

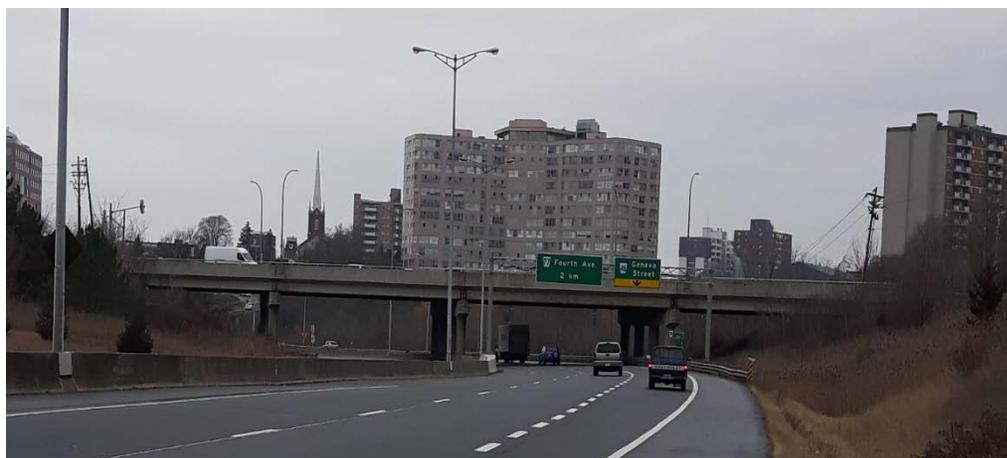


September 22, 2017

## FOUNDATION INVESTIGATION REPORT

### Westchester Avenue Underpass Structure Site 18-172 Highway 406 Structural Rehabilitation from Fourth Avenue to Westchester Avenue St. Catharines Ontario G.W.P. 2453-13-00

**Submitted to:**  
AECOM  
30 Leek Crescent  
Richmond Hill, ON  
L4B 4N4



REPORT

**GEOCRES NO: 30M3-298**

**Report Number: 1541610-2**

**Distribution:**

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## 1.0 INTRODUCTION

Golder Associates Ltd. (Golder) has been retained by AECOM on behalf of the Ministry of Transportation, Ontario (MTO) to provide foundation engineering services for the future rehabilitation of nine structures along Highway 406 from Fourth Avenue to Westchester Avenue in the City of St. Catharines, Ontario.

This report presents the geotechnical site conditions at the site of the existing Westchester Avenue underpass structure at Highway 406. It was developed with information from a foundation investigation completed in 1961 at the Westchester Avenue underpass site, reported as follows:

- **MTO GEOCREs No. 30M3-42:** *Foundation Investigation for Underpass for Hwy #58, (Line 'B') at Westchester Ave, near St. Catharines, W.J. 60-F-100, W.P. 47-61, District #4, dated October 3, 1961.*
- **MTO GEOCREs No. 30M3-42:** *Westchester Avenue Underpass, Hwy #58, Line 'C', Dist. #4, W.P. 47-61, W.J. 60-F-100(B), dated January 8, 1962.*

The Terms of Reference and Scope of Work for the foundation engineering services are outlined in MTO's Request for Proposal, dated September 2015 that form part of the Consultant's Agreement (Number 2014-E-0075) for this project. The Scope of Work for the Westchester Avenue underpass is comprised of a Desktop Study as reported herein. The work has been carried out in accordance with Golder's Supplementary Specialty Plan for this project, dated June 2016.

## 2.0 SITE DESCRIPTION

The Westchester Avenue underpass structure is located about 330 m east of the Geneva Street underpass, near the downtown core of St. Catharines (see Key Plan on Drawing 1). The existing underpass is a four-span structure with a total length of approximately 83 m and was constructed in about 1962.

The underpass structure is located on what was the left bank of the old second Welland Canal (now called Canal Valley; adjacent to Twelve Mile Creek). Westchester Avenue traverse the crest of the Canal Valley and grades gently downward to the east. The *General Plan* (Drawing D4565-1, dated 1961) shows topographic contour lines adjacent to the Westchester Avenue, suggesting that local gullies bounded the east abutment of the underpass. From interpretation of the General Plan 1961 borehole investigation results described in Section 4.2, it is inferred that Westchester Avenue was partially constructed across an infilled gully. Prior to construction of the underpass structure the ground surface in the vicinity of the west abutment was at approximately Elevation 104.8 m, descending to about Elevation 101 m at the east abutment. The current grade of Highway 406 is at about Elevation 100 m; therefore, Highway 406 was constructed in a 1 m to 4 m cut. The grade of the Westchester Avenue underpass is about Elevation 105 m suggesting an approximately 4 m high east approach embankment.

Drawing No. D4965-2 titled "Details of Abutments and Pier Footings" indicates that the west abutment is supported on a spread footing about 2.1 m wide founded at approximate Elevation 101.6 m, on native very stiff silty clay deposit, and the three piers are supported on 3.0 m wide spread footings founded at about Elevation 95.6 m. The east abutment is supported on a pile cap founded on 0.3 m diameter steel tube piles filled with concrete driven to Elevation 88.4 m in the transition between the very stiff silty clay to clay and the underlying firm to stiff clayey silt to silty clay.



Documents obtained from GEOCRETS indicate that in 1965 it was reported that there was observed distortion of the bearings and bending of the anchor bolts at the east abutment; however, there was no observed movement at the west abutment. In 1966 a berm was constructed by infilling a gully adjacent to the east abutment on the north side of the roadway.

Golder visited the site on December 2016, to perform a cursory review of the structure from a foundations perspective. There were no obvious visual signs of foundation-related issues in terms of foundation and embankment instability and settlement at that time.

### **3.0 PREVIOUS INVESTIGATION**

The foundation investigations for the GEOCRETS reports referenced in Section 1.0 were conducted in January and October 1961. During those periods, a skid-mounted drill rig advanced a total of eight boreholes using wash-boring techniques to depths between about 9 m and 32.3 m below the ground surface. Borehole 1 is located east of the structure site and are not relevant to the present desktop study nor shown on the Borehole Locations and Soil Strata.

The foundation investigation report indicates that soil samples were obtained at 0.75 m to 4.5 m depth intervals using 50 mm outside diameter split-spoon samplers driven by manual hammers, in accordance with the Standard Penetration Test (SPT) procedure. In the firm to stiff cohesive deposit thin-walled Shelby tube samples were also taken and in situ field vane testing was conducted to measure the undrained shear strength in the cohesive deposits, Dynamic Cone Penetration Testing (DCPT) was conducted in Borehole 5 from the bottom of the sampled borehole at Elevation 91.2 m to refusal at Elevation 76.7 m, using a 50 mm diameter cone with an energy of 350 ft.lb (475 joules) per blow.

Observations of the water levels in the boreholes were recorded on some boreholes logs; however, piezometers were not installed in any of the boreholes.

Selected samples obtained in from the boreholes were subjected to classification testing, strength testing and consolidation testing and the results are shown on the borehole logs and/or on laboratory test figures attached to the report(s).

The boreholes locations as provided on the borehole record sheets in Station and Off-set were plotted on the General Arrangement drawing (Dwg R2-1, dated Nov. 2016) provided by AECOM on January 18, 2017 and the borehole coordinates were interpreted from the coordinate system superimposed on the plan. The borehole locations in MTM NAD 83 (Zone 10) coordinates, ground surface elevations referenced to Geodetic datum and the drilled depths are as follows:



Borehole No.	MTM NAD83 Northing (m)	MTM NAD 83 Easting (m)	Ground Surface Elevation (m)	Borehole Depth (m) (DCPT Depth (m))
2	4,779,845.1	326,339.9	102.1	32.3
3	4,779,835.0	326,319.0	101.9	29.4
4	4,779,822.5	326,293.0	102.4	26.4
5	4,779,805.2	326,266.0	103.8	12.6 m (12.6 to 27.1)
6	4,779,793.1	326,263.0	104.2	15.7
7	4,779,800.3	326,293.1	104.1	15.7
8	4,779,814.8	326,321.2	101.1	9.0

## 4.0 SITE GEOLOGY AND SUBSURFACE CONDITIONS

### 4.1 Regional Geology

This section of Highway 406 is located within the Iroquois Plain physiographic region, as delineated in the *Physiography of Southern Ontario* (Chapman and Putnam, 1984)<sup>1</sup>. The Iroquois Plain extends around the western shores of Lake Ontario and is comprised of the flat to undulating lakebed and beaches of the former glacial Lake Iroquois, which occupied this area during the last glacial recession. This site is bound to the north by shoreline beach deposits from Glacial lake Iroquois such as the Homer Bar on which downtown St Catharines is located, and the Niagara Escarpment located some 3 km to the south.

Surficial soil in this area of the Iroquois Plain is typically comprised of silty and clayey till of the Halton Till sheet according to the *Quaternary Geology of the Niagara-Welland Area* (Ontario Geological Survey Map 2496; Feenstra, 1984)<sup>2</sup>. The Halton Till sheet is underlain by an older, red, sandy and silty till, possibly the Wentworth Till sheet (OGS Preliminary Map 764, Feenstra 1972)<sup>3</sup>. Shallow depressions on the surface of the clay plain upslope of the Homer Bar are infilled with bog sediments while fill materials comprised of earth and rock fill associated with the canal construction occur in the vicinity of the former Welland Canal (OGS Preliminary Map 764, Feenstra 1972).

### 4.2 Subsoil Conditions

The detailed subsurface soil and groundwater conditions encountered in the boreholes in the vicinity of and pertinent to the Westchester Avenue underpass structure (i.e. Boreholes 2 to 8, inclusive) and the results of in situ and geotechnical laboratory testing, where available from the 1961 and 1962 reports, are given on the borehole records and laboratory test figures contained in Appendix A, following the text of this report. In the discussion below, the depth below ground surface is referenced to the ground surface on the borehole logs at the time of the investigation in 1961 and is not referenced to current ground surface.

<sup>1</sup> Chapman, L.J. and Putnam, D. F. 1984. The Physiography of Southern Ontario, Ontario Geological Survey, Special Volume 2, Third Edition. Accompanied by Map P.2715, Scale 1:600,000.

<sup>2</sup> Feenstra, B.H. 1984. Quaternary Geology of the Niagara-Welland Area. Ontario Geological Survey, Map 2496, Quaternary Geology Series. Scale 1:50,000

<sup>3</sup> Feenstra, B.H. 1972. Quaternary Geology of the Niagara Area, Southern Ontario. Ontario Division of Mines, Preliminary Map P.764, Geological Survey. Scale 1:50,000



#### **4.2.1 Fill**

Fill was encountered in Boreholes 2, 3, 4 and 8 in the vicinity of the east abutment and approach, and east pier of the underpass at the ground surface (in 1961) extending to depths of between about 4.6 m to 8.5 m below ground surface, fill to between about Elevations 97.5 m and 96.5 m. Fill was also encountered in Borehole 5, advanced between the west and central piers, to Elevation 102.3 m (a depth of 1.5 m below ground surface).

The fill material is described as consisting of silty clay to clay and containing, sand, ashes, fine gravel and organic material.

Standard Penetration Test (SPT) “N”-values in the fill are variable, ranging between about 5 and 20 blows per 0.3 m of penetration, suggesting a firm to very stiff consistency. Undrained triaxial tests on the silty clay fill indicated undrained shear strengths ranging between about 40 kPa and 50 kPa.

Atterberg limits tests were carried out on three (3) samples of the cohesive fill material, and measured plastic limits between 24 per cent and 26 per cent, liquid limits between about 43 per cent and 57 per cent, and plasticity indices between about 17 per cent and 31 per cent. These test results indicates that the fill material is classified as silty clay of intermediate plasticity to clay of high plasticity.

The reported average bulk weight of the cohesive material(s) is about 18.5 kN/m<sup>3</sup>. The average water content of the fill ranges between about 27 per cent and 30 per cent.

#### **4.2.2 Silty Clay to Clay**

Underlying the fill, a deposit of silty clay to clay was encountered in Boreholes 2 to 8 between about Elevations 102.3 m and 93.4 m, and extends to between about Elevations 77.1 m and 74.6 m, where fully penetrated. The silty clay to clay contains pockets of fine sand and silt as well as traces of fine gravel. The authors of the 1961 report characterized this as a native deposit; however, comments in the “Summary of Field & Laboratory Tests” information sheets appended to the 1961 report indicated that glass was extracted from a Shelby tube sample obtained from Borehole 3 at a depth of about 9.1 m to 9.6 m below the ground surface (Elevation 92.8 m to 92.3 m), suggesting that a portion of the deposit may be fill. The deposit as shown in the Records of Borehole sheets is considered to be comprising of two layers / zones: an upper crust of higher consistency; and a lower layer of somewhat lower consistency, as described below.

##### ***Upper Weathered Crust – Cohesive Deposit***

The cohesive deposit contains an upper weathered crust that extends to between about Elevations 90.8 m and 88.5 m, suggesting that the thickness of the upper crust is 2.6 m thick at Borehole 2 and about 8.5 m thick at Boreholes 3 and 4. The measured SPT “N”-values range from 7 to 40 blows per 0.3 m of penetration. Two in situ field vane tests carried out within the upper crust of the cohesive deposit measured undrained shear strength of 86 kPa and greater than 95 kPa with sensitivities ranging between 2.9 and 10.0. The field vane test results together with the SPT “N”-values indicates that the upper weathered crust of the cohesive deposit has a stiff to hard consistency.

Atterberg limits tests were carried out on twenty-eight (28) samples of the upper crust of the cohesive deposit, and measured plastic limits between about 20 per cent and 27 per cent, liquid limits between about 47 per cent and 63 per cent, and plasticity indices between about 25 per cent and 38 per cent. These test results indicates that this material is silty clay of intermediate plasticity to clay of high plasticity. One Atterberg limits test was carried out on a clayey silt portion of the deposit and measured a plastic limit of 12 percent, a liquid limit of 24 per cent and a



plasticity index of 12 per cent, suggesting that it was a clayey silt of low plasticity. The results of the Atterberg limits testing are presented on Figures A1 and A2 in Appendix A.

**Firm to Stiff Cohesive Deposit**

Underlying the upper weathered crust, the cohesive deposit is less plastic and was originally classified as a clayey silt to silty clay. In situ field vane shear tests carried out within the cohesive deposit measured undrained shear strengths ranging from about 43 kPa to 86 kPa and the calculated sensitivity varies from about 1.9 to 5.0. Based on the field vane tests the cohesive deposit has a firm to stiff consistency.

Atterberg limits tests were carried out on fifteen (15) samples of the cohesive deposit, and measured plastic limits between about 16 per cent and 23 per cent, liquid limits between about 31 per cent and 56 per cent, and plasticity indices between about 6 per cent and 32 per cent. These test results indicates that this material is a clayey silt of low plasticity to clay of high plasticity. The results of the Atterberg limits testing are presented on Figures A1 and A2 in Appendix A.

Figure A3 presents the envelope of the results of grain size distribution analysis carried out by the DHO.

Undrained triaxial tests were carried out on several samples of the cohesive material(s) from Boreholes 2 and 3; however, it is not clear what depths they were samples obtained from - the results are presented on Figure A4 in Appendix A and the 1961 report indicates that the shear strength varies from 47 kPa to 84 kPa. Laboratory Consolidation tests were conducted on three samples of the silty clay to clay deposit obtained from Boreholes 2 and 3 at Elevations 93.9 m, 88.1 m and 75.6 m and the results are presented on Figure A5 in Appendix A and pertinent parameters are summarized below.

<b>BH/SA Elevation</b>	<b><math>\sigma_v'</math> (kPa)</b>	<b><math>\sigma_p'</math> (kPa)</b>	<b>OCR</b>	<b><math>e_o</math></b>	<b><math>c_c</math></b>	<b><math>c_r</math></b>
Borehole 3 / SA5 El. 93.9 m	84	95	1.1	0.827	0.260	0.077
Borehole 2 / SA10 El. 75 m	158	316	2.0	0.785	0.365	0.058
Borehole 2 / SA15 El. 88.1	229	364	1.6	0.576	0.295	0.068

Figure A6 presents a summary of the Atterberg limits and accompanying water content test results, in situ vane shear strength test results and the calculated overburden pressure and preconsolidation pressure versus elevation.

The reported bulk weight of the clayey silt to clay deposit ranges from about 18.2 kN/m<sup>3</sup> to 20.4 kN/m<sup>3</sup>. The water content of the clayey silt to clay deposit ranges from about 25 per cent to 41 per cent.

**4.2.3 Sandy Clayey Silt Till**

A till deposit consisting of sandy clayey silt was encountered underlying the clayey silt to clay deposit in Boreholes 2, 3 and 4, between Elevations 77.1 m and 75.2 m (between a depth of 25.2 m and 26.8 m below ground surface, at the time of the investigation). It is noted that on the borehole logs for Boreholes 2, 3 and 4 and in the “Summary of Field and Laboratory Tests” in Appendix A this material is described as a silty sand glacial till; however, based on the laboratory classification test results discussed below, the description of this deposit has been reinterpreted



as a sandy clayey silt till. Previous foundation investigations carried out by Golder in the vicinity of the Westchester underpass at Highway 406 indicate that a till deposit comprised of clayey silt is present at about Elevation 75 m, which is consistent with the presently reinterpreted classification of this deposit.

The measured SPT “N”-values within the sandy clayey silt till deposit range from about 36 to 95 blows per 0.3 m of penetration, suggesting a hard consistency. In Borehole 5 a DCPT was carried out from the bottom of the sampled borehole at Elevation 91.2 m to Elevation 76.7 m, where the DCPT value measured was 100 blows for 0.3m of penetration. The termination elevation of the DCPT is within the elevation where the sandy clayey silt till deposit was encountered and therefore it is likely that the DCPT terminated in the till deposit.

Grain size distribution analysis were carried out on samples of the sandy clayey silt till deposit and the envelope of the results is presented on Figure A7 in Appendix A.

Atterberg limits tests were carried out on four (4) samples of the cohesive till deposit, and measured plastic limits between about 12 per cent and 13 per cent, liquid limits between about 17 per cent and 20 per cent and plasticity indices between about 5 per cent and 7 per cent. These test results indicates that this material is a (sandy) clayey silt of low plasticity.

The reported bulk weight of four (4) samples of the sandy clayey silt till is about 22.0 kN/m<sup>3</sup> and the water content of five samples of the deposit ranges between about 8 per cent and 11 per cent.

#### **4.2.4 Groundwater**

The 1961 foundation investigation indicates that groundwater was encountered in Borehole 2 and Borehole 3 at approximate Elevations 100.2 m and 99.9 m, respectively. There was no indication that a piezometer was installed in these boreholes. As such, the groundwater elevations noted in the 1961 reports are inferred to represent the water level immediately after drilling and therefore do not represent stabilized levels.

As discussed in Section 2.0, Highway 406 was constructed in a cut; the water level measured in the boreholes during the 1961 foundation investigation corresponded to an elevation at about the highway grade to about 1 m above the highway grade. At the time of our site visit (December 2016) there was no visible evidence of seepage at the east or west abutment slopes or at the highway level.

The groundwater level is expected to fluctuate seasonally and to be higher during wet periods of the year. The present water level is expected to be lower than at the time of the investigation in 1961.



## 5.0 CLOSURE

This Foundation Investigation Report was prepared by Ms. Sandra McGaghran, M.Eng., P.Eng., a senior geotechnical engineer and Associate of Golder. Mr. Jorge M.A. Costa, P.Eng, a Designated MTO Foundations Contact and a Senior Consultant of Golder, conducted an independent technical and quality control review of this report.

### GOLDER ASSOCIATES LTD.



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Designated MTO Foundations Contact, Senior Consultant

SMM/JMAC/rb

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**METRIC**  
DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN. STATIONS IN KILOMETRES + METRES.

CONT No. GWP No. 2453-13-00

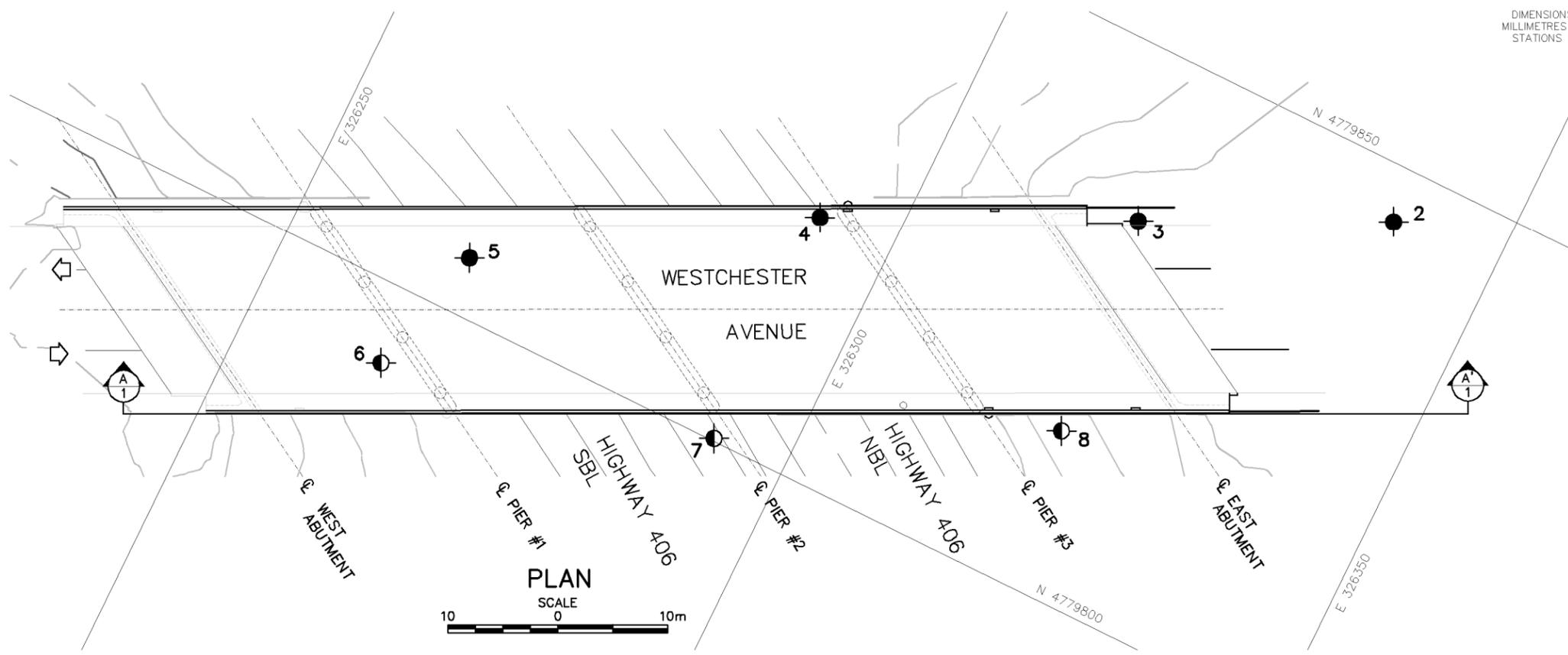
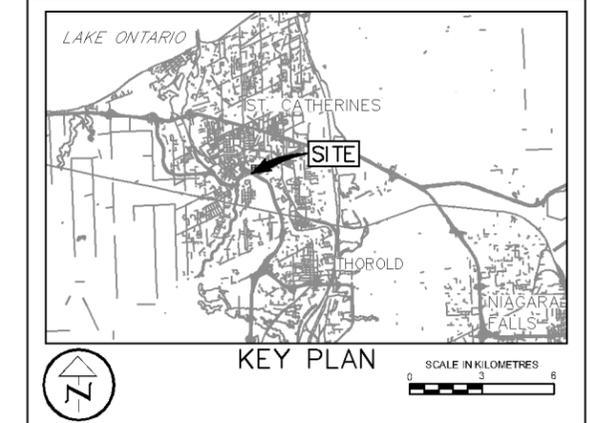


HIGHWAY 406  
WESTCHESTER AVENUE UNDERPASS  
BOREHOLE LOCATIONS AND SOIL STRATA

SHEET



**Golder Associates Ltd.**  
LONDON, ONTARIO, CANADA



**LEGEND**

- Borehole GEOCRETS 30M3-42 (October 1961)
- Borehole GEOCRETS 30M3-42 (January 1962)
- N Standard Penetration Test Value
- 16 Blows/0.3m unless otherwise stated (Std. Pen. Test, 475 j/blow)
- V Insitu Vane Test
- ∇ Water Level on completion or during drilling

No.	ELEVATION	BOREHOLE CO-ORDINATES (MTM ZONE 10)	
		NORTHING	EASTING
2	102.1	4 779 845.1	326 339.9
3	101.9	4 779 835.0	326 319.0
4	102.4	4 779 822.5	326 293.0
5	103.8	4 779 805.2	326 266.0
6	104.2	4 779 793.1	326 263.0
7	104.1	4 779 800.3	326 293.1
8	101.1	4 779 814.8	326 321.2

**NOTES**

This drawing is for subsurface information only. The proposed structure details/works are shown for illustration purposes only and may not be consistent with the final design configuration as shown elsewhere in the Contracts Documents.

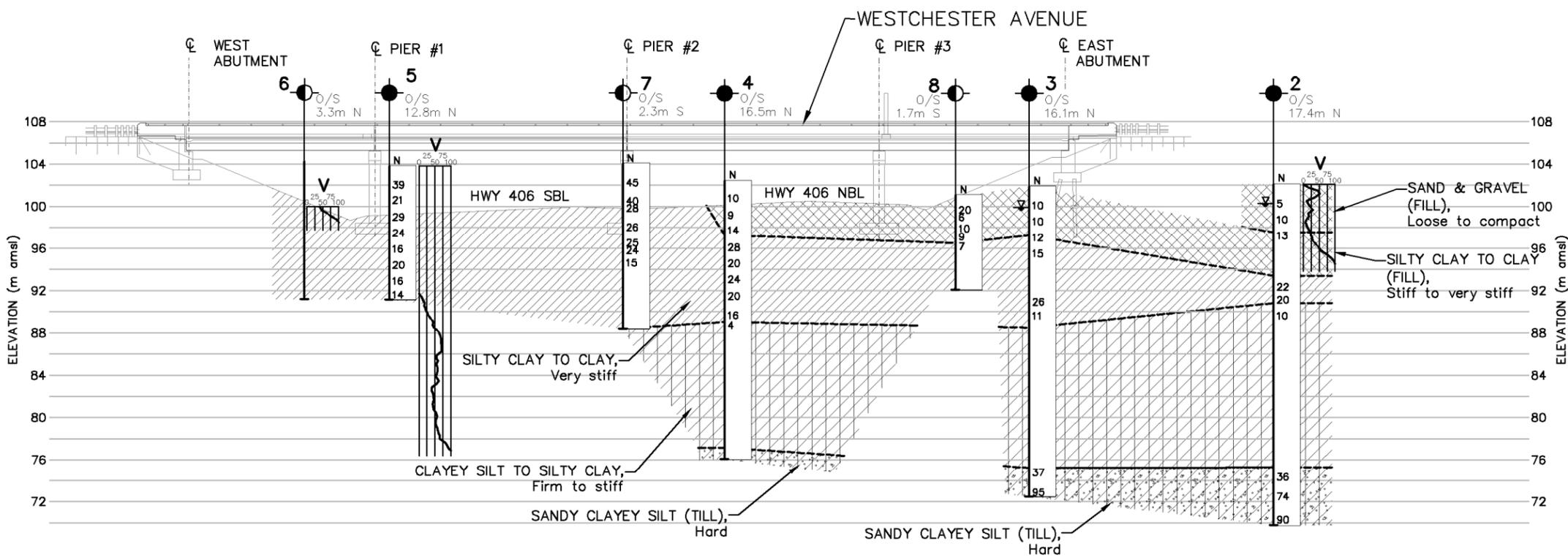
The boundaries between soil strata have been established only at borehole locations. Between boreholes the boundaries are assumed from geological evidence.

**REFERENCE**

The borehole locations as provided on the borehole record sheets in Station and Off-set were plotted on the General Arrangement drawing (Dwg R2-1, dated Nov. 2016) provided by AECOM on January 18, 2017 and the borehole coordinates were interpreted from the coordinate system superimposed on the plan. The borehole locations are approximate. AECOM, Drawing file No. R2-01\_WestChester Ave. Underpass\_GA.dwg, received January 18/17;

D.H.O Foundation Investigation Report (W.J. 60-F-100, W.P. 47-61, Site No. 18-172, dated October 3/61 (GEOCRETS 30M03-042); and

D.H.O Foundation Investigation Report (W.J. 60-F-100(B), W.P. 47-61, Site No. 18-172, dated January 8/62 (GEOCRETS 30M03-042).



**PROFILE ALONG SOUTH EDGE OF WESTCHESTER AVENUE BRIDGE**

HORIZONTAL & VERTICAL SCALE  
10 0 10m



NO.	DATE	BY	REVISION
Geocres No. 30M3-298			
HWY.	HIGHWAY 406	PROJECT NO.	1541610
SUBM'D.	SMM	CHKD.	SMM
DATE:	Sept. 20/17	SITE:	18-172
DRAWN:	DCH	APPD.	JMAC
DWG.	1		



# **APPENDIX A**

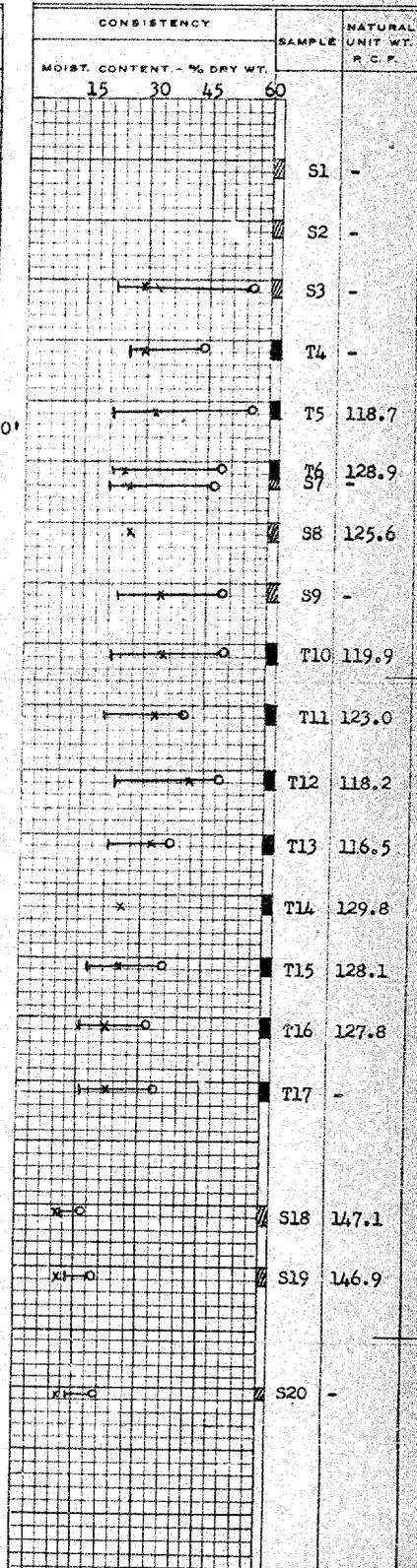
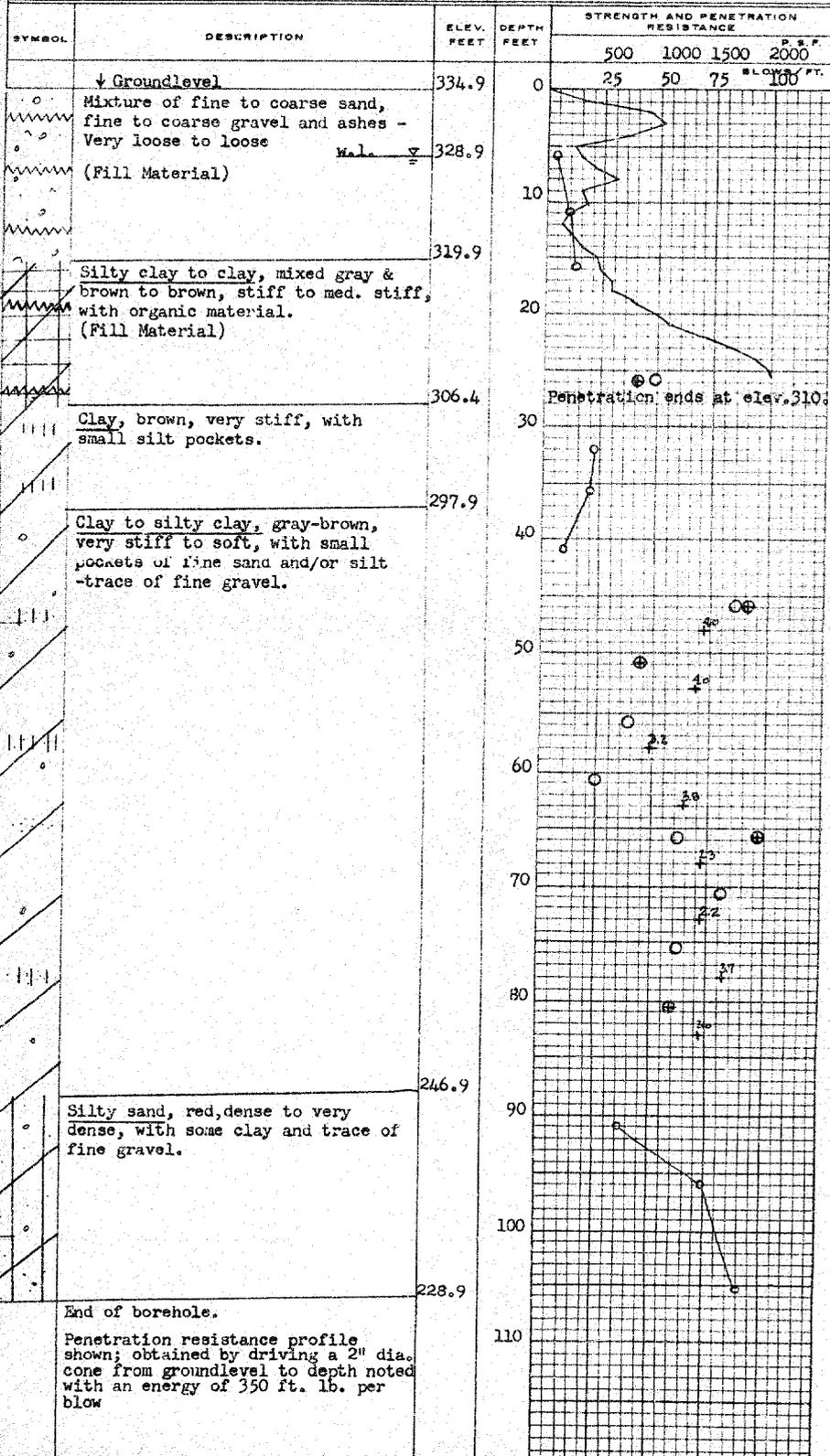
## **GEOCRES 30M3-42 Record of Borehole Sheets and Laboratory Data**

DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 2  
JOB 60-F-100 STATION <sup>(OnTangent)</sup> 254+64 - 98' Rt.  
DATUM 334.9' COMPILED BY B.K.  
BORING DATE Dec. 15/60 CHECKED BY G.G.C.

2" DIA. SPLIT TUBE  
2" SHELBY TUBE  
2" SPLIT TUBE  
2" DIA. CONE  
2" SHELBY CASING

LEGEND  
 Triaxial compression - - - - - O  
 1/2 UNCONFINED COMPRESSION (Qu) ●  
 VANE TEST (C) AND SENSITIVITY (S) +  
 NATURAL MOISTURE AND LIQUIDITY INDEX X  
 LIQUID LIMIT - - - - - O  
 PLASTIC LIMIT - - - - - O  
 Lab vane ⊕



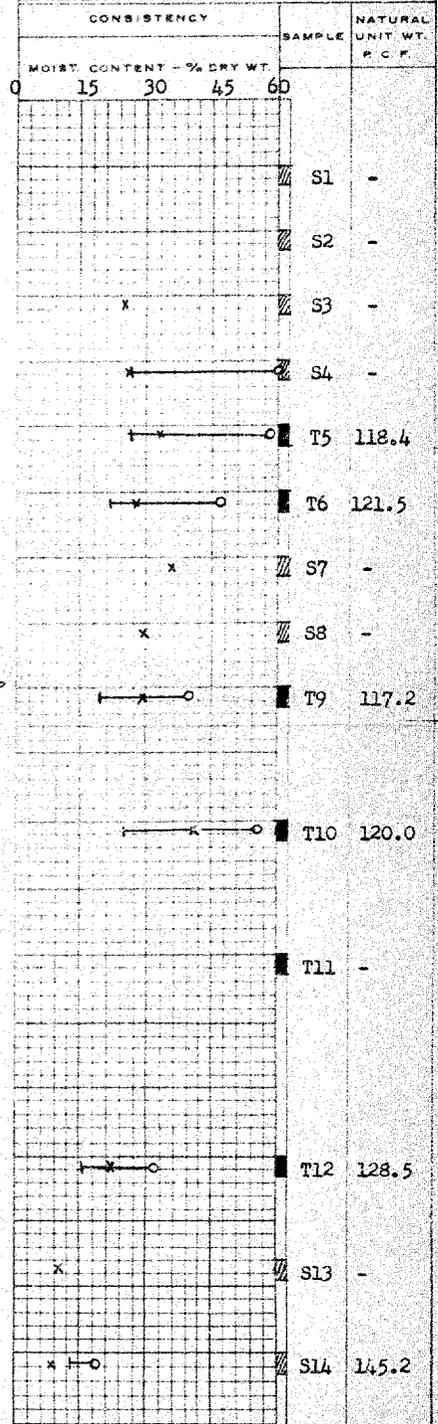
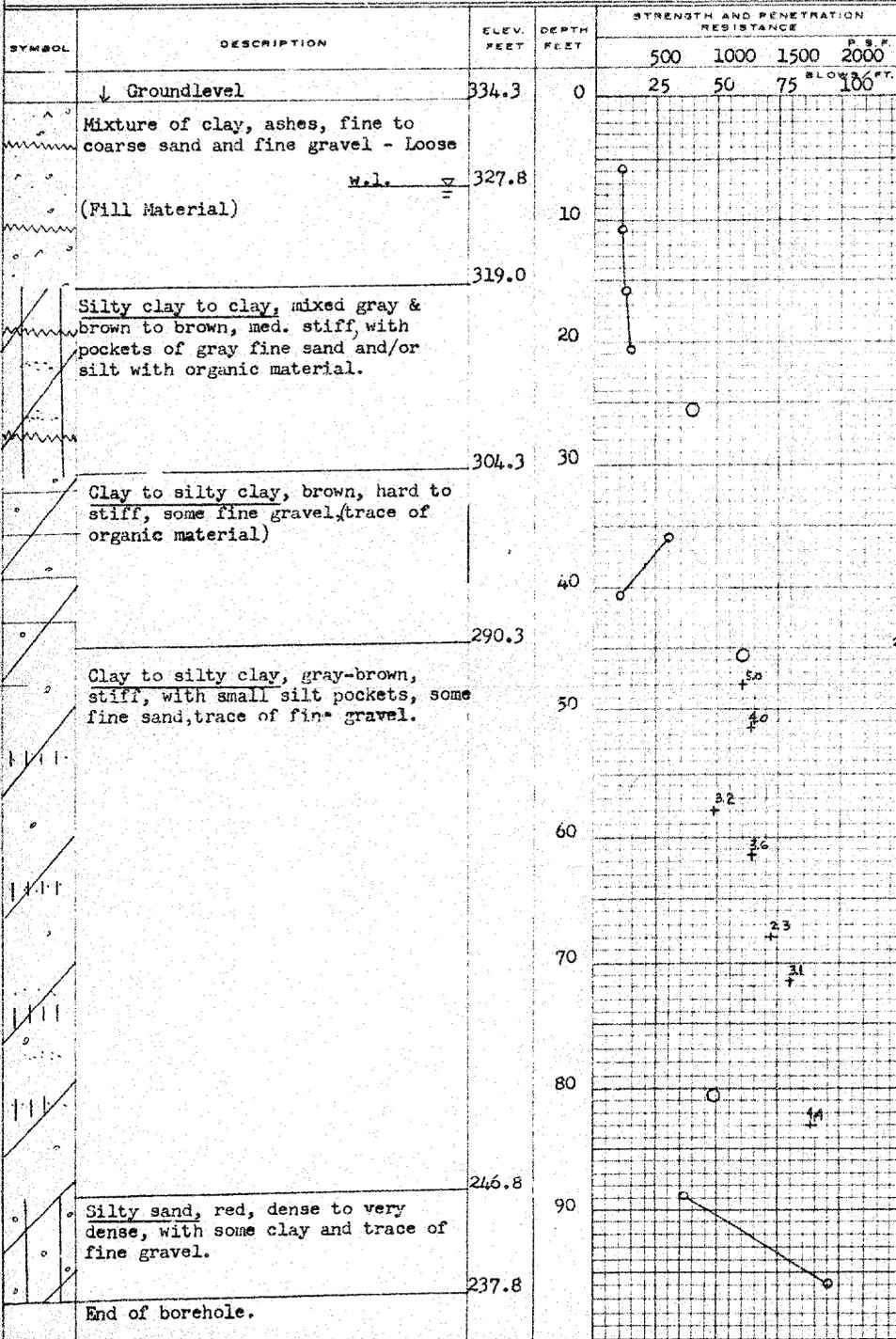
Penetration resistance profile shown; obtained by driving a 2" dia. cone from groundlevel to depth noted with an energy of 350 ft. lb. per blow

DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 3  
JOB 60-F-100 STATION (On Tangent) 224+83.24 Rt.  
DATUM 334.3' COMPILED BY B.K.  
BORING DATE Dec. 21/60 CHECKED BY G.G.C.

2" DIA SPLIT TUBE  
2" SHELBY TUBE  
2" SPLIT TUBE  
2" DIA CONE  
2" SHELBY  
CASING

LEGEND  
Triaxial compression - - - - - O  
1/2 UNCONSOLIDATED COMPRESSION (Qu) - - - - - O  
VANE TEST (C) AND SENSITIVITY (S) - - - - - + S  
NATURAL MOISTURE AND LIQUIDITY INDEX - - - - - X  
LIQUID LIMIT - - - - - O  
PLASTIC LIMIT - - - - - O  
Lab vane - - - - - ⊕

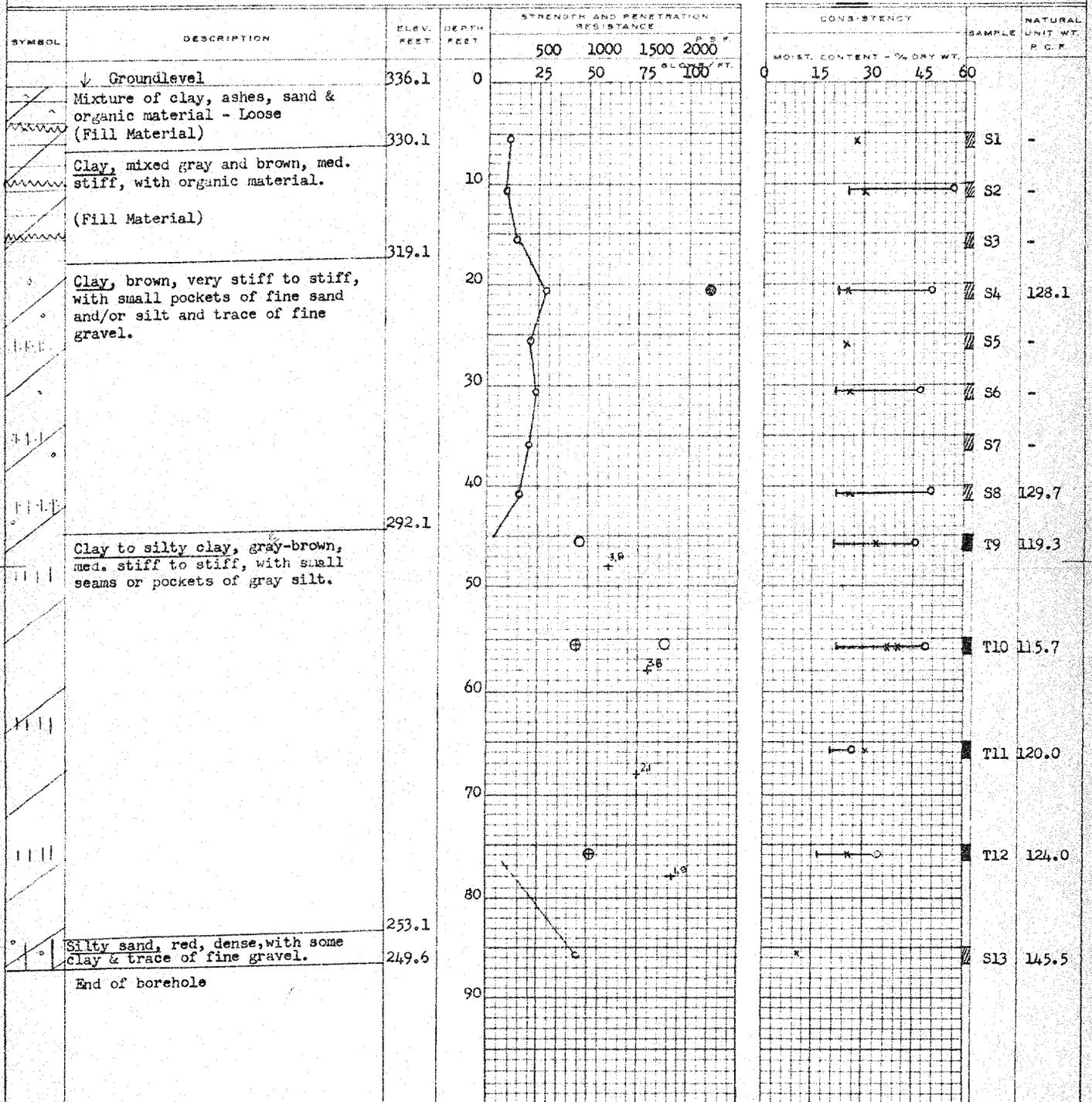


DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 4  
 JOB 60-F-100 STATION 255+07 - 70' Lt.  
 DATUM 336.1' COMPILED BY B.K.  
 BORING DATE Jan. 3/61 CHECKED BY G.G.C.

- 4" DIA SPLIT TUBE
- 2" SHELBY TUBE
- 2" SPLIT TUBE
- 4" DIA CONE
- 2" SHELBY CASING

- LEGEND
- Lab vane
  - 1/2 UNCONFINED COMPRESSION (Qu)
  - WANE TEST (C) AND SENSITIVITY (S)
  - NATURAL MOISTURE AND LIQUIDITY INDEX
  - LIQUID LIMIT
  - PLASTIC LIMIT
  - Triaxial compr.



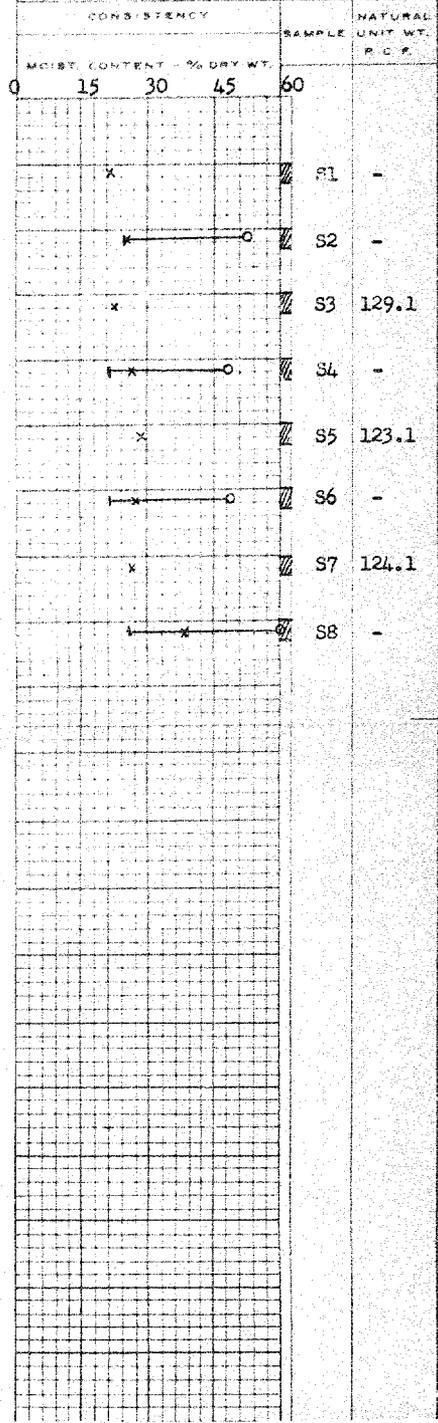
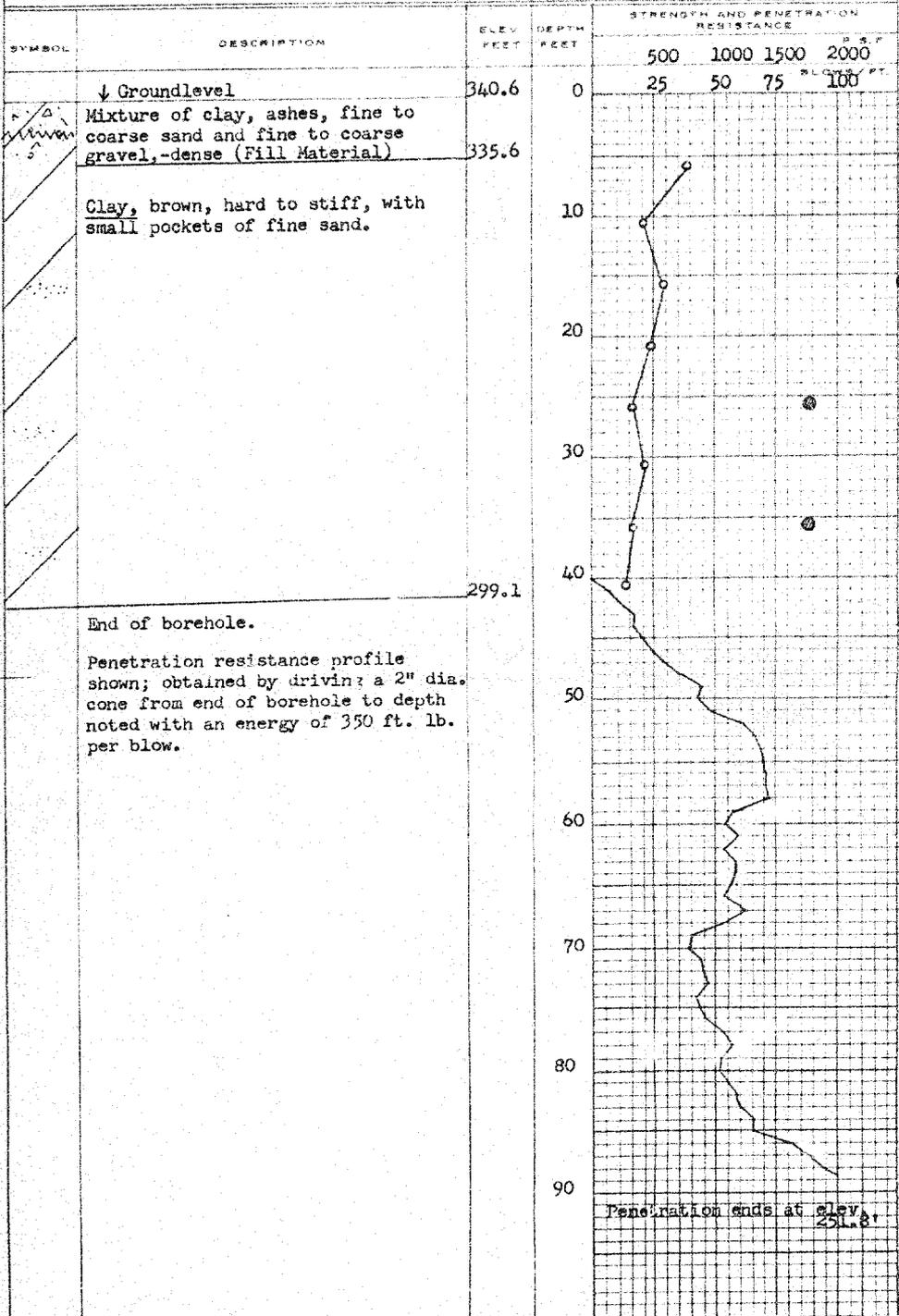
DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 5  
 JOB 60-F-100 (On Tangent) STATION 255+24 175' Lt.  
 DATUM 340.6' COMPILED BY B.K.  
 BORING DATE Jan. 6/61 CHECKED BY G.G.C.

2" DIA SPLIT TUBE  
 2" SHELBY TUBE  
 2" SPLIT TUBE  
 2" DIA. CONE  
 2" SHELBY  
 CASING

LEGEND

1/2 UNCONFINED COMPRESSION (QU) ⊙  
 VANE TEST (C) AND SENSITIVITY (S) ⊕  
 NATURAL MOISTURE AND LIQUIDITY INDEX X  
 LIQUID LIMIT ⊖  
 PLASTIC LIMIT ⊕  
 1/2 Triaxial compression - - - ⊙





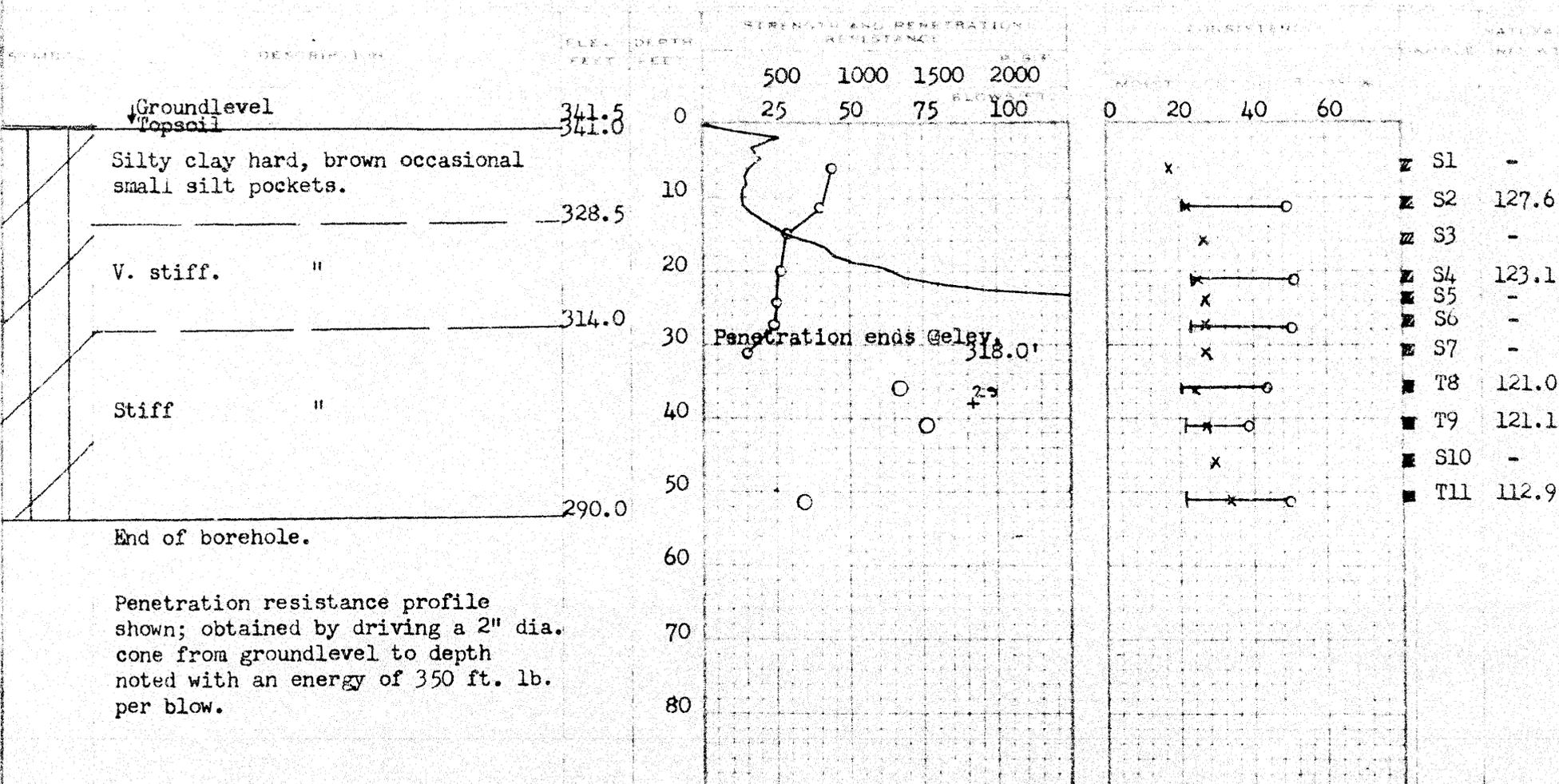
DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 7  
 JOB 60-F-100B STATION 13+90 (39' Rt.)  
 DATUM 341.5' COMPILED BY B.K.  
 BORING DATE Oct. 27/61. CHECKED BY

2" DIA. SPLIT TUBE  
 2" SHELBY TUBE  
 2" SPLIT TUBE  
 2" DIA. CONE  
 2" SHELBY  
 CAS NO.

LEGEND

UNCONFINED COMPRESSION (QU) ○  
 VANE TEST (C AND SENSITIVITY) \*  
 NATURAL MOISTURE AND LIQUIDITY INDEX  
 LIQUID LIMIT  
 PLASTIC LIMIT



Penetration resistance profile shown; obtained by driving a 2" dia. cone from groundlevel to depth noted with an energy of 350 ft. lb. per blow.

DEFECTS IN NEGATIVE DUE TO

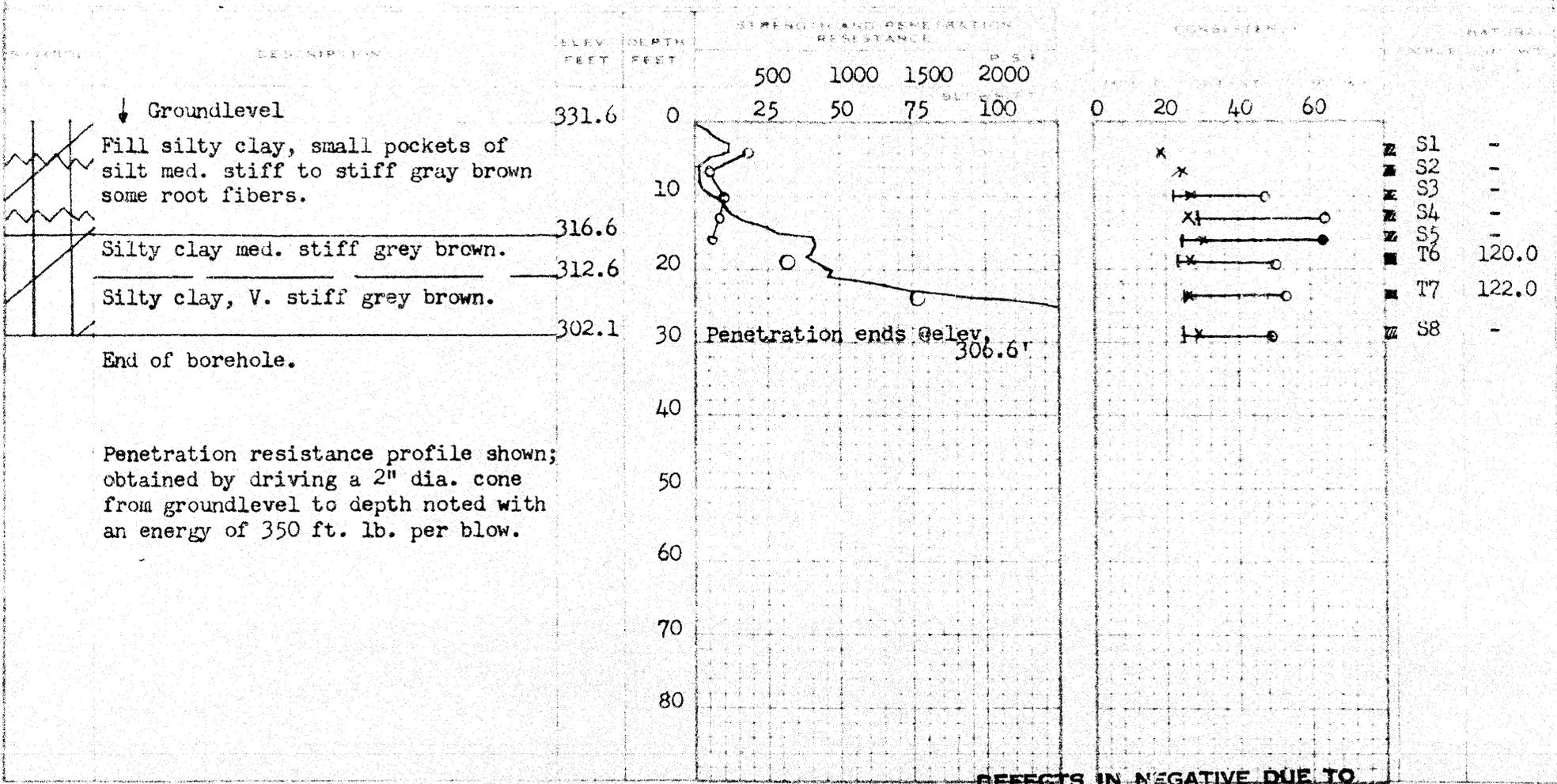
DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS AND RESEARCH SECTION

W.P. 47-61 BORE HOLE NO. 8  
 JOB 60-F-100B STATION 14795 (37' Rt.)  
 DATUM 331.6' COMPILED BY B.K.  
 BORING DATE Oct. 23/61. CHECKED BY

2" DIA. SPLIT TUBE  
 2" SHELBY TUBE  
 2" SPLIT TUBE  
 2" DIA. CORE  
 2" SHELBY  
 CASING

LEGEND

UNSATURATED VERTICAL COMPRESSION (QU)  
 VANE TEST (STANDARD SENSITIVITY)  
 NATURAL MOISTURE AND  
 LIQUID LIMIT  
 PLASTIC LIMIT



SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
1	S1	5'-6.5'	Loose mixture of clay, ashes, sand, and fine to coarse gravel.	13	-	-	-	-	-	
	S2	10'-11.0' 11'-11.5'	As above. Clay, brown, very stiff, flaky.	24	25.6	-	-	-	-	
	S3	11.5'-13'	Clay, brown, hard, flaky.	34	-	-	-	-	-	
2	S1	5'-6.5'	Very loose mixture of ashes, sand and fine to coarse gravel.	4	-	-	-	-	-	
	S2	10'-11.5'	Loose mixture of fine to coarse sand and fine gravel.	9	-	-	-	-	-	
	S3	15'-16.5'	Silty clay, mixed gray & brown, stiff, with trace of fine gravel.	12	28.8	23.6	55.5	-	-	
	T4	20'-21.5'	Silty clay, brown, stiff with some fine sand and organic material.	Levered	29.7	25.8	43.4	-	-	
	T5	25'-26.5'	Clay, brown, med. stiff, with seams of silty clay and some organic material.	Levered	31.7	22.2	55.2	TR=975	118.7	
	VANE	28'	As above.	-	-	-	-	V=852 >2000	-	Sens: >10.0
	T6	30'-30.8'	Clay, brown, very stiff, flaky.	Levered 7 1/2" S/1 1/2"	24.5	22.2	48.3	-	128.9	Fine roots present in sample.
	S7	30.8'-32.3'	As above.	25	25.7	21.0	46.5	-	-	
S8	35'-36.5'	Clay, brown, very stiff, flaky, with small silt pockets.	20	25.7	-	-	-	125.6		

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
2	S9	40'-41.5'	Clay, gray-brown, very stiff, with small silt pockets.	8	33.5	23.2	48.6	-	-	
	VANE	43.5'	Clay gray-brown, very stiff, flaky.	-	-	-	-	>2000	-	Sens: >5.0
	T10	45'-46.5'	Clay gray-brown, stiff, flaky with seams of clay silt.	Levered	33.8	22.0	49.6	TR=1700 V=1810	119.9	
	VANE	48'	Clay, gray-brown, stiff.	-	-	-	-	1440	-	Sens: 4.0
	T11	50'-51.5'	Clay, gray-brown, med. stiff, with silt pockets.	Levered	32.7	20.5	39.7	V=866	123.0	
	VANE	53'	Clay, gray-brown, stiff.	-	-	-	-	1360	-	Sens: 4.0
	T12	55'-56.5'	Clay, gray-brown, med. stiff, flaky, with small silt pockets.	Levered	41.0	23.1	48.3	TR=770	118.2	
	VANE	58'	Clay, gray-brown, med. stiff.	-	-	-	-	960	-	Sens: 3.2
	T13	60'-61.5'	Silty clay, gray-brown, soft, with silt pockets.	Pushed	32.2	22.0	37.2	TR=480	116.5	
	VANE	63'	Clay, gray-brown, stiff, with layers of very silty clay.	-	-	-	-	1280	-	Sens: 3.8
	T14	65'-66.5'	Silty clay, gray-brown, stiff, with pockets of fine sand and trace of fine gravel.	Pushed	25.2	-	-	TR=1230 V=1935	129.8	Fragments of red shale in sample.

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH P.S.F.	UNIT WEIGHT P.C.F.	REMARKS
2	VANE	68'	Silty clay, gray-brown, stiff, with trace of fine gravel and fragments of red shale.	-	-	-	-	1440	-	Sens: 2.3 Fragments of red shale in sample.
	T15	70'-71.5'	Silty-clay, gray-brown, stiff, with trace of fine sand & fine gravel-fragments of red shale.	Levered	25.0	17.8	35.8	TR=1620	128.1	
	VANE	73'	As above.	-	-	-	-	1440	-	Sens: 2.2
	T16	75'-76.5'	" "	Levered	22.2	15.6	30.9	TR=1230	127.8	
	VANE	78'	" "	-	-	-	-	1640	-	Sens: 3.7
	T17	80'-81.5'	" "	Levered	22.7	16.5	33.6	V=1168	-	
	VANE	83'	" "	-	-	-	-	1440	-	Sens: 3.0
	S18	90'-91.5'	Silty sand, red-brown, dense with some clay and trace of fine gravel.	36	10.8	12.2	17.0	-	147.1	Glacial Till.
	S19	95'-96.5'	Silty sand, red-brown, very dense with some clay and trace of fine gravel.	74	10.2	12.3	18.9	-	146.9	Glacial Till.
	S20	105'-106'	As above.	90	10.6	13.1	20.1	-	-	Glacial Till.

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH PSI	UNIT WEIGHT P.C.F.	REMARKS
3	S1	5'-6.5'	Loose mixture of clay, sand, ashes, and some fine gravel.	11	-	-	-	-	-	
	S2	10'-11.5'	As above.	11	-	-	-	-	-	
	S3	15'-15.25'	As above.							
		15.25-16.5'	Clay, mixed gray & brown, with fine sand and fine gravel. (trace of organic material)	12	25.5	-	-	-	-	
	S4	20'-21.5'	Silty clay, brown, med. stiff, with trace of fine sand & fine gravel.	14	26.2	27.1	60.1	-	-	Fine roots present in sample.
	T5	25'-26.5'	Clay, brown, med. stiff, with pockets of gray fine sand & silt. (trace of organic material)	Levered	33.0	26.1	58.0	TR=790	118.4	
	T6	30'-31.25'	Clay, brown, hard, with fine gravel.	Levered	27.2	21.3	46.8	-	121.5	Pieces of broken glass in sample.
	S7	35'-36.5'	Clay, brown, hard, mixed with decayed wood fibers.	30	35.4	-	-	-	-	
	S8	40'-41.5'	Silty clay, brown, stiff.	10	29.8	-	-	-	-	Fine roots present in sample.
T9	45'-46.5'	Clay, gray-brown, stiff, with small silt pockets.	Levered	29.6	19.8	39.2	TR=1200 V=2940	117.2		
VANE	48'	As above.	-	-	-	-	1200	-	Sens: 5.0	
VANE	51.5'	As above.	-	-	-	-	1280	-	Sens: 4.0	

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT.	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS	
3	T10	55'-56.5'	Clay, gray-brown, stiff, with small silt pockets.	Pushed	41.1	24.5	55.8	-	120.0		
	VANE	58'	As above.	-	-	-	-	960	-	Sens: 3.2	
	VANE	61.5'	As above.	-	-	-	-	1280	-	Sens: 3.6	
	T11	65'-66.5'	Clay, gray-brown, stiff, with some silt and fine sand. - trace of fine gravel.	Levered	-	-	-	-	-		
	VANE	68'	As above.	-	-	-	-	1440	-	Sens: 2.3	
	VANE	71.5'	As above.	-	-	-	-	1600	-	Sens: 3.1	
	T12	80'-81.5'	Silty clay, gray-brown, stiff, with some fine sand.	Levered	22.2	16.2	32.1	TR=995	128.5		
	VANE	83'	As above.	-	-	-	-	1760	-	Sens: 4.4	
	S13	88'-89.5'	Silty sand, red-brown, dense, with some clay.	37	9.6	-	-	-	-	-	Glacial Till.
	S14	95'-96.5'	Silty sand, red-brown, very dense, with some clay.	95	8.6	12.1	19.6	-	145.2	-	Glacial Till.

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
4	S1	5'-6.5'	Loose mixture of clay, sand, ashes & organic material.	10	27.4	-	-	-	-	
	S2	10'-11.5'	Clay, mixed gray & brown, med. stiff containing organic material.	9	30.0	25.9	56.8	-	-	
	S3	15'-16.5'	As above.	14	-	-	-	-	-	
	S4	20'-21.5'	Clay, brown, very stiff, flaky, with small pockets of sand & silt.	29	25.0	23.1	50.3	U=2210	128.1	
	S5	25'-26.5'	Clay, brown, very stiff, flaky, with small pockets of fine sand.	20	25.0	-	-	-	-	
	S6	30'-31.5'	Clay, brown, very stiff, flaky, with trace of fine sand & fine gravel.	23	26.0	21.6	47.3	-	-	
	S7	35'-36.5'		20	-	-	-	-	-	Sample Lost.
	S8	40'-41.5'	Clay, brown, stiff, flaky.	15	25.0	22.4	50.5	-	129.7	
	T9	45'-46.5'	Clay, gray-brown, med. stiff, flaky.	Levered	33.3	21.0	45.3	TR=900	119.3	
VANE	48'	Clay, gray-brown, stiff, flaky.	-	-	-	-	V=906 1200	-	Sens: 3.8	

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
4	T10	55'-56.5'	Clay, gray-brown, stiff, with small silt pockets.	Levered	40.6 37.0	22.4	49.8	TR=1755 V=886	115.7	
	VANE	58'	As above.	-	-	-	-	1600	-	Sens: 3.8
	T11	65'-66.5'	Silty clay, gray-brown, stiff, with seams of clay silt.	Levered	30.7	20.6	26.2	-	120.0	
	VANE	68'	As above.	-	-	-	-	1520	-	Sens: 2.1
	T12	75'-76.5'	Silty clay, gray-brown, stiff, with some fine sand.	Levered	24.8	16.6	34.5	V=1020	124.0	
	VANE	78'	As above.	-	-	-	-	1840	-	Sens: 1.9
	S13	85'-86.5'	Sandy silt, red-brown, dense, with some clay and trace of fine gravel.	44	10.4	-	-	-	145.5	Glacial Till.
5	S1	5'-5.5'	Dense mixture of clay, ashes, fine to coarse sand and fine to coarse gravel. Clay, brown, hard, flaky.	38	21.6	-	-	-	-	
		5.5'-6.5'								
	S2	10'-11.5'	Clay, brown, very stiff, flaky.	21	25.0	24.1	53.5	-	-	
	S3	15'-16.5'	Clay, brown, very stiff, flaky, with trace of fine sand.	28	23.3	-	-	U=2680	129.1	
	S4	20'-21.5'	Clay, brown, very stiff, flaky, with pocket of fine sand & trace of fine gravel.	24	26.0	22.4	48.1	-	-	

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60- F-100

W.P. 47-61

HOLE NO.	SAMP NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS FT	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
5	S5	25'-26.5'	Clay, brown, stiff, flaky.	17	28.8	-	-	U=1755	123.1	
	S6	30'-31.5'	Clay, brown, stiff, flaky, with small pockets of red fine sand.	21	27.4	21.2	48.9	-	-	
	S7	35'-36.5'	Clay, brown, stiff, flaky, with trace of fine sand.	17	26.8	-	-	U=1760	124.1	
	S8	40'-41.5'	Clay, brown, stiff, flaky, with small pockets of fine gray sand.	13	38.4	25.0	63.0	-	-	
			S denotes split spoon sample							
			T " Shelby tube "							
			U " unconfirmed compression							
			V " lab vane							
			TR " triaxial compression							

## SUMMARY OF FIELD &amp; LABORATORY TESTS

JOB 60-F-100BW.P. 47-61

HOLE NO.	SAMP. NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS/FT.	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
6	S1	10'-11.5'	Silty clay, v. stiff grey-brown.	24	25.2	23.4	49.3	-	-	
	T2	20'-21'	"	Levered	27.2	20.6	47.2	-	-	
	S3	25'-26.5'	"	20	27.1	-	-	-	-	
	T4	30'-31'	"	Levered	26.9	23.4	47.7	2665	125.4	
	T5	35'-36'	"	Levered	28.0	-	-	-	-	
	T6	40'-41.5'	Silty clay, stiff grey-brown.	Levered	29.0	20.9	42.7	1535	123.4	
	T7	45'-46.5'	"	Levered	35.9	-	-	-	-	
	T8	50'-51.5'	"	Levered	17.7	13.8	23.9	1930	135.3	
7	S1	5'-6.5'	Silty clay, hard brown occasional small silt pockets.	44	17.4	-	-	-	-	
	S2	10'-11.5'	"	39	22.3	21.2	49.4	-	127.6	
	S3	15'-16.5'	Silty clay with traces of fine gravel and occasional silt pockets - v. stiff-grey brown.	28	26.1	-	-	-	-	
	S4	20'-21.5'	"	27	24.8	23.4	51.1	-	123.1	
	S5	23'-24.5'	"	25	26.7	-	-	-	-	
	S6	26'-27.5'	"	24	26.8	23.0	50.1	-	-	
	S7	30'-31.5'	Silty clay, stiff, grey-brown.	15	26.5	-	-	-	-	

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100B

W.P. 47-61

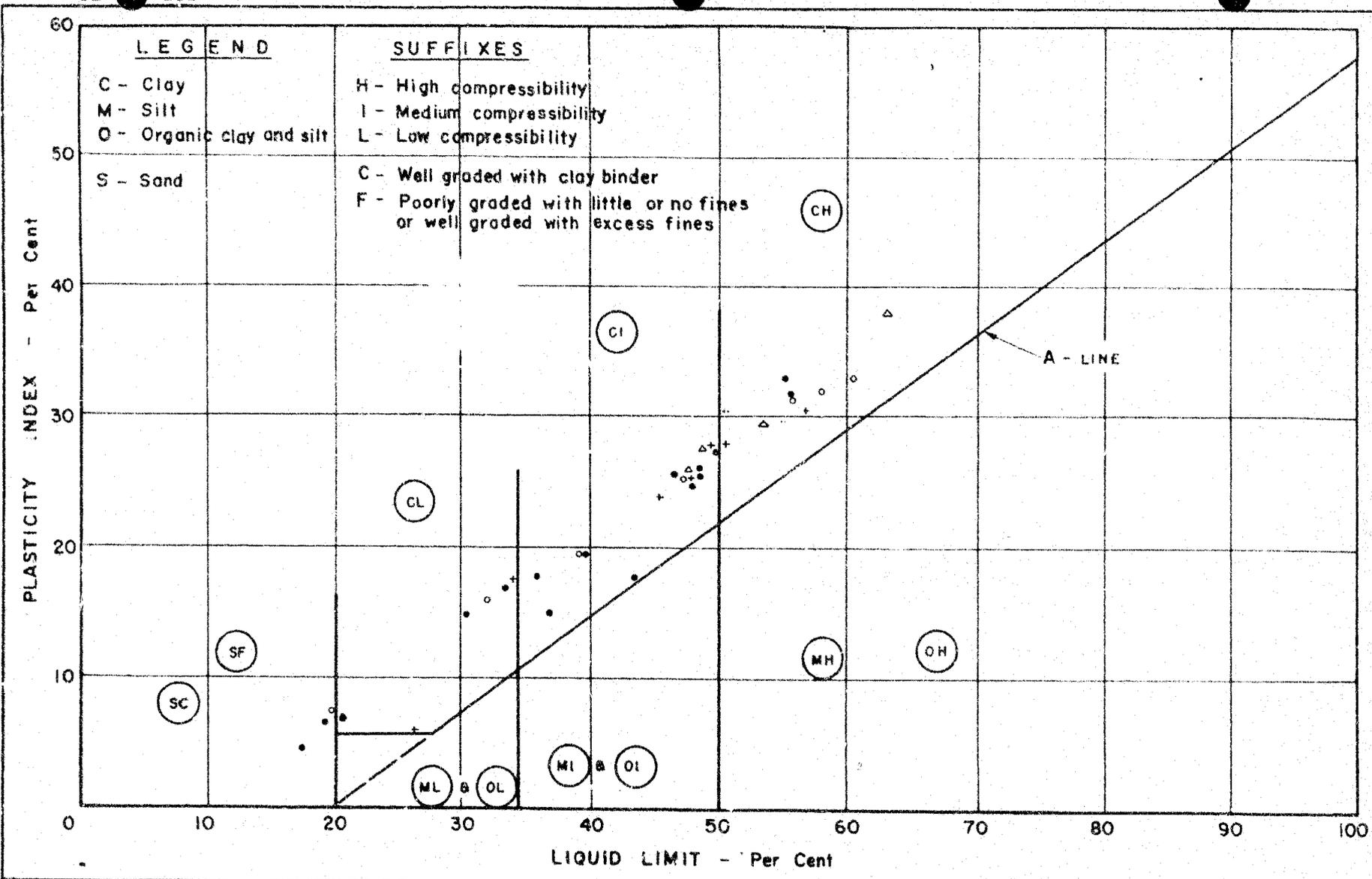
HOLE NO.	SAMP. NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS/FT.	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
7	VANE	33'		-	-	-	-	>2000	-	Sens: >2.3
	T8	35'-36.5'	Silty clay, stiff, grey-brown.	Levered	24.7	20.7	43.6	1330	121.0	
	VANE	38'		-	-	-	-	1840	-	Sens: 2.9
	T9	40'-41.5'	"	Levered	27.4	21.5	38.7	1520	121.1	
	S10	45'-46.5'	"	11	29.6	-	-	-	-	
	T11	50'-51.5'	"	Levered	34.7	21.4	50.0	685	112.9	
8	S1	3'-4.5'	Silty clay, small pockets of silt, some root fibers, med. stiff to stiff. (fill)	18	19.2	-	-	-	-	
	S2	6'-7.5'	"	5	24.4	-	-	-	-	
	S3	9'-10.5'	"	10	27.0	22.0	49.5	-	-	
	S4	12'-13.5'	"	7	26.0	28.1	64.0	-	-	
	S5	15'-16.5'	Silty clay, med. stiff, grey-brown with some topsoil.	6	30.6	24.6	63.4	-	-	
	T6	18'-19.5'	Silty clay, med. stiff, grey-brown.	Levered	26.9	23.4	50.3	640	120.0	
	VANE	21'		-	-	-	-	>2000	-	Sens: >7.1
	T7	23'-24'	Silty clay, stiff, grey brown.	Levered	26.0	25.3	54.9	1540	122.0	

SUMMARY OF FIELD & LABORATORY TESTS

JOB 60-F-100B

W.P. 47-61

HOLE NO.	SAMP. NO.	SAMPLE DEPTH (FEET)	MATERIAL DESCRIPTION	PENET'N RESIST. BLOWS/FT.	MOIST. CONT. %	PLASTIC LIMIT %	LIQUID LIMIT %	SHEAR STRENGTH p.s.f.	UNIT WEIGHT p.c.f.	REMARKS
8	S8	28'-29.5'	Silty clay, v. stiff.	21	28.6	24.2	49.6	-	-	
			S denotes split spoon sample. T " shelby tube "							



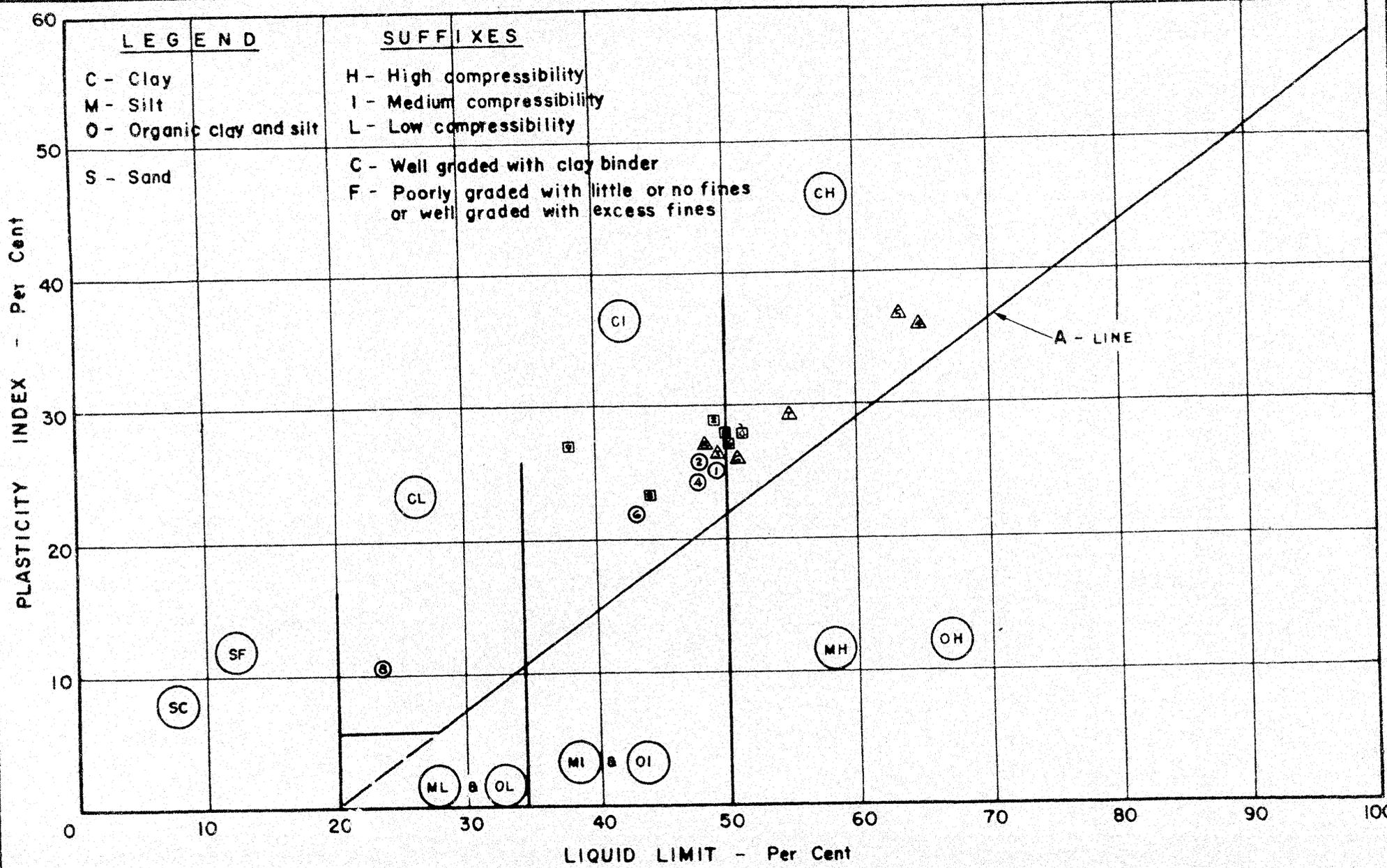
NOTES

- - B.H.#2
- - B.H.#3
- + - B.H.#4
- △ - B.H.#5

Figure A1

DEPARTMENT OF HIGHWAYS - ONTARIO  
 MATERIALS & RESEARCH SECTION  
 PLASTICITY CHART

Job No. 60-F-100      W.P. No. 47-61  
 Location Hwy. 56 at Westchester Ave.



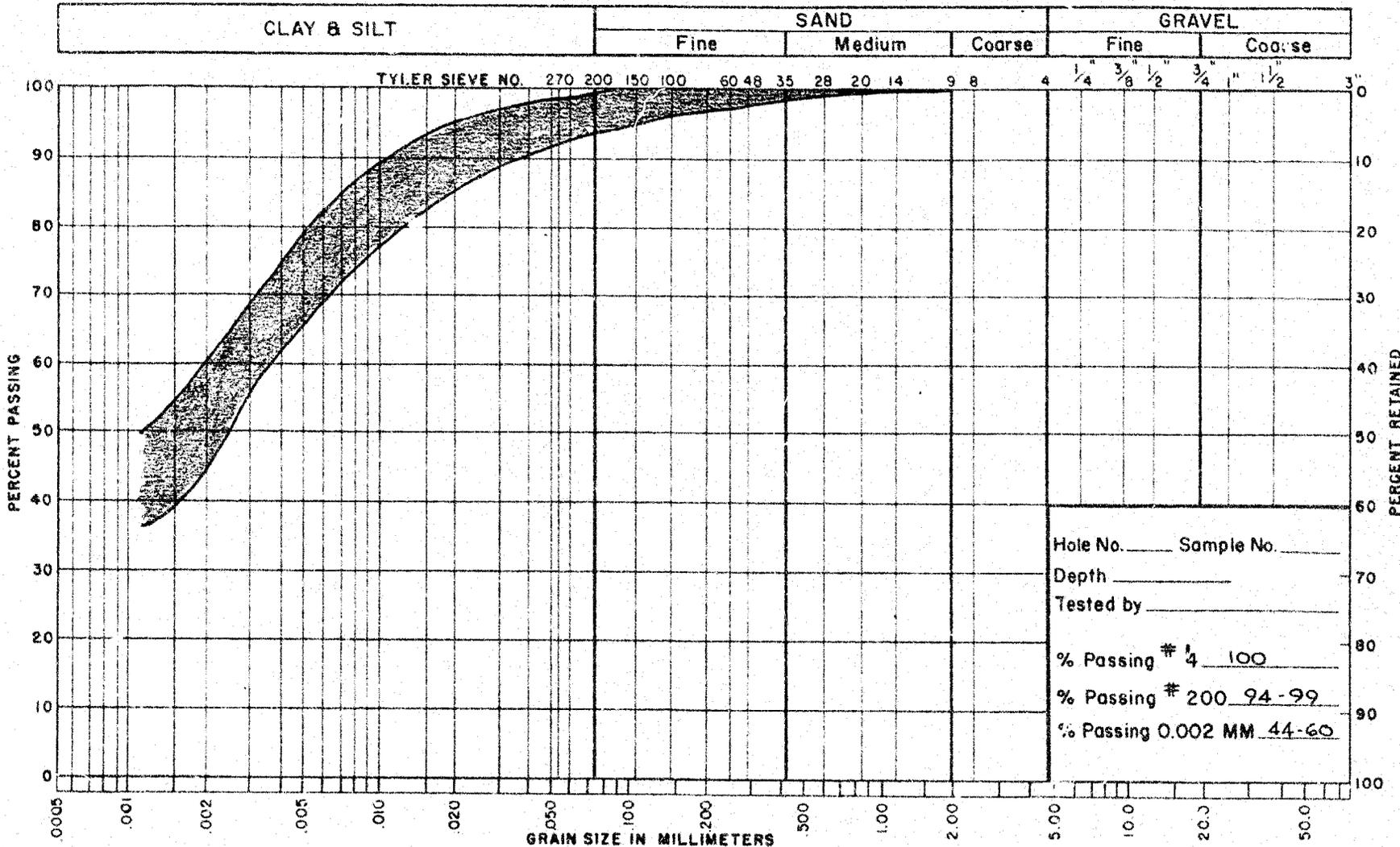
**NOTES**

BORE HOLE NO 6 - SAMPLES	①	②	④	⑥	⑧
" " " 7 -	②	④	⑥	⑧	⑩
" " " 8 -	①	③	⑤	⑦	⑨

Figure A2

DEPARTMENT OF HIGHWAYS - ONTARIO  
 MATERIALS & RESEARCH SECTION  
**PLASTICITY CHART**  
 Job No. 60-F-100 B      W.P. No. 47-61  
 Location WESTCHESTER CRES.

### UNIFIED SOIL CLASSIFICATION SYSTEM



NOTES Composite Grading Chart  
 Brown to Grey Brown Clay to Silty Clay  
 (C.L. - C.I. - C.H. Material)

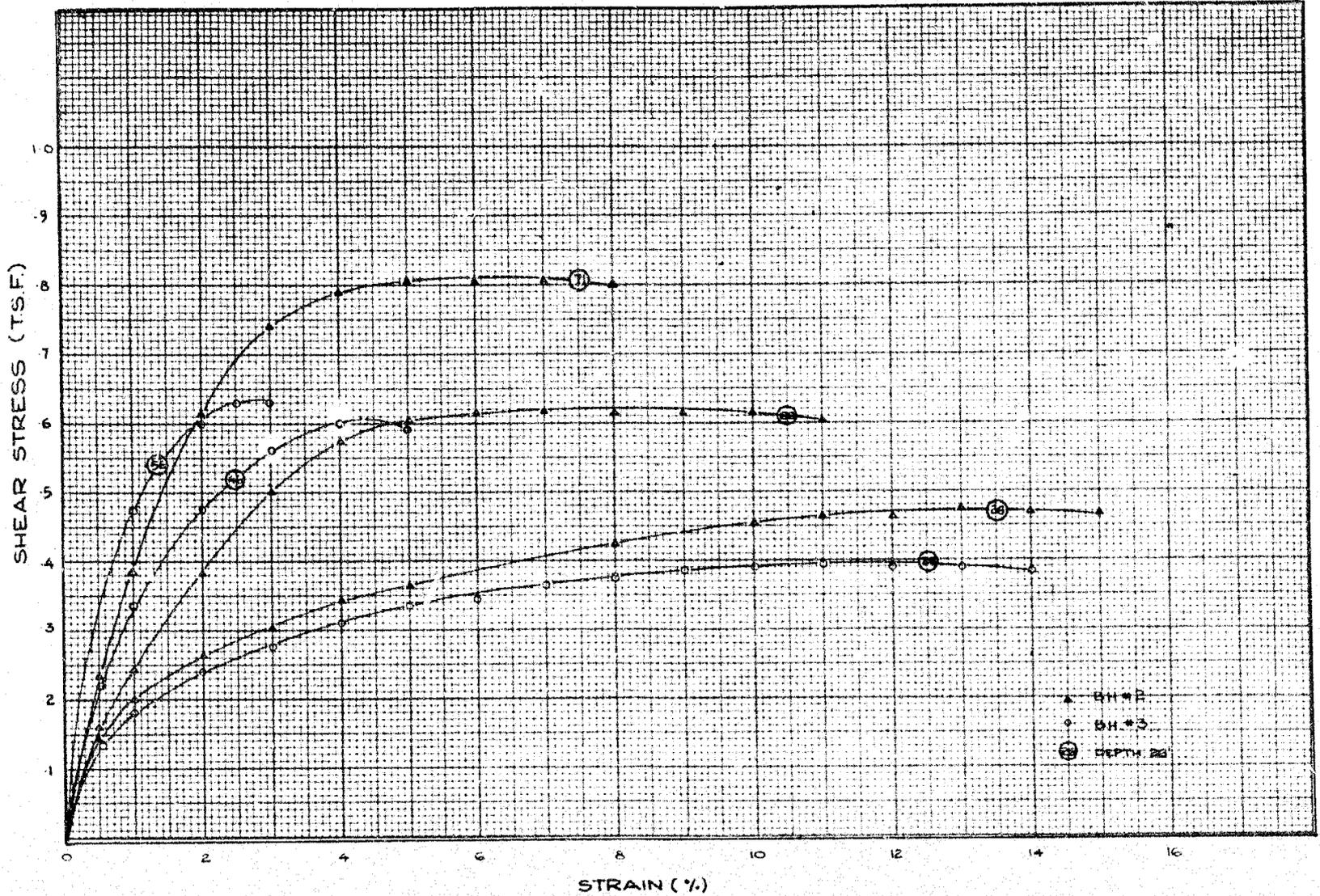
Figure A3

DEPARTMENT OF HIGHWAYS - ONTARIO  
 MATERIALS & RESEARCH SECTION  
**GRAIN SIZE DISTRIBUTION**

Job No. 60-F-100 W.P. No. 47-61  
 Location Hwy. 58 at Westchester Ave.

STRESS - STRAIN CURVES

Figure A4



W.P. 47-61  
JOB 60-F-100  
Hwy. 58 at Westchester Ave.  
BORE HOLES 2 & 3

TYPICAL e - LOG P CURVES

W.P. 47-61 JOB 60-F-100 HWY 58 AT WESTCHESTER AVENUE

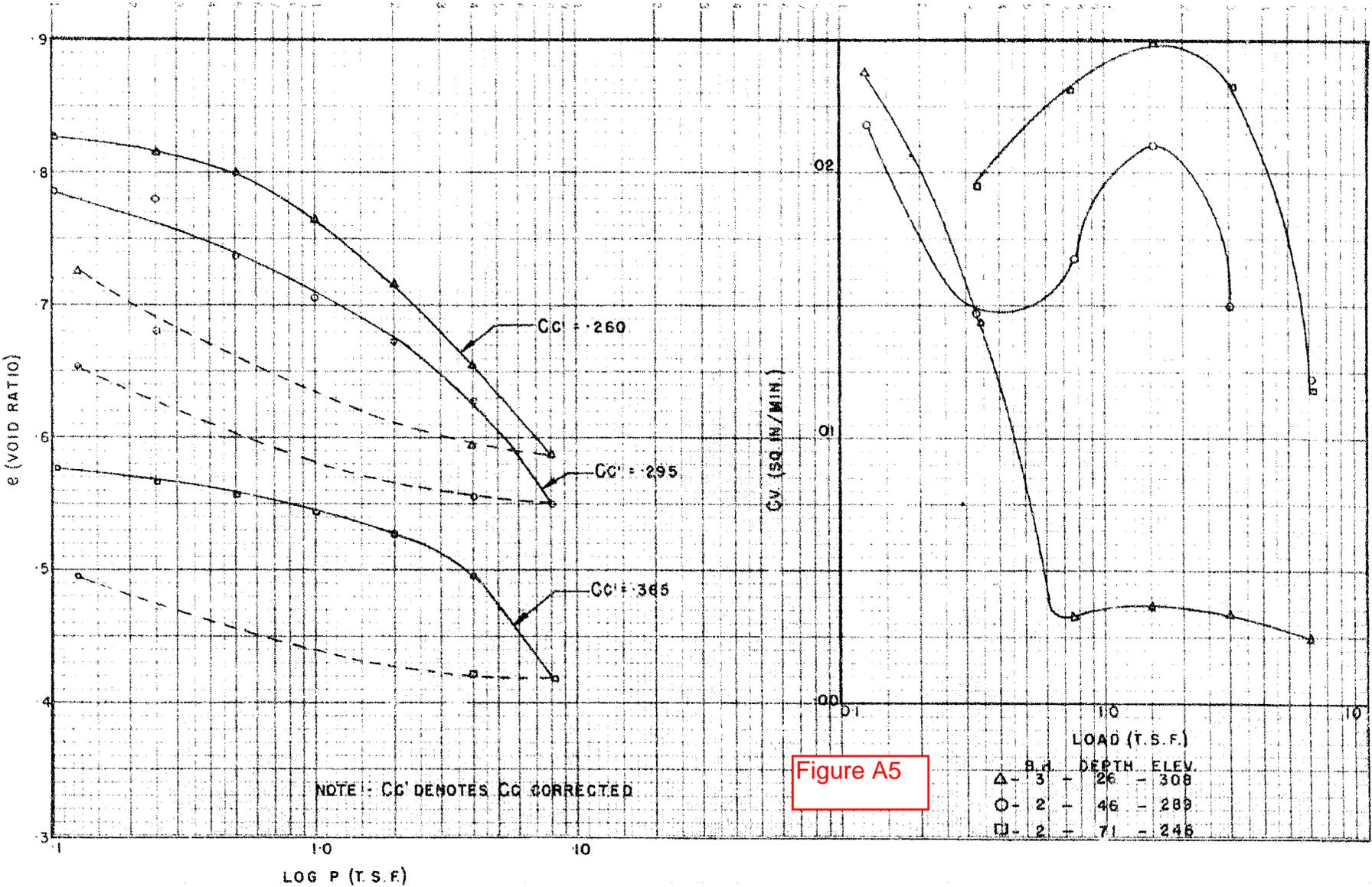


Figure A5

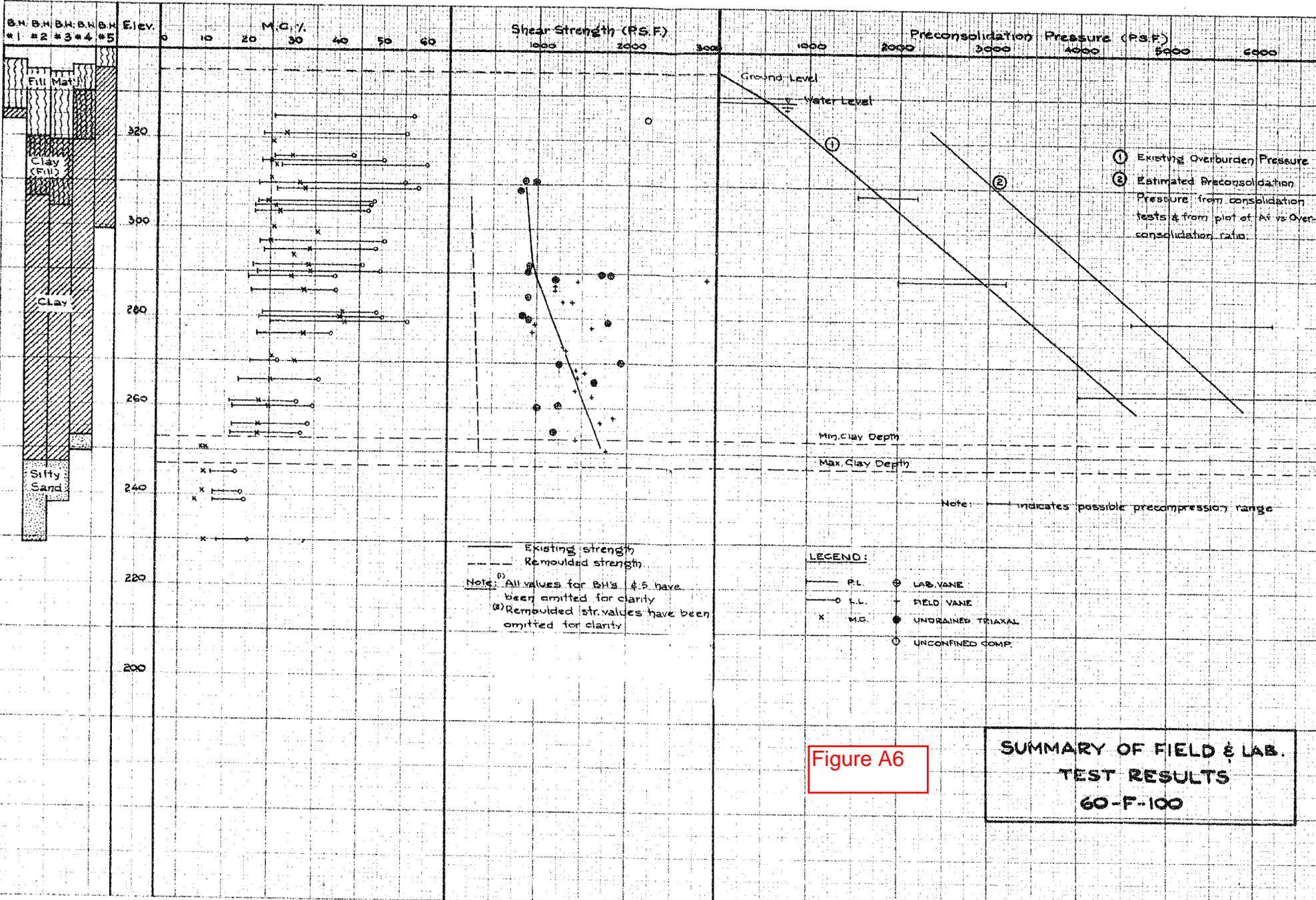
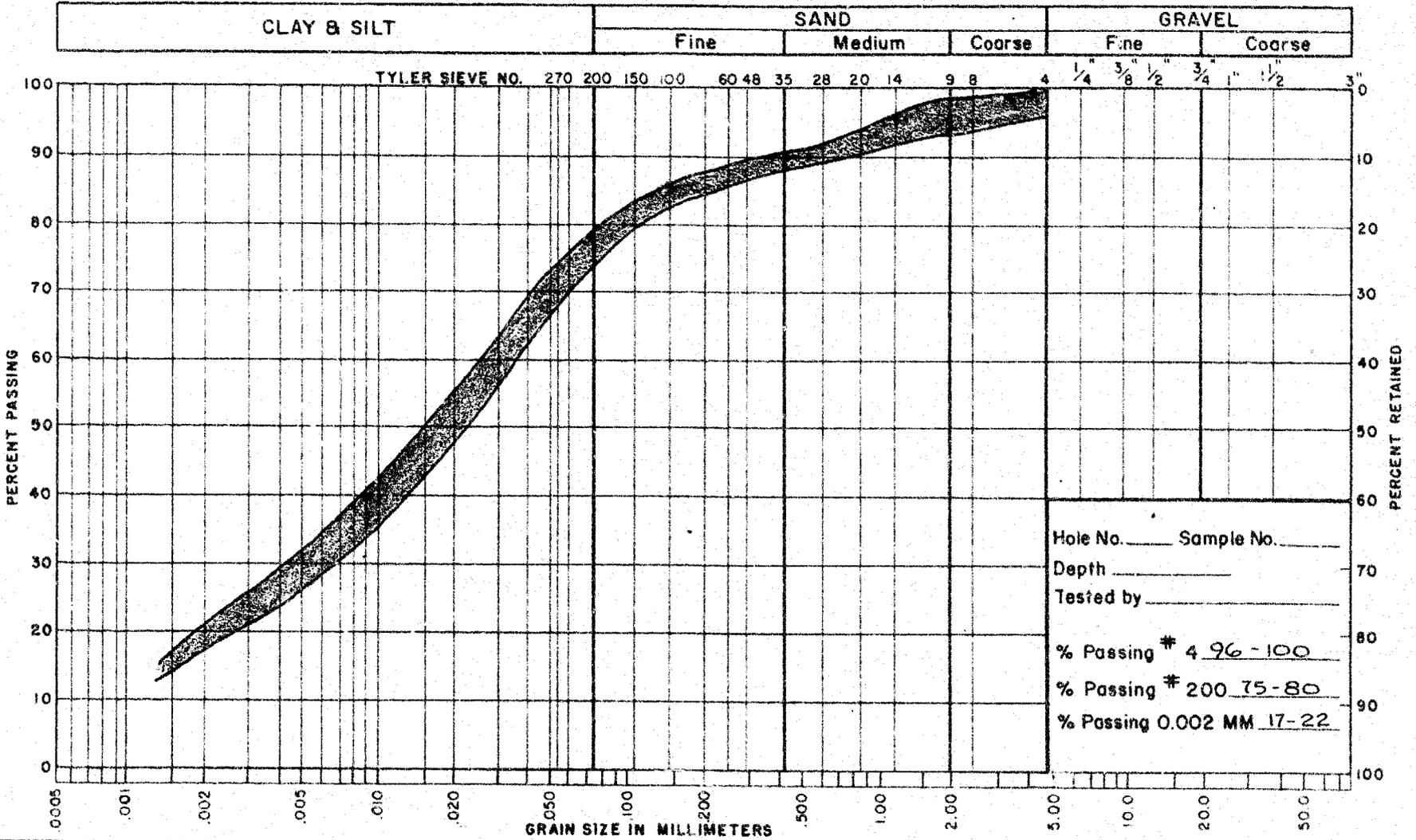


Figure A6

SUMMARY OF FIELD & LAB. TEST RESULTS  
60-F-100

### UNIFIED SOIL CLASSIFICATION SYSTEM

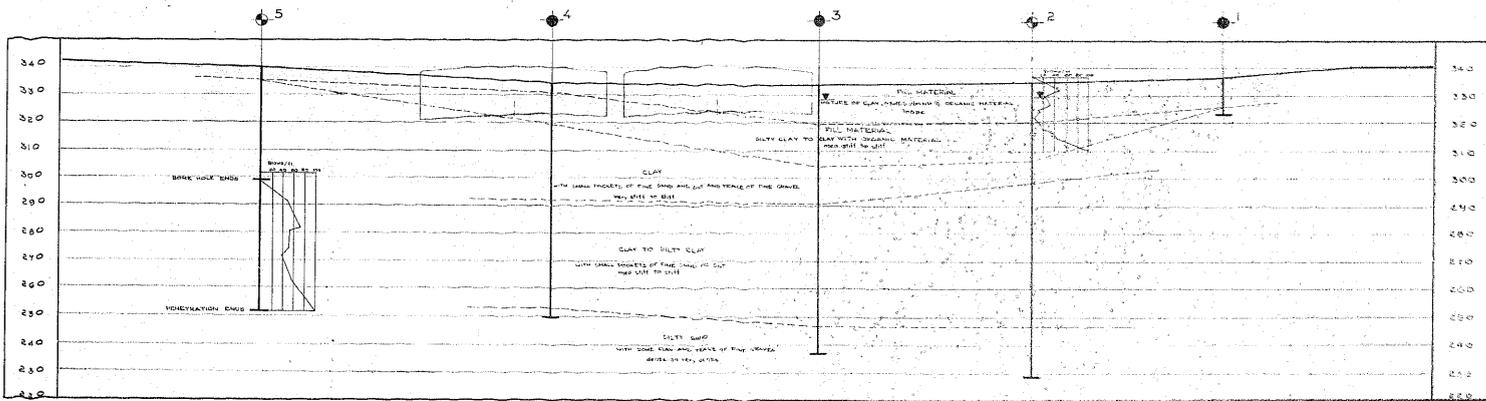
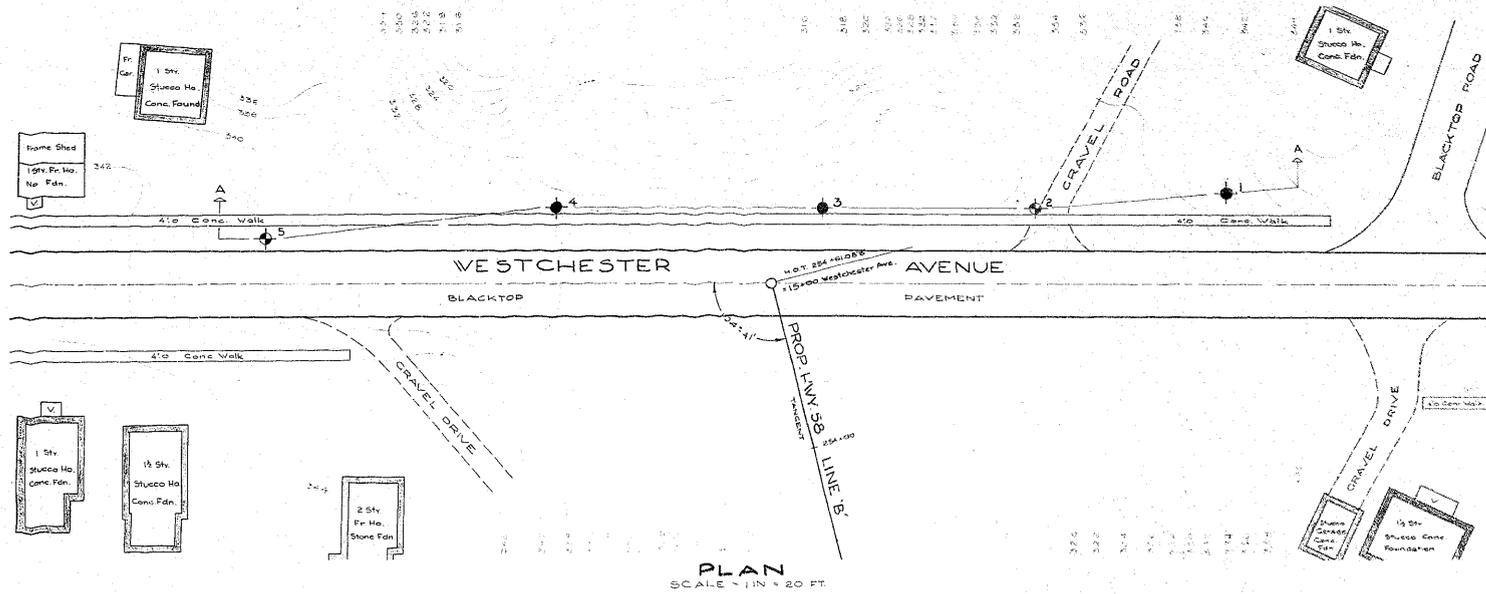


NOTES Composite Grading Chart  
Red Silty Sand (s.F.-CL Material)

Figure A7

DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS & RESEARCH SECTION  
**GRAIN SIZE DISTRIBUTION**

Job No. 60-F-100 W.P. No. 47-61  
Location Hwy. 58 at Westchester Ave.

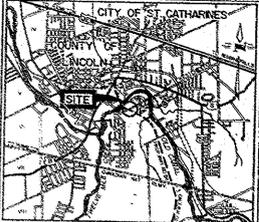
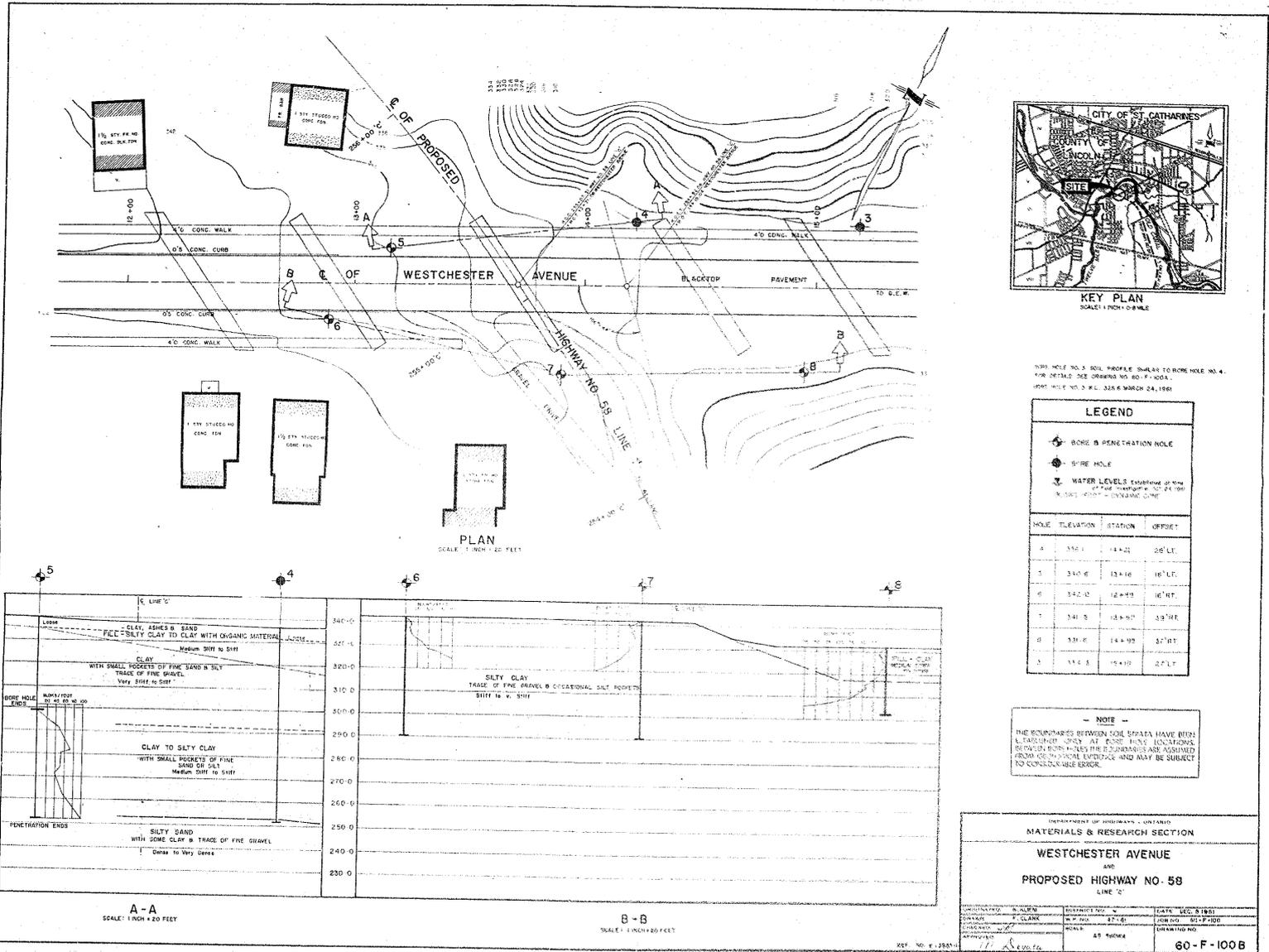


LEGEND			
	BORE HOLE		
	BORE AND PENETRATION HOLE		
NO.	ELEVATION	STATION	DISTANCE FROM TANGENT
1	326.0'	254+05	165' RT
2	324.9'	254+54	95' RT
3	324.5'	254+85	24' RT
4	324.1'	255+07	75' LT
5	340.6'	255+24	175' LT

DEPARTMENT OF HIGHWAYS - ONTARIO  
MATERIALS & RESEARCH SECTION

**WESTCHESTER AVE**  
AND  
PROPOSED HIGHWAY NE55 LINE 'B'

COORDINATOR: CHEBARRINGTON	DRAWN BY: M. SEYMOUR	DATE: 24 MARCH 1951
CHECKED BY: [Signature]	SCALE: 1 IN. = 20 FEET	JOB NO. 50-F-100
APPROVED: [Signature]		DRAWING NO. 60-F-100A



KEY PLAN  
SCALE: 1" = 100' HORIZ.

BORE HOLE NO. 3 SOIL PROFILE SHOWN TO BORE HOLE NO. 4.  
FOR DETAILS SEE DRAWING NO. 60-F-100A.  
BORE HOLE NO. 3 W.L. 325.6 MARCH 24, 1961

**LEGEND**

- BORE & PENETRATION HOLE
- SPIRE HOLE
- WATER LEVELS (ELEVATION OF HOLE TOP OF HOLE) (DATE OF TEST)

HOLE ELEVATION	STATION	OFFSET
4	330.1	18+22 28' LT.
5	340.6	13+16 16' LT.
6	342.12	12+49 16' RT.
7	341.5	13+57 19' RT.
8	331.6	14+39 37' RT.
3	344.3	15+19 27' LT.

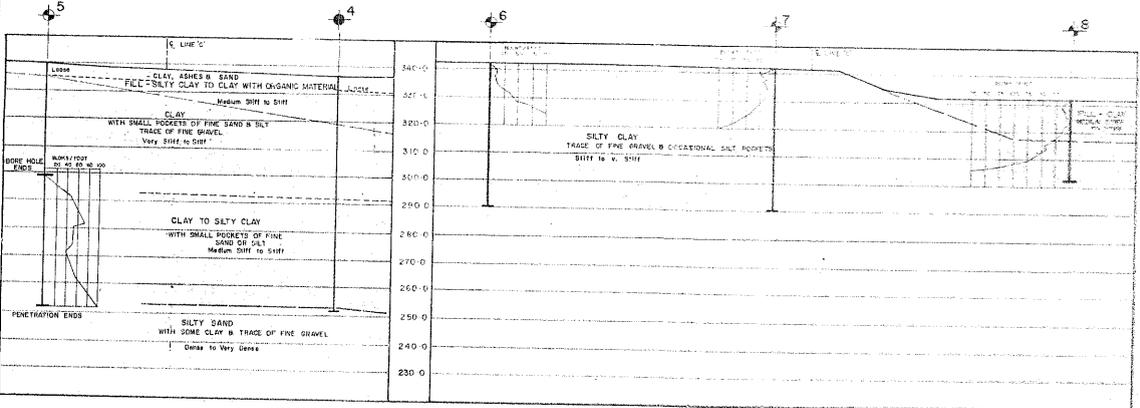
**NOTE**  
THE BOUNDARIES BETWEEN THE STRATA HAVE BEEN ESTABLISHED ONLY AT BORE HOLE LOCATIONS. BOUNDARIES BETWEEN THE STRATA ARE ASSUMED FROM USUAL SOIL BEHAVIOR AND MAY BE SUBJECT TO CONSIDERABLE ERROR.

DEPARTMENT OF HIGHWAYS - ONTARIO  
**MATERIALS & RESEARCH SECTION**

**WESTCHESTER AVENUE**  
AND  
**PROPOSED HIGHWAY NO. 58**  
LINE 'C'

DESIGNED BY	DATE	REV. 1
CHECKED BY	DATE	REV. 2
APPROVED BY	DATE	REV. 3

60-F-100B



A-A  
SCALE: 1" = 40' FEET

B-B  
SCALE: 1" = 40' FEET

REF. NO. P. 3585



# **APPENDIX B**

**GEOCRE 30M3-42 - Letter Dated November 29, 1965 and  
Drawing D-4965-1 General Plan, dated December 1961 and  
Drawing D-4965-2 Details of Abutment and Pier Footings**

Department of Highways Ontario

Copy for the information of

Mr. A. Stermac, Principal Foundation Engineer,  
Room 107, Lab. Building.

Mr. H. Greenland,  
District Engineer,  
Hamilton District.

Bridge Division,  
Downsview, Ontario.

November 29, 1965.

Westchester St. Bridge  
W.P. 47-61, Contract No. 62-316  
Hwy. #406, District No. 4

During a recent tour of inspection of the bridges in the St. Catherines area we observed that the east abutment Andre bearings of the above structure were distorted and the anchor bolts bent. As the distortion was by no means uniform, we came to the conclusion that this could only come about by a combination of differential settlement and rotation of the footing, although the latter is sitting on piles.

We suggest that this abutment be kept under observation and measurements taken periodically to determine the actual movements.

The west abutment bearings do not appear to be affected.

WMcF:rd

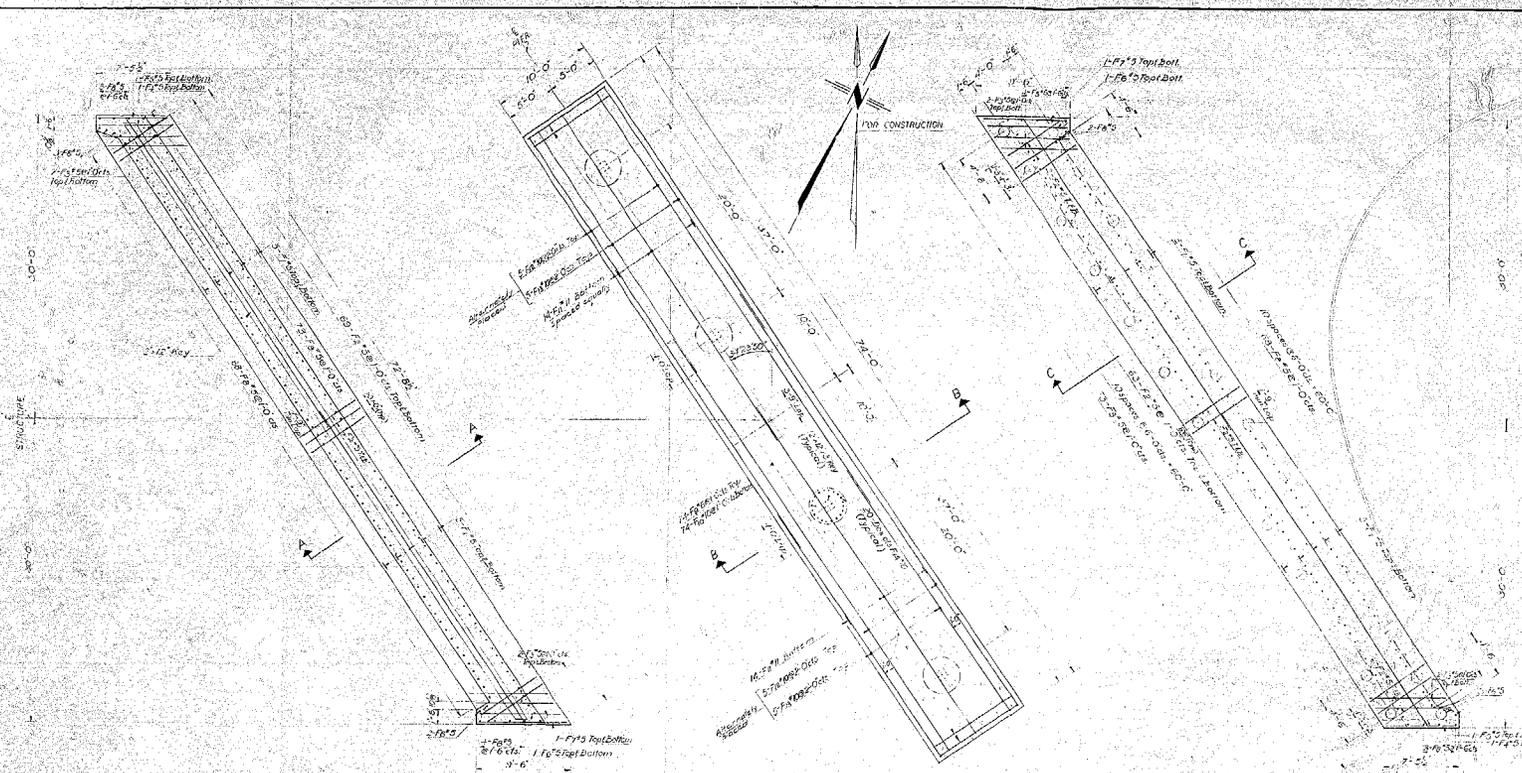
W. McFarlane,  
Senior Bridge Design Services Engineer.

c.c. A. Stermac  
W. Birch

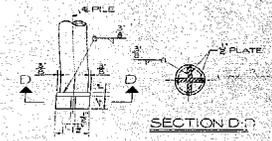
For the above mentioned project the District people ~~should~~<sup>to</sup> be contacted are as follows:-

- 1) Joe Castellona MU4-0252 (St. Catherines) - Project Supervisor during construction
- 2) Roger Verscheure 684-5245 ( " " )
- 3) Lynn Fisher Dist. office Hamilton





**NOTE**  
 ALL PILES 125 Ø STEEL TUBES  
 0.003 WALL THICKNESS FILLED  
 WITH 4000 P.S.I. CONCRETE  
 PILES TO BE GIVEN TO MIN. TIP  
 EL. 280.00  
 DESIGN LOAD PER PILE = 25 TONS  
 24 PILES 45'-0" LONG



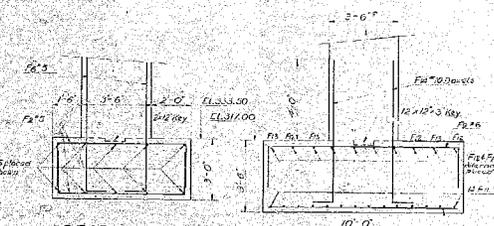
PILE SHOE DETAILS

PLAN OF WEST ABUTMENT FOOTING  
 SCALE: 3/8"=1'-0" FT.

PLAN OF PIER FOOTING  
 (TYPICAL)  
 SCALE: 3/8"=1'-0" FT.

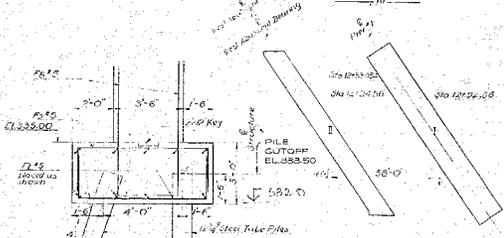
PLAN OF EAST ABUTMENT FOOTING  
 SCALE: 3/8"=1'-0" FT.

PRINT RECORD	NO.	REV.	DATE
	1	REVISED	1/20/62



SECTION A-A  
 SCALE: 3/8"=1'-0" FT.

SECTION B-B  
 SCALE: 3/8"=1'-0" FT.



SECTION C-C  
 SCALE: 3/8"=1'-0" FT.

FOOTING LAYOUT  
 SCALE: 1/8"=1'-0" FT.

REVISIONS	DATE	BY	DESCRIPTION

DEPARTMENT OF HIGHWAYS ONTARIO  
 BRIDGE DIVISION

**WESTCHESTER AVE. UNDERPASS**  
 (3.0 MI. NORTH OF ST. CATHARINES SOUTH LIMITS)

KING'S HIGHWAY No. #06 DIST. No. 4  
 CO. LINCOLN  
 CITY OF ST. CATHARINES LTD. CON.

**DETAILS OF ABUTMENT & PIER FOOTINGS**

APPROVED: [Signature] DATE: 1/20/62 SHEET: 27-61

DESIGN	A.K.	CHECK	J.L.H.	CONTRACT	
DRAWING	P.J.S.	CHECK	G.B.	NO.	47-61
DJR	J.A.N.	LOADING	120°/36	DRAWING	D-4965-2

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