBOLS AND ABBREVIATIONS FOR SAMPLE TYPE	SS Split Spoon Sample	WS Wash Sample	AS Auger (Grab) Sample
	TW Thin Wall Shelby Tube Sample	TP Thin Wall Piston Sample	
	PH Sampler Advanced by Hydraulic Pressure	PM Sampler Advanced by Manual Pressure	
	WH Sampler Advanced by Self Static Weight	RC Rock Core	SC Soil Core

$$\text{Sensitivity} = \frac{\text{Undisturbed Shear Strength}}{\text{Remoulded Shear Strength}}$$

 Water Level
 Shear Strength Determination by Pocket Penetrometer

- (1) SPT 'N' Value Standard Penetration Test 'N' Value – refers to the number of blows from a 63.5kg hammer free falling a height of 0.76m to advance a standard 50 mm outside diameter split spoon sampler for 0.3 m depth into undisturbed ground.
- (2) DCPT Dynamic Cone Penetration Test – Continuous penetration of a 50 mm outside diameter, 60° conical steel point attached to "A" size rods driven by a 63.5 kg hammer free falling a height of 0.76 m. The resistance to cone penetration is the number of hammer blows required for each 0.3 m advance of the conical point into undisturbed ground.

EXPLANATION OF ROCK LOGGING TERMS

<u>ROCK WEATHERING CLASSIFICATION</u>		<u>SYMBOLS</u>	
Fresh (FR)	No visible signs of weathering.		
Fresh Jointed (FJ)	Weathering limited to the surface of major discontinuities.		CLAYSTONE
Slightly Weathered (SW)	Penetrative weathering developed on open discontinuity surfaces, but only slight weathering of rock material.		SILTSTONE
Moderately Weathered (MW)	Weathering extends throughout the rock mass, but the rock material is not friable.		SANDSTONE
Highly Weathered (HW)	Weathering extends throughout the rock mass and the rock is partly friable.		COAL
Completely Weathered (CW)	Rock is wholly decomposed and in a friable condition, but the rock texture and structure are preserved.		Bedrock (general)

<u>DISCONTINUITY SPACING</u>		<u>STRENGTH CLASSIFICATION</u>			
Bedding	Bedding Plane Spacing	Rock Strength	Approximate Uniaxial Compressive Strength		Field Estimation of Hardness*
			(MPa)	(psi)	
Very thickly bedded	Greater than 2m	Extremely Strong	Greater than 250	Greater than 36,000	Specimen can only be chipped with a geological hammer
Thickly bedded	0.6 to 2m				
Medium bedded	0.2 to 0.6m	Very Strong	100-250	15,000 to 36,000	Requires many blows of geological hammer to break
Thinly bedded	60mm to 0.2m	Strong	50-100	7,500 to 15,000	Requires more than one blow of geological hammer to break
Very thinly bedded	20 to 60mm				
Laminated	6 to 20mm	Medium Strong	25.0 to 50.0	3,500 to 7,500	Breaks under single blow of geological hammer.
Thinly Laminated	Less than 6mm				
<u>TERMS</u>		Weak	5.0 to 25.0	750 to 3,500	Can be peeled by a pocket knife with difficulty
Total Core Recovery: (TCR)	Core recovered as a percentage of total core run length.	Very Weak	1.0 to 5.0	150 to 750	Can be peeled by a pocket knife, crumbles under firm blows of geological pick.
Solid Core Recovery: (SCR)	Percent Ratio of solid core of full cylindrical shape recovered. Expressed with respect to the total length of core run.	Extremely Weak (Rock)	0.25 to 1.0	35 to 150	Indented by thumbnail
Rock Quality Designation: (RQD)	Total length of sound core recovered in pieces 0.1m in length or larger as a percentage of total core run length.				
Uniaxial Compressive Strength (UCS)	Axial stress required to break the specimen				
Fracture Index: (FI)	Frequency of natural fractures per 0.3m of core run.				

RECORD OF BOREHOLE No 23-03

1 OF 1

METRIC

GWP# 5246-18-00 LOCATION HWY 69 & Shawanaga River Bridge N 5 041 646.4 E 247 769.3 ORIGINATED BY JF
 DIST Parry Sound HWY 69 BOREHOLE TYPE Hollow Stem Auger and HQ Coring COMPILED BY AS
 DATUM Geodetic DATE 2023.08.29 - 2023.08.29 LATITUDE 45.512405 LONGITUDE -80.229918 CHECKED BY MB

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa							
213.8	GROUND SURFACE							20	40	60	80	100			
0.0	TOPSOIL: (75mm)							20	40	60	80	100			
0.1	SAND and SILT , trace clay, trace gravel, trace organics Very Loose to Compact Brown Moist		1	SS	2		213								0 56 36 8
212.7			2	SS	66/										
1.1	BEDROCK (GNEISS), slightly weathered to fresh, very strong, grey and pink		1	RUN		0.300	212								RUN #1 TCR=100% SCR=18% RQD=0% UCS=118.4MPa UCS=136.7MPa (PLT)
	Vertical fracture from 2.4m to 2.6m		2	RUN			211								RUN #2 TCR=100% SCR=84% RQD=70% UCS=138.0MPa (PLT)
			3	RUN			210								RUN #3 TCR=100% SCR=93% RQD=89% UCS=135.2MPa (PLT)
209.5															
4.3	END OF BOREHOLE AT 4.3m. Well installation consists of 25mm diameter Schedule 40 PVC pipe with a 1.52m slotted screen. WATER LEVEL READINGS DATE DEPTH(m) ELEV.(m) 2023.09.21 0.4 213.4														

+³, ×³: Numbers refer to Sensitivity
 20
 15
 10
 (%) STRAIN AT FAILURE

METRIC

[illegible]

+³, ×³: Numbers refer to Sensitivity

APPENDIX C

Laboratory Test Results

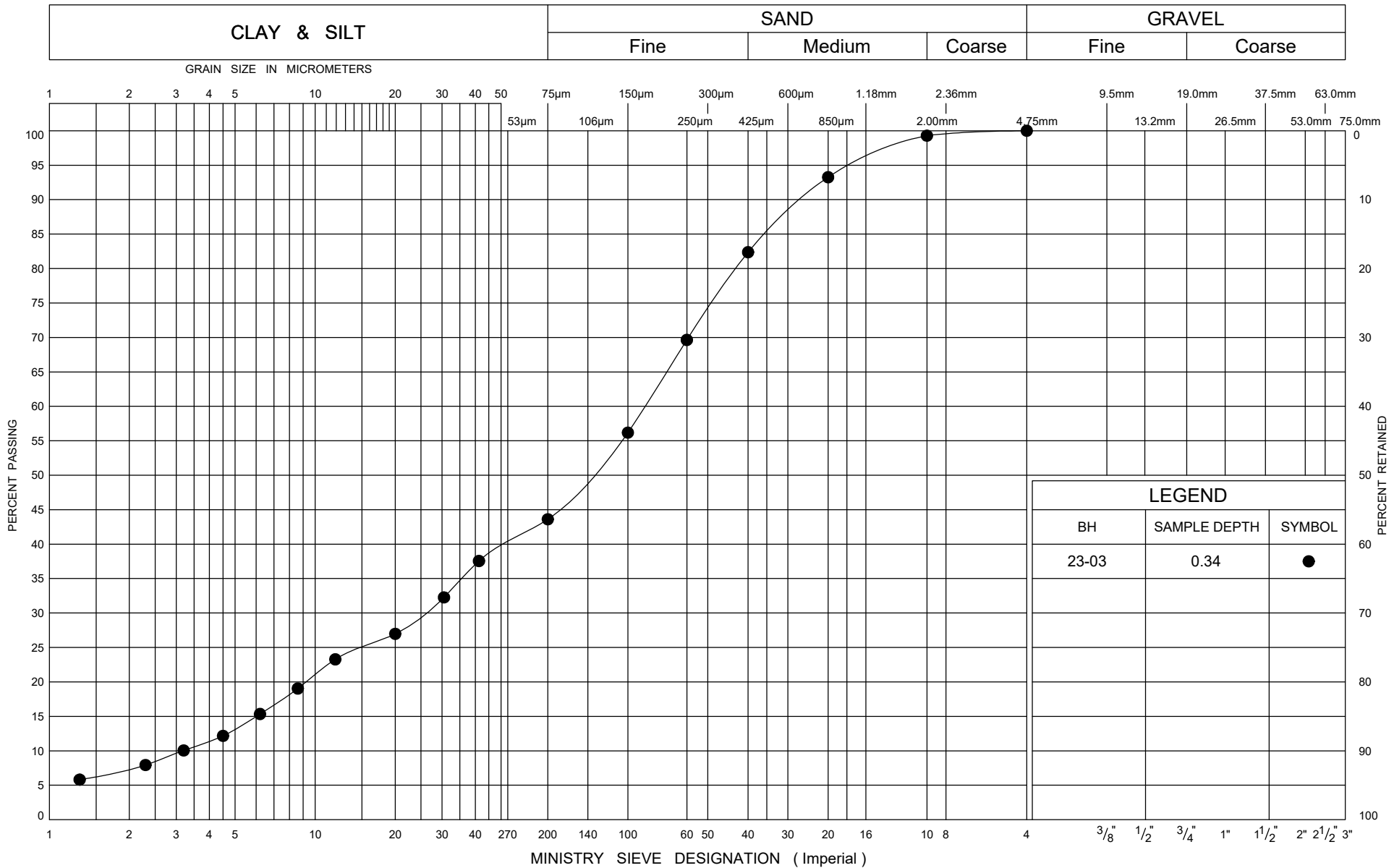




FIG No C2
GWP# 5246-18-00
HWY 69 & Shawanaga River Bridge



THURBER ENGINEERING LTD.

POINT LOAD TEST SHEET

ASTM D5731-08

Job No: 30351
 Client: Egis
 Project Name: Highway 69 and Shawanaga River Bridge Rehab
 Core Size: HQ BH No : 23-03

Date Drilled: aug 29, 2023
 Date Tested: sept 14, 2023
 Tester: AK
 Reviewed by: MC

Test No.	Run No.	Depth (m)	Axial or Diametral	Gauge (MPa)	Diameter (mm)	Length (mm)	I _{s(50)} (MPa)	UCS (MPa)	Rock Type	Rock Strength (after Hoek & Brown, 1997)
1	1	1.1	D	17.3	60.9	85.5	4.8	116.3	Gneiss	Very Strong
2	1	1.4	D	23.4	60.9	89.5	6.5	157.1	Gneiss	Very Strong
3	2	1.7	D	21.3	60.9	109.3	5.9	142.7	Gneiss	Very Strong
4	2	1.8	D	19.9	60.9	89.3	5.6	133.3	Gneiss	Very Strong
5	3	3.1	D	20.5	60.9	110.1	5.7	137.3	Gneiss	Very Strong
6	3	3.3	D	19.9	60.9	105.0	5.5	133.1	Gneiss	Very Strong
9					Average Run 1			136.7		
10					Average Run 2			138.0		
11					Average Run 3			135.2		
12										
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										
23										
24										
25										
26										
27										
28										
29										
30										
31										
32										
33										
34										
35										

* It is ideal to perform axial test on core specimens with D/L ratio of 1.1 ± 0.1

* Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

* Diametral Test should have $0.7 \times D$ on either side of test point.

* Correlation factor to obtain UCS values is 24.



THURBER ENGINEERING LTD.

POINT LOAD TEST SHEET

ASTM D5731-08

Job No: 30351
 Client: Egis
 Project Name: Highway 69 and Shawanaga River Bridge Rehab
 Core Size: HQ BH No : 23-04

Date Drilled: AUG 10,2023
 Date Tested: 06-Sep-23
 Tester: AK
 Reviewed by: MC

Test No.	Run No.	Depth (m)	Axial or Diametral	Gauge (MPa)	Diameter (mm)	Length (mm)	I _{s(50)} (MPa)	UCS (MPa)	Rock Type	Rock Strength (after Hoek & Brown, 1997)
1	1	2.2	D	14.5	61.9	136.1	4.0	95.0	Gneiss	Strong
2	1	2.3	D	18.1	61.9	114.2	4.9	118.2	Gneiss	Very Strong
3	2	3.0	D	19.2	61.9	96.2	5.2	125.5	Gneiss	Very Strong
4	2	3.1	D	16.6	61.9	93.0	4.5	108.2	Gneiss	Very Strong
5	3	3.9	D	20.0	61.9	109.6	5.5	131.0	Gneiss	Very Strong
6	3	4.1	D	8.4	61.9	105.2	2.3	55.0	Gneiss	Strong
7	4	4.7	D	6.1	61.9	118.3	1.7	40.0	Gneiss	Medium Strong
8										
9					Average Run 1			106.6		
10					Average Run 2			116.9		
11					Average Run 3			93.0		
12					Average Run 4			40.0		
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										
23										
24										
25										
26										
27										
28										
29										
30										
31										
32										
33										
34										
35										

* It is ideal to perform axial test on core specimens with D/L ratio of 1.1 ± 0.1

* Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

* Diametral Test should have $0.7 \times D$ on either side of test point.

* Correlation factor to obtain UCS values is 24.

November 9, 2023

Madisan Chiarotto
Thurber Engineering Ltd.
103, 2010 Winston Park Drive
Oakville ON
L6H 5R7

Re: UCS testing (Thurber Project No. 30551)

Dear Madisan:

On October 20th, 2023, fourteen (14) rock core samples (HQ and NQ sized) were received by Geomechanica Inc. via drop-off by Thurber personnel. These samples were identified as being from Thurber project 30551 (Highway 69 and Shawanaga River Bridge Rehabilitation). From these samples, fourteen (14) UCS tests were completed.

Details regarding the steps of specimen preparation and testing are presented in the accompanying laboratory report and summary spreadsheet.

Sincerely,



Bryan Tatone Ph.D., P. Eng.

Geomechanica Inc.
Tel: (647) 478-9767
Email: bryan.tatone@geomechanica.com

Rock Laboratory Testing Results

A report submitted to:

Madisan Chiarotto
Thurber Engineering Ltd.
103, 2010 Winston Park Drive
Oakville, Ontario
Canada L6H 5R7

Prepared by:

Bryan Tatone, PhD, PEng
Omid Mahabadi, PhD, PEng
Geomechanica Inc.
#14-1240 Speers Rd.
Oakville ON
L6L 2X4 Canada
Tel: +1-647-478-9767
lab@geomechanica.com

November 9, 2023
Project number: 30351

Abstract

This document summarizes the results of laboratory testing, including 14 Uniaxial Compressive Strength (UCS) tests. The UCS values along with photographs of specimens before and after testing are presented herein.

In this document:

1 Uniaxial Compressive Strength Tests	1
Appendices	3

1 Uniaxial Compressive Strength Tests

1.1 Overview

This section summarizes the results of uniaxial compressive strength (UCS) testing. The testing was performed in Geomechanica's rock testing laboratory using a 150 ton (1.3 MN) Forney loading frame equipped with pressure-compensated control valve to maintain an axial displacement rate of approximately 0.05 mm/min (Figure 1). The preparation and testing procedure for each specimen included the following:

1. Unwrapping the core sample and inspecting it for damage.
2. Diamond cutting the core sample to obtain a cylindrical specimen with an appropriate length (length:diameter = 2:1) and nearly parallel end faces.
3. Diamond grinding the specimen to obtain flat (within ± 0.025 mm) and parallel end faces (within 0.25°).
4. Placing the specimen into the loading frame and axially loading the specimen to rupture while continuously recording axial force and axial deformation to determine the peak strength (UCS).



Figure 1: Forney loading frame setup for UCS testing.

Using a precision V-block mounted on the magnetic chuck of the surface grinder, test specimens met the end flatness, end parallelism, and perpendicularity criteria set out in ASTM D4543-19. The side straightness criteria, as checked with a feeler gauge, and the minimum length:diameter criteria were met for all specimens unless noted otherwise in Table 1. Testing of the specimens followed ASTM D7012-14 Method C.

1.2 Results

The results of UCS testing are summarized in Table 1. Additional specimens and testing details are included in the summary spreadsheet that accompanies this report.

Table 1: Summary of Uniaxial Compression test results.

Sample	Depth (m)	Bulk density ρ (g/cm ³)	UCS (MPa)	Lithology	Failure description
BH23-01	32'10" - 33'10"	2.769	90.5	Granite gneiss	1, 2
BH23-02	2'3" - 3'2"	2.770	117.4	Granite gneiss	1
BH23-03	3'10" - 4'8"	2.792	118.4	Granite gneiss	3
BH23-04	8'0" - 8'10"	2.753	101.2	Granite gneiss	1
BH23-05	21'0" - 22'11"	2.646	117.3	Pegmatite	4
BH23-07	13'5" - 14'4"	2.746	90.2	Granite gneiss	3
BH23-09	7'10" - 8'5"	2.753	113.8	Granite gneiss	3, 5
BH23-10	27'6" - 28'5"	2.661	129.2	Granite gneiss	3
BH23-12	21'5" - 22'4"	2.775	86.7	Granite gneiss	1
BH23-13	6'6" - 7'2"	2.674	129.4	Granite gneiss	6
BH23-15	17'2" - 17'10"	2.416	98.6	Granite gneiss	3, 5
BH23-16	7'3" - 7'11"	2.648	147.0	Granite gneiss	3, 2
BH23-17	12'6" - 13'5"	2.751	75.9	Granite gneiss	1, 5
BH23-18	28'7" - 29'7"	2.707	68.2	Granite gneiss	3, 5

¹ Inclined shear failure

² Partial hourglass failure

³ Inclined shear fracture and axial splitting failure

⁴ Axial splitting failure

⁵ Failure partly along pre-existing structure

⁶ Hourglass failure

1.3 Specimen photographs



Photographs of the specimens before and after testing are presented in the Appendix of this report.

Appendices



Specimen sheets

- BH23-01
- BH23-02
- BH23-03
- BH23-04
- BH23-05
- BH23-07
- BH23-09
- BH23-10
- BH23-12
- BH23-13
- BH23-15
- BH23-16
- BH23-17
- BH23-18



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-01	Depth	32' 10" - 33' 10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	60.80		
Length (mm) ^a	129.90		
Bulk density ρ (g/cm ³)	2.769		
UCS (MPa)	90.5		
Lithology	Granite gneiss		
Failure description ^b	1, 2		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ¹ Inclined shear failure; ² Partial hourglass failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-02	Depth	2'3" - 3'2"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	62.92		
Length (mm) ^a	129.18		
Bulk density ρ (g/cm ³)	2.770		
UCS (MPa)	117.4		
Lithology	Granite gneiss		
Failure description ^b	1		
<div><div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ¹ Inclined shear failure;</div></div></div>			
Remarks: Loading Rate: 0.05mm/min. Specimen experienced pre-peak localized failure(s).			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-03	Depth	3' 10" - 4' 8"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	60.75		
Length (mm) ^a	129.54		
Bulk density ρ (g/cm ³)	2.792		
UCS (MPa)	118.4		
Lithology	Granite gneiss		
Failure description ^b	3		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-04	Depth	8'0" - 8'10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	61.85		
Length (mm) ^a	129.19		
Bulk density ρ (g/cm ³)	2.753		
UCS (MPa)	101.2		
Lithology	Granite gneiss		
Failure description ^b	1		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ¹ Inclined shear failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-05	Depth	21'0" - 22'11"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	47.52		
Length (mm) ^a	102.95		
Bulk density ρ (g/cm ³)	2.646		
UCS (MPa)	117.3		
Lithology	Pegmatite		
Failure description ^b	4		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ⁴ Axial splitting failure;</div>			
Remarks: Loading Rate: 0.05mm/min. Specimen experienced pre-peak localized failure(s).			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-07	Depth	13'5" - 14'4"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	46.74		
Length (mm) ^a	103.86		
Bulk density ρ (g/cm ³)	2.746		
UCS (MPa)	90.2		
Lithology	Granite gneiss		
Failure description ^b	3		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-09	Depth	7'10" - 8'5"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	48.54		
Length (mm) ^a	103.59		
Bulk density ρ (g/cm ³)	2.753		
UCS (MPa)	113.8		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-10	Depth	27'6" - 28'5"
<div>Specimen parameters</div> <div><div>Diameter (mm)^a</div><div>Length (mm)^a</div><div>Bulk density ρ (g/cm³)</div><div>UCS (MPa)</div><div>Lithology</div><div>Failure description^b</div><div></div></div>		<div>Prior to testing</div> <div></div>	<div>After testing</div> <div></div>
<div><div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ³ Inclined shear fracture and axial splitting failure;</div></div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-12	Depth	21'5" - 22'4"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	46.99		
Length (mm) ^a	103.61		
Bulk density ρ (g/cm³)	2.775		
UCS (MPa)	86.7		
Lithology	Granite gneiss		
Failure description ^b	1		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ¹ Inclined shear failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-13	Depth	6'6" - 7'2"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	48.89		
Length (mm) ^a	103.89		
Bulk density ρ (g/cm ³)	2.674		
UCS (MPa)	129.4		
Lithology	Granite gneiss		
Failure description ^b	6		
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ⁶ Hourglass failure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

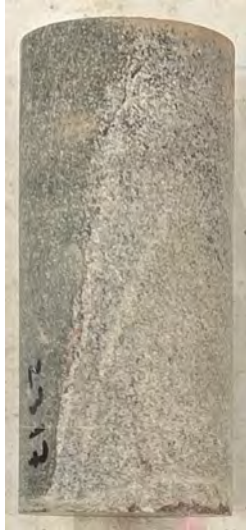

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-15	Depth	17'2" - 17'10"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	49.61		
Length (mm) ^a	103.77		
Bulk density ρ (g/cm ³)	2.416		
UCS (MPa)	98.6		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div> <div>^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-16	Depth	7'3" - 7'11"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	47.11		
Length (mm) ^a	103.57		
Bulk density ρ (g/cm ³)	2.648		
UCS (MPa)	147.0		
Lithology	Granite gneiss		
Failure description ^b	3, 2		
<div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet. ^b Failure description: ³ Inclined shear fracture and axial splitting failure; ² Partial hourglass failure;</div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-17	Depth	12'6" - 13'5"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	46.83		
Length (mm) ^a	102.54		
Bulk density ρ (g/cm ³)	2.751		
UCS (MPa)	75.9		
Lithology	Granite gneiss		
Failure description ^b	1, 5		
<div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ¹ Inclined shear failure; ⁵ Failure partly along pre-existing structure;</div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08

Uniaxial Compression Test

Client	Thurber Engineering Ltd.	Project	30351
Sample	BH23-18	Depth	28'7" - 29'7"
<div>Specimen parameters</div>		Prior to testing	After testing
Diameter (mm) ^a	62.81		
Length (mm) ^a	129.38		
Bulk density ρ (g/cm ³)	2.707		
UCS (MPa)	68.2		
Lithology	Granite gneiss		
Failure description ^b	3, 5		
<div><div><div><div><div><div>^a Additional specimen measurement/details provided in accompanying summary spreadsheet.</div><div>^b Failure description: ³ Inclined shear fracture and axial splitting failure; ⁵ Failure partly along pre-existing structure;</div></div></div></div></div></div>			
Remarks: Loading Rate: 0.05mm/min.			
Performed by	SD	Date	2023-11-08



FINAL REPORT

CA40126-AUG23 R

30351, Parry Sound (North)

Prepared for

Thurber Engineering Ltd.



FINAL REPORT

CA40126-AUG23 R

First Page

CLIENT DETAILS		LABORATORY DETAILS	
Client	Thurber Engineering Ltd.	Project Specialist	Jill Campbell, B.Sc.,GISAS
Address	103, 2010 Winston Park Drive	Laboratory	SGS Canada Inc.
	Oakville, ON	Address	185 Concession St., Lakefield ON, K0L 2H0
	L6H 5R7, Canada		
Contact	Madisan Chiarotto	Telephone	2165
Telephone	647-548-8390	Facsimile	705-652-6365
Facsimile		Email	jill.campbell@sgs.com
Email	mchiarotto@thurber.ca	SGS Reference	CA40126-AUG23
Project	30351, Parry Sound (North)	Received	08/11/2023
Order Number		Approved	08/17/2023
Samples	Solution (7)	Report Number	CA40126-AUG23 R
		Date Reported	08/17/2023

COMMENTS
Temperature of Sample upon Receipt: 9 degrees C
Cooling Agent Present: Yes
Custody Seal Present: Yes
Chain of Custody Number: 036865


SIGNATORIES
Jill Campbell, B.Sc.,GISAS




TABLE OF CONTENTS

First Page..... 1-2

Index..... 3

Results..... 4

QC Summary..... 5-7

Legend..... 8

Annexes..... 9



FINAL REPORT

CA40126-AUG23 R

Client: Thurber Engineering Ltd.

Project: 30351, Parry Sound (North)

Project Manager: Madisan Chiarotto

Samplers: Jakob Flood

MATRIX: WATER

Sample Number	6	7	8	9	10	11	12
Sample Name	Culvert 24 STA 11+449	Culvert 25 STA 11+680	Culvert 35 STA 14+631	Culvert 38 STA 15+250	Culvert 43 STA 16+534	Culvert 44 STA 16+764	Culvert 47 STA 17+808
Sample Matrix	Solution	Solution	Solution	Solution	Solution	Solution	Solution
Sample Date	09/08/2023	09/08/2023	09/08/2023	11/08/2023	11/08/2023	11/08/2023	08/08/2023

Parameter	Units	RL	Result	Result	Result	Result	Result	Result	Result
General Chemistry									
Conductivity	uS/cm	2	657	171	76	1120	80	66	1180
Redox Potential	mV	no	143	152	117	127	136	240	152
Sulphide	µg/L	6	< 6	< 6	16	7.0	23	41	< 6
Metals and Inorganics									
Sulphate	mg/L	0.04	12	8.1	3.0	28	0.74	5.8	16
Other (ORP)									
pH	No unit	0.05	5.80	6.28	5.68	6.32	5.98	5.77	6.41
Chloride	mg/L	0.04	210	38	17	380	14	10	380



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

Anions by IC
Method: EPA300/MA300-Ions1.3 | Internal ref.: ME-CA-IENVIIC-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Chloride	DIO0332-AUG23	mg/L	0.04	<0.04	6	20	102	90	110	108	75	125
Sulphate	DIO0332-AUG23	mg/L	0.04	<0.04	ND	20	97	90	110	92	75	125
Sulphate	DIO0393-AUG23	mg/L	0.04	<0.04	3	20	97	90	110	91	75	125

Conductivity
Method: SM 2510 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Conductivity	EWL0270-AUG23	uS/cm	2	< 2	0	20	100	90	110	NA		



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

pH
Method: SM 4500 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
pH	EWL0265-AUG23	No unit	0.05	NA	0		100			NA		
pH	EWL0277-AUG23	No unit	0.05	NA	0		100			NA		

Redox Potential
Method: SM 2580 |

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Redox Potential	EWL0243-AUG23	mV	no	NA	2	20	104	80	120	NA		

Sulphide by SFA
Method: SM 4500 | Internal ref.: ME-CA-IENVISFA-LAK-AN-008

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Sulphide	SKA0140-AUG23	ug/L	6	<0.006	ND	20	110	80	120	NA	75	125



FINAL REPORT

CA40126-AUG23 R

QC SUMMARY

Method Blank: a blank matrix that is carried through the entire analytical procedure. Used to assess laboratory contamination.

Duplicate: Paired analysis of a separate portion of the same sample that is carried through the entire analytical procedure. Used to evaluate measurement precision.

LCS/Spike Blank: Laboratory control sample or spike blank refer to a blank matrix to which a known amount of analyte has been added. Used to evaluate analyte recovery and laboratory accuracy without sample matrix effects.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate laboratory accuracy with sample matrix effects.

Reference Material: a material or substance matrix matched to the samples that contains a known amount of the analyte of interest. A reference material may be used in place of a matrix spike.

RL: Reporting limit

RPD: Relative percent difference

AC: Acceptance criteria

Multielement Scan Qualifier: as the number of analytes in a scan increases, so does the chance of a limit exceedance by random chance as opposed to a real method problem. Thus, in multielement scans, for the LCS and matrix spike, up to 10% of the analytes may exceed the quoted limits by up to 10% absolute and the spike is considered acceptable.

Duplicate Qualifier: for duplicates as the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Matrix Spike Qualifier: for matrix spikes, as the concentration of the native analyte increases, the uncertainty of the matrix spike recovery increases. Thus, the matrix spike acceptance limits apply only when the concentration of the matrix spike is greater than or equal to the concentration of the native analyte.



LEGEND

FOOTNOTES

NSS Insufficient sample for analysis.

RL Reporting Limit.

↑ Reporting limit raised.

↓ Reporting limit lowered.

NA The sample was not analysed for this analyte

ND Non Detect

Results relate only to the sample tested.

Data reported represent the sample as submitted to SGS. Solid samples expressed on a dry weight basis.

"Temperature Upon Receipt" is representative of the whole shipment and may not reflect the temperature of individual samples.

Analysis conducted on samples submitted pursuant to or as part of Reg. 153/04, are in accordance to the "Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act and Excess Soil Quality" published by the Ministry and dated March 9, 2004 as amended.

SGS provides criteria information (such as regulatory or guideline limits and summary of limit exceedances) as a service. Every attempt is made to ensure the criteria information in this report is accurate and current, however, it is not guaranteed. Comparison to the most current criteria is the responsibility of the client and SGS assumes no responsibility for the accuracy of the criteria levels indicated.

SGS Canada Inc. statement of conformity decision rule does not consider uncertainty when analytical results are compared to a specified standard or regulation.

This document is issued, on the Client's behalf, by the Company under its General Conditions of Service available on request and accessible at http://www.sgs.com/terms_and_conditions.htm.

The Client's attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein. Any other holder of this document is advised that information contained hereon reflects the Company's findings at the time of its intervention only and within the limits of Client's instructions, if any. The Company's sole responsibility is to its Client and this document does not exonerate parties to a transaction from exercising all their rights and obligations under the transaction documents. Reproduction of this analytical report in full or in part is prohibited.

This report supersedes all previous versions.

-- End of Analytical Report --

Industries & Environment - Lakefield: 185 Concession St., Lakefield, ON K0L 2H0 Phone: 705-652-2000 Fax: 705-652-6365 Web: www.sgs.com/environment
- London: 657 Consortium Court, London, ON, N6E 2S8 Phone: 519-672-4500 Toll Free: 877-848-8060 Fax: 519-672-0361

Received By: Scott
Received Date: 08/11/23 (mm/dd/yy)
Received Time: 13:05 (hr:min)
Cooling Agent Present: Yes ☐ No ☒
Custody Seal Intact: Yes ☐ No ☒
Temperature Upon Receipt (°C): 9.3
Type: ice
LAB LIMS #: CA46126-AUG23

Company: THUNDER ENGINEERING LTD
Contact: MADISON CHAMBERLAIN
Address: 2010 WINSTON PARK DR
Phone: 905-829-8666
Fax: 905-829-8666
Email: MCHAMBERLAIN@THUNDER.CA
Received By (signature): Scott
Custody Seal Present: Yes ☐ No ☒
Custody Seal Intact: Yes ☐ No ☒
Invoice Information
☐ (same as Report Information)
Company: THUNDER ENGINEERING LTD
Contact: MADISON CHAMBERLAIN
Address: 2010 WINSTON PARK DR
Phone: 905-829-8666
Fax: 905-829-8666
Email: MCHAMBERLAIN@THUNDER.CA

Quotation #: 30351
Project #: 30351
Site Location/ID: PARRY SOUND (NORTH)
TURNAROUND TIME (TAT) REQUIRED
☐ Regular TAT (5-7 days)
☐ 1 Day ☐ 2 Days ☐ 3 Days ☐ 4 Days
RUSH TAT (Additional Charges May Apply):
PLEASE CONFIRM RUSH FEASIBILITY WITH SGS REPRESENTATIVE PRIOR TO SUBMISSION

Specify Due Date: 08/11/23
NOTE: DRINKING (POTABLE) WATER SAMPLES FOR HUMAN CONSUMPTION MUST BE SUBMITTED WITH SGS DRINKING WATER CHAIN OF CUSTODY

REPORT INFORMATION				INVOICE INFORMATION			
Company: <u>THUNDER ENGINEERING LTD</u>				<input type="checkbox"/> (same as Report Information)			
Contact: <u>MADISON CHAMBERLAIN</u>				Company: <u>THUNDER ENGINEERING LTD</u>			
Address: <u>2010 WINSTON PARK DR</u>				Contact: <u>MADISON CHAMBERLAIN</u>			
Phone: <u>905-829-8666</u>				Address: <u>2010 WINSTON PARK DR</u>			
Fax: <u>905-829-8666</u>				Phone: <u>905-829-8666</u>			
Email: <u>MCHAMBERLAIN@THUNDER.CA</u>				Email: <u>MCHAMBERLAIN@THUNDER.CA</u>			
Other Regulations: <input type="checkbox"/> Reg 347/558 (3 Day min TAT) <input type="checkbox"/> PWQO <input type="checkbox"/> MMER <input type="checkbox"/> CCME <input type="checkbox"/> Other: <input type="checkbox"/> MISA <input type="checkbox"/> ODWS Not Reportable *See note				Sewer By-Law: <input type="checkbox"/> Sanitary <input type="checkbox"/> Storm Municipality: <u>THUNDER</u>			
Soil Volume <input type="checkbox"/> <350m3 <input type="checkbox"/> >350m3				Soil Texture: <input type="checkbox"/> Res/Park <input type="checkbox"/> Ind/Com <input type="checkbox"/> Coarse <input type="checkbox"/> Agri/Other <input type="checkbox"/> Medium/Fine Appx. <u>11+44</u>			
RECORD OF SITE CONDITION (RSC) <input type="checkbox"/> YES <input type="checkbox"/> NO				Other Regulations: <input type="checkbox"/> Reg 347/558 (3 Day min TAT) <input type="checkbox"/> PWQO <input type="checkbox"/> MMER <input type="checkbox"/> CCME <input type="checkbox"/> Other: <input type="checkbox"/> MISA <input type="checkbox"/> ODWS Not Reportable *See note			
SAMPLE IDENTIFICATION				DATE SAMPLED			
1 CULVERT 24 STA 11+44				08/09/23 17:00			
2 CULVERT 25 STA 11+680				08/09/23 16:45			
3 CULVERT 35 STA 14+631				08/09/23 9:30			
4 CULVERT 38 STA 15+250				08/11/23 9:10			
5 CULVERT 43 STA 16+534				08/11/23 9:05			
6 CULVERT 44 STA 16+764				08/11/23 9:00			
7 CULVERT 47 STA 17+808				08/08/23 10:00			
8							
9							
10							
11							
12							

Observations/Comments/Special Instructions

Signature: Scott
Signature: Scott
Date: 08/11/23 (mm/dd/yy)
Date: 08/11/23 (mm/dd/yy)
Pink Copy - Client
Yellow & White Copy - SGS

Sampled By (NAME): JAYOB FLOOD
Relinquished by (NAME): JAYOB FLOOD
Note: Submission of samples to SGS is acknowledged that you have been provided direction on sample collection/handling and transportation of samples. (2) Submission of samples to SGS is considered authorization for completion of work. Signatures may appear on this form or be retained on file in the contract, or in an alternative format (e.g. shipping documents). (3) Results may be sent by email to an unlimited number of addresses for no additional cost. Fax is available upon request. This document is issued by the Company under its General Conditions of Service accessible at http://www.sgs.com/terms_and_conditions.htm. (Printed copies are available upon request.) Attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein.



FINAL REPORT

CA40212-AUG23 R1

30351

Prepared for

Thurber Engineering Ltd.



FINAL REPORT

CA40212-AUG23 R1

First Page

CLIENT DETAILS		LABORATORY DETAILS	
Client	Thurber Engineering Ltd.	Project Specialist	Jill Campbell, B.Sc.,GISAS
Address	103, 2010 Winston Park Drive	Laboratory	SGS Canada Inc.
	Oakville, ON	Address	185 Concession St., Lakefield ON, K0L 2H0
	L6H 5R7, Canada		
Contact	Madisan Chiarotto	Telephone	2165
Telephone	647-548-8390	Facsimile	705-652-6365
Facsimile		Email	jill.campbell@sgs.com
Email	mchiarotto@thurber.ca	SGS Reference	CA40212-AUG23
Project	30351	Received	08/22/2023
Order Number		Approved	08/29/2023
Samples	Soil (6)	Report Number	CA40212-AUG23 R1
		Date Reported	08/29/2023

COMMENTS
Temperature of Sample upon Receipt: 8 degrees C
Cooling Agent Present: Yes
Custody Seal Present: Yes
Chain of Custody Number: 036623
Corrosivity Index is based on the American Water Works Corrosivity Scale according to AWWA C-105. An index greater than 10 indicates the soil matrix may be corrosive to cast iron alloys.


SIGNATORIES
Jill Campbell, B.Sc.,GISAS




TABLE OF CONTENTS

First Page..... 1-2

Index..... 3

Results..... 4

QC Summary..... 5-6

Legend..... 7

Annexes..... 8



FINAL REPORT

CA40212-AUG23 R1

Client: Thurber Engineering Ltd.

Project: 30351

Project Manager: Madisan Chiarotto

Samplers: Madisan Chiarotto

MATRIX: SOIL

Sample Number	5	6	7	8	9	10
Sample Name	23-02 SS#1	23-04 SS#2	23-05 SS#5B	23-12 SS#7	23-16 SS#2B	23-17 SS#3
	CORR	CORR	CORR	CORR	CORR	CORR
Sample Matrix	Soil	Soil	Soil	Soil	Soil	Soil
Sample Date	10/08/2023	09/08/2023	26/07/2023	02/08/2023	02/08/2023	04/08/2023

Parameter	Units	RL	Result	Result	Result	Result	Result	Result
Corrosivity Index								
Corrosivity Index	none	1	1	1	3	1	1	1
Soil Redox Potential	mV	no	287	304	295	249	453	368
Sulphide (Na ₂ CO ₃)	%	0.04	< 0.04	< 0.04	< 0.04	< 0.04	< 0.04	< 0.04
pH	pH Units	0.05	7.12	6.09	6.47	6.54	5.90	7.02
Resistivity (calculated)	ohms.cm	-9999	3750	3890	2260	5920	26300	22200
General Chemistry								
Conductivity	uS/cm	2	267	257	443	169	38	45
Metals and Inorganics								
Moisture Content	%	0.1	13.7	18.4	15.4	19.8	11.6	22.4
Sulphate	µg/g	0.4	17	16	15	10	7.9	9.0
Other (ORP)								
Chloride	µg/g	0.4	130	170	170	77	7.1	25



FINAL REPORT

CA40212-AUG23 R1

QC SUMMARY

Anions by IC
Method: EPA300/MA300-Ions1.3 | Internal ref.: ME-CA-IENVIIC-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Chloride	DIO0693-AUG23	µg/g	0.4	<0.4	3	35	101	80	120	100	75	125
Sulphate	DIO0693-AUG23	µg/g	0.4	<0.4	22	35	99	80	120	96	75	125

Carbon/Sulphur
Method: ASTM E1915-07A | Internal ref.: ME-CA-IENVIARD-LAK-AN-020

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Sulphide (Na2CO3)	ECS0120-AUG23	%	0.04	< 0.04	5	20	106	80	120			

Conductivity
Method: SM 2510 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Conductivity	EWL0460-AUG23	uS/cm	2	< 2	0	20	100	90	110	NA		



FINAL REPORT

CA40212-AUG23 R1

QC SUMMARY

pH
Method: SM 4500 | Internal ref.: ME-CA-IENVIEWL-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
pH	EWL0460-AUG23	pH Units	0.05	NA	0		100			NA		

Method Blank: a blank matrix that is carried through the entire analytical procedure. Used to assess laboratory contamination.

Duplicate: Paired analysis of a separate portion of the same sample that is carried through the entire analytical procedure. Used to evaluate measurement precision.

LCS/Spike Blank: Laboratory control sample or spike blank refer to a blank matrix to which a known amount of analyte has been added. Used to evaluate analyte recovery and laboratory accuracy without sample matrix effects.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate laboratory accuracy with sample matrix effects.

Reference Material: a material or substance matrix matched to the samples that contains a known amount of the analyte of interest. A reference material may be used in place of a matrix spike.

RL: Reporting limit

RPD: Relative percent difference

AC: Acceptance criteria

Multielement Scan Qualifier: as the number of analytes in a scan increases, so does the chance of a limit exceedance by random chance as opposed to a real method problem. Thus, in multielement scans, for the LCS and matrix spike, up to 10% of the analytes may exceed the quoted limits by up to 10% absolute and the spike is considered acceptable.

Duplicate Qualifier: for duplicates as the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Matrix Spike Qualifier: for matrix spikes, as the concentration of the native analyte increases, the uncertainty of the matrix spike recovery increases. Thus, the matrix spike acceptance limits apply only when the concentration of the matrix spike is greater than or equal to the concentration of the native analyte.



LEGEND

FOOTNOTES

NSS Insufficient sample for analysis.

RL Reporting Limit.

↑ Reporting limit raised.

↓ Reporting limit lowered.

NA The sample was not analysed for this analyte

ND Non Detect

Results relate only to the sample tested.

Data reported represent the sample as submitted to SGS. Solid samples expressed on a dry weight basis.

"Temperature Upon Receipt" is representative of the whole shipment and may not reflect the temperature of individual samples.

Analysis conducted on samples submitted pursuant to or as part of Reg. 153/04, are in accordance to the "Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act and Excess Soil Quality" published by the Ministry and dated March 9, 2004 as amended.

SGS provides criteria information (such as regulatory or guideline limits and summary of limit exceedances) as a service. Every attempt is made to ensure the criteria information in this report is accurate and current, however, it is not guaranteed. Comparison to the most current criteria is the responsibility of the client and SGS assumes no responsibility for the accuracy of the criteria levels indicated.

SGS Canada Inc. statement of conformity decision rule does not consider uncertainty when analytical results are compared to a specified standard or regulation.

This document is issued, on the Client's behalf, by the Company under its General Conditions of Service available on request and accessible at http://www.sgs.com/terms_and_conditions.htm.

The Client's attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein. Any other holder of this document is advised that information contained hereon reflects the Company's findings at the time of its intervention only and within the limits of Client's instructions, if any. The Company's sole responsibility is to its Client and this document does not exonerate parties to a transaction from exercising all their rights and obligations under the transaction documents. Reproduction of this analytical report in full or in part is prohibited.

This report supersedes all previous versions.

-- End of Analytical Report --

Industries & Environment - Lakeland: 185 Concession St., Lakeland, ON K0L 2H0 Phone: 705-652-2000 Fax: 705-652-6365 Web: www.sgs.com/environement
- London: 657 Concession Court, London, ON, N6E 2S8 Phone: 519-672-4500 Toll Free: 877-848-8060 Fax: 519-672-0361

Laboratory Information Section - Lab use only

Received By: ED	Received (signature):	Cooling Agent Present: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>	Type: ICG
Received Date: 8 Dec 08 (mm/dd/yy)	Custody Seal Present: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>	Temperature Upon Receipt (°C): 8.3	LAB LIMS #: CA40212-AUG 23
Received Time: 11:00 (hr : min)	Custody Seal Intact: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>		
REPORT INFORMATION		INVOICE INFORMATION	
Company: Thurber Engineering Ltd		(same as Report Information)	
Contact: Madisen Chirapat		Company:	
Address: 2010 Winston Park Dr		Contact:	
Unit 103, Oakville ON, L6H 5R7		Address:	
Phone: (905) 829-8666		Phone:	
Fax:			
Email: mchirapat@thurber.ca		Email: accounting@thurber.ca	
Email: mboucher@thurber.ca			

REGULATIONS		ANALYSIS REQUESTED		COMMENTS:	
<input type="checkbox"/> O.Reg 153/04	<input type="checkbox"/> O.Reg 406/19	Other Regulations:		Sewer By-Law:	
<input type="checkbox"/> Table 1	<input type="checkbox"/> Res/Park	<input type="checkbox"/> Reg 347/558 (3 Day min TAT)		<input type="checkbox"/> Sanitary	
<input type="checkbox"/> Table 2	<input type="checkbox"/> Ind/Com	<input type="checkbox"/> PWQO		<input type="checkbox"/> Storm	
<input type="checkbox"/> Table 3	<input type="checkbox"/> Agri/Other	<input type="checkbox"/> CCME		<input type="checkbox"/> Municipality:	
<input type="checkbox"/> Table	<input type="checkbox"/> Appx.	<input type="checkbox"/> MISA			
Soil Volume: <input type="checkbox"/> <350m3 <input type="checkbox"/> >350m3		<input type="checkbox"/> ODWS Not Reportable *See note			
RECORD OF SITE CONDITION (RSC)		<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO			
SAMPLE IDENTIFICATION		DATE SAMPLED		TIME SAMPLED	
1 23-0255#1 CORR		08/10/23		1	
2 23-0455#2 CORR		08/10/23		1	
3 23-0555#3 CORR		07/26/23		1	
4 23-1255#7 CORR		08/10/23		1	
5 23-1655#2 CORR		08/10/23		1	
6 23-1755#3 CORR		08/10/23		1	
7					
8					
9					
10					
11					
12					

Observations/Comments/Special Instructions		Signature: Madisen Chirapat		Date: 08.12.23 (mm/dd/yy)	Pinck Copy - Client
Sampled By (NAME): Madisen Chirapat		Signature: Madisen Chirapat		Date: 08.12.23 (mm/dd/yy)	Yellow & White Copy - SGS
Relinquished by (NAME): Madisen Chirapat		Signature: Madisen Chirapat		Date: 08.12.23 (mm/dd/yy)	Yellow & White Copy - SGS
Note: Submission of samples to SGS is acknowledgement that you have been provided direction on sample collection/handling and transportation of samples. (2) Submission of samples to SGS is considered authorization for completion of work. Signatures may appear on this form or be retained on file in the contract, or in an alternative format (e.g. shipping documents). (3) Results may be sent by email to an unlimited number of addresses for no additional cost. Fax is available upon request. This document is issued by the Company under its General Conditions of Service accessible at http://www.sgs.com/terms_and_conditions.htm . (Printed copies are available upon request.) Attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein.					

APPENDIX D

Bedrock Core Photos

HIGHWAY 69 AND REHABILITATION OF SHAWANAGA RIVER BRIDGE
Photographs of Rock Core

Borehole 23-03 – Run 1 – 1.07 – 1.68 m



Borehole 23-03 – Run 2 – 1.68 – 3.10 m



Borehole 23-03 – Run 3 – 3.10 – 4.27 m



HIGHWAY 69 AND REHABILITATION OF SHAWANAGA RIVER BRIDGE
Photographs of Rock Core

Borehole 23-04 – Runs 1 and 2 – 1.98 – 5.18 m



Top of Run 1

Run 3 Continues

Start of Run 2

Start of Run 3

Start of Run 4

APPENDIX E

Site Photographs



Photograph 1: East shoulder of Highway 69 over Culvert 25, looking north.



Photograph 2: Culvert 25 inlet, east embankment slope looking north.



Photograph 3: Culvert 25 outlet, west embankment slope looking south.



Photograph 4: Borehole 23-03 after completion.



Photograph 5: Borehole 23-04 after completion.