



FOUNDATION INVESTIGATION AND DESIGN REPORT

**Highway 66, Station 16+994, Township of Lebel
Culvert Replacement
Ministry of Transportation, Ontario
GWP 5210-14-00**

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PART A

FOUNDATION INVESTIGATION REPORT
HIGHWAY 66, STA 16+994, TOWNSHIP OF LABEL
CULVERT REPLACEMENT
MINISTRY OF TRANSPORTATION, ONTARIO
GWP 5210-14-00

1.0 INTRODUCTION

Golder Associates Ltd. (Golder) has been retained by AECOM Canada Ltd. (AECOM) on behalf of the Ministry of Transportation, Ontario (MTO) to provide foundation engineering services related to the replacement of the culvert on Highway 66 at Station 16+994, in the Township of Lebel, approximately 1.2 km east of Bidgood Road near King Kirkland, Ontario. The Key Plan of the general location of this section of Highway 66 and the location of the investigated area are shown on Drawing 1.

The purpose of this investigation is to establish the subsurface conditions at the culvert replacement site by borehole drilling with laboratory testing carried out on selected soil samples.

The Terms of Reference (TOR) and the scope of work for the foundation investigation are outlined in MTO's Request for Proposal, dated February 2018, and the subsequent clarifications/addenda, which forms part of the Consultant's Assignment Number 5017-E-0039 for this project. The work has been carried out in accordance with Golder's Supplementary Specialty Plan for foundation engineering services for this project dated November 2018.

2.0 SITE DESCRIPTION

It should be noted that the orientation (i.e., north, south, east, west) stated in the text of the report is typically referenced to project north and therefore may differ from magnetic north shown on Drawing 1. For the purpose of this report, Highway 66 is oriented in a west-east direction with the culvert positioned perpendicular to the highway generally in a north-south orientation. At the culvert location, the creek flows in a south to north direction.

The existing culvert consists of a 800 mm diameter, 28 m long Corrugated Steel Pipe (CSP). The culvert invert from AECOM's centreline survey profile is approximately Elevation 321.4 m. In general, the topography within the vicinity of the culvert consists of forested hills with exposed bedrock near the site. The culvert is located about 100 m south of Mud Lake, on a curve of the highway and the highway grade at the culvert centreline is Elevation 327.3 m. The embankment is approximately 6 m high relative to the culvert invert and the embankment/side slopes appear to be performing well, with no visible signs of slope instability or roadway settlement issues. The ground surface conditions at select locations near the culvert area are shown on Photographs 1 to 3.

3.0 INVESTIGATION PROCEDURES

Field work for this subsurface exploration was carried out between May 8 and 28, 2019, during which time five boreholes (Boreholes C227-1 to C227-5) were advanced at the approximate locations shown on Drawing 1. Boreholes C227-1 to C227-3 were advanced through the roadway embankment and Borehole C227-4 was advanced near the south toe of the highway embankment adjacent to the culvert inlet using a track mounted CME-55LC drilling rig supplied and operated by George Downing Estate Drilling (Downing) of Grenville-Sur-La-Rouge, Quebec. Borehole C227-5 was advanced near the north toe of the highway embankment adjacent to the culvert outlet using a portable tripod rig supplied and operated by Landcore Drilling (Landcore) of Chelmsford, Ontario. Traffic control, where required, was performed in accordance with MTO's Ontario Traffic Control Manual Book 7 – Temporary Conditions.

Boreholes C227-1 to C227-4 were advanced using 76 mm I.D. Hollow Stem Augers, NW casing with wash boring techniques, and NQ coring; and Borehole C227-5 was advanced using NW casing with wash boring techniques. Two Dynamic Cone Penetration Tests (DCPT) were conducted adjacent to Borehole C227-5 to confirm refusal to

split spoon sampling in the borehole. Soil samples were obtained in the boreholes at 0.75 m and 1.5 m intervals of depth using 50 mm outer diameter split-spoon samplers driven by an automatic or cathead hammer in accordance with the Standard Penetration Test (SPT) procedure (ASTM D1586). The groundwater level inside the augers/casing was observed during the drilling operations. The boreholes were backfilled upon completion in accordance with Ontario Regulation 903 (wells) as amended. The roadway surface at the boreholes drilled through Highway 66 were capped at ground surface using cold patch asphalt.

Field work was supervised on a full-time basis by a member of Golder's technical staff who: located the boreholes in the field; arranged for the clearance of underground services; supervised the drilling and sampling operations; logged the boreholes; and examined the soil samples. The soil samples were identified in the field, placed in labelled containers and transported to Golder's geotechnical laboratory in Sudbury for further examination and laboratory testing. Index and classification testing consisting of water content determinations, grain size distributions, Atterberg limits, and organic content was carried out on selected soil samples. The geotechnical laboratory testing was completed according to ASTM and MTO LS standards, as applicable. One soil sample was submitted to Bureau Veritas Laboratories (formerly Maxxam) of Mississauga, an accredited analytical laboratory, for testing a suite of corrosivity indicator parameters.

The as-drilled borehole locations were measured relative to highway chainages/station marked on the pavement by a member of our technical staff and converted into northing/easting coordinates on the plan drawing. The ground surface elevations at the borehole locations were surveyed by Golder relative to the highway and culvert centreline, with the elevation of the centrelines provided by AECOM. The MTM NAD 83-CSRS CBN v6-2010.0 (Zone 12) northing and easting coordinates, geographical coordinates, ground surface elevations referenced to Geodetic datum, and borehole depths at each borehole location are presented on the borehole records in Appendix A and summarized below.

Borehole Number	MTM NAD 83 Northing (m) (Latitude)	MTM NAD 83 Easting (m) (Longitude)	Ground Surface Elevation (m)	Borehole Depth (m)
C227-1	5335892.7 (48.156968)	383855.5 (-79.937299)	326.2	13.9*
C227-2	5335885.5 (48.156901)	383867.6 (-79.937137)	326.2	14.9*
C227-3	5335888.6 (48.156932)	383847.5 (-79.937407)	325.7	12.1*
C227-4	5335876.9 (48.156827)	383846.3 (-79.937425)	322.8	9.9*
C227-5	5335903.6 (48.157064)	383865.0 (-79.937169)	319.4	3.8 (2 DCPTs to 4.0 m and 4.6 m Depth)

Notes:

*Including coring for lengths between 3.2 m and 3.7 m.

4.0 SITE GEOLOGY AND SUBSURFACE CONDITIONS

4.1 Regional Geology

Based on Northern Ontario Engineering Geology Terrain Study (NOEGTS)¹ mapping, the subsoils in the vicinity of the culvert site are glacially derived ground moraine comprising primarily of till.

Based on geological mapping (MNDM)², the site is underlain by foliated tonalite suite bedrock, or tonalite to granodiorite-foliated to massive bedrock.

4.2 Subsurface Conditions

The detailed subsurface soil and groundwater conditions encountered in the boreholes and the summary results of in situ and laboratory testing are given on the Record of Borehole sheets contained in Appendix A. The plotted results of geotechnical laboratory testing are contained in Appendix B. The results of the in-situ field tests (i.e., SPT 'N' values) as presented on the Record of Borehole sheets and discussed in Section 4.2 are uncorrected. The stratigraphic boundaries shown on the Record of Borehole sheets and on the interpreted stratigraphic profile shown on Drawing 1 are inferred from non-continuous sampling and, therefore, represent transitions between soil types rather than exact planes of geological change. The results of the analytical laboratory testing (by Bureau Veritas Laboratories) are summarized in Section 4.4 and the detailed laboratory testing report is included in Appendix B.

The subsurface conditions will vary between and beyond the borehole locations, however, the factual data presented on the Record of Borehole Sheets governs any interpretation of the site conditions. A summary description of the soil deposits and groundwater conditions encountered in the boreholes is provided below. It should be noted that the interpreted stratigraphy shown on Drawing 1 is a simplification of the subsurface conditions.

4.2.1 Asphalt/Fill

An approximately 170 mm to 180 mm thick layer of asphalt pavement was encountered in the roadway boreholes C227-1 to C227-3 between Elevations 326.2 m and 325.7 m. A 5.2 m to 6.8 m thick layer of embankment fill, consisting of various layers of silty gravelly sand to gravelly sand to sand and gravel and silt to silt and sand to sandy gravelly silt was encountered below the asphalt. Asphalt coated particles were encountered in all three road boreholes between depths of 0.2 m to 0.4 m below the roadway surface. Cobbles and/or boulders were noted as follows at the various roadway boreholes:

- Borehole C227-1: Cobbles from 2.0 m to 2.2 m depth, a 300 mm boulder at 4.0 m depth and cobbles from 5.4 m to 5.6 m depth and cobble from 6.8 m to 7.0 m depth.
- Borehole C227-2: Auger grinding from 0.8 m to 6.1 m depth, with the potential presence of cobbles as inferred from auger grinding.
- Borehole C227-3: 500 mm boulder at 3.3 m depth, cobbles from 4.7 m to 5.0 m depth.

¹ Ontario Ministry of Natural Resources and Forestry. Northern Ontario Engineering Geology Terrain Study. Ontario Geological Society Electronic Mapping. Map 41PNE

² Ontario Ministry of Northern Development and Mines. Bedrock Geology of Ontario, East-Central Sheet. Map 2543

A 1.5 m thick layer of organic silty sandy fill was encountered from ground surface in Borehole C227-4 at Elevation 322.8 m. Borehole C227-5 encountered a 50 mm thick layer of topsoil from ground surface at Elevation 319.4 m, underlain by 1.4 m of silty sand fill containing cobbles for the fill zone from 0.7 m to 1.5 m depth.

The SPT “N”-values measured within the non-cohesive fill range between 3 blows to 110 blows per 0.3 m of penetration with blow counts up to 100 blows per 0.15 m of penetration on cobbles/boulders, indicating a loose to very dense compactness condition.

Grain size distribution testing was carried out on: four samples of sand and gravel to silty gravelly sand fill and the results are presented on Figure B-1 in Appendix B; two samples of the sandy gravelly silt to silt and sand fill and the results are presented on Figure B-2; and one sample of the silt fill and the results are presented on Figure B-3.

The natural moisture content measured on four samples of the sand and gravel to silty gravelly sand fill ranges from 6 per cent and 19 per cent. The natural moisture content measured on the two samples of the sandy gravelly silt to silt and sand fill is 19 per cent and 24 per cent. The natural moisture content of a sample of sandy silt fill is 32 per cent. An Atterberg limits test was carried out on a sample of the silt fill and the result was non-plastic. An organic content test was carried on a sample of the sand and gravel to silty gravelly sand fill and the result is 3.0 per cent organics.

4.2.2 Sand and Gravel Till

A 2.3 m to 4.7 m thick deposit of sand and gravel till was encountered underlying the fill in all five boreholes between Elevations 321.3 m and 317.9 m. Cobbles were encountered within the till deposit in Boreholes C227-2 and C227-3 and boulders (500 mm and 600 mm size) were encountered in Borehole C227-3. Borehole C227-5 was terminated within the till deposit at 3.8 m depth due to split spoon refusal after penetrating 2.3 m into the deposit; two DCPTs were advanced adjacent to the borehole and refusal to further penetration was encountered at depths of 4.0 m and 4.6 m below ground surface after penetrating 2.5 m and 3.1 m into the deposit (inferred from the adjacent borehole).

The SPT “N”-values measured within the till deposit range from 18 blows to 64 blows per 0.3 m of penetration with “N” values up to 80 blows for 0.08 m of penetration, indicating that the deposit has a compact to very dense compactness condition.

Grain size distribution analysis was carried out on four samples of the till deposit and the results are presented on Figure B-4 in Appendix B. The natural moisture content measured on samples of the till deposit ranges from 9 per cent to 13 per cent.

4.2.3 Bedrock

Bedrock was encountered underlying the sand and gravel till in Borehole C227-1 to C227-4 at depths ranging from 6.2 m to 11.0 m below ground surface, between Elevations 317.0 m and 314.7 m and between 3.2 m and 3.7 m of diorite bedrock was cored. The retrieved bedrock cores are described as fine to medium grained, fresh, grey diorite, as described on the Record of Drillholes presented in Appendix A. Photographs of the retrieved bedrock core samples are shown on Figure B-5A and B-5B in Appendix B. The Total Core Recovery (TCR) of the bedrock samples is 100 per cent. The Rock Quality Designation (RQD) of the bedrock samples ranges between

91 per cent and 100 per cent and based on the Quality Classification from Table 3.10 of CFEM 2006³, the bedrock quality is considered excellent.

4.3 Groundwater Conditions

The unstabilized groundwater levels relative to ground surface measured inside the casing or augers upon completion of drilling are summarized below. The creek was dry at the time of the subsurface exploration. Groundwater and creek water levels in the area are subject to seasonal fluctuations and variations due to precipitation events.

Borehole No.	Depth to Unstabilized Groundwater Level (m)	Approximate Groundwater Elevation (m)
C227-1	5.2	321.0
C227-2	5.5	320.7
C227-3	4.3	321.4
C227-4	0.6	322.2
C227-5	0.7	318.7

4.4 Analytical Laboratory Testing Results

Analytical testing was carried out on a sample of the silt and sand fill recovered from Borehole C227-1 (Sample #7). The soil sample was submitted to Bureau Veritas Laboratories of Mississauga, Ontario for corrosivity testing. The analytical laboratory test results are summarized below, and the detailed analytical laboratory test report is included in Appendix B.

Borehole No.	Borehole Sample No.	Depth (m)	Parameters					
			Resistivity (ohm-cm)	Electrical Conductivity (µmho/cm)	Soluble Sulphate (SO ₄) Content (µg/g)	Chloride (Cl) Content (µg/g)	Sulphide (µg/g)	pH
C227-1	7	6.1 – 6.7	2,500	405	<20 ¹	250	<0.30 ²	7.00

Notes:

1. The sulphate concentration is below the reportable detection limit of 20 µg/g.
2. The sulphide was below the reportable detection limit of 0.30 µg/g.

³ Canadian Geological Society, 2006. Canadian Foundation Engineering Manual, 4th Edition.

5.0 CLOSURE

The field drilling program was carried out under the supervision of Mr. Mathew Riopelle and Yusuf Soliman, EIT, under the overall direction of Mr. André Bom, P.Eng., an Associate of Golder. This Foundation Investigation Report was prepared by Mr. Gavin Mundry, and Mr. André Bom provided a technical review of the report. Mr. Jorge Costa, P.Eng., an MTO Foundations Designated Contact and Senior Consultant for Golder, conducted an independent quality control review of this report.

Signature Page

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PART B
FOUNDATION DESIGN REPORT
HIGHWAY 66, STA 16+994, TOWNSHIP OF LABEL
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6.0 DISCUSSION AND ENGINEERING RECOMMENDATIONS

This section of the report provides foundation design recommendations for the replacement of the culvert crossing Highway 66 at about Station 16+994, Township of Lebel, Ontario, approximately 1.2 km east of Bidgood Road near King Kirkland, Ontario. These recommendations are based on interpretation of the factual data obtained from the boreholes advanced during the current subsurface exploration. The discussion and recommendations presented are intended to provide the designer with sufficient information to assess feasible foundation alternatives and culvert types and to design the proposed replacement culvert. This foundation investigation and design report, discussion and recommendations are intended for the use of the Ministry of Transportation, Ontario (MTO) and shall not be used or relied upon for any other purpose or by any other parties, including the construction or design-build contractor. The contractor must make their own interpretation based on the factual data in Part A (Foundation Investigation) of the report. Where comments are made on construction, they are provided to highlight those aspects that could affect the design of the project, and for which special provisions may be required in the Contract Documents. Those requiring information on the aspects of construction must make their own interpretation of the factual information provided as such interpretation may affect equipment selection, proposed construction methods, scheduling and the like.

6.1 Proposed Culvert Alignment and Installation Options

The existing culvert to be replaced consists of a 0.8 m diameter, 28 m long Corrugated Steel Pipe (CSP). Based on the drawings provided by AECOM via email on July 2, 2019, and our site observations during the foundation exploration work, the existing culvert crosses the existing Highway 66 embankment on a perpendicular alignment; and we understand from AECOM that the proposed replacement culvert will cross the highway on or near the same alignment and will be of similar length and diameter as the existing culvert. The existing embankment at the culvert location is sloped at about 2H:1V to 4H:1V on the south side and at about 2H:1V on the north side and is between about 5.1 m and 6.0 m high relative to the culvert invert at the inlet (south end) and outlet (north end). The invert at the inlet and outlet of the existing culvert is about Elevation 320.9 m and 320.2 m, respectively. Based on documentation supplied by AECOM (Culvert Inspection Report) and our site observations, the existing culvert appears to be in good condition with minor rusting at the bottom. There were no visual signs of embankment or pavement distress in the immediate vicinity of the culvert at the time of the foundation investigation field work. We understand from AECOM that a temporary roadway protection system is being considered for staging of the culvert replacement in open cut, and that a permanent grade raise or widening of the roadway embankment is not required. We further understand that a full road closure and temporary detour is not being considered for traffic staging during culvert replacement operations.

Depending on construction staging and as an alternative to open cut construction of the new culvert, consideration may also be given to replacing the culvert using a trenchless method. Based on the existing road surface profile along the proposed culvert alignment, the existing soil cover over the top of the culvert is about 5.0 m, and the ratio of existing cover-to-pipe diameter is about 6. The replacement culvert is also proposed to be 0.8 m in diameter, resulting in a soil cover-to-tunnel diameter ratio of about 5.0, if an approximate 1 m diameter liner is used, which is typical for many trenchless installation methods. The actual soil cover to culvert diameter ratio will depend on the installation method and size of liner actually used.

A comparison between an open cut (cut and cover) method of culvert installation and the preferred trenchless installation method at this site is presented in Table 1 following the text of this report.

6.2 Consequence and Site Understanding Classification

In accordance with Section 6.5 of the *Canadian Highway Bridge Design Code* (CHBDC, 2014) and its *Commentary*, Highway 66 and culvert / foundation system are expected to carry medium traffic volumes and their performance will have potential impacts on other transportation corridors; hence, the culvert foundation system is classified as having a “typical consequence level” associated with exceeding limits states design. In addition, given the typical project-specific foundation investigation carried out at this site (as presented in Part A of the report), in comparison to the degree of site understanding in Section 6.5 of *CHBDC* (2014), the level of confidence for design is considered to be a “typical degree of site and prediction model understanding.” Accordingly, the appropriate corresponding ultimate limit state (ULS) and serviceability limit state (SLS) consequence factor, Ψ , and geotechnical resistance factors, ϕ_{gu} and ϕ_{gs} , from Tables 6.1 and 6.2 of the *CHBDC* have been used for design, as applicable.

6.3 Circular Culvert Installation by Open Cut Excavation

6.3.1 Settlement and Stability

Settlement of the foundation soils beneath the culvert is not anticipated to be a concern.

The proposed reconstructed embankment (after culvert replacement) is considered to be stable from a slope stability perspective if it is reconstructed of granular material and with side slopes at an inclination of 2 horizontal to 1 vertical (2H:1V) or flatter.

6.3.2 Bedding / Embedment and Cover

Pipe culverts (less than 3 m diameter) should be designed in general accordance with the MTO Gravity Pipe Design Guidelines (2014). It is not practical or necessary to found a non-structural pipe culvert below the depth of frost penetration, as pipe culverts are generally tolerant of small magnitudes of movement related to freeze-thaw cycles.

A circular, concrete pipe culvert installed by open cut method should be completed in accordance with Ontario Provincial Standard Drawing (OPSD) 802.031 (*Rigid Pipe Bedding, Cover and Backfill*). If the replacement culvert is to consist of a Corrugated Steel Pipe (CSP) or plastic (HDPE or PVC) pipe installed by open cut method, it should be constructed in accordance with OPSD 802.010 (*Flexible Pipe Embedment and Backfill*) for Type 3 Soil.

All unsuitable, deleterious, organic materials and fill materials are to be removed from the base/below the culvert (and bedding) footprint along its entire alignment. Based on the foundation exploration and existing/proposed culvert invert levels, the subgrade will generally consist of compact to very dense sand and gravel (till) and/or loose to very dense silty sand to silt and sand (fill) and is considered suitable for support of the culvert and bedding materials. The bedding should be 300 mm thick and be compatible with the type/class of pipe material, the surrounding subsoil and anticipated loading conditions and should consist of OPSS.PROV 1010 (Aggregates) Granular ‘A’ material. Depending on the time of year and success of the contractor’s surface water diversion / groundwater control methods, and the quality of the bearing stratum exposed at the base of the excavation, a thicker bedding layer may be required if wet and softened soil conditions, unsuitable fill, or organic material are present at the base of the excavation.

From the top of the bedding to 300 mm above the top of the culvert, Granular ‘A’ should be used around the culvert. All bedding, embedment and cover materials should be placed, and culvert construction carried out in accordance with OPSS.PROV 421 (*Pipe Culvert Installation in Open Cut*) and OPSS 401 (*Trenching, Backfilling*

and Compacting), and the bedding/embedment/cover soil should be compacted in accordance with OPSS.PROV 501 (*Compacting*) as amended by Special Provision (SP) 105S22. If the bottom of the excavation is wet and surface water diversion / dewatering is not satisfactorily maintaining the water level sufficiently below the base of the excavation to allow compaction, it is recommended that OPSS.PROV 1010 (*Aggregates*) Granular 'B' Type II material be used for bedding and as additional sub-excavation backfill below the bedding, as may be required.

6.3.2.1 Trench Backfill

The excavated embankment fill materials from the culvert site will vary in quality and composition, and are predominantly comprised of silt, sandy gravelly silt, silt and sand, and sand and gravel with local deposits of cobbles and boulders. The excavated embankment fills are generally considered suitable for backfill to re-instate the embankment provided the target moisture contents and compaction levels are achieved and provided cobbles / boulders larger than 150 mm in dimension are removed. Silty soils within the depth of frost penetration should be avoided and the existing organic fill materials should not be reused as backfill for reconstruction of the highway embankment in the immediate vicinity or over the new culvert.

Alternatively, granular material which meets the requirements of OPSS.PROV 1010 (*Aggregates*) Select Subgrade Material (SSM) or Granular 'B' Type I may be used as trench backfill.

These materials should also be placed and compacted in accordance with OPSS.PROV 421 (*Pipe Culvert Installation in Open Cut*), OPSS 401 (*Trenching, Backfilling and Compacting*) and OPSS.PROV 501 (*Compacting*), as amended by SP 105S22.

6.4 Assessment of Trenchless Installation Methods

6.4.1 Subsurface Conditions and Tunnelman's Ground Classification

We understand that consideration may be given to replacement of the culvert by a trenchless method. The soil conditions encountered along the proposed culvert alignment are relatively consistent, and generally the soils encountered along the proposed tunnel horizon (vertical culvert liner alignment) include loose to very dense sandy gravelly silt, silt and sand, and sand and gravel (with cobbles and boulders) embankment fill. On the south side of the tunnel horizon, the dense to very dense gravelly sand to sand and gravel (till) deposit may also be encountered. The groundwater level measured inside the hollow stem auger casing on completion of drilling Boreholes C227-1, C227-4, and C227-5 (nearest to the existing culvert) ranges between Elevation 322.2 m and 318.7 m. Therefore, the groundwater level is expected to be slightly above or within the proposed culvert/tunnel horizon, especially on the southern half of the plan alignment.

Correlating the soil classification noted above with the Tunnelman's Ground Classification System (Heuer, 1974, modified from Terzaghi, 1950), the existing embankment fill along the tunnel horizon (i.e., sandy gravelly silt, silt and sand, and sand and gravel) and the relatively uniform, native granular soils (sand and gravel till) can be considered "cohesive running" to "running" above the groundwater level and "flowing" below the groundwater level. It is expected that these soils, when exposed fully above groundwater or after dewatering, may be able to stand unsupported for a limited length of time prior to "running" or exhibiting "cohesive running" behaviour (in the order of a few minutes to hours). Below groundwater levels, the existing embankment fill and native soils will be unable to stand unsupported for any length of time and will exhibit "flowing" behaviour and destabilize any overlying materials.

6.4.2 General Description of trenchless Technologies

The contractor should be responsible for choosing the method and equipment for the crossing trenchless installation, unless specific methods are otherwise prohibited. Ground behaviour will be, in part, dependent on the installation method adopted by the contractor, however this report provides guidance on the influence of ground behaviour on some possible pipe culvert/tunnel installation methods. It should not be construed that the contractor is restricted to the particular methods considered herein, and in the event of alternative methods, the contractor must make his own interpretation of the anticipated ground behaviour, based on the factual information from the investigation. Trenchless work should be carried out in accordance with MTO's Non-Standard Special Provision (NSSP) titled "Pipe Installation by Trenchless Method" presented in Appendix C.

For the proposed culvert installation for this project, a number of trenchless installation methods were considered for completeness, though the practicality of some of these techniques for this site may be doubtful if not entirely unsuitable. Given the diameter of the new culvert is about 0.8 m, the trenchless techniques considered from a general perspective include horizontal directional drilling (HDD), horizontal auger boring – Jack and Bore, pipe ramming, micro-tunneling by MTBM, and pilot tube micro-tunneling (PTMT). In brief, these construction methods involve the following:

- **Horizontal Directional Drilling (HDD):** HDD involves drilling of a relatively small diameter pilot hole (on the order of 100 to 150 mm) using a remotely controlled and steerable drill bit on a flexible string of drill rods, while the bore is supported using a bentonite slurry. Once the pilot hole is complete, the bore is typically reamed in one or more passes to a larger diameter, and then the final pipe is pulled through the bore (using the drill rods to pull the pipe into place). HDD equipment is available for drilling in both bedrock and overburden, but drilling is very challenging in bouldery ground. Deep entrance and exit pits are generally not required; however, larger laydown areas are required to install the final pipe, and the crossing typically needs to be longer to accommodate the shallow entry and exit angles for the drilling equipment (on the order of 10 to 30 degrees from horizontal). Bores are typically limited to less than 1 m in diameter. Control of line and grade to the degree needed for gravity flow at shallow storm or sanitary sewer slopes may be problematic. Given the geometry of the site and set-up required for HDD, this option is not considered to be practical at this site.
- **Horizontal Auger Boring – "Jack and Bore":** In Ontario, a traditional "jack and bore" operation involves pushing a steel pipe (casing) horizontally into the ground by jacking while simultaneously cutting the ground with an auger head operating near the leading end of the steel pipe. The spoil is generally removed from within the casing using an auger boring machine. The cutting head is driven by, and is positioned at, the leading end of an auger string that is established within the casing pipe. Jacking and receiving pits are required. Typically, there is limited ability to steer the casing during jacking. This method is only applicable to construction in soils and may not be feasible in bouldery soils (e.g., glacial till). In some cases, contractors will run the auger cutting head in front of the lead end of the casing to advance the pipe in difficult ground; however, this approach can lead to high risks for ground losses (settlement, sinkholes). This method is also not feasible in running or flowing ground (dry or saturated sand and silt).

In some cases, traditional "jack and bore" equipment is supplemented with a specialized rotating cutting head, sometimes referred to as a "small boring unit". These cutting heads are welded to the lead end of steel casings, can sometimes include limited alignment adjustment capabilities, and can be fitted with rock disc cutters. In the right ground conditions (e.g., hard glacial till, weathered rock), the small boring heads can be advantageous. However, these systems are not well suited to and should not be used in saturated and potentially flowing ground conditions. Further, these systems should not be confused with microtunnelling systems that operate using very different principles of ground support. Given that flowing / running soils are

anticipated to be encountered at this site jack and bore is considered to be high risk option, and although it may be considered as marginally feasible with adequate controls / equipment, is not recommended.

- **Pipe Ramming:** Pipe ramming uses a pneumatic tool to hammer a steel pipe or casing into the ground. The pipe is almost always driven “open” to thereby direct the soil into the pipe interior instead of compacting it outside the pipe. The leading edge of the pipe typically has a small overcut to reduce friction between the casing and soil and to improve the load conditions on the pipe. Soil/pipe friction reduction can also be achieved with lubrication, and different types of bentonite and/or polymers can be used for this purpose. Depending on the length of the installation, the soils inside the pipe can be removed either during or after the installation by augering, compressed air or water jetting. Pipe ramming methods are also better suited for penetrating through/displacing potential obstructions, such as cobbles and boulders in comparison to jack and bore installation method, though this method can still be obstructed by cobbles and boulders depending on their size, number, and their positions relative to the pipe leading edge. Pipe ramming has also been used to accomplish culvert replacement in which a larger diameter pipe is rammed through the ground immediately around an existing pipe and both the existing pipe and ground are removed from within the rammed pipe by manual methods. This technique is sometimes called “pipe eating” or “pipe swallowing.” Partial or full removal of materials from within the pipe, to facilitate driving, should not be carried out if the ground through which the pipe is being driven consists of saturated granular soils (silt, sand, gravel). As with traditional jack and bore methods, flowing ground conditions and/or operating the cleanout augers beyond, at or near the leading edge of the casing can result in significant ground losses, excessive surface settlement and, in some cases, sinkholes that propagate to the surface. Given that cohesionless saturated soils are anticipated and the total run length is about 25 m, pipe ramming is considered to be a feasible option at this site.
- **Micro-tunnelling Boring Machine (MTBM):** MTBM is a method of installing pipes in bores ranging from 0.6 to 3 metres in diameter behind a steerable remote-controlled shield that is pressurized with a bentonitic slurry at the cutting face to minimize ground losses. The process is essentially remote-controlled pipe jacking where all operations are controlled from the surface, cuttings are removed by the circulating slurry, and the necessity for personnel to enter the bore is eliminated. Micro-tunnelling equipment is generally more suited to tunnelling through overburden. Availability of this equipment in the project area is limited. Some MTBMs are promoted as being able to “crush” cobbles with internal cone crushing systems. Others have been promoted as capable of passing boulders of as much as 1/3 of the bore diameter. However, both approaches to managing larger stones can be highly problematic and incapable of completing construction in boulder ground. Large numbers of cobbles can also “choke” these machines and result in failure of the bore. In bouldery ground, where the boulders can be firmly held in place by the surrounding soil matrix, equipping MTBMs with rock-disc cutters can be successful. In all cases, detailed review of the conditions and equipment configuration are needed prior to construction to achieve a reasonable probability of success. At this site, a MTBM is considered feasible; however, the short length of the run and presence of loose cohesionless soils combined with cobbles / boulders is considered to be a high risk that may limit steerability, advancement, and will create challenges in designing appropriate slurry viscosity to avoid inadvertent returns through the gravelly embankment fill.
- **Pilot Tube Micro-tunnelling (PTMT):** PTMT employs augers for excavation and soil removal and a jacking system for advancing the drill pipes, casings and final pipes. The guidance system comprises a target with LEDs mounted in the steering head of the equipment that is monitored through a TV monitor. The PTMT operation includes pilot boring and reaming; and since this technique is used for smaller size pipes, the equipment and space required for this operation is smaller than what is normally required for pipe jacking or microtunnelling. PTMT can obtain an accuracy of 10 mm per 100 m of pipe length; however, the accuracy

depends on the ground conditions, the accuracy of the guidance system, and the operator's skill. The "pilot tube" is advanced in a similar fashion to horizontal directional drilling with a guidance system used to control alignment and grade.

In this method, a bore hole is drilled with a steering head connected to pilot tubes whose size is smaller than the required casing size. A steering head is used for pilot boring and adjustment of alignment and grade, and the bore hole is subsequently enlarged by a reamer with an auger string inside the casing used to remove cuttings. Temporary casings, if applicable, or the final pipe follows the reamer into the ground. Configurations of "reamer" tools varies widely within the industry, with some including rotating cutting tools, while others are a simplified cage-like head that allows soils to be forced into the openings as the larger diameter pipe is pulled and pushed into the ground. These reamer systems can have a significant influence on both the feasibility and risks of using this method and should be evaluated with caution.

6.4.3 Assessment for Feasible Installation Methods

Based on the ground conditions at the culvert site and the anticipated relatively short tunnel distance of about 27.5 m, HDD and PTMT methods are likely not suitable and are not considered to be cost-effective. The cobbles and boulders (i.e., as encountered in Boreholes C227-1, C227-3, and C227-5) that may be present along the tunnel horizon would likely preclude the use of HDD, traditional "jack and bore," and possibly MTBM. The use of pipe ramming is likely to be the most feasible and suitable method for the culvert installation in the anticipated ground conditions. It should be noted that interventions to remove obstructions may be necessary. The following geotechnical issues/risks associated with the trenchless construction that should be considered and evaluated at this site are:

- Ground or Road Heave – culverts installed at shallow depth and with diameters that are relatively large as compared to the depth of burial are particularly susceptible to heaving the roadway if the pipe is rammed into place – i.e., having a cover to diameter (C/D) ratio of less than about 2, which is not the case at this site given the 1 m diameter liner required to accommodate the 0.75 m diameter culvert and 5.0 m thickness of soil cover.
- Obstructions – Cobbles and/or boulders were encountered within the embankment fill and native soil deposits in Boreholes C227-1, C227-2, C227-3, and C227-5 at this site. It should be noted that cobbles and boulders can obstruct or foul trenchless construction equipment if they are encountered while boring. As the culvert would be installed close to or along the interface between the original ground surface and the overlying roadway embankment there is a risk of encountering such obstructions, as well as wood, depending on how the original ground was cleared, stripped, and grubbed prior to embankment placement. In some instances, low and wet areas have historically been filled with larger rock materials in attempts to stabilize the ground. For culvert sites that have such cases, pipe ramming is considered to be the least susceptible to equipment fouling and damage if wood debris is encountered.
- Ground or Road Settlement – Given that the water level at this site would likely be at or below the culvert invert at the time of construction, there is the potential for flow of saturated granular soils (silt/silt and sand, gravelly silty sand) and groundwater back through the spoils within the casing and towards the entrance pit. Such flow could cause significant loss of ground at and above the face of excavation. For any method that requires groundwater control for managing risks of ground losses, gravity flow to sumps and pumps or into permeable linings should not be relied upon except as a supplement to a fully designed vacuum well point or eductor system. For pipe ramming, groundwater control for trenchless operations may not be required provided sufficient soil plug is maintained within the casing; however, temporary dewatering may be required if an obstruction is encountered and access to the face is required to progress the trenchless installation.

The trenchless methods that are to be considered not suitable at this site have been identified in the NSSP for Pipe Installation by Trenchless Methods, included in Appendix C.

As a general guideline, the depth of cover above the crown of the new pipe installation should be greater than or equal to the cut diameter (and ideally at a ratio of cover to culvert diameter greater than 2) of whatever trenchless system is used to excavate the ground or the largest pipe diameter that will be installed, whichever is greater. Similarly, the separation between newly installed pipes should be at least one, and preferably two tunnel diameters, in both the vertical and horizontal directions. Oversize casings (if separate casing and pipes will be used) or oversize final pipes should be installed to permit final adjustments of the invert channel elevations for final flow control given the challenges associated with maintaining alignment. Selection of casing size should consider the potential for mis-alignment over the tunnel length due to ground conditions (i.e., cobbles/boulders), access to the tunnel face (if potentially necessary), proposed tunneling methodology and the length of the tunnel drive. We understand from AECOM that a 2 pass-system is not typical for centreline culverts in MTO's Northeast Ontario. However, a 2-pass lining may be advantageous to facilitate some methods of tunnelling and for achieving final alignment control, depending on the final hydraulic opening and lining design requirements.

Settlement monitoring of the trenchless crossing (refer to Section 6.6.8) should be carried out over the entire centreline length of the new culvert alignment at the edge of all pavements, in landscape areas leading to the pavement from the entry pit and at perpendicular off-sets of about 2 m from the centre line at all pavement shoulder edges for a distance equal to the depth from road surface to invert as per the SP for Pipe Installation by Trenchless Method. Also, a settlement monitoring program should be implemented that is consistent with OPSS.PROV 539 (Temporary Protection Systems) for shoring systems at the pits if these are near the roadway, as discussed further in Section 6.6.3.

6.5 Analytical Testing of Existing Soil

The results of analytical tests on one sample of the silt and sand fill, recovered in Borehole C227-1 is summarized in Section 4.4. The potential for sulphate attack and corrosion are discussed in the following paragraphs; however, it is ultimately up to the designer to determine the appropriate construction materials, including the exposure class, and ensuring that all aspects of CSA A23.1-14 (2014) Section 4.1.1 "Durability Requirements" are followed when designing concrete elements. The culvert should be designed with consideration given to Table 7.1 of the MTO Gravity Pipe Design Guidelines (2014).

6.5.1 Potential for Sulphate Attack

The analytical test results were compared to CSA A23.1-14 Table 3 ("Additional requirements for concrete subjected to sulphate attack") for the potential sulphate attack on concrete. The water soluble-sulphate concentration measured in the soil sample is less than the reportable detection limit of 0.002 per cent, which is below the exposure class of S-3 (Moderate) and is considered Negligible according to Table 7.2 in the MTO Gravity Pipe Design Guidelines (2014). Therefore, based on the test result for the sample, when the designer is selecting the exposure class for the culvert/structure, the effects of sulphates from within the near surface/culvert invert native soil(s) may not need to be considered. However, as the culvert will extend under the roadway shoulders and be exposed to de-icing salt, concrete should be designed for a "C" type exposure class as defined by CSA A23.1-14 Table 1.

6.5.2 Potential for Corrosion

The soil has a pH of 7.0 and according to the MTO Gravity Pipe Design Guidelines (2014), the pH is not considered detrimental to culvert durability. The resistivity is 2,500 ohm-cm, which indicates that the soil corrosiveness is Moderate ($2,000 > R > 4,500$ ohm-cm), as per Table 3.2 of the MTO Gravity Pipe Design Guidelines (2014). It is also noted that sulphide concentration is below the reportable detection limit of 0.30 µg/g in the analyzed test sample; and sulphide is considered very corrosive to cast iron/steel materials (Cashman and Preene, 2001).

6.6 Construction Considerations

6.6.1 Open Cut Excavation

The proposed open cut excavation through the embankment and into the subgrade to the base of the culvert bedding level associated with the removal of the existing culvert and construction of the new culvert by open cut and re-construction of the embankment, will advance through the granular embankment fill and into a sand and gravel till deposit, and the excavation is anticipated to extend to or below the groundwater level. Where space permits for an open cut excavation into these materials, the excavation must be carried out in accordance with the guidelines outlined in the Occupational Health and Safety Act (OHSA) for Construction Activities in Ontario. Above the water table, the existing fill materials are classified as Type 3 soil, according to OHSA and temporary excavations (i.e., those which are open for a relatively short time period) should be made with side slopes no steeper than 1 horizontal to 1 vertical (1H:1V). Below the water table the existing fill materials and underlying native soils are classified as Type 4 soil according to OHSA, and temporary excavations (i.e., those which are open for a relatively short time period) into this soil type should be made with side slopes no steeper than 3 horizontal to 1 vertical (3H:1V).

Depending upon the construction procedures adopted by the contractor, groundwater seepage conditions, and weather conditions at the time of construction, some local flattening of the slopes of open cut excavations may be required, especially in looser/softer zones or where localized seepage is encountered. Further, layering of soils and the effectiveness of the contractor's surface diversion / dewatering systems could affect the OHSA classification and, therefore, the classification of soils for OHSA purposes must be made at the time the excavation is open and can be directly observed during construction.

6.6.2 Groundwater / Surface Water Control

The groundwater level is expected to be at or above the proposed culvert along most of its alignment; the excavation for the culvert replacement should be expected to extend below the groundwater level. The groundwater should be lowered to at least 1 m below the base of the excavation to maintain basal stability and allow for construction in dry conditions. Groundwater may be controlled by providing an active dewatering system consisting of an adequate number of sumps and pumps installed and operated in advance/during the excavation, or in combination with temporary support systems such as a sheet piling wall and/or cofferdam (as required).

The contractor is responsible for the assessment of dewatering requirements, which depends on their chosen method of open cut excavation for replacement, as well as on the method and procedure for construction/operation/maintenance and decommissioning. The contractor is also responsible for confirming that the radius of groundwater drawdown does not impact the existing embankment and any surrounding features. The design of dewatering, unwatering, and temporary flow passage system is the responsibility of the contractor.

Surface water should be directed away from open excavation areas to prevent ponding of water that could result in disturbance and weakening of the subgrade and/or affect construction or trenchless operations, as applicable. Depending on the water flow through the watercourse at the time of construction and staging/diversion requirements/limitations, temporary cofferdams may also be required.

Groundwater and surface water control will be required for excavation and construction of the culvert replacement and for any trenchless option being considered. Dewatering operations must be in accordance with OPSS.PROV 517 (*Dewatering*) as amended by SP 517F01 (*Dewatering System/Temporary Flow Passage System*), recommending that a design engineer be required to carry out the design of the system. The return period flow estimates in Table A of SP 517F01 (included in Appendix C) shall be filled-in by the hydraulic design engineer. Given the loosening potential of the gravelly silty sand subgrade at this site, the Designer Engineer fill-in for Note 1 in Table A shall indicate “yes”. Given the apparent lack of infrastructure present in the vicinity of the culvert, a preconstruction survey is not considered to be required at this site and the fill-in for Note 2 should be N/A.

If construction dewatering is required and the quantity of water taken is between 50,000 litres/day and 400,000 litres/day under normal operations, an Environmental Activity and Sector Registry (EASR) is required in accordance with Part II.2 of the *Environmental Protection Act (EPA or Act)*, *Ontario Regulation (O. Reg.) 245/11* (Registrations Under II.2 of the Act – General) and *O. Reg. 63/16* (Registrations under II.2 of the Act – Water Taking). The need for an EASR should be evaluated by the contractor, given the construction methods/operations and groundwater control system, in advance of construction operations to minimize any potential impacts on the construction schedule.

6.6.3 Temporary Protection Systems

In order to replace the existing culvert and allow at least one lane of live traffic to pass during construction, temporary protection systems will likely be required. Temporary protection systems may also be required for entry / exit pits if trenchless installation is considered. The temporary excavation protection and support systems shall be designed and constructed in accordance with OPSS.PROV 539 (*Temporary Protection Systems*) and SP 105S09. The lateral movement of the protection systems shall meet Performance Level 2 as specified in OPSS.PROV 539, provided that any utilities, if present, can tolerate this magnitude of deformation.

It is anticipated that a driven interlocking steel sheet pile system may not be suitable at this site due to the presence of cobbles and boulders within the embankment fill, dense to very dense glacial till soils containing cobbles/boulders. Pre-drilling and/or removal of obstructions may be required if a steel sheet pile system is considered. The contractor may use a soldier pile and lagging system with piles socketed into the glacial till / bedrock as required. Pre-drilling for soldier piles may be required and dewatering may be necessary in the lower portion of the wall prior to installation of the lagging boards as the cohesionless fills and saturated silt deposit will not have adequate stand-up time to permit installation of the lagging boards.

The sheet piles or soldier piles will need to extend to a sufficient depth to provide the necessary passive resistance for the retained soil height, plus any surcharge loads behind the protection system. Lateral support to the sheet pile wall or soldier pile wall could be provided in the form of struts, rakers, or temporary anchors, if and as required.

Vibratory equipment for the installation of temporary protection systems may be used at this site provided that it does not impact the embankment or nearby buried infrastructure or structures, if present. The installation of temporary protection systems by vibratory equipment should be monitored to ensure the vibration levels produced

by such construction activity are within tolerable limits and in consultation with the infrastructure/utility and property owners within the zone of influence of the site.

While the selection and design of the temporary protection system will be the responsibility of the contractor, the following information is provided to MTO and its designers to aid in the assessment of feasible alternatives.

Stratigraphic Unit	Bulk Unit Weight, γ (kN/m ³)	Angle of Internal Friction, ϕ (degrees)	Undrained Shear Strength, s_u (kPa)	Lateral Earth Pressure Coefficients ^{1/2}		
				Passive, K_p^3	Active, K_a	At-rest, K_o
Embankment Fill – Compact to very dense sand and gravel	20	32	-	3.25	0.31	0.47
Embankment Fill – Loose to very dense sandy gravelly silt to silt to silt and sand	19	29	-	2.88	0.35	0.52
Compact to very dense sand and gravel till	20	34	-	3.54	0.28	0.44

Notes:

1. The design groundwater level may be assumed to be at Elevation 322.5 m and 321.0 m near the inlet and outlet respectively, based on the ground surface and water levels in the boreholes.
2. The lateral earth pressure coefficients presented above are based on a horizontal surface adjacent to the excavation. If sloped surfaces are expected, the coefficients should be corrected accordingly.
3. The total passive resistance below the base of the excavation (i.e., adjacent to the temporary protection system) may be calculated based on the values of K_p indicated above but reduced by an appropriate factor that considers the allowable wall movement in accordance with Figure C6.16 of the CHBDC (2014) to account for the fact that a large strain would be required for mobilization of the full passive resistance.

It is recommended that the ground surface extending back/upwards from the top of the protection system to the existing Highway 66 surface be graded to an inclination no steeper than 2 horizontal to 1 vertical (2H:1V). This should be shown on the Contract staging drawings.

The loading from construction equipment as well as any material stockpiles within a distance defined by a 1 horizontal to 1 vertical line drawn from the bottom of the excavation to the existing ground surface should be included as a surcharge in the design of the temporary protection system.

Consideration could be given to either partial or full removal of the temporary protection system upon completion of construction or each stage of construction (as required). Vibration and noise controls during extraction of any temporary systems should meet the same tolerable limits used for installation.

6.6.4 Obstructions

Cobbles and/or boulders were encountered or interred throughout the embankment fill in Boreholes C227-1, C227-2, C227-3, and C227-5 and in the native sand and gravel till soils in Boreholes C227-2 and C227-3 during the drilling exploration. The presence of these obstructions could affect the excavation and installation of temporary protection systems. There is also the potential for the presence of wood debris and organic material (as encountered in the silty sandy organic and silty sand fill materials in Boreholes C227-4 and C227-5), including roots and tree stumps, at the interface of fill and native soils under the existing embankment, due to possible poor stripping practices during the embankment construction. A Notice to Contractor to identify the presence of cobbles

and boulders within the fill and native soils and wood/trees near the fill / native soil interface, should be included in the Contract Documents; a copy of which is included in Appendix C.

6.6.5 Subgrade Protection

For open cut culvert installation, the subgrade soils will be susceptible to disturbance from construction traffic and/or ponded water. To limit this degradation, it is recommended that the granular bedding layer be placed immediately after preparation and approval of the subgrade.

6.6.6 Embankment Reconstruction

Engineered fill for reconstruction of the embankment after open cut culvert replacement should consist of OPSS.PROV 1010 (Aggregates) Granular 'A' or Granular 'B' Type I or Type II material. The embankment fill should be placed and compacted in accordance with OPSS.PROV 501 (Compacting) as amended by special Provision (SP) 105S22 and OPSS.PROV 206 (Grading). Embankment side slopes should be constructed no steeper than 2 Horizontal to 1 Vertical (2H:1V) in granular fill. Consideration can be given to using existing embankment fill materials provided all excessive organic material, deleterious material, and cobbles/boulders greater than 150 mm in dimension are removed and moisture contents are suitable for adequate compaction. In addition, any frost susceptible soils should not be used within the depth of frost penetration.

6.6.7 Surficial Embankment Stability and Erosion Protection

If the culvert is replaced by open cut method, depending on the selected embankment reconstruction fill material type, slope geometry, surface treatment and weather conditions (i.e., precipitation, cycles of wetting-drying and/or freezing-thawing), surficial instability of the embankment side slopes may occur, which could include localized sloughing and erosion. As such, in order to maintain the integrity of the new embankments, erosion protection measures may be required depending on the fill type selected for construction.

Based on the specified material types and hence the gradation envelope, granular fill such as OPSS.PROV 1010 (Aggregates) Granular 'A', Granular 'B' Type I or Type II, or the existing sand and gravel fill have a low potential for erosion. For embankments constructed of granular fill, erosion control may be limited to seeding following the construction specifications of OPSS 802 (Topsoil) and OPSS.PROV 804 (Seed and Cover). On-going maintenance for embankments constructed of this material is not expected to be required once the vegetation has been established.

The specification for OPSS.PROV 1010 (Aggregates) SSM allows for much more variation in the gradation of the material compared to Granular 'A', or Granular 'B' Type I or Type II, and therefore has the potential to be low - erodible to moderate - erodible. Erosion protection for slopes constructed of SSM or the existing silt and sand fill should consist of erosion control blankets and seeding. Slopes constructed of SSM and properly protected from erosion should require limited on-going maintenance.

Inlet / outlet erosion and scour protection requirements surrounding the new culvert and associated design details should be provided by the hydrotechnical engineer.

6.6.8 Instrumentation and Monitoring Program

An instrumentation and monitoring program is recommended at the culvert location if a trenchless installation method is to be used for culvert construction to:

- Document the effects of the culvert installation on the overlying highway and adjacent underground utilities/services, if applicable.
- Potentially identify adverse ground movement trends that could occur due to the construction methods and equipment or unforeseen ground conditions.
- Evaluate the contractor's compliance with the settlement limits specified in the Contract Documents.
- Allow adjustments to be made to the culvert installation methods such that the settlement limits established are not exceeded.

The SP for "Pipe Installation by Trenchless Method" (Appendix C) contains the details of the settlement monitoring program to be implemented to measure ground settlement at the existing roadway prior to, during and following the proposed installation. If a trenchless installation method is adopted, a site-specific Supply/Installation of Instrumentation and a Settlement Monitoring Plan should be prepared for the contractor and Contract Administrator assignment for inclusion in the Contract Documents/Provided to the CA. The proposed "Settlement Monitoring Instrument Location Plan" and "Settlement Monitoring Instrumentation Details" are shown on Drawings 2 and 3, attached.

It is also recommended that, to the extent practicable and possible, the weight or volume of ground removed from beneath the highway be measured and compared to the theoretical cut hole volume on a frequency of at least once per 3 m section of tunnel / pipe installed. On-site observations of construction operations and measurements of grout and/or lubricant volumes should assist in identifying atypical conditions that could be indicative of unacceptable ground losses.

Provision should be included in the Contract Documents for rehabilitation of the Highway 66 embankment along the culvert alignment in the event of excessive settlement during the installation process. It is understood from AECOM that granular driving restrictions will govern timing of paving such that if settlement occurs, repairs would be performed during the Contract.

Further, the location (depth/alignment), type and tolerances to movement and vibrations of any existing buried utilities (functioning or decommissioned) would have to be clearly established prior to any trenchless installation operation, and the Review Level and Alert Level tolerances for settlement revised and/or confirmed in the "Pipe Installation by Trenchless Method" NSSP.

6.6.9 Grouting

Post installation grouting to fill the annular space between the carrier pipe (culvert) and the casing may need to be carried out after the permanent culvert pipe is installed within the casing as noted in the NSSP for "Pipe Installation by Trenchless Method" included in Appendix C. For any installations when the settlement monitoring or excavation volume monitoring indicates that excessive pavement settlement or ground loss has occurred, or where signs of ground loss have been noted, provision should also be made for a program of compensation grouting above / surrounding the casing pipe and/or repair of the pavements.

In addition, provision should be made in the Contract to grout / fill the abandoned CSP culvert during construction activities.

7.0 CLOSURE

This foundation design report was prepared by Mr. Tibor Berecz, a geotechnical EIT with Golder, and the technical aspects were reviewed by Mr. André Bom, P.Eng., a senior geotechnical engineer and Associate of Golder. Mr. Kevin J. Bentley, P.Eng., and Jorge Costa, P.Eng., a Senior Consultant, an MTO Foundations Designated Contacts with Golder conducted an independent and quality control review of the report.

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- Ontario Provincial Standard Drawings (OPSD)
- | | |
|--------------|---|
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| OPSD 802.031 | Rigid Pipe Bedding, Cover, And Backfill, Type 3 Soil - Earth Excavation |
- Ontario Provincial Standard Specifications (OPSS) – Provincial Oriented
- | | |
|----------------|---|
| OPSS.PROV 206 | Construction Specification for Grading |
| OPSS.PROV 401 | Construction Specification for Trenching, Backfilling and Compacting |
| OPSS.PROV 421 | Construction Specification for Pipe Culvert Installation in Open Cut |
| OPSS.PROV 501 | Construction Specification for Compacting |
| OPSS.PROV 517 | Construction Specification for Dewatering of Pipeline, Utility, and Associated Structure Excavation |
| OPSS.PROV 539 | Construction Specification for Temporary Protection Systems |
| OPSS 802 | Construction Specification for Topsoil |
| OPSS.PROV 804 | Construction Specification for Seed and Cover |
| OPSS.PROV 1010 | Material Specification for Aggregates – Base, Subbase, Select Subgrade and Backfill Material |
- Special Provisions
- | | |
|-----------|-----------------------|
| SP 105S22 | Amendment to OPSS 501 |
| SP 105S09 | Amendment to OPSS 539 |

Table 1: Culvert Replacement Alternatives – Highway 66 Station 16+994 Township of Lebel

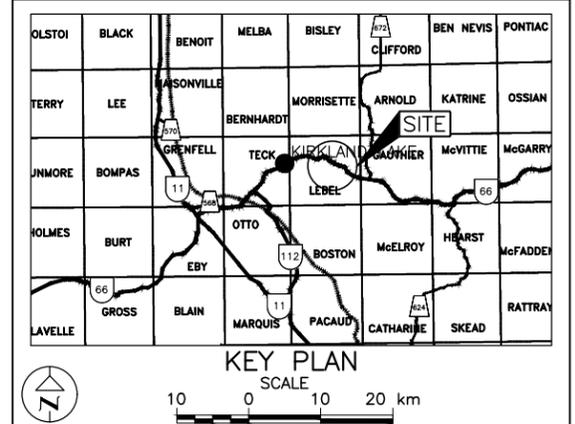
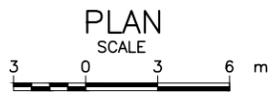
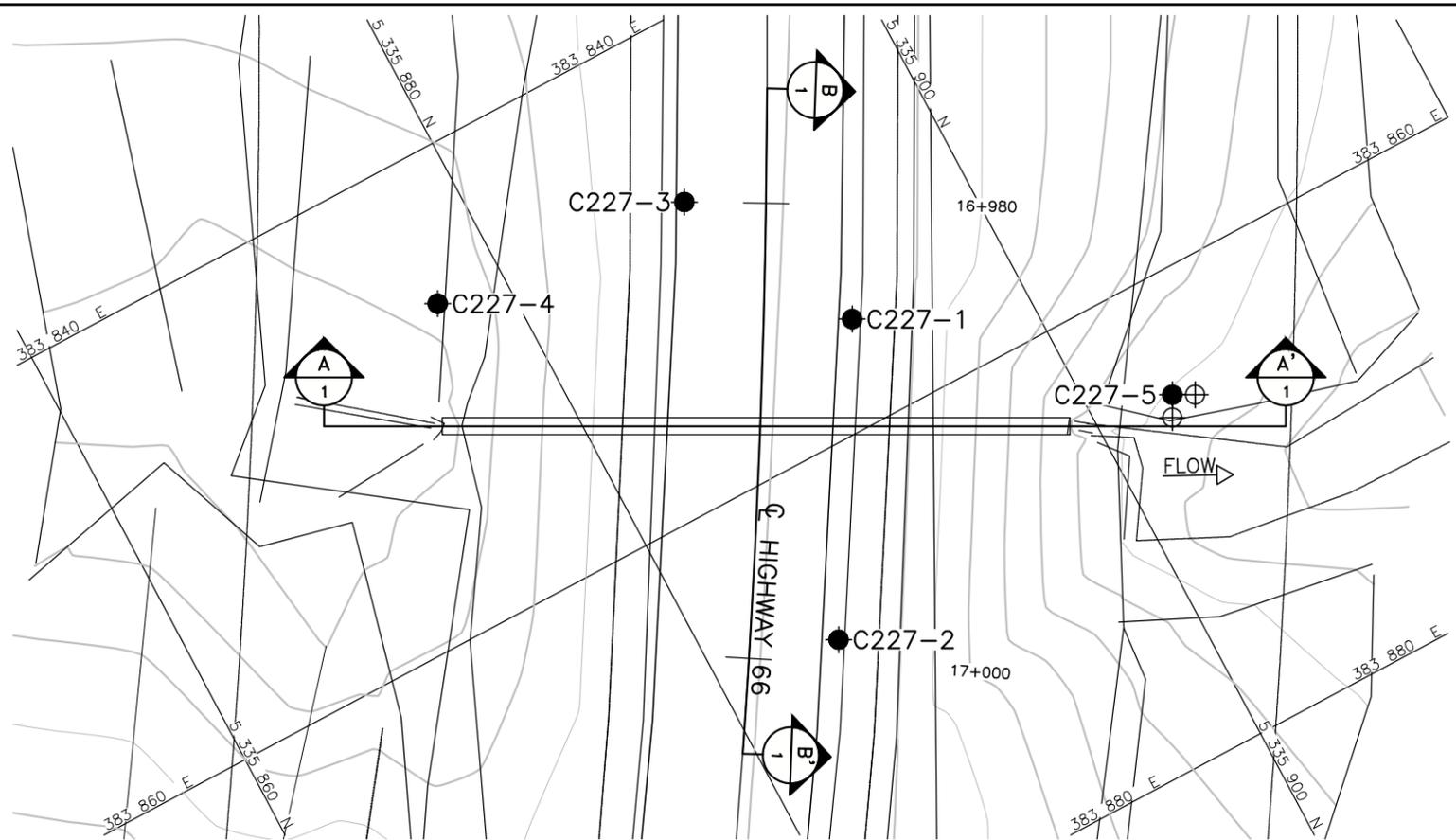
Replacement Alternatives	Advantages	Disadvantages	Risks
Trenchless - Pipe Ramming	<ul style="list-style-type: none"> ■ Installed without significant removal of soils prior to full casing penetration through embankment. ■ Relatively low cost. ■ Relatively small site operations footprint. ■ Within size range typical in Ontario. ■ Better than other low-cost technologies for penetrating ground that potentially includes limited number of cobbles and small boulders. 	<ul style="list-style-type: none"> ■ Density of ground in some areas may encourage premature removal of soils from within the casing. ■ Concentrations of cobbles and boulders or large boulders could obstruct / slow operations. ■ Vibrations from pipe driving can lead to densification and settlement of loose granular materials surrounding and overlying pipe and result in settlement of roadway surface. ■ Alignment control can be difficult when penetrating soils of differing densities, when encountering cobbles and boulders or when mixed soil and rock exist within the alignment. ■ Any voids / loosening of embankment soils caused by corrosion / poor performance of existing CSP will not be addressed. 	<ul style="list-style-type: none"> ■ Obstruction refusal may require removal of soil to expose face or shaft excavation from the ground/highway surface to progress advancement. ■ Need for removal of soil from within casings because of driving resistance (binding of casings, weight of spoil, obstructions) could lead to excess ground losses and surface settlement.
Cut and Cover	<ul style="list-style-type: none"> ■ Depth of temporary excavation is within typical limits for conventional excavation support, slopes and dewatering systems. ■ With appropriate planning and execution, excavation and temporary shoring methods can address potential for cobbles / boulders or other obstructions ■ Any potential voids / loosening of soil in existing embankment due to inadequate performance of the existing CSP culvert will be addressed. ■ Allows for easy removal of cobbles and/or boulders from the embankment fill materials and native soil deposits as encountered presented in Boreholes C227-1 to C227-3 and C224-5. 	<ul style="list-style-type: none"> ■ Traffic disruption (staging of crossing). ■ Roadway protection required for staged excavation. ■ Advanced dewatering prior / during excavation may be required in silty soils. ■ Pre-drilling / removal of cobbles/boulders and/or socket into bedrock may be required to advance temporary protection system. 	<ul style="list-style-type: none"> ■ Difficulties / delays installing roadway protection given cobbles/boulders in embankment fill and native soils. ■ Traffic staging challenges (e.g., close proximity to temporary protection system and minimum offset of temporary concrete barriers, seasonal construction, etc.).

METRIC
 DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN. STATIONS IN KILOMETRES + METRES.

CONT No. GWP No. 5210-14-00

HIGHWAY 66
 STATION 16+994
 TOWNSHIP OF LABEL CULVERT
 BOREHOLE LOCATIONS AND SOIL STRATA

SHEET

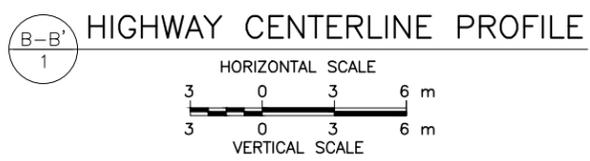
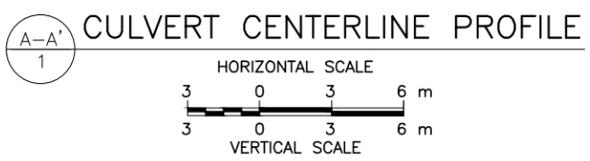
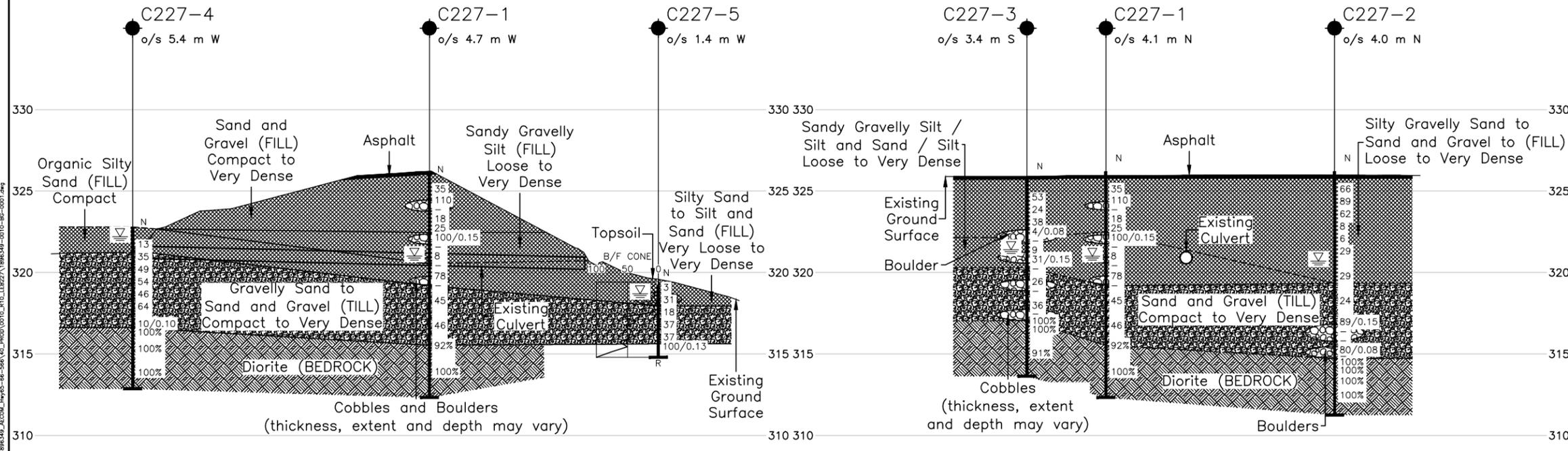


LEGEND

- Borehole - Current Investigation
- ⊕ Dynamic Cone Penetration Test - Current Investigation
- N Standard Penetration Test Value
- 16 Blows/0.3m unless otherwise stated (Std. Pen. Test, 475 j/blow)
- R Refusal
- 100% Rock Quality Designation (RQD)
- ∇ WL upon completion of drilling

BOREHOLE CO-ORDINATES (NAD 83 MTM ZONE 12)

No.	ELEVATION	NORTHING	EASTING
C227-1	326.2	5335892.7	383855.5
C227-2	326.2	5335885.5	383867.6
C227-3	325.7	5335888.6	383847.5
C227-4	322.8	5335876.9	383846.3
C227-5	319.4	5335903.6	383865.0



NOTES

This drawing is for subsurface information only. The proposed structure details/works are shown for illustration purposes only and may not be consistent with the final design configuration as shown elsewhere in the Contracts Documents.

The boundaries between soil strata have been established only at borehole locations. Between boreholes the boundaries are assumed from geological evidence.

REFERENCE

Base plans provided in digital format by CALLON DIETZ LTD. drawing file no. gwp521001400a.dwg, received AUGUST 14, 2019.

NO.	DATE	BY	REVISION

Geocres No. 32D-24

HWY. 66	PROJECT NO. 1896349	DIST. .
SUBM'D.	CHKD. TB	DATE: 2/7/2020
DRAWN: TR	CHKD. AB	APPD. JMAC
		DWG. 1



Photograph 1: Drilling Area for Borehole C227-4, South of Hwy 66, Facing East (May 2019)



Photograph 2: South end of culvert (inlet) (May 2018)



Photograph 3: Portable Tripod Setup at Borehole C227-5, North of Hwy 66 North Culvert Outlet (May 2019)

APPENDIX A

Record of Boreholes

**ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS
MINISTRY OF TRANSPORTATION, ONTARIO**

PARTICLE SIZES OF CONSTITUENTS

Soil Constituent	Particle Size Description	Millimetres	Inches (US Std. Sieve Size)
BOULDERS	Not Applicable	>300	>12
COBBLES	Not Applicable	75 to 300	3 to 12
GRAVEL	Coarse	19 to 75	0.75 to 3
	Fine	4.75 to 19	(4) to 0.75
SAND	Coarse	2.00 to 4.75	(10) to (4)
	Medium	0.425 to 2.00	(40) to (10)
	Fine	0.075 to 0.425	(200) to (40)
FINES	Classified by plasticity	<0.075	< (200)

MODIFIERS FOR SECONDARY COMPONENTS^{1,2}

Percentage by Mass	Modifier
> 35	Use 'and' to combine primary and secondary component (<i>i.e.</i> , SAND and gravel)
> 20 to 35	Primary soil name prefixed with "gravelly, sandy" as applicable
> 10 to 20	some (<i>i.e.</i> , some sand)
≤ 10	trace (<i>i.e.</i> , trace fines)

- Only applicable to components not described by Primary Group Name.
- Classification of Primary Group Name based on Unified Soil Classification System (ASTM D2487) for coarse-grained soils; fine-grained soils described per current MTO Soil Classification System.

PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.). Values reported are as recorded in the field and are uncorrected.

Cone Penetration Test (CPT)

An electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q_t), porewater pressure (u) and sleeve friction (f_s) are recorded electronically at 25 mm penetration intervals.

Dynamic Cone Penetration Resistance (DCPT); N_d:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

- PH:** Sampler advanced by hydraulic pressure
PM: Sampler advanced by manual pressure
WH: Sampler advanced by static weight of hammer
WR: Sampler advanced by weight of sampler and rod

SAMPLES

AS	Auger sample
BS	Block sample
CS	Chunk sample
DD	Diamond Drilling
DO or DP	Seamless open ended, driven or pushed tube sampler – note size
DS	Denison type sample
GS	Grab Sample
MC	Modified California Samples
MS	Modified Shelby (for frozen soil)
RC / SC	Rock core / Soil core
SS	Split spoon sampler – note size
ST	Slotted tube
TO	Thin-walled, open – note size (Shelby tube)
TP	Thin-walled, piston – note size (Shelby tube)
WS	Wash sample
OD / ID	Outer Diameter / Inner Diameter
HSA / SSA	Hollow-Stem Augers / Solid-Stem Augers

SOIL TESTS

w	water content
PL , w _p	plastic limit
LL , w _L	liquid limit
C	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test ¹
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
D _R	relative density (specific gravity, G _s)
DS	direct shear test
GS	specific gravity
M	sieve analysis for particle size
MH	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO ₄	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V (FV)	field vane (LV-laboratory vane test)
Y	unit weight

- Tests anisotropically consolidated prior to shear are shown as CAD, CAU.

COARSE-GRAINED SOILS

Compactness¹

Term	SPT 'N' (blows/0.3m) ²
Very Loose	0 to 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	> 50

- Definition of compactness terms are based on SPT 'N' ranges as provided in Terzaghi, Peck and Mesri (1996). Many factors affect the recorded SPT 'N' value, including hammer efficiency (which may be greater than 60% in automatic trip hammers), overburden pressure, groundwater conditions, and grain size. As such, the recorded SPT 'N' value(s) should be considered only an approximate guide to the soil compactness. These factors need to be considered when evaluating the results, and the stated compactness terms should not be relied upon for design or construction.
- SPT 'N' in accordance with ASTM D1586, uncorrected for the effects of overburden pressure.

FINE-GRAINED SOILS

Consistency

Term	Undrained Shear Strength (kPa)	SPT 'N' ^{1,2} (blows/0.3m)
Very Soft	< 12	0 to 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 200	15 to 30
Hard	> 200	> 30

- SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.
- SPT 'N' values should be considered ONLY an approximate guide to consistency; for sensitive clays (e.g., Champlain Sea clays), the N-value approximation for consistency terms does NOT apply. Rely on direct measurement of undrained shear strength or other manual observations.

Field Moisture Condition

Term	Description
Dry	Soil flows freely through fingers.
Moist	Soils are darker than in the dry condition and may feel cool.
Wet	As moist, but with free water forming on hands when handled.

LITHOLOGICAL AND GEOTECHNICAL ROCK DESCRIPTION TERMINOLOGY

WEATHERINGS STATE

Fresh: no visible sign of weathering

Faintly weathered: weathering limited to the surface of major discontinuities.

Slightly weathered: penetrative weathering developed on open discontinuity surfaces but only slight weathering of rock material.

Moderately weathered: weathering extends throughout the rock mass but the rock material is not friable.

Highly weathered: weathering extends throughout rock mass and the rock material is partly friable.

Completely weathered: rock is wholly decomposed and in a friable condition but the rock and structure are preserved.

BEDDING THICKNESS

<u>Description</u>	<u>Bedding Plane Spacing</u>
Very thickly bedded	Greater than 2 m
Thickly bedded	0.6 m to 2 m
Medium bedded	0.2 m to 0.6 m
Thinly bedded	60 mm to 0.2 m
Very thinly bedded	20 mm to 60 mm
Laminated	6 mm to 20 mm
Thinly laminated	Less than 6 mm

JOINT OR FOLIATION SPACING

<u>Description</u>	<u>Spacing</u>
Very wide	Greater than 3 m
Wide	1 m to 3 m
Moderately close	0.3 m to 1 m
Close	50 mm to 300 mm
Very close	Less than 50 mm

GRAIN SIZE

<u>Term</u>	<u>Size*</u>
Very Coarse Grained	Greater than 60 mm
Coarse Grained	2 mm to 60 mm
Medium Grained	60 microns to 2 mm
Fine Grained	2 microns to 60 microns
Very Fine Grained	Less than 2 microns

Note: * Grains greater than 60 microns diameter are visible to the naked eye.

CORE CONDITION

Total Core Recovery (TCR)

The percentage of solid drill core recovered regardless of quality or length, measured relative to the length of the total core run.

Solid Core Recovery (SCR)

The percentage of solid drill core, regardless of length, recovered at full diameter, measured relative to the length of the total core run.

Rock Quality Designation (RQD)

The percentage of solid drill core, greater than 100 mm length, recovered at full diameter, measured relative to the length of the total core run. RQD varied from 0% for completely broken core to 100% for core in solid sticks.

DISCONTINUITY DATA

Fracture Index

A count of the number of discontinuities (physical separations) in the rock core, including both naturally occurring fractures and mechanically induced breaks caused by drilling.

Dip with Respect to Core Axis

The angle of the discontinuity relative to the axis (length) of the core. In a vertical borehole a discontinuity with a 90° angle is horizontal.

Description and Notes

An abbreviation description of the discontinuities, whether naturally occurring separations such as fractures, bedding planes and foliation planes or mechanically induced features caused by drilling such as ground or shattered core and mechanically separated bedding or foliation surfaces. Additional information concerning the nature of fracture surfaces and infillings are also noted.

Abbreviations

JN Joint	PL Planar
FLT Fault	CU Curved
SH Shear	UN Undulating
VN Vein	IR Irregular
FR Fracture	K Slickensided
SY Stylolite	PO Polished
BD Bedding	SM Smooth
FO Foliation	SR Slightly Rough
CO Contact	RO Rough
AXJ Axial Joint	VR Very Rough
KV Karstic Void	
MB Mechanical Break	

RECORD OF BOREHOLE No C227-1 1 OF 2 **METRIC**

PROJECT 1896349 W.P. 5210-14-00 LOCATION N 5335892.7; E 383855.5 NAD83 MTM ZONE 12 (LAT. 48.156968; LONG. -79.937299) ORIGINATED BY MR

DIST HWY 66 BOREHOLE TYPE 76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring COMPILED BY GM

DATUM GEODETIC DATE May 11 to 12, 2019 CHECKED BY AB

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)					
ELEV. DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	SHEAR STRENGTH kPa			WATER CONTENT (%)
											○ UNCONFINED + FIELD VANE			○ 3% STRAIN AT FAILURE			GR	SA	SI	CL	
326.2	GROUND SURFACE																				
0.0	ASPHALT (170 mm)																				
0.2	Sand and gravel (FILL) Compact to very dense Grey to brown Wet																				
	Asphalt coated particles from 0.3 m to 0.4 m depth		1	SS		35															
			2	SS		110															
	- Two cobbles encountered from 2.0 m to 2.2 m depth		-	RC		-															
			3	SS		18															
			4	SS		25															
322.5																					
3.7	Sandy gravelly silt, trace clay (FILL) Loose to very dense Brown Wet		5	SS		100/0.15															
	- 300 mm diameter boulder encountered at 4.0 m		-	RC		-															
			6	SS		8															29 27 41 3
	- Two cobbles encountered from 5.4 m to 5.6 m depth		-	RC		-															
320.6																					
5.6	Silt and sand, some gravel, trace clay (FILL) Very dense Brown Wet		7	SS		78															9 48 41 2
	- Cobble encountered from 6.8 m to 7.0 m		-	RC		-															
319.2																					
7.0	SAND and GRAVEL, some silt, trace clay (TILL) Dense Brown Wet		8	SS		45															41 48 10 1
			9	SS		46															
315.5																					
10.7	DIORITE (BEDROCK)																				
	For coring details see Record of Drillhole C227-1.		1	RC		REC 100%															RQD = 92%

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Continued Next Page

 +³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>1896349</u>	RECORD OF BOREHOLE No C227-1	2 OF 2	METRIC
W.P. <u>5210-14-00</u>	LOCATION <u>N 5335892.7; E 383855.5 NAD83 MTM ZONE 12 (LAT. 48.156968; LONG. -79.937299)</u>	ORIGINATED BY <u>MR</u>	
DIST <u> </u> HWY <u>66</u>	BOREHOLE TYPE <u>76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring</u>	COMPILED BY <u>GM</u>	
DATUM <u>GEODETIC</u>	DATE <u>May 11 to 12, 2019</u>	CHECKED BY <u>AB</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	SHEAR STRENGTH kPa
--- CONTINUED FROM PREVIOUS PAGE ---							○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × REMOULDED					WATER CONTENT (%)						
							20	40	60	80	100	20	40	60				
312.3	DIORITE (BEDROCK)	1	RC	REC 100%		314											RQD = 92%	
13.9	For coring details see Record of Drillhole C227-1.	2	RC	REC 100%		313											RQD = 100%	
13.9	END OF BOREHOLE																	
	NOTE:																	
	1. Water level at a depth of 5.2 m below ground surface (Elev. 321.0 m) upon completion of drilling.																	

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+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT: 1896349
 LOCATION: N 5335892.7; E 383855.5
 NAD83 MTM ZONE 12 (LAT. 48.156968; LONG. -79.937299)
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: C227-1

SHEET 1 OF 1
 DRILLING DATE: May 12, 2019
 DATUM: GEODETIC

DRILL RIG: CME55 LC Track Mount
 DRILLING CONTRACTOR: Downing Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR FLUSH	RECOVERY		R.Q.D. %	FRACT. INDEX METRES	DISCONTINUITY DATA				HYDRALLIC CONDUCTIVITY		Diametral Point Load Index (MPa)	RMC -Q AVG.	
							TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION		Jr	Ja	Jun	k, cm/s			σ _p
							BD - Bedding	PL - Planar			PO - Polished	BR - Broken Rock							
		GROUND SURFACE		315.5															
11	NW	DIORITE Fine to medium grained Fresh Grey		10.7	1	Grey 100													
12																			
13	NQ Coaming May 12, 2018				2	Grey 100													
14		END OF DRILLHOLE		312.3 13.9															
15																			
16																			
17																			
18																			
19																			
20																			
21																			
22																			

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RECORD OF BOREHOLE No C227-2 1 OF 2 **METRIC**

PROJECT 1896349

W.P. 5210-14-00 LOCATION N 5335885.5; E 383867.6 NAD83 MTM ZONE 12 (LAT. 48.156901; LONG. -79.937137) ORIGINATED BY MR

DIST HWY 66 BOREHOLE TYPE 76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring COMPILED BY GM

DATUM GEODETIC DATE May 8 and 10, 2019 CHECKED BY AB

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	20
326.2	GROUND SURFACE																	
0.0	ASPHALT (180 mm)																	
0.2	Silty gravelly sand to sand and gravel (FILL) Loose to very dense Brown to dark brown Moist to wet Asphalt coated particles from 0.3 m to 0.4 m depth Auger grinding from 0.8 m to 6.1 m depth																	
			1	SS	66													
			2	SS	89							○						46 41 (13)
			3	SS	62													
			4	SS	8													
			5	SS	6													
			6	SS	29							○						26 51 (23)
						▽												
	- Trace organics in Sample 7		7	SS	29							○						OC=3.0% 39 40 18 3
319.3	SAND and GRAVEL, some silt, trace clay (TILL) Compact to very dense Grey Wet																	
6.9			8	SS	24													
			9	SS	89/0.15													
	- Approximately 600 mm diameter boulder encountered from 9.5 m to 10.1 m depth		-	RC	-													
	- Cobbles encountered from 10.1 m to 10.4 m depth		-	RC	-													
			10	SS	80/0.08													
	- Approximately 500 mm diameter boulder encountered from 11.0 m to 11.5 m depth		-	RC	-													
314.7	DIORITE (BEDROCK)		1	RC	REC 100%													
11.5																		RQD = 100%

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Continued Next Page

 +³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>1896349</u>	RECORD OF BOREHOLE No C227-2	2 OF 2	METRIC
W.P. <u>5210-14-00</u>	LOCATION <u>N 5335885.5; E 383867.6 NAD83 MTM ZONE 12 (LAT. 48.156901; LONG. -79.937137)</u>	ORIGINATED BY <u>MR</u>	
DIST <u></u> HWY <u>66</u>	BOREHOLE TYPE <u>76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring</u>	COMPILED BY <u>GM</u>	
DATUM <u>GEODETIC</u>	DATE <u>May 8 and 10, 2019</u>	CHECKED BY <u>AB</u>	

ELEV. DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100					
	--- CONTINUED FROM PREVIOUS PAGE ---																
	DIORITE (BEDROCK) For coring details see Record of Drillhole C227-2.	[Hatched Pattern]	1	RC			314										RQD = 100%
			2	RC	REC 100%												RQD = 100%
			3	RC	REC 100%		313										RQD = 100%
			4	RC	REC 100%		312										RQD = 100%
311.3 14.9	END OF BOREHOLE NOTE: 1. Water level at a depth of 5.5 m below ground surface (Elev. 320.7 m) upon completion of drilling.																

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+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT: 1896349
 LOCATION: N 5335885.5; E 383867.6
 NAD83 MTM ZONE 12 (LAT. 48.156901; LONG. -79.937137)
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: C227-2

SHEET 1 OF 1
 DATUM: GEODETIC

DRILLING DATE: May 10, 2019
 DRILL RIG: CME55 LC Track Mount
 DRILLING CONTRACTOR: Downing Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR	FLUSH	RECOVERY		R.Q.D. %	FRACT. INDEX METRES	DISCONTINUITY DATA				HYDRALLIC CONDUCTIVITY		Diametral Point Load Index (MPa)	RMC -Q AVG.	
								TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION		Jr	Ja	Jun	k, cm/s			σ _p
								80	85			90	95	100	0	1	2			3
		GROUND SURFACE		314.7																
	NW	DIORITE Fine to medium grained Fresh Grey		11.5	1	Grey	100													
12					2	Grey	100													
13					3	Grey	100													
14					4	Grey	100													
15	NQ Coring May 10, 2018	END OF DRILLHOLE		311.3 14.9																

SUD-MTO-RCK S:\CLIENTS\MTO\HWY65&66\02_DATA\GINTV\1896349.GPJ GAL-MISS.GDT 9-13-19_TR



RECORD OF BOREHOLE No C227-3 1 OF 2 **METRIC**

PROJECT 1896349

W.P. 5210-14-00 LOCATION N 5335888.6; E 383847.5 NAD83 MTM ZONE 12 (LAT. 48.156932; LONG. -79.937407) ORIGINATED BY MR

DIST HWY 66 BOREHOLE TYPE 76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring COMPILED BY GM

DATUM GEODETIC DATE May 12, 2019 CHECKED BY AB

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	SHEAR STRENGTH kPa								
						20	40	60	80	100	20	40	60		GR SA SI CL	
325.7	GROUND SURFACE															
0.0	ASPHALT (170 mm)															
0.2	Gravelly sand to sand and gravel (FILL) Compact to very dense Brown Wet Asphalt coated particles from 0.2 m to 0.4 m depth		1	SS	53											
			2	SS	24											
			3	SS	38						o				24 72 (4)	
			4	SS	4/0.08											
322.1	- 500 mm diameter boulder encountered from 3.3 m to 3.8 m depth		-	RC	-											
3.6	Silt, some gravel, some sand, trace clay (FILL) Loose to very dense Brown Wet		5	SS	9						o			NP	11 17 68 4	
			6	SS	31/0.15											
	- Two cobbles encountered from 4.7 m to 5.0 m depth		-	RC	-											
320.3	SAND and GRAVEL (TILL) Compact to dense Brown Wet															
5.4			7	SS	26											
	- Two cobbles encountered from 6.4 m to 6.7 m depth		-	RC	-											
			8	SS	36											
	- Three cobbles encountered from 8.3 m to 8.6 m depth		-	RC	-											
317.0	DIORITE (BEDROCK)		1	RC	REC 100%										RQD = 100%	
8.7	For coring details see Record of Drillhole C227-2.		2	RC	REC 100%										RQD = 100%	
			3	RC	REC 100%										RQD = 91%	

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Continued Next Page

 +³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>1896349</u>	RECORD OF BOREHOLE No C227-3	2 OF 2	METRIC
W.P. <u>5210-14-00</u>	LOCATION <u>N 5335888.6; E 383847.5 NAD83 MTM ZONE 12 (LAT. 48.156932; LONG. -79.937407)</u>	ORIGINATED BY <u>MR</u>	
DIST <u> </u> HWY <u>66</u>	BOREHOLE TYPE <u>76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring</u>	COMPILED BY <u>GM</u>	
DATUM <u>GEODETIC</u>	DATE <u>May 12, 2019</u>	CHECKED BY <u>AB</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV. DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80	100	W _p	W		
313.6 12.1	END OF BOREHOLE NOTE: 1. Water level at a depth of 4.3 m below ground surface (Elev. 321.4 m) upon completion of drilling.	///														

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+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT: 1896349
 LOCATION: N 5335888.6; E 383847.5
 NAD83 MTM ZONE 12 (LAT. 48.156932; LONG. -79.937407)
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: C227-3

SHEET 1 OF 1
 DATUM: GEODETIC

DRILLING DATE: May 12, 2019
 DRILL RIG: CME55 LC Track Mount
 DRILLING CONTRACTOR: Downing Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX METRES	DISCONTINUITY DATA				HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.		
							TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION		Jr	Ja	Jun	k, cm/s	σ ₁			σ ₂	σ ₃
							FLUSH				B Angle	DIP w.r.t. CORE AXIS									
		GROUND SURFACE		317.0																	
9	NW	DIORITE Fine to medium grained Fresh Grey		8.7	1	Grey 100															
10					2	Grey 100															
11					3	Grey 100															
12		END OF DRILLHOLE		313.6																	
13				12.1																	
14																					
15																					
16																					
17																					
18																					
19																					
20																					

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RECORD OF BOREHOLE No C227-4 1 OF 1 **METRIC**

PROJECT 1896349

W.P. 5210-14-00 LOCATION N 5335876.9; E 383846.3 NAD83 MTM ZONE 12 (LAT. 48.156827; LONG. -79.937425) ORIGINATED BY MR

DIST HWY 66 BOREHOLE TYPE 76 mm I.D. Hollow Stem Augers, NW Casing, NQ Coring COMPILED BY GM

DATUM GEODETIC DATE May 12 to 13, 2019 CHECKED BY AB

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	SHEAR STRENGTH kPa									WATER CONTENT (%)
						20	40	60	80	100	20	40	60		GR SA SI CL		
322.8 0.0	GROUND SURFACE Organic silty sand, some gravel (FILL) Compact Black / brown Wet		1	SS	13												
321.3 1.5	Gravelly SAND to SAND and GRAVEL, some silt (TILL) Dense to very dense Brown Wet		2	SS	35												
			3	SS	49							o				29 57 (14)	
			4	SS	54												
			5	SS	46												
			6	SS	64								o			45 49 (6)	
			7	SS	10/0/10												
316.6 6.2	DIORITE (BEDROCK) For coring details see Record of Drillhole C227-3.		1	RC	REC 100%											RQD = 100%	
			2	RC	REC 100%											RQD = 100%	
			3	RC	REC 100%											RQD = 100%	
312.9 9.9	END OF BOREHOLE NOTE: 1. Water level at a depth of 0.6 m below ground surface (Elev. 322.2 m) upon completion of drilling.																

SUD-MTO 001 S:\CLIENTS\MT\TOI\HWY65&66\02_DATA\GINT\1896349.GPJ GAL-MISS.GDT 9-13-19 TR

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT: 1896349
 LOCATION: N 5335876.9; E 383846.3
 NAD83 MTM ZONE 12 (LAT. 48.156827; LONG. -79.937425)
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: C227-4

SHEET 1 OF 1
 DATUM: GEODETIC

DRILLING DATE: May 13, 2019
 DRILL RIG: CME55 LC Track Mount
 DRILLING CONTRACTOR: Downing Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR FLUSH	RECOVERY		R.Q.D. %	FRACT. INDEX METRES	DISCONTINUITY DATA				HYDRALLIC CONDUCTIVITY		Diametral Point Load Index (MPa)	RMC -Q AVG.	
							TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION		Jr	Ja	Jun	k, cm/s			q
							80	80			90	90	90	90	90	90			90
		GROUND SURFACE		316.6															
7	NW	DIORITE Fine to medium grained Fresh Grey		6.2	1	Grey 100													
8	NQ Coring May 13, 2018				2	Grey 100													
9					3	Grey 100													
10		END OF DRILLHOLE		312.9 9.9															

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RECORD OF BOREHOLE No C227-5 1 OF 1 **METRIC**

PROJECT 1896349

W.P. 5210-14-00 LOCATION N 5335903.6; E 383865.0 NAD83 MTM ZONE 12 (LAT. 48.157064; LONG. -79.937169) ORIGINATED BY YS

DIST HWY 66 BOREHOLE TYPE Portable Equipment, NW Casing and Wash Boring COMPILED BY GM

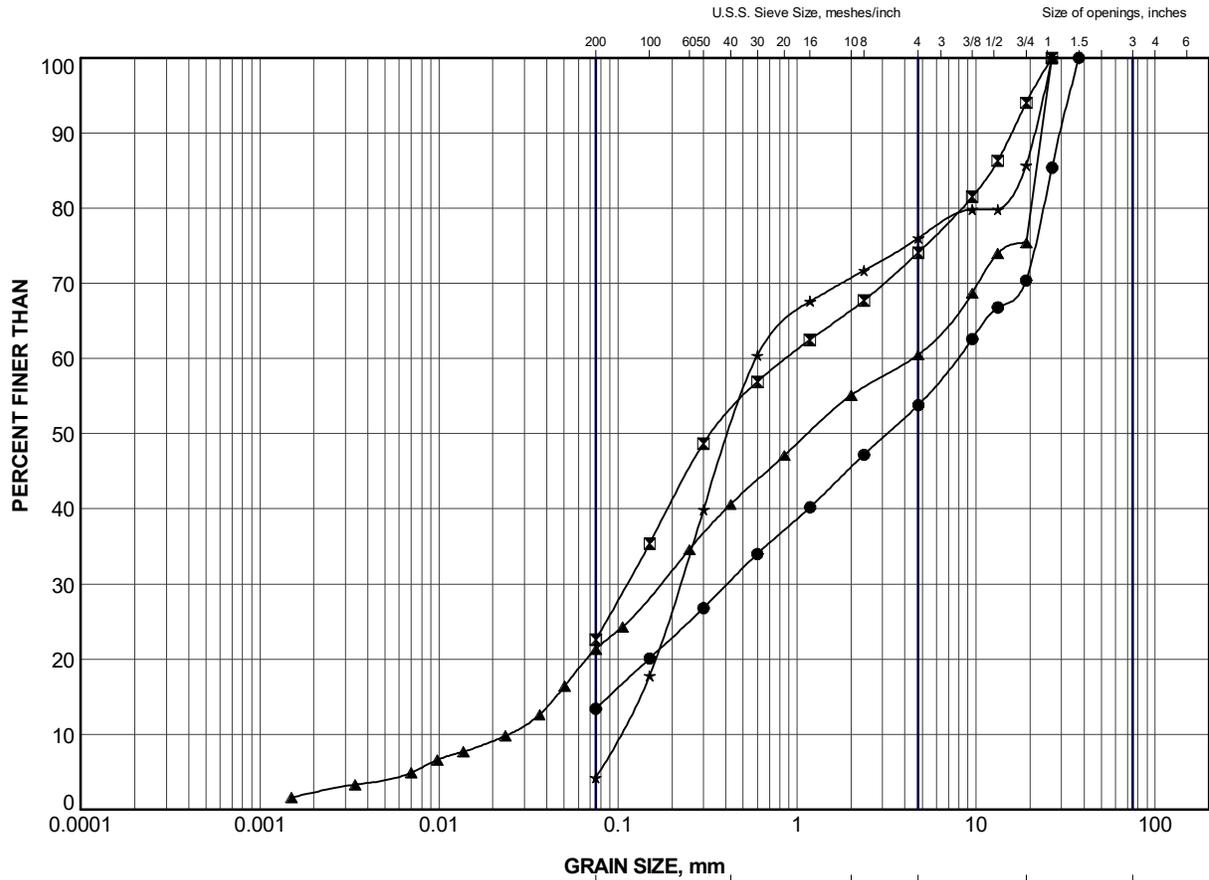
DATUM GEODETIC DATE May 27 and 28, 2019 CHECKED BY AB

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)					
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20						40	60	80	100	20
319.4	GROUND SURFACE																	
0.7	TOPSOIL (50 mm)		1	SS	3													
	Silty sand, some gravel, trace organics, containing cobbles (FILL) Very loose to dense Brown Moist to wet		2	SS	31													
317.9																		
1.5	SAND and GRAVEL, trace silt, trace clay (TILL) Compact to dense Brown Wet		3	SS	18													
			4	SS	37													
			5	SS	37													
315.6			6	SS	100/0.13													
3.8	END OF BOREHOLE SPLIT-SPOON REFUSAL																	
314.8	END OF DCPT (HAMMER BOUNCING)																	
4.6	NOTE: 1. Water level at a depth of 0.7 m below ground surface (Elev. 318.7 m) upon completion of drilling. 2. Advanced DCPT 1.0 m north of borehole. Refusal at 4.0 m depth. 3. Advanced second DCPT 1.0 m east of borehole. Refusal at 4.6 m depth.																	

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APPENDIX B

Laboratory Test Results



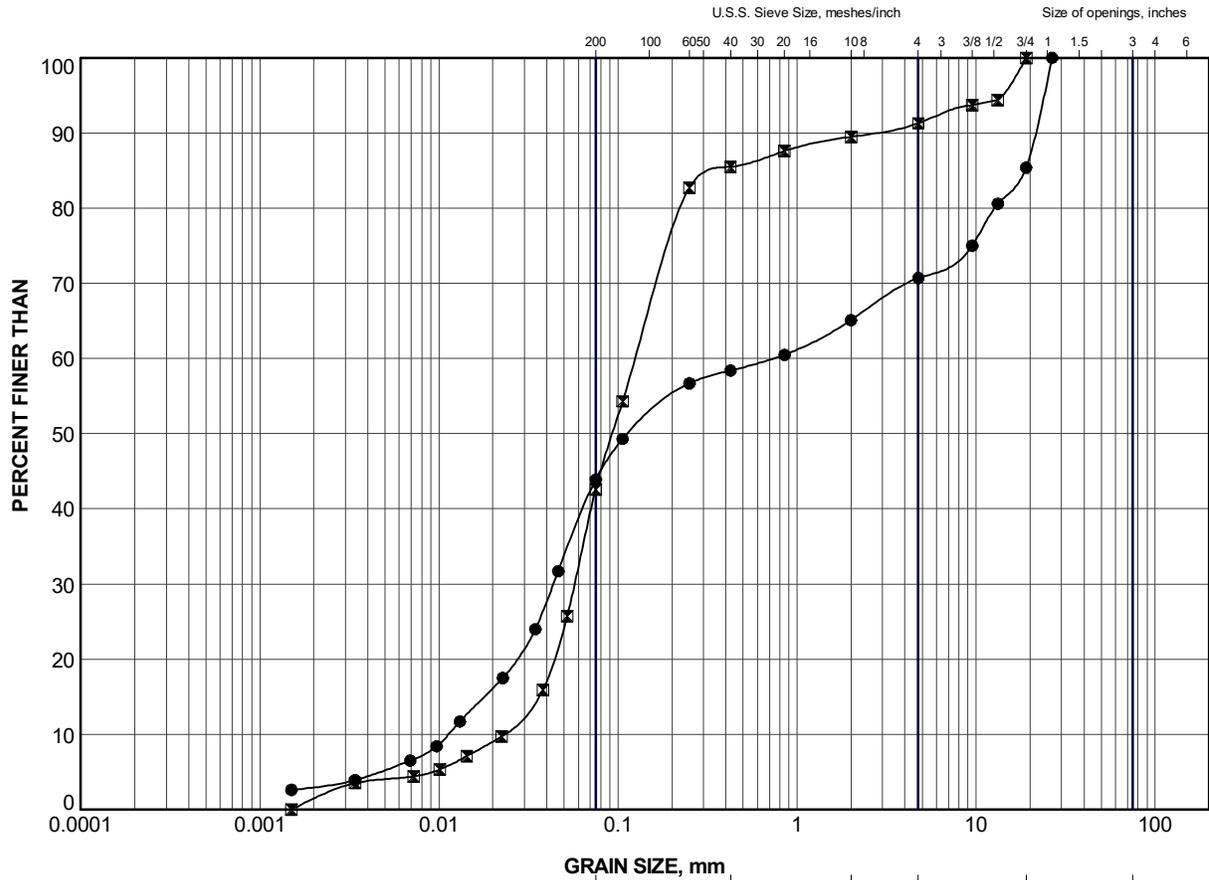
CLAY AND SILT	fine	medium	coarse	fine	coarse	Cobble Size
	SAND SIZE			GRAVEL SIZE		

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	C227-2	2	324.4
⊠	C227-2	6	321.3
▲	C227-2	7	319.8
★	C227-3	3	323.1

PROJECT						HIGHWAY 66 STATION 16+994 TOWNSHIP OF LABEL CULVERT					
TITLE						GRAIN SIZE DISTRIBUTION Silty Gravelly Sand to Sand and Gravel (FILL)					
PROJECT No.			1896349			FILE No.			1896349.GPJ		
DRAWN		TR		Sep 2019		SCALE		N/A		REV.	
CHECK		AB		Sep 2019		APPR		JMAC		Sep 2019	
 GOLDER SUDBURY, ONTARIO						FIGURE B-1					

SUD-MTO GSD_GLDR_LDN.GDT



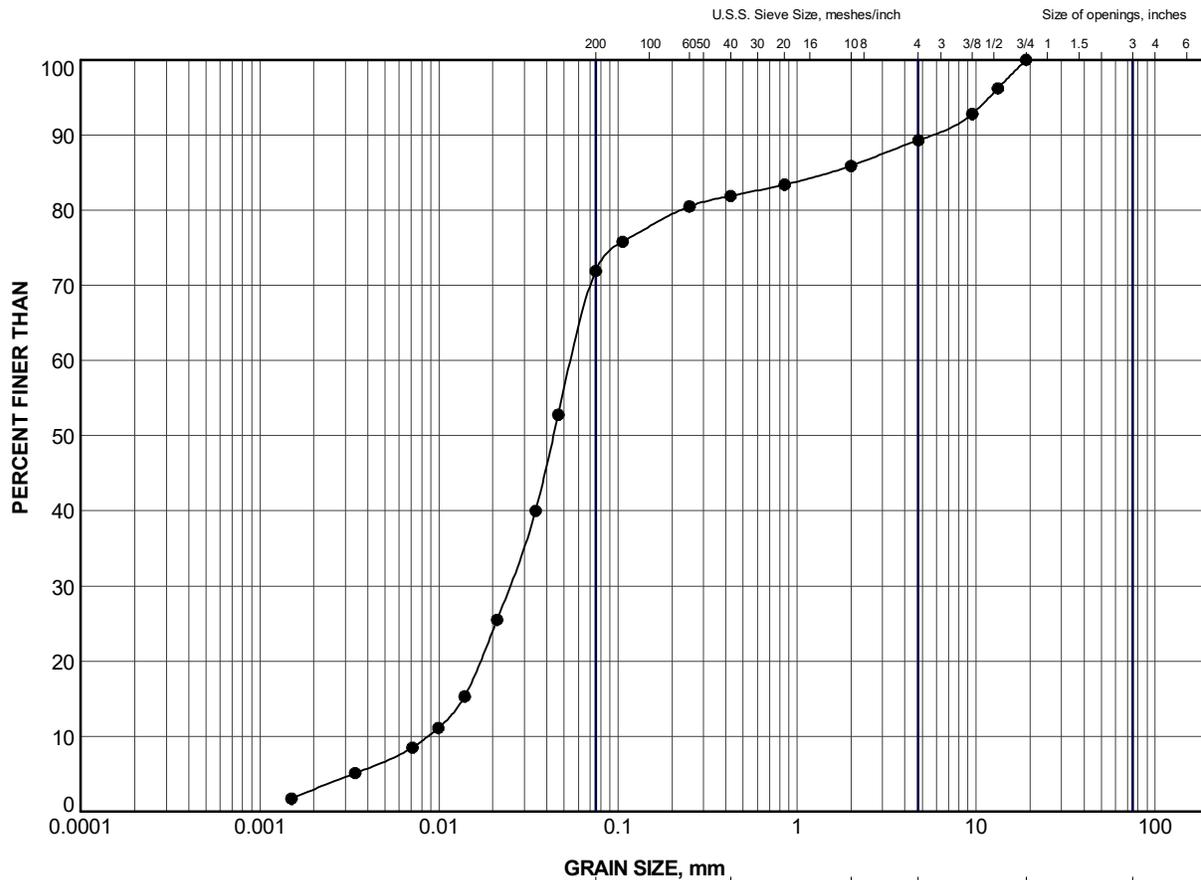
CLAY AND SILT	fine	medium	coarse	fine	coarse	Cobble Size
	SAND SIZE			GRAVEL SIZE		

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	C227-1	6	321.3
■	C227-1	7	319.8

PROJECT						HIGHWAY 66 STATION 16+994 TOWNSHIP OF LABEL CULVERT					
TITLE						GRAIN SIZE DISTRIBUTION Sandy Gravelly Silt to Silt and Sand (FILL)					
PROJECT No.			1896349			FILE No.			1896349.GPJ		
DRAWN		TR		Sep 2019		SCALE		N/A		REV.	
CHECK		AB		Sep 2019		APPR		JMAC		Sep 2019	
 GOLDER SUDBURY, ONTARIO						FIGURE B-2					

SUD-MTO GSD GLDR_LDN.GDT

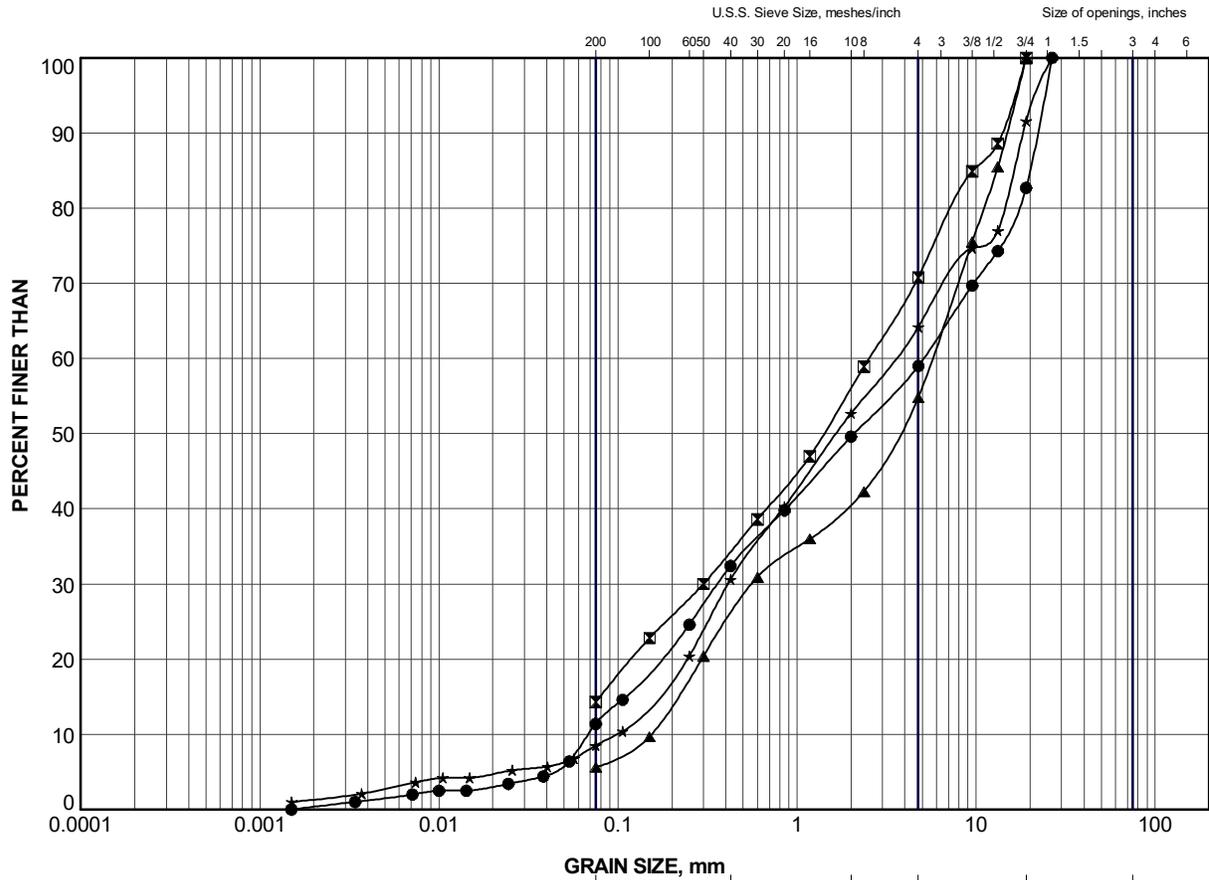


CLAY AND SILT	fine	medium	coarse	fine	coarse	Cobble Size
	SAND SIZE			GRAVEL SIZE		

LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	C227-3	5	321.6

PROJECT	HIGHWAY 66 STATION 16+994 TOWNSHIP OF LABEL CULVERT				
TITLE	GRAIN SIZE DISTRIBUTION Silt (FILL)				
 GOLDER SUDBURY, ONTARIO	PROJECT No. 1896349		FILE No. 1896349.GPJ		
	DRAWN	TR	Sep 2019	SCALE	N/A
	CHECK	AB	Sep 2019	REV.	
	APPR	JMAC	Sep 2019	FIGURE B-3	



LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEV (m)
●	C227-1	8	318.3
⊠	C227-4	3	320.2
▲	C227-4	6	317.9
★	C227-5	4	316.8

PROJECT						HIGHWAY 66 STATION 16+994 TOWNSHIP OF LABEL CULVERT					
TITLE						GRAIN SIZE DISTRIBUTION Gravelly SAND to SAND and GRAVEL (TILL)					
PROJECT No.			1896349			FILE No.			1896349.GPJ		
DRAWN		TR		Sep 2019		SCALE		N/A		REV.	
CHECK		AB		Sep 2019		APPR		JMAC		Sep 2019	
 GOLDER SUDBURY, ONTARIO						FIGURE B-4					



GOLDER

Bedrock Core Photographs

Figure B-5A

Highway 66, Station 16+994, Township of Lebel Culvert



Borehole C227-1
Elevation 315.5 m to 312.3 m





Bedrock Core Photographs

Figure B-5B

GOLDER

Highway 66, Station 16+994, Township of Lebel Culvert

317.0 m



Borehole C227-3
Elevation 317.0 m to 313.6 m

313.6 m

316.6 m



Borehole C227-4
Elevation 316.6 m to 312.9 m

312.9 m

0 m

0.75 m

1.5 m



BUREAU
VERITAS

BV Labs Job #: B9D3975
Report Date: 2019/06/03

Golder Associates Ltd
Client Project #: 1896349(2100)
Site Location: HWY 66
Sampler Initials: MR

RESULTS OF ANALYSES OF SOIL

BV Labs ID		JTI432			JTI433	JTI434	JTI435	JTI436		
Sampling Date		2019/05/03 10:45			2019/05/04 14:47	2019/05/08 12:39	2019/05/11 16:36	2019/05/14 08:49		
COC Number		127611			127611	127611	127611	127611		
	UNITS	C236-1 Lab-Dup	RDL	QC Batch	C267-1	C228-1	C227-1	C256-1	RDL	QC Batch
CONVENTIONALS										
Sulphide	ug/g				<0.30	<0.30	<0.30	<0.30	0.30	6150574
Calculated Parameters										
Resistivity	ohm-cm				23000	12000	2500	22000		6129977
Inorganics										
Soluble (20:1) Chloride (Cl-)	ug/g				<20	29	250	<20	20	6133046
Conductivity	umho/cm				43	84	405	46	2	6135430
Available (CaCl2) pH	pH				7.74	6.56	7.00	6.30		6133358
Soluble (20:1) Sulphate (SO4)	ug/g				<20	<20	<20	<20	20	6133048
Physical Testing										
Moisture-Subcontracted	%	15	0.30	6150575	21	20	20	20	0.30	6150575
RDL = Reportable Detection Limit QC Batch = Quality Control Batch Lab-Dup = Laboratory Initiated Duplicate										

APPENDIX C

**Non-Standard Special Provisions
and Notice to Contractor**

PIPE INSTALLATION BY TRENCHLESS METHOD – Item No.

Special Provision

January 2019

CONSTRUCTION SPECIFICATION FOR THE INSTALLATION OF PIPES BY TRENCHLESS METHODS

TABLE OF CONTENTS

1.0	SCOPE
2.0	REFERENCES
3.0	DEFINITIONS
4.0	DESIGN AND SUBMISSION REQUIREMENTS
5.0	MATERIALS
6.0	EQUIPMENT
7.0	CONSTRUCTION
8.0	QUALITY ASSURANCE- Not Used
9.0	MEASUREMENT FOR PAYMENT
10.0	BASIS OF PAYMENT
1.0	SCOPE

This specification covers the requirements for the installation of pipe by a selected trenchless method.

2.0 REFERENCES

This specification refers to the following standards, specifications, or publications:

Ontario Provincial Standard Specifications, General

OPSS 180 Management of Disposal of Excess Material

Ontario Provincial Standard Specifications, Construction

OPSS 401 Trenching, Backfilling, and Compacting
OPSS 402 Excavating, Backfilling, and Compacting for Maintenance Holes, Catch Basins, Ditch Inlets and Valve Chambers
OPSS 403 Rock Excavation for Pipelines, Utilities, and Associated Structures in Open Cut
OPSS 404 Support Systems
OPSS 409 Closed-Circuit Television (CCTV) Inspection of Pipelines

OPSS 491	Preservation, Protection, and Reconstruction of Existing Facilities
OPSS 492	Site Restoration Following Installation of Pipelines, Utilities and Associated Structures
OPSS 517	Dewatering
OPSS 539	Temporary Protection Systems

Ontario Provincial Standard Specifications, Material

OPSS 1004	Aggregates - Miscellaneous
OPSS 1350	Concrete - Materials and Production
OPSS 1440	Steel Reinforcement for Concrete
OPSS 1802	Smooth Walled Steel Pipe
OPSS 1820	Circular and Elliptical Concrete Pipe
OPSS 1840	Non-Pressure Polyethylene (PE) Plastic Pipe Products

CSA Standards

B182.6	Profile polyethylene (PE) sewer pipe and fittings for leak-proof sewer applications
A3000	Cementitious Materials Compendium
W59	Welded Steel Construction (Metal Arc Welding)

American Society for Testing and Materials (ASTM) International Standards

A 252	Standard Specification for Welded and Seamless Steel Pipe Piles
D 2657	Standard Practice for Heat Fusion Joining of Polyolefin Pipe and Fittings
D 3350	Standard Specification for Polyethylene Plastics Pipe and Fittings Materials
D6910	Standard Specification for Marsh Funnel Viscosity of Clay Construction Slurries
F 894	Standard Specification for Polyethylene Large Diameter Profile Wall Sewer and Drain Pipe

International Organization for Standardization/International Electrotechnical Commission (ISO/IEC)

17025	General Requirements for the Competence of the Testing and Calibration Laboratories
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3.0 DEFINITIONS

For the purpose of this specification, the following definitions apply:

Auger Jack & Bore means a method of forming a horizontal bore in the subsurface by simultaneously or alternately jacking into the ground a casing pipe and rotating a cutter head at the lead end of an auger flight with removal of material from inside the casing by using continuous-flight augers.

Backreamer or Reamer means a cutting head suitably designed for the subsurface conditions that is attached to drilling equipment and used to enlarge the bore

Bore Path means a drilled path according to the grade and alignment tolerances specified in the Contract Documents.

Design Engineer means the Engineer retained by the Contractor who produces the design and working drawings and other engineering documents required of the Contractor. The Design Engineer shall be licensed to practice in the Province of Ontario.

Design Checking Engineer means the Engineer retained by the Contractor who checks the original design and working drawings. The design checking engineer shall be licensed to practice in the Province of Ontario, shall not be an employee of the Contractor and shall be independent from the Design Engineer.

Digger Shield/Hand Mining means a method of forming a horizontal bore in the subsurface by essentially simultaneously jacking a casing pipe, with or without a protective shield at the lead end, into the ground while tunnelling and removal of earth and rock is completed using manually-operated tools (e.g., pneumatic spades, rams, shovels, breaker bars, etc.) or a “digger” type shield with a hydraulic excavator arm or “road-header” rock cutting machine to remove materials from inside the shield and liner pipe.

Horizontal Directional Drilling (HDD) means horizontal directional boring or guided boring.

Drilling Fluids means a mixture of water and additives, such as bentonite, polymers, surfactants, and soda ash, designed to block the pore space on a bore wall, reduce friction in the bore, and to suspend and carry cuttings to the surface.

Drilling Fluid Hydraulic Fracture or “Frac Out” means a condition where the drilling fluid’s pressure in the bore is sufficient to fracture the soil and/or rock materials and allow the drilling fluids to migrate to the surface at an unplanned location.

Earth Pressure Balance (EPB) means a tunnelling system that provides support to the excavated face of the ground and resistance to groundwater inflow through the pressure of mixed earth, rock and any drilling fluids or additives (spoil) as maintained by and in a chamber behind the cutting face of a tunnel boring machine through which spoil can pass only by manner of controlled-load relieving gates or an internal screw-conveyor that is separate from subsequent spoil conveyance systems (e.g., flight augers, belt conveyor, spoil bucket rail cars, etc.). Trenchless systems that apply pressure to the excavated face of the ground only through mechanical and jacking forces on metal parts of the machinery (e.g., steel parts of cutting tools, adjustable gates or doors at cutting face, etc.) will not be considered equivalent to EPB systems.

Excavation means all materials encountered regardless of type and extent and shall include removal of natural soil, boulders, cobbles, wood and fill regardless of means necessary to break consolidated materials for removal.

Environmentally Sensitive Area (ESA) means areas specified in the Contract Documents that are prohibited from entry or use.

Fill means man-made mixture of previously placed or handled materials such as sand, clay, silt, gravel, broken rock, sometimes containing organic and/or deleterious materials, placed in an excavation or other area to raise the surface elevation.

Guidance System means an electronic system capable of indicating the position, depth and orientation of the drill head during the directional drilling process.

Hand Mining means a method of forming a horizontal bore in the subsurface by simultaneously jacking ahead while tunnelling advances using hand-mining (man-entry operation or “Jack and Mine”) or a “digger” type shield with a hydraulic excavator arm to remove materials from inside the liner pipe.

Inadvertent Returns means the unexpected flow of fluids, saturated materials (or flowing soil) towards the drilling rig that typically originated from an artesian aquifer encountered during the drilling process.

Loss of Circulation means the discontinuation of the flow of drilling fluid in the bore back to the entry or exit point or other planned recovery points.

Microtunnelling means an underground method of constructing a passage by using a microtunnel boring machine (MTBM) or hand mining using a shield to support the opening.

Pilot Bore means the initial bore to set directional controlled horizontal and vertical alignment between the connecting points.

Pipe Jacking means a method for installing steel casing, concrete pipe or other acceptable material in the subsurface utilizing hydraulically operated jacks of adequate number and capacity for the smooth and uniform advancement of the casing or pipe.

Pipe means pipe culverts, pipe storm and sanitary sewers, watermain pipe, conduits and ducts.

Pipe Ramming means a method for installing steel casings utilizing the energy from a percussion hammer to advance a steel casing with a cutting shoe attached at the front end of the casing.

Project Superintendent means an individual representing the Contractor that oversees the trenchless or tunnelling operation qualified to provide the services specified in the Contract Documents.

Pullback means that part of the HDD method in which the drilling equipment is pulled back through the bore path to the entry point.

Reaming means a process for enlarging the bore path

Rock means natural beds or massive fragments, or the hard, stable, cemented part of the earth's crust, igneous, metamorphic, or sedimentary in origin, which may or may not be weathered and includes boulders having a volume of 0.5 m³ or greater.

Shaft means an excavation used as entry and/or exit points, alternatively called entry/exit pits, from which the trenchless method is initiated for the installation of the pipe product.

Slurry Pressure Balance (SPB) means a tunnelling system that provides support to the excavated face of the ground and resistance to groundwater inflow through the pressure of slurry as maintained by and in a chamber behind the cutting face of a TBM or MTBM through which spoil can pass only by manner of controlled-pressure and controlled flow slurry pumping systems.

Strike Alert means a system that is intended to alert and protect the operator in the case of inadvertent drilling into an electrical utility cable. The strike alert system consists of a sensor and an alarm connected to the drill rig and a grounding stake. The alarm may be audio or visual or both.

Slurry means a mixture of soil and/or rock cuttings, and drilling fluid.

Soil means all soils except those defined as rock, and excludes stone masonry, concrete, and other manufactured materials.

Spoil means mix of earth cuttings, rock cuttings, water (groundwater or added water), bentonite, polymers and/or other additives that is discharged from the trenchless construction systems.

Trenchless Installation means an underground method of constructing a passage open at both ends that involves installing a pipe product by auger jack & boring, pipe ramming, horizontal directional drilling, or tunnelling.

Trenchless Contractor means the subcontractor retained by the Prime Contractor qualified to provide the

services specified in the Contract Documents.

Tunnelling means an underground method of constructing a passage using a tunnel boring machine (TBM) operated by personnel within the tunnel, a microtunnel boring machine (MTBM) operated by personnel at a remote control station or excavation using a shield to support the opening and protect workers.

Zone of Influence means a zone defined by lines projected outward and upward at 45 degrees from horizontal to the ground surface from the vertical and horizontal alignment of the pipe constructed using trenchless/tunnel methods.

4.0 DESIGN AND SUBMISSION REQUIREMENTS

4.01 Design

4.01.01 General

The Contractor shall determine the most appropriate method of installation for each location within the terms of this specification.

The installation method selected for each pipe crossing shall be designed for the subsurface conditions as reported in the Contract Documents.

The detailed design of the installation method selected to carry out the work as specified in the Contract Documents shall be completed.

Based on the subsurface conditions encountered during the foundation investigation at the culvert site crossing Highway 66, Station 16+994, Township of Lebel, combined with the proposed carrier pipe diameter (0.8 m), and anticipated relatively short tunnel length of about 28 m, HDD, TBM, Tunnel Digging Machine (TDM), Pilot Tube Microtunnelling (PTMT), and Auger Jack and Bore methods are considered not suitable for the trenchless installation of the culvert.

4.02 Submission Requirements

4.02.01 Qualifications

At least two weeks prior to construction, the names and the demonstrated project experience of the Project Superintendent, Trenchless contractor, Design Engineer, and Design Checking Engineer shall be submitted to the Contract Administrator.

4.02.01.01 Project Superintendent

The Project Superintendent shall have a minimum of five years' demonstrated experience on projects with similar scope and complexity.

During construction, the project superintendent shall not change without written permission from the Contract Administrator. A proposal for a change in the project superintendent shall be submitted at least one week prior to the actual change in project superintendent.

4.02.01.02 Trenchless Contractor

The Trenchless Contractor shall have a minimum of five years' demonstrated experience on projects with similar scope and complexity

4.02.01.03 Design Engineer

The Design Engineer shall have a minimum of five years' demonstrated experience on projects with similar scope and complexity

4.02.01.04 Design Checking Engineer

The Design Checking Engineer shall have a minimum of five years' demonstrated experience on projects with similar scope and complexity

4.02.02 Working Drawings

Three sets of Working Drawings for the trenchless installation method selected shall be submitted to the Contract Administrator (CA) for purposes of documentation and quality assurance at least two week prior to the commencement of the work. All Working Drawings shall bear the seal and signature of the Design Engineer and Design Checking Engineer.

The working drawings shall be submitted to the Contract Administrator under cover with a Request to Proceed.

The Contractor shall not proceed with the work until a Notice to Proceed has been received from the Contract Administrator

A copy of the Working Drawings shall be kept at the site during construction.

Information and details shown on the Working Drawings shall include, but not be limited to:

a) Plans and Details:

- i. Plans and profiles defining all horizontal and vertical alignment positions and positions of all utilities and other infrastructure within the zone of influence of the work;
- ii. A work plan outlining the materials, procedures, methods and schedule to be used to execute the work.
- iii. A list of personnel, including backup personnel, and their qualifications and experience.
- iv. A safety plan including the company safety manual and emergency procedures.
- v. The work area layout.
- vi. An erosion and sediment control plan that includes a contingency plan in the event the erosion and sediment control measures fail.
- vii. A contingency plan with specific details of the manner in which rock or boulders will be broken and removed from the face and the face will be protected to prevent soil loss into the liner.
- viii. A drilling fluid management plan, if applicable, that addresses control of frac-out pressures, any potential environmental impacts and includes a contingency plan detailing emergency procedures in the event that the fluid management plan fails.
- ix. Lighting, ventilation and fire safety details as may be required by applicable occupational health and safety regulations.
- x. Excavated materials disposal plan.
- xi. Locations of protection systems.

b) Designs

- i. Primary liner design (e.g., steel liner plates, steel ribs and wood lagging, steel casing pipe, etc.),
- ii. Design assumption and material data when materials other than those specified are proposed for use.
- iii. Drill path design, details of alignment and alignment control, maximum curvature and reaming stages.

c) Materials:

- i. Certification from the manufacturer that the product furnished on the contract meets the specifications cited in the manufacturer's product specification and that the materials supplied are suitable for the application.
- ii. Manufacturer data sheets for all drilling fluids and additives for use in Earth Pressure Balance, Slurry Pressure Balance
- iii. Manufacturer data sheets for drilling systems.
- iv. Mix designs, target rheology criteria (e.g., viscosity, density, shear strength, gel time, pressure-filtration – fluid losses under pressure, etc.) and additive dosage rates for all slurries and EPB TBM and MTBM operations.
- v. The proposed grout mix design for grouts to be used for lubricating jacking pipe and for filling of voids and annular spaces.
- vi. Compressive strength of concrete pipe products.
- vii. Pipe class for all steel pipe products.
- viii. Steel for Permanent Casings
 - One copy of a mill test certificate certifying that the steel meets the requirements for the appropriate standards for permanent casings shall be submitted to the Contract Administrator at the time of delivery.
 - Where mill test certificates originate from a mill outside Canada or the United States of America, the information on the mill certificates shall be verified by testing by a Canadian laboratory. The laboratory shall be certified by an organization accredited by the Standards Council of Canada to comply with the requirements of ISO/IEC 17025 for the specific tests or type of tests required by the material standard specified on the mill test certificate.
 - The mill test certificates shall be stamped with the name of the Canadian testing laboratory and appropriate wording stating that the material conforms to the specified material requirements. The stamp shall include the appropriate material specification number, the date (i.e., yyyy-mm-dd), and the signature of an authorized officer of the Canadian testing laboratory
- ix. The Contractor shall submit the followings to the Contract Administrator two weeks prior to construction:
 - type, source, and physical and chemical properties of bentonite, polymer or other additives;
 - source of water;
 - method of mixing;
 - the water to solids ratio and the mass and volumes of the constituent parts, including any chemical admixtures or physical treatment employed to achieve required physical properties;
 - details of procedure to be used for monitoring physical properties of slurry, drilling fluids and tunnelling fluids or EPB spoil; and method of disposal of the slurry, drilling fluids and associated spoil

d) Upstream/Downstream Portal Installation Procedure:

- i. The access shaft or entry/exit pit details, as applicable.
- ii. Face support and other temporary support details, if applicable.

e) Primary Liner/Secondary Liner Installation and Grouting Procedure:

- i. Excavation and pipe installation procedures, including methods to handle obstructions and prevent soil cave-in.
- ii. Details of tunnelling equipment/methods to be used for the works.

f) Excavation and Dewatering:

- i. Equipment and methods for control, handling, treatment, and disposal of groundwater and water or fluids introduced by the Contractor;
- ii. Equipment and methods for maintaining control of ground inflow at the excavation face during excavation;
- iii. Equipment and methods for removal of cobbles and boulders;
- iv. Manufacturer data sheets for each TBM, shield, tunnelling system or drilling system noting all intermediate and final cut dimensions, and methods and equipment for controlling and measuring drilling fluid, SPB and EPB pressures;
- v. Methods for measuring excavated volumes or weights of earth and rock materials cut from ground on a per meter or per pipe basis up to a maximum of 3 m long intervals per measurement;
- vi. Target operating pressures (minimum and maximum) and range of expected pressure variation for slurry or EPB spoil at excavated face or drilling fluids at lead end of drilling equipment and in annular gap between maximum excavated dimensions and outside dimensions of tunnelling equipment, drilling equipment and primary liner systems;
- vii. Basis for setting target operating conditions (pressures, flow rates, advance rates) and the relationship of target operating conditions to ground conditions;
- viii. Basis for selection of excavation tools (e.g., bits, TBM face tools, MTBM face tools, excavator fittings, etc.) as related to expected ground conditions;
- ix. Jacking forces for installation of pipe, for driving of trenchless equipment forward and, in the case of Auger Jack & Bore, for advancing the lead end of the casing ahead of the lead end of the auger cutting tools.

g) Monitoring Method:

Methods, equipment, frequency and repeatability (accuracy and precision) of data collection to be employed for measuring and monitoring shall be submitted for:

- i. Maintaining the alignment of the installation;
- ii. EPB, SPB and drilling fluid pressures at the leading edge of excavation (face), flow rates and volume or weights of spoil;
- iii. Jacking forces on pipes, linings and cutting tools;
- iv. Torque, total revolutions and revolution rates on rotating equipment such as TBM or MTBM heads, auger flights, drill bits, etc.
- v. Grout injection pressures and volumes;
- vi. Longitudinal position of all casings and excavation cutting tools (auger flight heads, TBM face, drill bit position, etc.);
- vii. Ground displacements (heave and settlement); and noise and ground vibrations induced by trenchless construction

4.02.03 Quality Control Certificate

The Contractor shall submit a Quality Control Certificate to the Contract Administrator for documentation and quality assurance purposes, prepared and stamped by the Design and Design Checking Engineers, a minimum of two weeks prior to commencement of work under this item. The Certificate shall state that the construction procedures are in conformance with the requirements and specifications of the contract documents.

The Contractor shall submit to the Contract Administrator a Quality Control Certificate sealed and signed by the Design and Design Checking Engineer upon completion of each of the following operations and prior to commencement of each subsequent operation for each pipe installation:

- Site Surveying (as noted in Section 4.02)
- Excavation for pits including dewatering of excavations
- Jacking/Ramming/Directional Drilling of Casing/Liner
- Installation of the Product
- Grouting Operations

Each Quality Control Certificate shall state that the work has been carried out in general conformance with the contract documents, specifications and/or stamped working drawings.

The Contractor shall submit a Request to Proceed to the Contract Administrator upon completion of each of the milestones.

The Contractor shall not proceed to the subsequent operation until a Notice to Proceed has been received from the Contract Administrator

In addition, upon completion of the installation of the pipe at each location, the Contractor shall submit to the Contract Administrator a final Quality Control Certificate sealed and signed by the Design and Design Checking Engineer. The Certificate shall state that the pipe has been installed in general conformance with the Contractor's Submission and Design Requirements, stamped working drawings and contract documents.

5.0 MATERIALS

5.01 Pipe

5.01.01 General

The product shall be concrete pipe, steel pipe or high density polyethylene pipe as specified.

All joints shall be suitable for jacking operations as specified in the working drawings.

Fittings shall be suitable and compatible with the class and type of pipe with which they will be used.

All fittings shall be designed to be watertight.

5.01.02 Steel Pipe

Steel pipe shall be according to ASTM A252.

All steel casing pipe shall be square cut.

Steel casing pipe shall meet a straightness tolerance of 1.5 mm/m. When placed anywhere on the pipe parallel to the pipe axis, there shall not be a gap more than 1.5 mm between a 1 m long straightedge and the pipe.

5.01.03 HDPE Pipe

High density polyethylene (HDPE) pipe according to OPSS 1840 shall be used in accordance with ASTM D3350.

Fittings shall be according to CAN/CSA-B182.6 or ASTM F894 and suitable for the class and type of pipe with which they will be used.

Jointing of HDPE piping shall be completed according to the manufacturer's recommended procedures and ASTM D2657. Where conflicts exist between the manufacturer's instructions and ASTM D2657, the manufacturer's instructions are to be followed.

Jointing of HDPE piping to other piping materials or appurtenances shall be completed using flanged connections.

5.01.04 Concrete Pipe

Concrete pipe shall be according to OPSS 1820.

5.02 Concrete

Concrete shall be according to OPSS 1350. The concrete strength shall be as specified on the Working Drawings.

5.03 Steel Reinforcement

Steel reinforcement for concrete work shall be according to OPSS 1440.

5.04 Wood

Wood shall be according to OPSS 1601.

5.05 Drilling Fluids

Drilling fluid shall be mixed according to the working drawings.

Selection of drilling fluid type shall be based on the soils encountered in the subsurface investigation.

The drilling fluids shall be mixed according to the manufacturer's recommendations.

Slurry shall be mixed according to the submitted slurry design and be appropriate for the anticipated subsurface conditions. The viscosity of slurry used for SPB tunnelling shall be no less than 40 seconds Marsh Funnel viscosity, as defined by ASTM D6910, measured prior to introduction of groundwater and spoil and as required to ensure:

- a) development of appropriate filter cake at excavation face to provide slurry support pressures exceeding

- ground and groundwater pressures at excavation face;
- b) lubricate installation of primary liners as required;
- c) transport spoil through pipe systems;

5.06 Grout

Purging grout shall conform to the requirements of OPSS 1004 wetted with only sufficient water to make the mixture plastic

6.0 EQUIPMENT

6.01 Auger Jack & Bore

Except in the case of dewatering to at least 1 m below the tunnel/bore invert for the full length of the pipe alignment, Auger Jack & Bore shall not be used and will not be permitted where subsurface conditions indicate that saturated gravel, sand and silt soils may be encountered at pipe level or within one pipe diameter above or below outside pipe dimensions.

Pipe auger jack & bore equipment shall be determined by the Contractor and shall be identified in the submission requirements specified herein.

Specific details of the equipment with which rock or boulders will be broken and removed from the face and the face will be protected to prevent soil loss into the liner shall be submitted to the Contract Administrator for information purposes prior to proceeding with the works.

The lead end of the auger shall be maintained at least one pipe diameter inside the lead end of the casing. The auger cutting tools shall not extend to or beyond the lead end of the casing at any time unless specific exception is provided by the Ministry prior to construction. Submittals shall identify anticipated jacking forces for advancing casing ahead of leading edge of auger cutting tools in addition to friction forces that are to be overcome by jacking systems

6.02 Pipe Ramming

Pipe ramming equipment shall be determined by the Contractor and shall be identified in the submission requirements specified herein.

The pipe ramming hammer(s) shall be capable of driving the pipe casing from the entry pit to the exit pit through the existing subsurface conditions at the site without removal of soil from within the casing until the lead end of the pipe is outside the zone of influence for any overlying infrastructure.

Specific details of the equipment with which rock or boulders will be broken and removed from the face and the face will be protected to prevent soil loss into the pipe shall be submitted to the Contract Administrator for information purposes prior to proceeding with the works.

6.03 Horizontal Directional Drilling

6.03.01 General

The Horizontal Directional Drilling equipment shall consist of a directional drilling rig and a drilling fluid

mixing and delivery system to successfully complete the product installation without exceeding the maximum tensile strength of the product being installed.

6.03.02 Drilling Rig

The horizontal directional drilling rig shall:

- a) Consist of a leak free hydraulically powered boring system to rotate, push, and pull hollow drill pipe into the ground at a variable angle while delivering a pressurized fluid mixture to a guidable drill head.
- b) Have drill rod that is suitable for both the drill and the product pipe installation.
- c) Contain a drill head that is steerable, equipped with the necessary cutting surfaces and fluid jets, and be suitable for the anticipated ground conditions.
- d) Have adequate reamers and down-bore tooling equipped with the necessary cutting surfaces and fluid jets to facilitate the product installation and be suitable for the anticipated ground conditions.
- e) Contain a guidance system to accurately guide boring operations.
- f) Be anchored to the ground to withstand the rotating, pushing, and pulling forces required to complete the product installation.
- g) Be grounded during all operations unless otherwise specified by the drilling rig manufacturer.

6.03.03 Drill Head

The drill head shall be steerable by changing its rotation, be equipped with the necessary cutting surfaces and drilling fluid jets, and be of the type for the anticipated subsurface conditions,

6.03.04 Guidance System

The guidance system shall be setup, installed, and operated by trained and experienced personnel. The operator shall be aware of any magnetic or electromagnetic anomalies and shall consider such influences in the operation of the guidance system when a magnetic or electromagnetic system is used.

6.03.05 Drilling Fluid Mixing System

The drilling fluid mixing system shall be of sufficient size to thoroughly and uniformly mix the required drilling fluid.

6.03.06 Drilling Fluid Delivery System

The delivery system shall have a means of measuring and controlling fluid pressures and be of sufficient flow capacity to ensure that all slurry volumes are adequate for the length and diameter of the final bore and the anticipated subsurface conditions. Connections between the delivery pump and drill pipe shall be leak-free.

6.04 Tunnelling

Tunnelling equipment shall be determined by the Contractor and shall be identified in the submission requirements specified herein. Specific details of tunnelling equipment included in the submission shall be provided for:

- a) rock or boulder breaking and removal;
- b) equipment used within shields for spilling, fore-poling, face drainage, breasting boards/plates and for otherwise maintaining support of the tunnel crown and face under all anticipated conditions;
- c) jacking systems;
- d) alignment control systems;

Use of rock fracturing chemicals shall only be considered subject to a field demonstration satisfactory to the Ministry prior to its use. Use of explosives is prohibited without specific application and acceptance by the Ministry prior to construction.

6.05 Microtunnelling Equipment

The Contractor shall be responsible for selecting microtunnelling equipment which, based on past experience, has proven to be satisfactory for excavation of the soils that will be encountered.

The Contractor shall employ microtunnelling equipment that will be capable of handling the various anticipated ground conditions.

The MTBM shall also be capable of controlling loss of soil ahead of and around the machine and shall provide continuous pressurized support of the excavated face.

- a) Remote Control System – The Contractor shall provide a MTBM that includes a remote control system with the following features:
 - i. Allows for operation of the system without the need for personnel to enter the microtunnel. Has a display available to the operator, at a remote operation console, showing the position of the shield in relation to a design reference together with other information such as face pressure, roll, pitch, steering attitude, valve positions, thrust force cutter head torque, rate of advance and installed length.
 - ii. Integrates the system of excavation and removal of spoil and its simultaneous replacement by Product Pipe. As each pipe section is jacked forward, the control system shall synchronize all of the operational functions of the system.
 - iii. The system shall be capable of adjusting the face pressure to maintain face stability for the particular soil condition encountered.
 - iv. The system shall monitor and continuously balance the soil and ground water pressure to prevent loss of soil or uncontrolled ground water inflow.
 - v. The pressure at the excavation face shall be managed by controlling the volume of spoil removal with respect to the advance rate.
 - vi. The system shall include a separation process designed to provide adequate separation of the spoil from the slurry so that slurry with a sediment content within the limits required for successful microtunnelling, can be returned to the cutting face for reuse. Appropriately contain spoil at the site prior to disposal.
 - vii. The type of separation process shall be suited to the size of microtunnel being constructed, the soil type being excavated, and the work space available at each work area.
 - viii. The system shall allow the composition of the slurry to be monitored to maintain the slurry weight and viscosity limits required.

- b) Active Direction Control - Provide an MTBM that includes an active direction control system with the following features:
 - i. Controls line and grade by a guidance system that relates the actual position of the MTBM to a design reference Provides active steering information that shall be monitored and transmitted to the operating console and recorded.
 - ii. Provides positioning and operation information to the operator on the control console.

6.05.01 Pipe Jacking Equipment

Provide a pipe jacking system with the following features:

- a) Has the main jacks mounted in a jacking frame located in the launch shaft.
- b) Has a jacking frame that successively pushes towards a receiving shaft, a string of Product Pipe that follows the microtunnelling excavation equipment.
- c) Has sufficient jacking capacity to push the microtunnelling excavation equipment and the string of pipe through the ground.
- d) The main jack station may be complemented with the use of intermediate jacking stations as required.
- e) Has a capacity at least 20 percent greater than the calculated maximum jacking load.
- f) Develops a uniform distribution of jacking forces on the end of the casing pipe.
- g) Provides and maintains a pipe lubrication system at all times to lower the friction developed on the surface of the pipe during jacking.
- h) Jack Thrust Blocking shall adequately support the jacking pressure developed by the main jacking system.
- i) Special care shall be taken when setting the pipe guide rails in the jacking shaft to ensure correctness of the alignment, grade, and stability.

6.05.02 Spoil Separation System

The Contractor shall determine the type of spoil separation equipment needed for each drive based on the geotechnical information available and other project constraints.

6.05.03 Electrical Equipment, Fixtures and Systems

Electrical equipment shall be suitably insulated for noise reduction. Noise produced by electrical equipment must comply with local municipal noise by-laws.

Electrical systems shall conform to requirements of the Canadian Electrical Code – CSA C22.1.

7.0 CONSTRUCTION

7.01 General

The Contractor shall notify the Contract Administrator at least 48 hours in advance of starting work. The proposed method of pipe installation to be used by the Contractor shall be subject to the limitations presented in the following subsections.

The Project Superintendent shall supervise the work at all times.

7.01.01 Layout, Alignment and Depth Control

The location of the installation shall be established from the lines, elevations and tolerances specified in the

Contract Documents. The pipe installation shall be to the horizontal and vertical alignments specified in the Contract Drawings. Deviations from location, alignment, grades and/or invert levels shall be corrected by the Contractor at no cost to the Ministry.

All reference points necessary to construct the pipe installation and appurtenances shall be laid out.

The Contractor shall calibrate tracking and locating equipment at the beginning of each work day, and shall monitor and record the alignment and depth readings provided by the tracking system every 2 m.

The Contract Administrator shall be provided with the assistance and access necessary to check the layout of the pipe installation and associated appurtenances.

The Contractor shall submit records of the alignment and depth of the installation to the Contract Administrator at the completion of the installation.

7.01.02 Construction Shafts

Construction shafts shall be specified in the Contractor's submission. The boundaries and protection of these shall be as required to contain all disturbances to areas outside of the ESA limits.

Shafts shall be maintained in a drained condition.

A minimum 2.4 m high secure fence shall be installed around the perimeter of the construction shaft area with gates and truck entrances. The fence shall be removed on completion of the work.

7.01.03 Protection Systems

The construction of all protection systems shall be according to OPSS539. Where the stability, safety, or function of an existing roadway, watercourse, other works, proposed works or ESA's may be impaired due to the method of operation, protection shall be provided. Protection may include sheathing, shoring, and piles where necessary to prevent damage to such works or proposed works.

7.01.04 Settlement or Heave

Any disturbance to the ground surface (settlement or heave) as a result of the pipe installation shall be immediately corrected by the Contractor, at no additional cost to the Ministry.

7.01.05 Stability of Excavation

The construction methods, plant, procedures, and precautions employed shall ensure that excavations are stable, free from disturbance, and maintained in a drained condition.

The construction methods, plant, procedures, and materials employed shall prevent the migration of soil and/or rock material into the excavation from adjacent ground.

7.01.06 Preservation and Protection of Existing Facilities

Preservation and protection of existing facilities shall be according to OPSS 491.

Minimum horizontal and vertical clearances to existing facilities as specified in the Contract Documents shall be maintained. Clearances shall be measured from the nearest edge of the largest cut diameter required to the nearest edge of the facility being paralleled or crossed.

Existing underground facilities shall be exposed to verify its horizontal and vertical locations when the outlet pipe path comes within 1.0 m horizontally or vertically of the existing facility. Existing facilities shall be exposed by non-destructive methods. The number of exposures required to monitor work progress shall be as specified in the Contract Documents.

7.01.07 Transporting, Unloading, Storing and Handling Materials

Manufacturer's handling and storage recommendations shall be followed.

7.01.08 Trenching, Backfilling and Compacting

Trenching, backfilling, and compacting for entry and exit points or other locations along the pipe path shall be according to OPSS 401.

7.01.09 Support Systems

Support systems shall be according to OPSS 404.

If any open excavation will encroach into the highway embankment the protection system shall satisfy the requirements for Performance Level 2 as specified in OPSS 539.

7.01.10 Dewatering

The work of this Section includes control, handling, treatment, and disposal of groundwater. The Contractor shall review the foundation investigation report for reference to soil and groundwater conditions on the project site and plan a dewatering scheme accordingly.

The Contractor shall control groundwater inflows to excavations to maintain stability of surrounding ground, to prevent erosion of soil, to prevent softening of ground exposed in the excavation, and to avoid interfering with execution of the work.

The Contractor shall maintain excavations free of standing water at all times during excavation, including while concrete is curing.

Should water enter the excavation in amounts that could adversely affect the performance of the work or could cause loss of ground, the Contractor shall take immediate steps to control the inflow.

The Contractor is alerted that seepage zones of perched water within the fill materials should be expected, particularly where granular materials are excavated.

Dewatering shall be according to OPSS 517.

7.01.11 Removal of Cobbles and Boulders

The Contractor is alerted that cobbles and boulders are expected within the soil deposits at the site. Accordingly, the Contractor shall address the removal of cobbles and boulders in the proposed method of construction. Removal of cobbles shall be expected to be routine and will not be considered cause for delay or additional compensation and the Contractor's trenchless equipment shall be appropriately equipped and operated for these conditions. The Contractor shall immediately inform the Contract Administrator of any obstruction encountered.

7.01.12 Removal of Obstructions

The Contractor is alerted that obstructions such as, but not limited to wood debris, roots, and stumps, and construction debris consisting of (broken asphalt, concrete etc.) are expected within the trenchless alignment as identified in the Contract Documents. Accordingly, the Contractor shall address methods for the removal of obstructions in the proposed method of construction. The Contractor shall immediately inform the Contract Administrator of any obstruction encountered and the Contractor's expected method of and schedule for removal.

7.01.13 Management of Excess Material

Management of excess material shall be according to OPSS 180. Satisfactory re-usable excavated material required for backfill shall be separated from unsuitable excavated material.

7.01.14 Site Restoration

Site restoration shall be according to OPSS 492.

7.02 Auger Jack & Bore Installation

7.02.01 Method of Installation Procedure

The installation procedure to be used shall be subject to the following limitations:

- a) Hydraulically operated jacks of adequate number and capacity shall be provided to ensure smooth and uniform advancement without over-stressing of the pipe.
- b) A suitably padded jacking head or collar shall be provided to transfer and distribute jacking pressure uniformly over the entire end bearing area of the pipe.
- c) The jacking pipe shall be fully supported in the jacking pit at the specified line and grade.
- d) Selection of the excavation method and jacking equipment shall take into consideration the conditions at each pipe crossing.

7.02.02 Pipe Installation

Concrete pipe joints shall be water tight and according to OPSS 1820 and must withstand jacking forces, determined by the Contractor.

During the jacking of the liner the space between the liner and the wall of the excavated volume (e.g., maximum cut diameter) shall be kept filled with bentonite slurry. Upon completion of jacking, the space between the liner and the wall of the excavated volume shall be filled with grout or slurry with gel strength properties demonstrated to be sufficient to form a semi-solid or solid gap filling material, prevent ground convergence around the pipe and subsequent ground surface subsidence and prevent long-term water flow at the outside boundary of any pipe and ground.

The annular space between the liner and the product shall be fully grouted with a water tight, expandable and stable grout.

7.03 Pipe Ramming Installation

For pipe ramming installation the following requirements apply:

Only smooth walled steel pipe shall be used. Butt welding of pipe joints shall conform to CAS W59.

Ramming equipment of adequate capacity shall be provided to ensure smooth and uniform advancement between the shafts/pits without overstressing of the pipe. Delays shall be avoided between ramming operations.

A ramming head shall be provided to transfer and distribute jacking pressure uniformly over the entire end bearing area of the pipe.

Two or more lubricated guide rails or sills shall be provided of sufficient length to fully support the pipe at the specified line and grade in the ramming pit. Pipe shall be installed to the line and grade specified.

Removal of materials from within the pipe shall not be undertaken until the lead end of the pipe has passed fully through and beyond the zone of influence of any overlying infrastructure.

Following installation of the liner pipe, all material shall be removed from the pipe to the satisfaction of the Contract Administrator. Any voids remaining between the pipe and the excavation wall shall be grouted as soon as the pipe is rammed. The annular space between the liner pipe and the product shall be fully grouted with a water tight, expandable and stable grout.

7.04 Horizontal Directional Drilling Installation

7.04.01 General

When strike alerts are provided on a drilling rig, they shall be activated during drilling and maintained at all times.

For horizontal directional drilling, the contractor shall ensure that during pilot hole drilling the maximum degree of deviation or “dog-leg” shall be 2.5 degrees per 9 m drill pipe length. Any deviation exceeding 2.5 degrees will necessitate a pull-back and straightening of the alignment at the Contractor’s sole expense. The pilot hole exit location shall be within 0.5m of the target location.

7.04.02 Site Preparation

The work site shall be graded or filled to provide a level working area for the drilling rig. No alterations beyond what is required for HDD operations are to be made. All activities shall be confined to designated work areas.

7.04.03 Pilot Bore

The pilot bore shall be drilled along the bore path in accordance with the grade, alignment, and tolerances as indicated on the Contractor’s submitted drilling plan to ensure that the product is installed to the line and grade shown on the Contract Drawings. The Contractor’s methods shall take into consideration the conditions at each crossing within the pipe alignment and shall be suitable to advance through such obstructions such as cobbles and boulders and address the potential for deflection off these obstruction and/or soil conditions.

In the event the pilot bore deviates from the submitted path, the Contract Administrator shall be notified. The Contract Administrator may require the Contractor to pullback, fill and abandon the hole and re-drill from the

location along the bore path before the deviation.

If a drill hole beneath highways, roads, watercourses or other infrastructure must be abandoned, the hole shall be backfilled with grout or bentonite to prevent future subsidence and subsurface water conveyance.

The Contractor shall maintain drilling fluid pressure and circulation throughout the HDD process, including during the initial pilot bore and during the reaming process.

The Contractor shall at all times and for the entire length of the installation alignment be able to demonstrate the horizontal and vertical position of the alignment, the fluid volume used, return rates and pressures.

7.04.04 Drilling Fluid Losses to Surface (“Frac-Out”)

To reduce the potential for hydraulic fracturing of the hole during horizontal directional drilling, a minimum depth of cover of 5 m shall be maintained between the top of pipe and the surface of any pavements or beds of water courses. Sections of the pipe close to the entry and exit pit with less than 5 m cover shall be cased. The Contractor shall ensure that drilling fluid pressures are properly set and controlled for the full length of the bore to prevent frac-out for the depth of cover available between the bottom of the pavement structure (bottom of the subbase material) and the top of the bore.

Once a fluid loss or frac-out event is detected, the Contractor shall halt operations immediately and conduct a detailed examination of the drill path and implement measures to collect all fluids discharged to surface, mitigate and prevent additional fluid loss.

7.04.05 Reaming

The bore shall be reamed using the appropriate tools to a diameter at least 50% greater than the outside diameter of the product.

7.04.06 Product Installation

7.04.06.0 General

The product shall be jointed according to manufacturer’s recommendations. The length of the product to be pulled shall be jointed as one length before commencement of the continuous pulling operation.

The product shall be protected from damage during the pullback operation.

The minimum allowable bending radius for the product shall not be contravened.

Product shall be allowed to recover to static conditions from thermal and installation stresses before connections to new or existing facility are made. Product recovery time shall be according to manufacturers recommendations.

7.04.06.02 Pullback and Grouting

After successfully reaming the bore to the required diameter, the product pipe shall be pulled through the bore path. Once the pullback operation has commenced, it shall continue without interruption until the product pipe is completely pulled into bore unless otherwise approved by the Contract Administrator.

A swivel shall be used between the reamer and the product being installed to prevent rotational forces from

being transferred to the product. A weak link or breakaway connector shall be used to prevent excess pulling force from damaging the product.

The product pipe shall be inspected for damage where visible at excavation pits and where it exits the bore. Any damage noted shall be rectified to the satisfaction of the Contract Administrator.

The pull back and reaming operations shall not exceed the fluid circulation rate capabilities. Reaming and back pulling operations shall be planned to insure that, once started, all reaming and back pulling operations are completed without stopping and within the permitted work hours.

The space between the pipe and the walls of the excavated volume shall be filled with grout or slurry with gel strength properties demonstrated to be sufficient to form a semi-solid or solid gap filling material, prevent ground convergence around the pipe and subsequent ground surface subsidence and prevent long-term water flow at the outside boundary of any pipe and ground.

7.05 Tunnelling Installation

7.05.01 General

Excavation of native soil and fill shall be done in a manner to control groundwater inflow to the excavation and to prevent loss of ground into the excavation.

Methods of excavating the tunnel shall be capable of fully supporting the face and shall accommodate the removal of boulders and other oversize objects from the face. Continuous ground support shall be maintained during excavation.

As the excavation progresses, the Contractor shall continuously monitor (every 2 m) indications of support distress, such as cracking, deflection or failure of support system and subsidence of ground near the excavation.

The Contractor shall provide ventilation and lighting in accordance with OHSA requirements for the entire length of the tunnel installed as tunneling progresses.

The tunnel is to be kept sufficiently dry at all times to permit work to be performed in a safe and satisfactory manner.

The Contractor shall maintain clean working conditions at all times in tunnels.

If excavation threatens to endanger personnel, the Work, or adjacent property, the Contractor shall cease excavation and make the excavation face secure. The Contractor shall then evaluate methods of construction and revise as necessary to ensure the safe continuation of the work.

The Contractor shall maintain tunnel excavation line and grade to provide for construction of final lining within specified tolerances.

7.05.01 Tunnelling Method

The tunnelling method shall be suitable to provide face support in changing ground conditions that may be encountered during the progress of the work. The selection of the tunnelling method should consider the soil conditions at each pipe crossing and the presence of obstructions, such as cobbles and boulders, with respect to the tunnel alignment.

7.05.02 Primary Liner (Support System)

Primary support systems shall prevent deterioration, loosening, or unravelling of ground surfaces exposed by excavation.

The primary liner support system shall be designed and installed to achieve the intended performance requirements.

Primary liner support system shall maintain the safety of personnel, minimize ground movement into the excavation, ensure stability and maintain strength of ground surrounding the excavation.

The primary liner shall be designed to support all subsurface conditions and hydrostatic pressures and to withstand any additional loads caused by installation and grouting, and shall ensure that no ground loading or other loading will be placed on the new work until after design strength has been reached.

The primary liner shall be installed so that the exterior is as tight as possible to the excavated surface of the tunnel and allows the placement of the full design thickness of the secondary lining.

Primary support systems shall be compatible with the encountered ground conditions, with the method of excavation, with methods for control of water, and with placement of the permanent lining.

All voids between the primary lining and the wall of the excavated volume shall be filled with cement grout or slurry with gel strength properties demonstrated to be sufficient to form a semi-solid or solid gap filling material, prevent ground convergence around the pipe and subsequent ground surface subsidence and prevent long-term water flow at the outside boundary of any pipe and ground. If an unexpanded liner is used, the space outside the liner plates shall be filled at least daily.

7.05.03 Secondary Liner

7.05.03.01 Placing of Grout

The void outside the finished secondary liner shall be filled with cement grout according to the Contractor's submission.

Grout shall not be placed until the lining has achieved 85% of its specified strength or 30 MPa. Grouting shall be limited to such sequences and programs as are necessary to avoid damaging any part of the works or any other structure or property. Grout mix design shall be chemically and thermally compatible with all pipe systems.

7.06 Microtunnelling

7.06.01 General

Excavation of soil, rock and fill shall be done in a manner to control and prevent groundwater inflow to the tunnel.

The MTBM shall be capable of fully supporting the face and shall accommodate the removal of boulders and other obstructions from the face. Continuous ground support shall be maintained during excavation.

The tunnel is to be kept well drained at all times to permit work to be performed in a safe and satisfactory manner.

The Contractor shall maintain clean working conditions at all times.

In the event that excavation threatens to endanger personnel, the Work, adjacent property, roadways, railways, waterways, or the public in any way, the Contractor shall cease excavation. The Contractor shall then evaluate the methods of construction and revise as necessary to ensure the safe continuation of the Work.

The Contractor shall maintain the tunnel excavation line and grade to provide for construction of the product within the specified tolerances.

7.06.02 Method of Installation

The installation procedure to be used shall be subject to the following limitations:

- The jacking pipe shall be fully supported in the jacking pit at the specified line and grade.
- Selection of the excavation method and jacking equipment shall take into consideration the subsurface conditions within the tunnel alignment.
- Perform microtunnelling operations in a manner that will minimize the movement of the ground in front of and surrounding the tunnel in conformance with the limits listed in the Contract Documents.
- Prevent damage to structures and utilities above and in the vicinity of the microtunnelling operations.
- Excavated diameter should be the minimum size required to permit pipe installation by jacking.
- Whenever there is a condition encountered which could endanger the microtunnel excavation or adjacent structures if tunnelling operations cease, continue to operate without intermission including 24-hour working days, weekends and holidays, until the condition no longer exists.
- Maintain an envelope of lubricant around the exterior of the pipe during the jacking and excavation operation to reduce the exterior soil/pipe friction and possibility of the pipe seizing in place.
- In the event a section of pipe is damaged during the jacking operation or a joint failure occurs, as evidenced by inspection, visible ground water inflow or other observations, the Contractor shall submit for approval his methods for repair or replacement of the pipe.

7.06.03 Casing Installation

Casing must withstand the jacking forces determined by the Contractor.

The space between the Casing and the wall of the excavation shall be kept filled with lubricant during the pipe jacking operation. Upon completion of pipe jacking, the space between the Casing and the wall of the excavation shall be filled with grout that is compatible with the Casing.

The Casing shall act as a support system to maintain the safety of personnel, minimize ground movement into the excavation, ensure stability and maintain strength of ground surrounding the Casing.

The Casing shall be designed to support all subsurface conditions and hydrostatic pressures and to withstand any additional loads caused by installation and grouting.

7.07 Instrumentation and Monitoring

The work specified in this Section includes furnishing and installing instruments for monitoring of settlement (and heave) and ground stability.

7.07.01 Surface Monitoring Points

Surface settlement points for monitoring ground stability shall be installed at the pavement/ground surface level on the shoulder, side slope and pavement at intervals of 5 m or less along the tunnel alignment centreline and as arrays of three points in each shoulder of the highway crossing and centred on the tunnel alignment. The equipment and procedures used for settlement monitoring during construction must be capable of surveying the settlement point elevations to within a repeatability (combined accuracy and precision of equipment and methods) ± 2 mm of the actual elevation.

Surface settlement markers shall be hardened steel markers treated or coated to resist corrosion, with an exposed convex head having a minimum diameter of 12 mm and similar to surveyor's PK nails. Markers shall be rigidly affixed so as not to move relative to the surface to which it is attached. Traffic shall be managed by the contractor using short-term lane closures in accordance with the Ontario Traffic Manual (OTM). Surface markers shall be recessed or otherwise designed for safe passage of vehicles at highway speeds and protected from snow removal equipment in the event that work occurs during snow removal seasons.

7.07.02 In-Ground Monitoring Points

In-ground settlement monitoring points shall be 12-18 mm rebar encased in a 50-70 mm, SCH40 PVC pipe, set to a depth of 1.5 m below ground surface or below frost penetration depth whichever is greater. The assembly shall be placed in a drill hole, backfilled with uniform sand and provided with protective covers suitable for high vehicular traffic areas.

7.07.03 Installation, Replacement and Abandonment

The Contractor shall install all settlement monitoring points a minimum of two weeks prior to the start of works to permit baseline surveying to be completed. The settlement monitoring points shall be clearly labelled for easy field identification. The Contractor shall submit to the Contract Administrator a site plan showing the locations of the monitoring points, a geodetic survey of the settlement monitoring points including station, offset and elevation. Instruments damaged by the Contractor's operations or other causes shall be replaced and surveyed at the time of installation within 24 hours at no additional cost. At the completion of the job, the Contractor shall abandon all instrumentations installed during the course of the Work and restore the surface at instrument locations.

7.07.03 Monitoring and Reporting Frequency

The Contractor shall survey and otherwise obtain elevations of all settlement monitoring points at the following time intervals:

- a) Three consecutive readings at least one week prior to commencement of the work (Baseline Reading);
- b) Once per shift or once daily during tunnelling operations period whichever results in the more frequent reading intervals; and
- c) Weekly after completion of the work for one month, or until such time at which all parties agree that further movement has stopped.

All readings shall be submitted to the Contract Administrator for information purposes on a weekly basis.

Each report shall include all survey data collected in tabular and graphical format as plots of time versus settlement in comparison to survey data collected prior to commencement of the work.

7.07.03 Benchmarks

Two independent benchmarks shall be used for all settlement monitoring surveying and shall be located sufficiently outside the zone of influence such that the benchmarks are not influenced by any trenchless or other construction activity or weather conditions (e.g., frost heave). All surveying shall be reported using the geodetic datum and coordinate system as defined in the Contract Documents.

7.08 Criteria for Assessment of Roadway Subsidence/Heave

Based on the monitoring of ground movement as specified in Subsections 4.02 and 7.07, the following represents trigger levels that define magnitude of movement and corresponding action:

- a) Review Level: If a maximum value of 10 mm relative to the baseline readings is reached, the Contractor shall review or modify the method, rate or sequence of construction or ground stabilization measures to mitigate further ground displacement. If this Review Level is exceeded, the Contractor shall immediately notify the CA and review and discuss response actions. The Contractor shall submit a plan of action to prevent Alert Levels from being reached. All construction work shall be continued such that the Alert Level is not reached.
- b) Alert Level: If a maximum value of 15 mm relative to the baseline readings is reached, the Contractor shall cease construction operations, inform the Contract Administrator and execute pre-planned measures to secure the site, to mitigate further movements and to assure safety of public and maintain traffic. No construction shall take place until all of the following conditions are satisfied:
 - i. The cause of the settlement has been identified.
 - ii. The Contractor submits a corrective/preventive plan.
 - iii. Any corrective and/or preventive measure deemed necessary by the Contractor is implemented.
 - iv. The CA deems it is safe to proceed.

9. MEASUREMENT FOR PAYMENT

Measurement shall be by Plan Quantity Payment as may be revised by Adjusted Plan Quantity Payment in metres, following along the centre line of the pipes from centre to centre of maintenance holes or chambers (catch basins) or from/to the end of the pipe where no maintenance hole or chamber is installed, of the actual length of pipe installed by trenchless methods.

10. BASIS OF PAYMENT

Payment at the contract price shall be full compensation for all labour, equipment and materials required for excavation (regardless of material encountered), dewatering, sheathing and shoring, settlement instrumentation and monitoring, site restoration, and all other work necessary to complete the installation as specified.

Where a protection system is made necessary because of the Contractor's operations (e.g., choice of trenchless installation method), the cost shall be included in this item and shall be full compensation for all labour, equipment and materials required to carry out the work including subsequently removing the temporary protection system and performing any necessary restoration work.

Payment for connecting intercepted drains and service connections shall be made on the following basis:

- (a) Where such drains and service connections are shown on the contract drawings the cost of connections shall be included in the contract price for pipe installation.
- (b) Where such drains and service connections are not shown on the contract drawings, the cost of connections will be considered an allowable extra to the contract.

DEWATERING SYSTEM - Item No.
TEMPORARY FLOW PASSAGE SYSTEM - Item No.

Special Provision No. 517F01

July 2017

Amendment to OPSS 517, November 2016

Design Storm Return Period and Preconstruction Survey Distance

517.01 SCOPE

Section 517.01 of OPSS 517 is deleted in its entirety and replaced with the following:

This specification covers the requirements for the design, operation, and removal of a dewatering or temporary flow passage system or both to control water during construction, and the control of the water prior to discharge to the natural environment and sewer systems.

517.04 DESIGN AND SUBMISSION REQUIREMENTS

517.04.01 Design Requirements

Subsection 517.04.01 of OPSS 517 is amended by deleting the first paragraph in its entirety and replacing it with the following:

A dewatering or temporary flow passage system or both shall be designed to control water at the locations specified in the Contract Documents and at any other location where a system is necessary to complete the work. The design of the system shall be sufficient to permit the work at each location to be carried out as specified in the Contract Documents.

Subsection 517.04.01 of OPSS 517 is further amended by deleting the second last paragraph in its entirety and replacing it with the following:

Temporary flow passage systems shall be designed, as a minimum, for a 2 year design storm return period and groundwater discharge, except for the work specified in Table A. For the work specified in Table A, the temporary flow passage system shall be designed, as a minimum, for the design storm return period specified in Table A and groundwater discharge. A longer return period shall be used when determined appropriate for the work.

Intensity-Duration Factor (IDF) curve location, site specific minimum return period, return period flow estimates, and other information is provided in Table A. The IDF information can be accessed through the MTO IDF Curve Look up Tool on the Drainage and Hydrology page of MTO's website. The return period flow estimates do not include flow volumes from groundwater discharge. The Owner specifically excludes these flow estimates from the warranty in the Reliance on Contract Documents subsection of OPSS 100, MTO General Conditions of Contract.

Table A

IDF Curve Location	Latitude: 48.156935	Longitude: -79.937218				
Temporary Flow Passage Systems						
Site Name / Station Reference	Minimum Return Period (Years)	Return Period Flow Estimates (m ³ /s)				Design Engineer Requirements (Note 1)
		2 Year	5 Year	10 Year	25 Year	
Highway 66, Station 16+994, Township of Lebel Culvert Replacement	***	****	****	****	****	Yes
Dewatering Systems						
Site Name / Station Reference	Preconstruction Survey Distance (Note 2) (m)				Design Engineer Requirements (Note 1)	
Highway 66, Station 16+994, Township of Lebel Culvert Replacement	N/A				Yes	
<p>Note:</p> <p>1. "Yes" means the design Engineer and design-checking Engineer shall have a minimum of 5 years of experience in designing systems of similar nature and scope to the required work. "No" means a minimum experience level is not required for the design Engineer and design-checking Engineer.</p> <p>2. "N/A" indicates a preconstruction survey is not required.</p>						

NOTES TO DESIGNER:

Designer Fill-in for Table A:

- * Enter the latitude and longitude co-ordinates of the IDF Curve as obtained using the MTO IDF Curve Look up Tool. Create additional tables, as necessary, if more than one (1) IDF curve was used on the contract (i.e. on a very long contract there may be two IDF curves used to better represent rainfall events for two (2) different sections of the contract).
- ** Fill-in site name, work, and station reference as appropriate for the dewatering system and/or temporary flow passage system item locations.
- *** For temporary flow passage system item locations, fill-in the minimum design storm return period for the site based on MTO Drainage Design Standard TW-1.
- **** For temporary flow passage system item locations, fill-in the design flow rate estimates for the various return periods.
- ***** Insert "Yes" when recommended by the Foundation Engineer. Insert "No" otherwise.
- ***** Fill-in the required distance for preconstruction survey if recommended by the Foundation Engineer. Fill-in "N/A" if not recommended.

WARRANT: Always with these tender items.

OBSTRUCTIONS – Item No.

Notice to Contractor

The contractor shall be alerted to the presence of cobbles and boulders within the existing embankment fill and within the native cohesionless till soils along the alignment of the Highway 66, Station 16+994, Township of Lebel culvert. In addition, the presence of wood debris and organic material near the fill / native soil interface is anticipated. Consideration of the presence of these obstructions must be made in the selection of appropriate equipment and procedures for open cut excavations and installation of temporary protection systems and/or cofferdams, as necessary.



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