



THURBER ENGINEERING LTD.

FINAL
FOUNDATION INVESTIGATION AND DESIGN REPORT
HIGHWAY 118 CULVERT STA. 11+494, OAKLEY TOWNSHIP
ASSIGNMENT NO. 5017-E-0003
G.W.P. 5287-14-00

Geocres No.: 31E-397

Report to:

McIntosh Perry Consulting Engineers Limited

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PART 1. FACTUAL INFORMATION

1 INTRODUCTION

This section of the report presents the factual findings obtained from a foundation investigation completed at a culvert at Sta. 11+494 on Highway 118. The culvert crossing is located approximately 0.3 km west of Milne Road within Oakley Township in the District of Muskoka. Thurber Engineering Limited (Thurber) carried out the current field investigation as a sub-consultant to McIntosh Perry Consulting Engineers Ltd. (MPCE) under Assignment No. 5017-E-0003.

The purpose of this investigation was to explore the subsurface conditions at the site and, based on the data obtained, to provide a borehole location plan, records of boreholes, stratigraphic profile, laboratory test results and a written description of the subsurface conditions. A model of the subsurface conditions influencing design and construction was developed in the course of the current investigation.

No previous foundation investigation information was available for the subject culvert site within the online Geocres Library. A Project Assessment Report (PAR) and a historical base plan survey were provided by MPCE.

2 SITE DESCRIPTION

For project purposes, Highway 118 will be considered to be oriented east-west with chainage increasing to the east. The existing culvert conveys (unnamed) creek flow from the north to the south under a high fill embankment supporting Highway 118. As described within the historical baseplan drawings provided by MPCE, the existing culvert is a non-structural corrugated steel pipe (CSP) culvert with a measured diameter of 1.1 m and a length of 29.9 m. The invert of the culvert is at approximate elevation 321.1 m at the inlet (north) and 320.9 m at the outlet (south). The culvert has a skew angle of approximately 18 degrees. Transverse cracking of the pavement was noted at the culvert. No signs of erosion or slope instability were noted on the existing highway embankments during the field investigation. The roadway surface over the culvert was generally in good condition with no dips or bumps noted during the field investigation. The condition of the existing

culvert, as assessed by MPCE, did show some signs of corrosion and the south end of the culvert has a substantial dent at the exposed end.

At the location of the culvert, Highway 118 is a two-lane highway with paved shoulders. The highway has a horizontal and vertical curve at the location of the culvert. The Highway 118 fill height above the culvert ranges from approximately 4.8 to 5.0 m with the road surface at approximate elevation 327.0 m. The existing northern and southern embankment slopes are inclined at approximately 2.0H:1V and 1.8H:1V, respectively. Cobbles and boulders were observed on the embankment slopes. Cable guidewires with wooden posts are present on both sides of the highway in the vicinity of the culvert. The land adjacent to the highway is undeveloped and is densely vegetated with shrubs and trees. A single family dwelling is located approximately 250 m west of the culvert. Bedrock outcrops and minor rock cuts are present at multiple locations within close proximity to the culvert site. Overhead utility lines run parallel to the north side of the highway. Traffic volumes on this section of Highway 118 are understood to be 4,300 AADT (2016).

Photographs showing the existing conditions in the area of the culvert at the time of the field investigation are included in Appendix D for reference.

3 SITE INVESTIGATION AND FIELD TESTING

The site investigation and field testing program was carried out from September 15th to 29th, 2018. The field investigation consisted of advancing four boreholes identified as 18-1 through 18-4. The drilling was carried out using portable equipment for off-road boreholes 18-1 and 18-4 and a truck mounted CME 75 drill rig for the on-road boreholes 18-2 and 18-3. Prior to commencement of drilling, utility clearances were obtained in the vicinity of the borehole locations.

The northing, easting and elevation of the boreholes are shown on the Borehole Location and Soil Strata Drawing No. 1 in Appendix A, the individual Record of Borehole sheets in Appendix B and within Table 3-1. The termination depth of each of the boreholes is also provided. The site is within MTM Zone 10. The borehole elevations were surveyed with a Nikon-AP-8 with an accuracy of +/- 1.5 mm. The survey referenced the top of the north end of the culvert which has an elev. 322.193 m, as provided by MPCE. Horizontal locations were measured relative to existing site features.

Table 3-1: Borehole Summary

Borehole No.	Drilled Location	Northing (m)	Easting (m)	Ground Surface Elevation (m)	Termination Depth * (m)
18-1	Near Culvert Inlet	4 984 994.7	335 619.8	322.3	9.2
18-2	Westbound Lane HWY 118	4 984 982.2	335 616.5	326.7	15.2
18-3	Eastbound Lane HWY 118	4 984 973.6	335 624.5	327.2	16.2
18-4	Near Culvert Outlet	4 984 961.5	335 620.1	321.0	8.6

Note: () depths provided in table are from the ground surface*

Soil samples were obtained at selected intervals using a split spoon sampler in conjunction with Standard Penetration Testing (SPT) following ASTM D1586. A half-weight (32 kg) hammer was used during SPT testing in Boreholes 18-1 and 18-4, which were drilled with portable equipment. The N-values reported herein for these off-road boreholes have been adjusted to an equivalent standard weight hammer (64 kg). A standard weight hammer was used during SPT testing for the on-road boreholes and no correction was necessary. All boreholes were advanced into bedrock with either NW or NWT casing in conjunction with coring techniques.

The drilling and sampling operations were supervised on a full-time basis by an experienced member of Thurber's technical staff. The drilling supervisor logged the boreholes and processed the recovered soil samples for transport to Thurber's Ottawa geotechnical laboratory for further examination and testing.

A 19 mm diameter standpipe piezometer was installed in Borehole 18-4 to allow for measurements of the groundwater level after completion of drilling. The piezometer installation details are illustrated on the respective Record of Borehole sheet provided in Appendix B. The boreholes were backfilled in accordance with MOE requirements (O.Reg. 903, as amended). Boreholes 18-2 and 18-3 were backfilled with granulars within the depth of pavement structure and capped with 150 mm of cold patch asphalt to reinstate the travelling surface.

4 LABORATORY TESTING

The recovered soil samples were subjected to visual identification and to natural moisture content determination. Selected samples were also subjected to gradation analysis



(hydrometer and/or sieve) and Atterberg Limit testing. The results of these tests are summarized on the respective Record of Borehole sheets included in Appendix B. One sample of soil recovered from within each of Boreholes 18-1 and 18-4 was selected and submitted for analytical testing of corrosivity parameters. One rock core sample underwent unconfined compressive strength testing. All laboratory test results are provided in Appendix C.

5 GENERAL DESCRIPTION OF SUBSURFACE CONDITIONS

Details of the encountered soil stratigraphy are presented on the Record of Borehole sheets included in Appendix B and the Borehole Location and Soil Strata drawing included in Appendix A. A general description of the stratigraphy, based on the conditions encountered in the boreholes, is given in the following paragraphs. However, the factual data presented on the Record of Borehole sheets takes precedence over the Soil Strata Drawing and the general description. It must be recognized that the soil and groundwater conditions may vary between and beyond borehole locations.

In general terms, the site was found to be underlain by a pavement structure, granular fill and rock fill overlying native deposits of sand and silty sand over glacial till. Granite bedrock was encountered at varying elevations within the depths of investigation in all boreholes.

5.1 Embankment

5.1.1 Asphalt

Boreholes 18-2 and 18-3 were drilled through the existing Highway 118 embankment and encountered a layer of asphalt with a thickness of 100 mm at the ground surface.

5.1.2 Sand with Gravel (Fill)

A layer of granular fill (pavement structure), classified as sand with gravel to gravel with sand, was encountered below the asphalt in Boreholes 18-2 and 18-3. The underside of this fill was at 1.0 and 1.4 m below the existing roadway surface (elev. 325.7 and 325.8 m) in Boreholes 18-2 and 18-3, respectively.

The SPT tests conducted in the granular fill gave N-values ranging from 55 to 79 blows, indicating a relative density of very dense. Recorded moisture contents ranged from 4 to 8%.

5.1.3 Rock Fill

A layer consisting of rock fill was encountered beneath the granular pavement structure in Boreholes 18-02 and 18-03. This layer had a thickness ranging from 1.9 to 3.2 m (underside elev. 322.5 to 323.9 m). Boreholes were advanced through the rock fill using casing and coring techniques. Split spoon sampling was attempted between core runs; however, sample recovery was poor. SPT refusal was routinely encountered.

Rock fill pieces were cored and indicated mainly granite particles with cored lengths of up to 900 mm.

5.1.4 Sand with Silt and Gravel (Fill)

A second granular fill layer, consisting of sand with silt and gravel and occasional to frequent cobbles and boulders was encountered below the rock fill in Boreholes 18-2 and 18-3. The thickness of this granular fill ranged from 0.5 m to 2.0 m (underside elev. 322.0 m to 321.9 m).

The SPT tests conducted in this granular fill layer gave N-values ranging from 3 to 38 blows, indicating a relative density of very loose to dense.

Recorded moisture contents of 10 and 15% were measured in this layer. The results of grain size analysis conducted on one sample of the granular fill indicated this material to consist of 26% gravel, 67% sand and 7% fines. These results are illustrated on Figure C1 in Appendix C.

5.2 Silty Sand (SM) with organics

A native deposit of silty sand with organics was encountered at the ground surface in off-road Boreholes 18-1 and 18-4 with a thickness of 1.5 m and 0.6 m (underside elev. 320.8 m and 320.4 m), respectively.

SPT tests conducted in the silty sand deposit gave N-values ranging from 2 to 5 blows indicating a relative density of very loose to loose.

Recorded moisture contents of the silty sand ranged from 24 to 59%. The results of grain size analyses conducted on two samples of the silty sand are summarized below and are illustrated on Figure C2 in Appendix C.

Soil Particle	Percentage (%)
Gravel	0
Sand	65 – 69
Silt	25 – 30
Clay	5 – 6

Atterberg Limit testing was completed on two samples of the silty sand deposit and indicated that the material is non-plastic.

5.3 Sand (SP to SP-SM)

A native deposit of sand with varying amounts of silt was encountered below the organic silty sand in Boreholes 18-1 and 18-4 and below the fill materials in Boreholes 18-2 and 18-

3. The thickness of this sand deposit ranged from 2.5 to 6.9 m with a bottom elevation of 315.0 to 318.3 m.

The SPT N-values conducted in this layer ranged from 12 to 75 blows indicating a relative density of compact to very dense.

The moisture content of the samples tested ranged from 16 to 28%. The results of grain size analyses conducted on five samples of the sand are summarized below and are illustrated on Figure C3 in Appendix C.

Soil Particle	Percentage (%)
Gravel	0 – 1
Sand	91 – 98
Silt	2 – 9
Clay	

5.4 Sandy Silt (ML)

A 0.3 m thick layer of sandy silt with traces of organics was encountered below the sand in Borehole 18-2. The underside elevation of this layer was at 315.7 m.

A moisture content was measured to be 100%.

5.5 Sand (SP-SM) to Silty Sand (SM) – (Glacial Till)

Below the sand in Boreholes 18-1, 18-3 and 18-4 and below the sandy silt layer in Boreholes 18-2 was a deposit of glacial till consisting of sand to silty sand with varying amounts of gravel. Occasional cobbles and boulders were encountered in Borehole 18-4 and the till deposit encountered in Borehole 18-1 consisted predominantly of cobbles and boulders. Coring techniques were required to advance through the cobbles and boulders in Borehole 18-1. The thickness of the till deposit ranged from 0.8 m to 1.7 m with an underside elevation ranging from 316.6 m to 314.2 m.

SPT tests conducted in the till deposits within Boreholes 18-3 and 18-4 gave N-values ranging from 18 to 25 blows indicating a relative density of compact. SPT testing was not feasible in Borehole 18-1 due to the presence of boulders.

Recorded moisture contents of the till ranged from 8 to 20%. The result of grain size analyses conducted on three samples of the till are summarized below and are illustrated on Figure C4 in Appendix C.

Soil Particle	Percentage (%)
Gravel	0 – 42
Sand	47 – 81
Silt	11 – 19
Clay	

5.6 Bedrock

Bedrock was proven by coring in all boreholes. Information on the bedrock surface from the current investigation is summarized in the following table.

Table 5-1: Summary of Bedrock Elevations

Borehole No.	Depth to Bedrock below Existing Ground Surface (m)	Bedrock Elevation (m)
18-1	5.7	316.6
18-2	11.8	314.9
18-3	13.0	314.2
18-4	5.6	315.4

The bedrock core samples consisted of slightly weathered to fresh granite. The Total Core Recovery (TCR) from all core samples ranged from 94 to 100%, the Solid Core Recovery (SCR) ranged from 79 to 100% and the Rock Quality Designation (RQD) ranged from 70 to 100%. Based on the measured RQD values, the bedrock is classified as fair to excellent in quality, but predominantly good to excellent (Table 3.10, Canadian Foundation and Engineering Manual 2006).

Unconfined Compressive Strength (UCS) testing was carried out on one sample of the intact bedrock. The result of UCS testing was 88 MPa, indicating the intact granite bedrock to be strong. Photographs of the bedrock core are provided in Appendix C.

5.7 Groundwater

Representative water levels could not be recorded in the open boreholes due to water continuously being introduced into the borehole as part of the drilling operations. The groundwater level measured in the standpipe piezometer installed in Borehole 18-4, 10 days after installation was recorded at a depth of 1.2 m below the ground surface

(elev. 319.8 m) on September 29, 2018. The culvert was dry during the time of the field investigation.

These observations are considered short term and it should be noted that the groundwater level at the time of construction may be different and seasonal fluctuations of the groundwater level are to be expected. In particular, the groundwater level may be at a higher elevation after periods of significant and/or prolonged precipitation.

5.8 Analytical Testing

Two samples from the native soils encountered at the site were submitted for analysis of pH, water soluble sulphate and chloride concentrations, and resistivity. The analysis results are summarized in the Table 5-2. A copy of the test results is provided in Appendix C.

Table 5-2: Results of Chemical Analysis

Borehole (Sample)	Depth(*) (mbgs)	Sulphate (µg/g)	pH (-)	Resistivity (Ohm-cm)	Conductivity (uS/cm)	Chloride (µg/g)	Sulphide (%)
18-1 (SS3B)	1.5 – 1.8	24	5.32	2,610	383	236	<0.02
18-4 (SS3)	1.2 – 1.8	7	6.22	4,450	225	124	<0.02

6 MISCELLANEOUS

Borehole locations were selected by Thurber relative to existing site features and the anticipated culvert location. The as-drilled locations and ground surface elevation of the boreholes were measured by Thurber following completion of the drilling. Elevation benchmarks were provided by MPCE.

George Downing Estate Drilling Ltd. and Forage M3 Drilling Services Inc., both of Hawksbury, Ontario, supplied and operated the drilling equipment to conduct the drilling, soil sampling, in-situ testing, standpipe installation and borehole decommissioning. NC Traffic Management Inc. of Kirkland Lake, Ontario supplied the traffic control equipment and personnel for lane closures required for the field work. The field investigation was supervised on a full time basis by Miss Allison Chow, EIT and Mr. Sean O'Bryan, C.E.T. of Thurber. Overall supervision of the investigation was provided by Miss Katya Edney, P.Eng.

Routine geotechnical laboratory testing was completed by Thurber's laboratory in Ottawa, Ontario. UCS testing was completed by Stantec's laboratory in Ottawa, Ontario. Analytical testing was completed by Paracel Laboratories in Ottawa, Ontario. Interpretation of the factual data and preparation of this report were carried out by Miss Katya Edney P.Eng. and



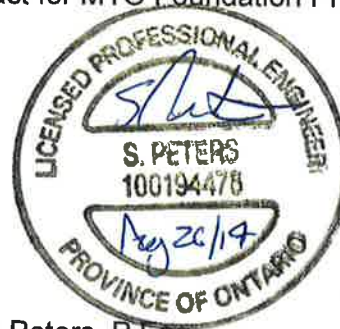
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PART 2. ENGINEERING DISCUSSION AND RECOMMENDATIONS

7 INTRODUCTION

This section of the report provides an interpretation of the factual data from Part 1 of this report and presents geotechnical recommendations to assist the project team in designing a suitable replacement of the existing culvert crossing Highway 118 at Station 11+494. The discussion and recommendations presented in this report are based on the information provided by McIntosh Perry Consulting Engineers Ltd. (MPCE) and on the factual data obtained during the course of the investigation.

This foundation investigation and design report with the interpretation and recommendations are intended for the use of the Ministry of Transportation, and shall not be used or relied upon for any other purposes or by any other parties including the construction or design-build contractor. The construction or design-build contractor must make their own interpretation based on the factual data in Part 1 of the report. Where comments are made on construction, they are provided only in order to highlight those aspects which could affect the design of the project. Contractors must make their own interpretation of the factual information provided as it may affect equipment selection, proposed construction methods and scheduling.

As shown on the historical base plan drawings, the existing culvert is located under a high fill embankment at Station 11+494 and is a 1.1 m diameter non-structural corrugated steel pipe (CSP) culvert with a length of 29.9 m. The invert of the culvert is at approximate elevation 321.1 and 320.9 m at the inlet (north) and outlet (south), respectively. The Highway 118 fill height above the culvert ranges from approximately 4.5 to 5.2 m with the road surface at approximate elevation 327.0 m. The existing northern and southern embankment slopes are inclined at approximately 2H:1V and 1.8H:1V, respectively. It is noted that no water was present in the culvert at the time of drilling.

It is noted that rock fill was observed within both of the on-road boreholes.

No previous foundation investigation information for the subject culvert was available within the online Geocres Library.

7.1 Proposed Structure

At the time of preparation of this Foundation Investigation and Design Report, it is expected that the existing culvert will be replaced with a non-structural culvert of similar size, length and alignment. It has also been assumed the invert elevations will be similar to that of the existing culvert. This culvert is located at a high fill embankment site, where the fill height above the culvert is approximately 4.8 m. As per the Culvert Reinstatement Typical Drawing of the 30% Drawing Package, received on May 28, 2019 from MPCE, the replacement will be carried out utilizing grade lowering.

7.2 Applicable Codes and Design Considerations

The geotechnical assessment presented below has been prepared based on the assumed replacement culvert, the existing ground conditions and in accordance with the Canadian Highway Bridge Design Code (CHBDC), version CSA S6-14.

It is understood that if the culvert were to be replaced with a structural culvert, the new culvert would have a consequence classification of *Typical Consequence*, in accordance with Section 6.5.1 of the CHBDC. Accordingly, a consequence factor (Ψ) of 1.0, as per Table 6.1 of the CHBDC, would be used in assessing factored geotechnical resistances.

The depth of frost and applicable recommendations are provided in Section 10.3.

8 SEISMIC CONSIDERATIONS

8.1 Spectral and Peak Acceleration Hazard Values

The seismic hazard data for the CHBDC is based on the fifth-generation seismic model developed by the Geological Survey of Canada (GSC). The seismic hazard for this site has been obtained from the GSC calculator. The data includes a peak ground acceleration (PGA), peak ground velocity (PGV) and the 5% spectral response acceleration values ($S_a(T)$) for the reference ground condition (Site Class C) for a range of periods (T) and for a range of return periods including 475-year, 975-year and 2475-year events. The GSC seismic hazard calculated data sheet for this site is included in Appendix F.

The site coefficients used to determine the design spectral acceleration and displacement values are a function of the Site Class and the peak ground acceleration (PGA). The PGA at this site for a *reference* Site Class C with a 2% probability of exceedance in 50 years (2475-year event) is 0.069g. This value is to be scaled by the $F(PGA)$ based on the site-specific Site Class, as discussed below.

8.2 Seismic Liquefaction Potential

Based on the low reference PGA, the subsurface conditions encountered at the drilled locations at this site and using the Seed & Idriss Simplified Method for liquefaction assessment, the foundation soils below the culvert inverts are not considered to be susceptible to liquefaction during the design seismic event.

8.3 CHBDC Seismic Site Classification

In accordance with the CHBDC, the selection of the seismic site classification is based on the soil and rock conditions encountered in the upper 30 m of the stratigraphy. This site has been classified as a Site Class D in accordance with Section 4.4.3.2 of the CHBDC (S6-14) utilizing the harmonic mean of the recorded SPT N-values.

9 DESIGN OPTIONS

9.1 Culvert Type and Foundation Alternatives

Selection of the culvert type must consider the proposed construction procedures, staging requirements, geotechnical resistance available in the foundation soils, the depth to suitable bearing stratum and post-construction settlement criteria. From a geotechnical perspective, the following culvert types were considered:

- Circular Pipes (Concrete, HDPE, Steel)
From a foundation engineering perspective, a pipe culvert is a technically feasible alternative. It is assumed that an internal pipe diameter of similar size (1.1 m) or slightly greater is likely to be proposed.
- Open Bottom Culvert (Box, Arch)
An open bottom culvert is considered feasible from a foundation engineering perspective but is not recommended for this site based on the anticipated size of the replacement culvert. An open bottom culvert would have the requirement for greater excavation depths and dewatering efforts during construction and there could be a potential for differential settlement following construction.
- Closed Bottom Culvert (Box)
A precast segmental box culvert is considered a feasible option from a foundation engineering perspective. Precast sections, rather than cast-in-place construction, can be installed expediently with less potential for disturbance of the founding soils during installation.
- Steel Sheet Pile Walls with Precast Concrete Slab
Contiguous sheet pile walls supporting a precast concrete slab is not considered feasible due to the presence of rock fill and boulders in the embankment and native soils.

A comparison of these alternatives, based on their respective advantages and disadvantages, is included in Appendix E. It is not considered to be economical or practical to support a culvert on deep foundations at this site and therefore this option is not presented in this report.

9.2 Construction Methodology Alternative

For the proposed culvert replacement, the following construction methods were considered.

- Open Cut with Full Road Closure and Detour
Installation of a new culvert using open cut techniques and a full road closure would allow for an expedited construction schedule and could reduce costs associated with roadway protection and water flow diversion. However, it is understood that an acceptable detour is not available and therefore this option is not feasible.
- Open Cut with Staged Temporary Widening and/or Temporary Detour Embankment
Widening of the existing highway and/or construction of a temporary detour embankment to accommodate traffic passage during construction is considered feasible from a geotechnical perspective. However due to the proximity of overhead utilities, the embankment widening or detour embankment would need to be located south of Highway 118 alignment otherwise the utilities may need to be relocated. Additionally, a review of the requirement for property acquisition and highway geometry is needed to assess this option. An additional borehole investigation program may be required to determine the subsurface conditions along a temporary detour alignment.
- Open Cut and Temporary Protection System (TPS)
The use of open cut techniques in conjunction with staged culvert replacement is a feasible construction option from a geotechnical perspective. This option includes roadway protection, as discussed further in Section 11.2, installed near the embankment centerline to maintain a single lane of traffic flow along the current highway alignment. The Contractor will need to consider the potential for obstructions in the embankment fill and underlying deposits during the design and installation of the roadway protection as rock fill, cobbles and boulders were encountered during the foundation investigation. Associated cost implications may need to be considered along with the risk of such obstructions. To reduce lateral deflections of the protection system, the roadway protection may need to include anchoring and/or bracing system. The TPS would need to support a temporary cut height in the order of 6 m. The height of the TPS could be reduced if a temporary grade lowering was also included.

Existing embankments at this culvert site are approximately 6 m high. Temporary grade lowering can be incorporated into the design to reduce the overall height of embankment above the base of the proposed excavation while maintaining traffic within the existing embankment footprint. However, the vertical road alignment and traffic speed constraints will need to be reviewed from a highway design perspective. Additionally, sub-excavation of the rock fill may be required to allow placement of a suitable temporary pavement structure. The project pavement engineer should be consulted if this approach is to be carried forward.

- Temporary Modular Bridge

A temporary modular bridge (TMB) could provide a single lane of traffic passage while allowing for full excavation and replacement of the culvert without staged culvert replacement. A reduced quantity of roadway protection is also anticipated. Additional boreholes would be required at the temporary abutment locations for the TMB to provide foundation design recommendations. The design length of the TMB must consider the need for stable excavation slopes and a horizontal offset between the TMB footings and the crests of the temporary slopes.

- Trenchless Techniques

A trenchless installation would likely encounter very loose to loose sand fill with silt, gravel, cobbles and boulders over very loose to compact silty sand and compact to very dense sand with silt. Groundwater was observed approximately 1 m below the pipe invert. It is highly likely that cobbles and boulders will be encountered in the embankment fill. Given the conditions, microtunneling would likely be the preferred trenchless approach, however, the presence of cobbles and boulders could result in challenges. Due to the risks, this option is not recommended for this site.

9.3 Recommended Approach for the Culvert Replacement

From a foundation engineering perspective, the preferred approach is to replace the existing culvert with either a circular or a closed box culvert using open cut techniques. TPS would be needed to facilitate construction. Design of the relatively high TPS will need to account for the presence of rock fill, the lateral capacity available in the native soils at this site and the need to anchor or brace the TPS to reduce lateral deflection. Obstructions are likely to be encountered in the embankment fill and native till. Grade lowering should be considered to reduce the height of the TPS.

Provided sufficient property exists, temporary embankment widening could also be considered in conjunction with grade lowering however, no geotechnical information is available to support the design of an embankment widening detour and further investigation may be warranted.

10 FOUNDATION DESIGN RECOMMENDATIONS

Foundation design aspects for the replacement culvert include subgrade conditions, geotechnical resistances, settlement of the founding soils, imposed loading pressures, erosion control, protection system design, groundwater control and design of staged construction. The culvert must be designed to resist loadings including lateral earth pressures, hydrostatic pressure, weight of embankment fill, traffic loading and any surcharge due to construction equipment and activities under static and seismic conditions.

10.1 Culvert Foundation Bearing Resistances

It is assumed that the existing culvert will be replaced on the same alignment and that the embankment will be reconstructed with no grade raise or widening (temporary or permanent). Therefore, it is anticipated that the subgrade soils within the culvert footprint will not be subjected to any significant additional loading.

10.1.1 Box Culvert

Pre-cast box culverts should be constructed in accordance with OPSS 422. The recommended geotechnical resistances at roadway centreline for a pre-cast box culvert up to 2 m in width, with a 0.2 m thick base slab and installed with an invert elevation similar to the current culvert (approximate elev. 320.9 m at the outlet) on an undisturbed native sand subgrade are as follows:

- Factored Geotechnical Resistance at ULS of 300 kPa
- Factored Geotechnical Resistance at SLS of 200 kPa

Lower resistance values are available near the inlet and outlet due to the presence of loose sands, however, the loads at these locations are also significantly lower and will not govern the design.

The factored geotechnical resistances include the following factors:

- Consequence factor (Ψ) of 1.0 (as per CHBDC Table 6.1)
- Geotechnical resistance factors (as per CHBDC Table 6.2):
 - $\phi_{gu} = 0.5$ (static analysis; typical degree of understanding)
 - $\phi_{gs} = 0.8$ (static analysis; typical degree of understanding)

The bearing resistance values are for vertical, concentric loading. In the case of eccentric or inclined loading, the bearing resistance must be reduced in accordance with CHBDC Clause 6.10.3 and Clause 6.10.4. Foundation settlement, based on the above SLS resistance, is expected to be less than 25 mm for subgrades prepared with good workmanship.

Resistance to lateral forces/sliding resistance between the precast concrete and the underlying Granular 'A' bedding (Section 10.2) should be evaluated in accordance with the CHBDC assuming an unfactored coefficient of friction of 0.45. A geotechnical resistance factor against sliding (ϕ_{gu}) of 0.8 (as per CHBDC Table 6.2) may be used.

Surface water diversion and dewatering (Section 11.3) should be provided as required to place the bedding material and install the culvert in the dry.

10.1.2 Pipe Culvert

Pipe culverts should be constructed in accordance with OPSS.PROV 421. Geotechnical resistance values are not typically required for pipe culverts. A modulus of subgrade reaction of 20 MN/m³ can be used for a pipe culvert installed at this site, if required.

10.2 Subgrade Preparation, Bedding and Backfilling

For a replacement culvert constructed along the same alignment as the current culvert, the existing culvert and bedding materials should be removed. After excavation and removal of the existing culvert and existing fill, any organics, soft or loose deposits, disturbed soils, loose alluvial deposits and deleterious materials must be stripped from the footprint of the new culvert to expose competent native undisturbed subgrade material at or below the desired founding elevations. Given the sand subgrade anticipated at the founding level in the areas near the inlet and outlet of the replacement culvert, construction equipment should not travel on the exposed final subgrade. If the new culvert is to be installed adjacent to the existing culvert, the excavation should not undermine the existing culvert when used as a temporary by-pass culvert.

The exposed final subgrade must be inspected to confirm that the subgrade is suitable and uniformly competent. Any deleterious materials at the subgrade level should be sub-excavated and backfilled with granular fill consisting of OPSS.PROV 1010 Granular A material as soon as practical to protect the subgrade from disturbance during construction. The granular fill should be compacted as per OPSS.PROV 501. In order to provide a more uniform foundation subgrade condition for the culvert, a minimum 300 mm thick layer of bedding material conforming to OPSS.PROV 1010 Granular A requirements must be provided under the base of the culvert as per OPSS 422 and OPSP 803.010 (box culvert) and OPSP 802.010 (pipe culvert).

It is noted that construction will extend below the ditch elevation. Water diversion and dewatering may be required to prepare the subgrade in the dry. Please refer to Section 11.3 for additional comments on groundwater and surface water control.

For box culverts, it is recommended that culvert cover be in accordance with OPSP 803.010 and OPSS 902 and consist of Granular A material meeting the requirements of OPSS.PROV 1010. Culvert backfill above the granular cover should be in accordance with OPSS 902 and consist of material meeting the requirements of OPSS Granular B Type I or Select Subgrade Material (SSM) and should be compacted as per OPSS.PROV 501. Care must be exercised when compacting the fill adjacent to and above the culvert in order not to damage the culvert. Heavy compaction equipment, used near the culvert, must be restricted in accordance with OPSS.PROV 501.

For flexible pipe culverts it is recommended that culvert embedment and cover be in accordance with OPSP 802.010 and OPSS.PROV 401 and consist of OPSS Granular A material. Culvert backfill should meet the requirements of OPSS Granular B Type I or SSM and constructed in accordance with OPSS.PROV. 401.

10.3 Frost Depth

The depth of frost penetration at this site is 1.8 m (as per OPSP 3090.101). It is not necessary to found a closed box or pipe culvert at a depth below frost penetration. Frost

taper treatment, if needed, should be as directed within the Pavement Design Report. Wing walls would require foundations founded below frost depth.

10.4 Lateral Earth Pressures

The Lateral Earth Pressure parameters provided in Table 10-1 and Table 10-2 are based on the assumptions that any head or wing wall is vertical and the backfill is fully drained so that there are no unbalanced hydrostatic pressures. If adequate drainage cannot be confirmed, the potential for buildup of hydrostatic pressures should be considered in design. Where ground surfaces are horizontal or sloped at 2H:1V behind vertical walls, the corresponding coefficients provided in Table 10-1 and Table 10-2 should be used.

10.4.1 Static Lateral Earth Pressure Coefficients

Lateral earth pressures acting on structures should be computed in accordance with the CHBDC but, under fully drained conditions, are given by the following general expression:

$$\sigma_h = K * (\gamma d + q)$$

where:

σ_h	=	static lateral earth pressure on the wall at depth d (kPa)
K	=	static earth pressure coefficient (see table below) (K_A for yielding walls, K_o for non-yielding walls)
γ	=	unit weight of retained soil (see table below), use submerged unit weight below groundwater level
d	=	depth below top of fill where pressure is computed (m)
q	=	value of any surcharge (kPa)

A lateral earth pressure due to backfill compaction should be added to the calculated lateral earth pressure in accordance with Clause 6.12.3 of the CHBDC. Typical earth pressure coefficients for backfill on vertical structures are shown in Table 10-1.

Table 10-1. Earth Pressure Coefficients

Condition	Earth Pressure Coefficient					
	OPSS Granular A or Granular B Type II $\phi = 35^\circ$ $\gamma = 22.8 \text{ kN/m}^3$		OPSS Granular B Type I $\phi = 32^\circ$ $\gamma = 21.2 \text{ kN/m}^3$		OPSS SSM $\phi = 30^\circ$ $\gamma = 20.0 \text{ kN/m}^3$	
	Horizontal Surface Behind Wall	Sloping Surface Behind Wall (2H:1V)	Horizontal Surface Behind Wall	Sloping Surface Behind Wall (2H:1V)	Horizontal Surface Behind Wall	Sloping Surface Behind Wall (2H:1V)
Active, K_A (Yielding Wall)	0.27	0.39	0.31	0.47	0.33	0.54
At Rest, K_O (Non-Yielding Wall)	0.43	-	0.47	-	0.50	-
Passive, K_P (Movement towards Soil Mass)	3.7	-	3.3	-	3.0	-
Soil Group(*)	"medium dense sand"		"loose to medium dense sand"		"loose sand"	

Note: (*) for use with Figure C6.16 of the Commentary to the CHBDC.

The use of a material with a high friction angle and low active earth pressure coefficient (Granular A or Granular B Type II) is preferred as it results in lower earth pressures acting on the culvert.

The parameters in the table correspond to full mobilization of active and passive earth pressures and require certain relative movements between the wall and adjacent soil to produce these conditions. The values to be used in design can be assessed from Figure C6.16 of the Commentary to the CHBDC using the soil group designation as outlined in the table above. Active pressures should be used for any head walls or unrestrained walls. For rigid structures such as a concrete box culvert, at-rest/non-yielding horizontal earth pressures should be used for design.

10.4.2 Combined Static and Seismic Lateral Earth Pressure Parameters

In accordance with Clause 4.6.5 of the CHBDC (S6-14), a structure should be designed using dynamic earth pressure coefficient that incorporate the effects of earthquake loading. The following recommendations are per Section C4.6.5 of the Commentary of the CHBDC which states that seismically induced lateral soil pressures may be calculated using Mononobe-Okabe Method with:

- $k_h = \frac{1}{2} * F(PGA) * PGA$, for structures that allow 25 to 50 mm of movement, and
- $k_h = F(PGA) * PGA$, for non-yielding walls

The coefficients of horizontal earth pressure for combined static and seismic loading presented in Table 10-2 may be used for vertical walls. The provided earth pressure coefficients are for a Seismic Site Class D, PGA_{ref} with a 2% probability of exceedance in 50 years (2475-year event) of 0.069g (Geological Survey of Canada – Fifth Generation) and an $F(PGA)$ of 1.29 as per Table 4.8 of the CHBDC (S6-14).

Table 10-2. Combined Static and Dynamic Earth Pressure Coefficients

Condition	Earth Pressure Coefficient			
	OPSS Granular A or OPSS Granular B Type II $\phi = 35^\circ, \gamma = 22.8 \text{ kN/m}^3$		OPSS Granular B Type I $\phi = 32^\circ, \gamma = 21.2 \text{ kN/m}^3$	
	Horizontal Surface Behind Wall	Slope Surface Behind Wall (2H:1V)	Horizontal Surface Behind Wall	Slope Surface Behind Wall (2H:1V)
Active, K_{AE} Yielding Wall	0.30	0.45	0.33	0.55
Active, K_{AE} Non-Yielding Wall	0.32	0.53	0.36	0.73

The total pressure due to combined static and seismic loads acting at a specific depth below the top of the wall may be determined using the following equation that includes consideration of material properties and the soils profile.

$$\sigma_h = K * \gamma * d + (K_{AE} - K_A) * \gamma * (H - d)$$

where:

- σ_h = combined static and dynamic lateral earth pressure on the wall at depth d (kPa)
- d = depth below the top of the wall where pressure is computed (m)
- K = static earth pressure coefficient (see Table 10-1)
(K_A for yielding walls, K_o for non-yielding walls)
- γ = unit weight of retained soil, use submerged unit weight below groundwater level
- K_{AE} = combined static and dynamic earth pressure coefficient
- H = total height of the wall (m)

10.5 Embankment Design and Reinstatement

10.5.1 Embankment Reconstruction

Embankment reconstruction after culvert replacement should be carried out in accordance with OPSS.PROV 206. The embankment should be reinstated with side slopes of 2H:1V (or flatter) if constructed using Select Subgrade Material (SSM) or Granular B Type I. If

steeper embankment slopes are required for property constraints or to match the southern embankment slope rock fill may be used. Embankments constructed full height with rock fill could be placed with slopes as steep as 1.25H:1V. In no case should rock fill be used within the culvert bedding, cover or embedment envelope. The fill should be placed and compacted in accordance with OPSS.PROV 501.

Where new embankment fill is placed against existing embankment slopes or on a sloping ground surface steeper than 3H:1V, benching of the existing slope should be carried out in accordance with OPSD 208.010.

10.5.2 Embankment Settlement and Stability

The condition of the existing embankment slopes was examined in the field during the field investigation and no evidence of instability (tension cracks etc.) was noted at that time. The embankment slopes did have coverage consisting of large vegetation such as trees as well as cobbles and boulders.

It is understood that no permanent grade raise is anticipated along the alignment of Highway 118 and therefore negligible settlement of the soils beneath the embankment is expected to occur. If embankment widening is required as part of the construction activities for the culvert replacement, settlements of the foundation soils will occur.

The magnitude of the embankment compression for fills constructed with granular materials is in the order of 0.5% of the embankment height and is expected to occur following fill placement. The magnitude of self compression for embankments constructed with compacted rock fill is in the order of 1.0% of the embankment height.

Provided no grade raise or embankment widening is required and proper construction methods are used, no long term or global stability issues are anticipated for reinstated embankments at this site. Material stockpiling above the existing grades is a temporary construction measure and any stability/settlement implications are the responsibility of the Contractor. Selection and placement of construction equipment (such as heavy cranes) are also the Contractor's responsibility.

10.6 Cement Type and Corrosion Potential

Analytical tests were completed to determine the potential for degradation of concrete in the presence of soluble sulphates and the potential for corrosion of exposed steel. The concentration of soluble sulphate provides an indication of the degree of sulphate attack that is expected for concrete in contact with soil and groundwater at the site. Soluble sulphate concentrations less than 1000 µg/g generally indicate that a low degree of sulphate attack is expected for concrete in contact with soil and groundwater. The class of concrete selected should consider the effects of road de-icing salts.

The pH, resistivity and chloride concentration provide an indication of the degree of corrosiveness of the sub-surface environment. The tests results provided in Section 5.8



may be used to aid in the selection of coatings and corrosion protection systems for buried steel objects. The corrosive effects of road de-icing salts should also be considered.

11 CONSTRUCTION CONSIDERATIONS

11.1 Excavation

All excavation must be carried out in accordance with the Occupational Health and Safety Act (OHSA). For the purposes of OHSA, the existing fills above the water level may be classified as Type 3 soil. Fill and native soils below the groundwater level are classified as Type 4 soils.

Excavation for the culvert replacement must be carried out in accordance with OPSS 902 and will be carried out through the existing embankment fill and will extend into the underlying native sand deposits. The sides of temporary excavations must be sloped in accordance with the requirement of OHSA. Selection of the equipment and methodology to excavate and prepare the founding surface is the responsibility of the Contractor. Protection of adjacent utilities will need to be taken into consideration when evaluating the excavation limits.

At locations where there are space restrictions, excavations will need to be carried out within a protection system. Further discussion is presented in Section 11.2.

11.2 Temporary Protection Systems

Temporary Protection Systems may be required during various stages of construction and must be implemented in accordance with OPSS.PROV 539 and designed for Performance Level 2 (maximum 25 mm horizontal deflection). The actual pressure distribution acting on the shoring system is a function of the construction sequence and the relative flexibility of the wall and these factors must be considered when designing the shoring system.

Lateral earth pressure coefficients, under fully mobilized conditions, that can be used in design of the protection system installed through existing fill and culvert backfill are provided in Table 10-1. The lateral earth pressure coefficients for the rockfill and native soil deposits assuming a vertical wall and horizontal soil slopes are given below:

Rockfill:

$$\begin{aligned}\gamma &= 18 \text{ kN/m}^3 \text{ (use submerged unit weight below groundwater level)} \\ K_A &= 0.24 \\ K_P &= 4.2\end{aligned}$$



Native Sand:

$$\begin{aligned}\gamma &= 20 \text{ kN/m}^3 \text{ (use submerged unit weight below groundwater level)} \\ K_A &= 0.33 \\ K_P &= 3.0\end{aligned}$$

Temporary protection systems are the responsibility of the Contractor and should be designed by a licensed Professional Engineer experienced in such designs and retained by the Contractor. Rockfill, as well as cobbles and boulders were encountered during the drilling investigation, which will interfere with the installation of sheet piles. A suggested NSSP to alert the Contractor is provided in Appendix G. Therefore, drilled-in soldier piles with lagging are considered a feasible option at this site from a geotechnical perspective. A suitable anchoring and/or bracing system may need to be incorporated into the temporary protection design to resist the lateral earth pressure loadings including traffic loading and surcharge loading due to construction equipment and operations.

It is recommended that the TPS be left in place and cut off in accordance with OPSS 539.

11.3 Surface and Groundwater Control

The culvert was dry at the time of the borehole investigations. A reading from the piezometer installed within Borehole 18-4 on September 29, 2018 showed the groundwater level at elevation 319.8 m which is below the culvert invert. However, this was a short term reading and the water level is expected to fluctuate. Accordingly, provisions must be made for diversion of flow and measures must be included to dewater the temporary excavation so that the culvert bedding and culvert construction may be carried out in the dry. Groundwater perched within the embankment and surface water will tend to seep into and accumulate in excavations. The Contractor must be prepared to control the groundwater and surface water at the site to permit construction in a dry and stable environment.

The design of dewatering systems is the responsibility of the Contractor. The Contract Documents must alert the Contractor to this responsibility. For box culverts the dewatering system should be designed in accordance with SP FOUN0003 which amends OPSS 902. A preconstruction survey is not required, thus Designer Fill-In ** in SP FOUN0003 should be "N/A".

For pipe culverts, the dewatering system is to be designed in accordance with OPSS.PROV 517 and SP517F01 should be invoked for this site. The hydrogeology is not considered to be complex, thus Designer Fill-In ***** in SP517F01 should be "No". A preconstruction survey is not required, thus Designer Fill-In ***** in this SP should be "N/A".

The groundwater level will fluctuate and the minimum groundwater elevation for the site at the time of construction should be taken as the water level from the design storm period defined in SP517F01 and SP FOUN0003.

Construction of cofferdams may be required to divert flow away from the area of the new culvert. A sand bag coffer dam and sump pumps are anticipated to be sufficient if the groundwater conditions at the time of construction are as they were during the foundation investigation. Alternatively, a sheet piled cofferdam can be designed following the recommendations provided in Sections 11.1 and 11.2. Note that driving of sheet piles extending into the embankment may be difficult due to the presence of rock fill, cobbles and boulders in the fill. The use of vibration during the removal of sheet pile cofferdams could result in settlement of a new culvert particularly at the inlet and outlet and must be carried out cautiously. It may be prudent to cut off the sheet piles in accordance with OPSS.539.

Excavation below the groundwater level to replace the existing culvert without prior dewatering is not recommended since the inflow of groundwater will make it difficult to maintain a dry, sound base on which to work. Disturbance of the subgrade soils is considered to be a significant risk without proper consideration of groundwater lowering. The groundwater level should be lowered to 0.5 m below the planned base of excavation for each stage of excavation.

The need for a Permit to Take Water (PTTW) should be carried out by specialists experienced in this field.

11.4 Scour Protection and Erosion Control

Scour and erosion protection should be provided for the culvert inlet and outlet areas. Design of the scour and erosion protection measures must consider hydrologic and hydraulic concerns and should be carried out by specialists experienced in this field.

Based on the subsurface conditions encountered at the drilled locations through the embankment at this site the embankment fill materials are considered to have low susceptibility to erosion as per the Wischmeier Nomograph. The native soils are considered to have low to moderate susceptibility to erosion.

Typically, rock protection should be provided over all earth surfaces in contact with flowing water. Treatment at the outlet should be in accordance with OPSD 810.010. A vegetation cover should be established on all other exposed earth surfaces to protect against surficial erosion in general accordance with OPSS.PROV 804.

It is recommended that a concrete cut-off wall be used to minimize the potential for piping and erosion around the inlet of the culvert.

12 CONSTRUCTION CONCERNS

Potential construction concerns include, but are not necessarily limited to:

- Rock fill, cobbles and boulders were encountered as part of the embankment fill. These and potentially other buried obstructions may be encountered during



excavation in the embankment fill and will interfere with driving of protection systems.

- Groundwater levels will fluctuate. Excavation may require lowering the groundwater level below the excavation base to maintain a reasonably dry excavation and stable side slopes.
- The Contractor's selection of construction equipment and methodology must include assessment of the capability of the existing embankment to support the proposed construction equipment and any temporary structure fill (i.e., as a pad for crane support).

The successful performance of the culvert installation will depend largely upon good workmanship and quality control during construction. Subgrade examination in accordance with SP109S12 should be carried out by qualified geotechnical personal during construction to confirm that foundation recommendations are correctly implemented, and material specifications are met.

13 CLOSURE

Engineering analysis and preparation of this report were carried out by Miss Katya Edney, P.Eng. and Mr. Stephen Peters, P.Eng. The report was reviewed by Dr. Fred Griffiths, P.Eng and Dr. P.K. Chatterji, P.Eng a Designated Principal Contact for MTO Foundation Projects.

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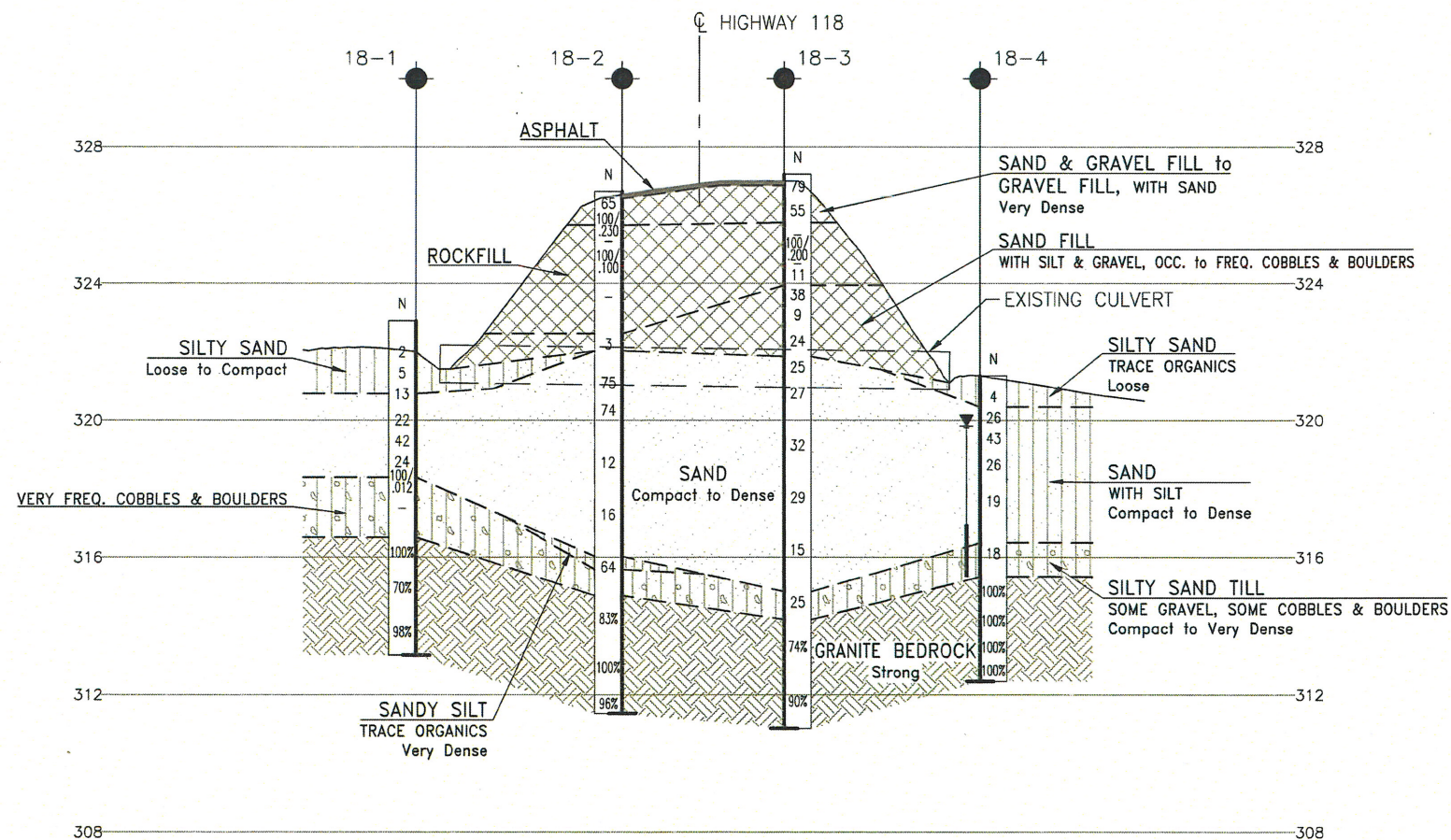
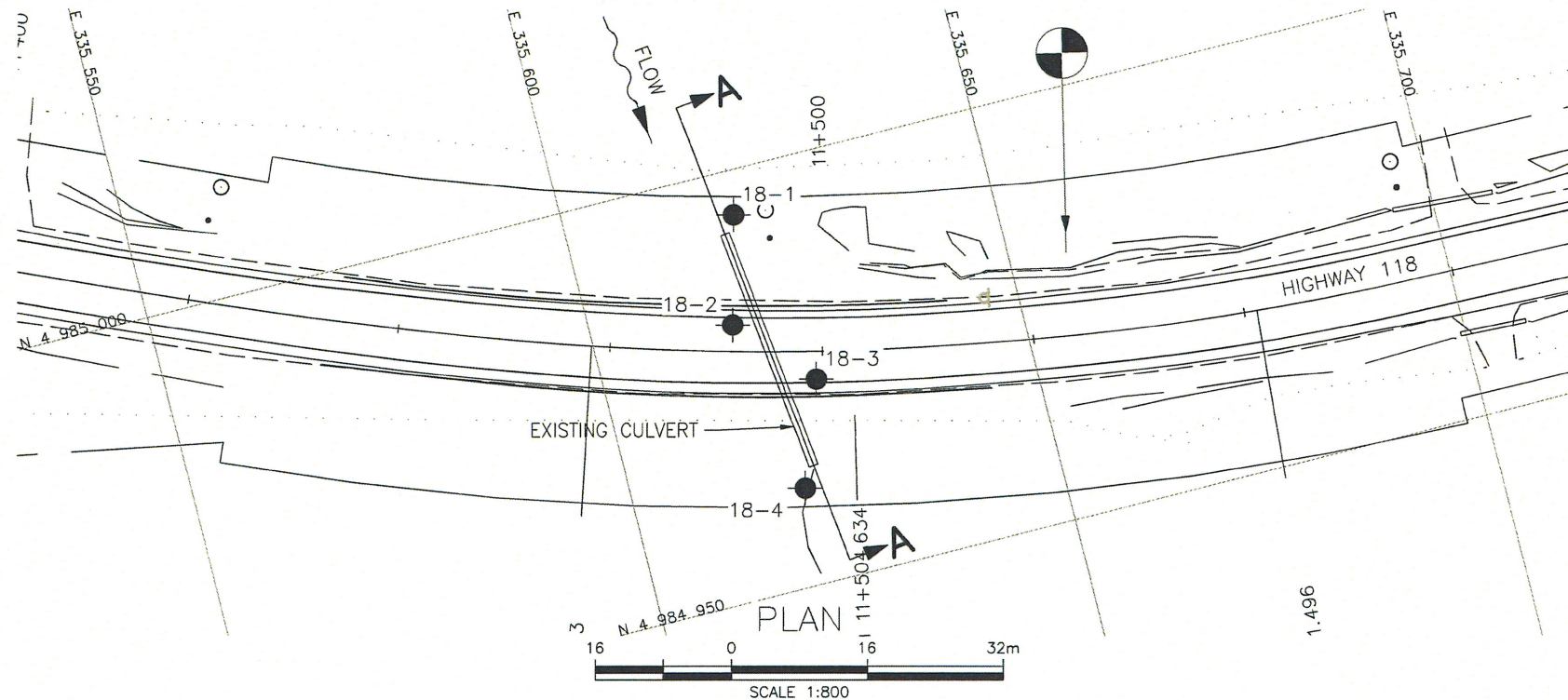


Dr. P.K. Chatterji, P.Eng.
Review Principal
Senior Geotechnical Engineer



Appendix A.

Borehole Location Plan and Stratigraphic Drawing



METRIC
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES
UNLESS OTHERWISE SHOWN



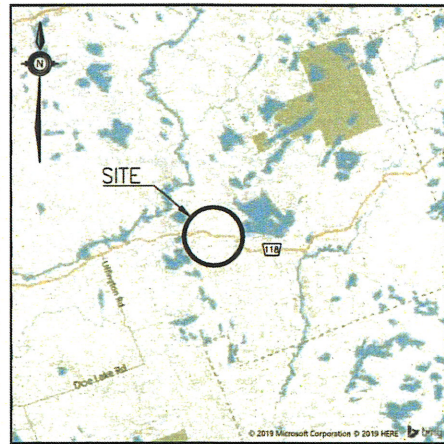
CONT No
GWP No 5287-14-00

HIGHWAY 118
STATION 11+494
CULVERT REPLACEMENT
BOREHOLE LOCATIONS AND SOIL STRATA

McINTOSH PERRY



THURBER ENGINEERING LTD.



LEGEND

- Current Borehole by Thurber
- ⊕ Previous Borehole by Others (Approx.)
- N Blows /0.3m (Std Pen Test, 475J/blow)
- CONE Blows /0.3m (60' Cone, 475J/blow)
- PH Pressure, Hydraulic
- ⊕ Water Level
- ⊕ Head Artesian Water
- ⊕ Piezometer
- 90% Rock Quality Designation (RQD)
- A/R Auger Refusal

NO	ELEVATION	NORTHING	EASTING
18-1	322.3	4 984 994.7	335 619.8
18-2	326.7	4 984 982.2	335 616.5
18-3	327.2	4 984 973.6	335 624.5
18-4	321.0	4 984 961.5	335 620.1

-NOTES-

- The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.
- This drawing is for subsurface information only. Surface details and features are for conceptual illustration.
- Coordinate system is MTM NAD 83 Zone 10.

GEOCREs No. 31E-397

DATE	BY	DESCRIPTION
DESIGN	KE	CHK SP
DRAWN	MFA	CHK KE
		CODE
		LOAD
		DATE
		APR 2019
		STRUCT
		DWG



Appendix B.
Record of Borehole Sheets



SYMBOLS, ABBREVIATIONS AND TERMS USED ON TEST HOLE RECORDS

TERMINOLOGY DESCRIBING COMMON SOIL GENESIS

Topsoil	mixture of soil and humus capable of supporting vegetative growth
Peat	mixture of fragments of decayed organic matter
Till	unstratified glacial deposit which may include particles ranging in sizes from clay to boulder
Fill	material below the surface identified as placed by humans (excluding buried services)

TERMINOLOGY DESCRIBING SOIL STRUCTURE:

Desiccated	having visible signs of weathering by oxidization of clay materials, shrinkage cracks, etc.
Fissured	having cracks, and hence a blocky structure
Varved	composed of alternating layers of silt and clay
Stratified	composed of alternating successions of different soil types, e.g. silt and sand
Layer	> 75 mm in thickness
Seam	2 mm to 75 mm in thickness
Parting	< 2 mm in thickness

RECOVERY:

For soil samples, the recovery is recorded as the length of the soil sample recovered.

N-VALUE:

Numbers in this column are the field results of the Standard Penetration Test: the number of blows of a 63.5 kg hammer falling 0.76 m, required to drive a 50 mm O.D. split spoon sampler 0.3 m into undisturbed soil. For samples where insufficient penetration was achieved and N-value cannot be presented, the number of blows are reported over the sampler penetration in millimetres (e.g. 50/75).

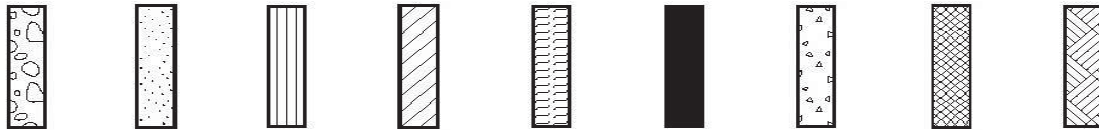
DYNAMIC CONE PENETRATION TEST (DCPT):

Dynamic cone penetration tests are performed using a standard 60 degree apex cone connected to an "A" size drill rods with the same standard fall height and weight as the Standard Penetration Test. The DCPT value is the number of blows of the hammer required to drive the cone 0.3 m into the soil. The DCPT is used as a probe to assess soil variability.



STRATA PLOT:

Strata plots symbolize the soil and bedrock description. They are combinations of the following basic symbols. The dimensions within the strata symbols are not indicative of the particle size, layer thickness, etc.



Boulders
Cobbles
Gravel Sand Silt Clay Organics Asphalt Concrete Fill Bedrock

TEXTURING CLASSIFICATION OF SOILS

Classification	Particle Size
Boulders	Greater than 200 mm
Cobbles	75 – 200 mm
Gravel	4.75 – 75 mm
Sand	0.075 – 4.75 mm
Silt	0.002 – 0.075 mm
Clay	Less than 0.002 mm

TERMS DESCRIBING CONSISTENCY (COHESIVE SOILS ONLY)

Descriptive Term	Undrained Shear Strength (kPa)
Very Soft	12 or less
Soft	12 – 25
Firm	25 – 50
Stiff	50 – 100
Very Stiff	100 – 200
Hard	Greater than 200

NOTE: Clay sensitivity is defined as the ratio of the undisturbed strength over the remolded strength.

SAMPLE TYPES

SS	Split spoon samples
ST	Shelby tube or thin wall tube
DP	Direct push sample
PS	Piston sample
BS	Bulk sample
WS	Wash sample
HQ, NQ, BQ etc.	Rock core sample obtained with the use of standard size diamond coring equipment

TERMS DESCRIBING CONSISTENCY (COHESIONLESS SOILS ONLY)

Descriptive Term	SPT “N” Value
Very Loose	Less than 4
Loose	4 – 10
Compact	10 – 30
Dense	30 – 50
Very Dense	Greater than 50

MODIFIED UNIFIED SOIL CLASSIFICATION

Major Divisions		Group Symbol	Typical Description
COARSE GRAINED SOIL	GRAVEL AND GRAVELLY SOILS	GW	Well-graded gravels or gravel-sand mixtures, little or no fines.
		GP	Poorly-graded gravels or gravel-sand mixtures, little or no fines.
		GM	Silty gravels, gravel-sand-silt mixtures.
		GC	Clayey gravels, gravel-sand-clay mixtures.
	SAND AND SANDY SOILS	SW	Well-graded sands or gravelly sands, little or no fines.
		SP	Poorly-graded sands or gravelly sands, little or no fines.
		SM	Silty sands, sand-silt mixtures.
		SC	Clayey sands, sand-clay mixtures.
FINE GRAINED SOILS	SILT AND CLAY SOILS $W_L < 35\%$	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
		OL	Organic silts and organic silty-clays of low plasticity.
	SILT AND CLAY SOILS $35\% < W_L < 50\%$	MI	Inorganic compressible fine sandy silt with clay of medium plasticity, clayey silts.
		CI	Inorganic clays of medium plasticity, silty clays.
		OI	Organic silty clays of medium plasticity.
	SILT AND CLAY SOILS $W_L > 50\%$	MH	Inorganic silts, micaceous or diatomaceous fine sandy of silty soils, elastic silts.
		CH	Inorganic clays of high plasticity, fat clays.
		OH	Organic clays of high plasticity, organic silts.
HIGHLY ORGANIC SOILS		Pt	Peat and other organic soils.

Note - W_L = Liquid Limit



EXPLANATION OF ROCK LOGGING TERMS

ROCK WEATHERING CLASSIFICATION

Fresh (FR)	No visible signs of weathering.
Fresh Jointed (FJ)	Weathering limited to surface of major discontinuities.
Slightly Weathered (SW)	Penetrative weathering developed on open discontinuity surfaces, but only slight weathering of rock materials.
Moderately Weathered (MW)	Weathering extends throughout the rock mass, but the rock material is not friable.
Highly Weathered (HW)	Weathering extends throughout the rock mass and the rock is partly friable.
Completely Weathered (CW)	Rock is wholly decomposed and in a friable condition, but the rock texture and structures are preserved.

TERMS

Total Core Recovery: (TCR)	Core recovered as a percentage of total core run length.
Solid Core Recovery: (SCR)	Percent ratio of solid core of full cylindrical shape recovered. Expressed with respect to the total length of core run.
Rock Quality Designation: (RQD)	Total length of sound core recovered in pieces 0.1 m in length or larger, as a percentage of total core length
Unconfined Compressive Strength: (UCS)	Axial stress required to break the specimen.
Fracture Index: (FI)	Frequency of natural fractures per 0.3 m of core run.

DISCONTINUITY SPACING

Bedding	Bedding Plane Spacing
Very thickly bedded	Greater than 2 m
Thickly bedded	0.6 to 2 m
Medium bedded	0.2 to 0.6 m
Thinly bedded	60 mm to 0.2 m
Very thinly bedded	20 to 60 mm
Laminated	6 to 20 mm
Thinly laminated	Less than 6 mm

STRENGTH CLASSIFICATION

Rock Strength	Approximate Uniaxial Compressive Strength (MPa)
Extremely Strong	Greater than 250
Very Strong	100 – 250
Strong	50 – 100
Medium Strong	25 – 50
Weak	5 – 25
Very Weak	1 – 5
Extremely Weak	0.25 – 1

RECORD OF BOREHOLE No 18-1

1 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004269°, Long: -79.109051°
St. 11+494 N 4 984 994.7 E 335 619.8 ORIGINATED BY SOB
HWY 118 BOREHOLE TYPE Portable NWT Washboring COMPILED BY AC
DATUM Geodetic DATE 16.09.2018 - 17.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa						
								20 40 60 80 100						
322.9														
0.0	STAND													
322.3														
0.6	SILTY SAND (SM) with organics very loose to compact brown		1	SS	2		322							0 65 30 5 non-plastic
			2	SS	5		321							
320.8			3	SS	13									
2.1	SAND (SP) compact to dense brown													0 97 3 (SI+CL)
			4	SS	22		320							
			5	SS	42		319							
			6	SS	24									
318.3			7	SS	100/		318							RUN #1 TCR=100% SCR=100% RQD=100%
4.6	SAND TILL cobbles and boulders very dense grey													
			8	NQ	-		317							
316.6														
6.3	BEDROCK GRANITE fresh medium to coarse grained strong grey with red seams		1	RUN			316							
			2	RUN			315							
			3	RUN			314							
313.1														RUN #3 TCR=98% SCR=87% RQD=98%
9.8	End of Borehole													

DOUBLE LINE ST 11+490.GPJ 2012TEMPLATE(MTO).GDT 21/8/19

Continued Next Page

+³, ×³: Numbers refer to
Sensitivity 20
15 10 5 (%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-1

2 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004269°, Long: -79.109051°
St. 11+494 N 4 984 994.7 E 335 619.8 ORIGINATED BY SOB
HWY 118 BOREHOLE TYPE Portable NWT Washboring COMPILED BY AC
DATUM Geodetic DATE 16.09.2018 - 17.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa									
	Continued From Previous Page																
	A half-weight (32 kg) drop hammer was used to advance the split-spoon sampler. The N values presented have been adjusted to provide an equivalent N value that would have been obtained with a standard 64 kg hammer.																

DOUBLE LINE ST 11+490.GPJ 2012TEMPLATE(MTO).GDT 21/8/19

RECORD OF BOREHOLE No 18-2

1 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004157°, Long: -79.109093° St. 11+494 N 4 984 982.2 E 335 616.5 ORIGINATED BY AC
 HWY 118 BOREHOLE TYPE NW Washboring COMPILED BY AC
 DATUM Geodetic DATE 16.09.2018 - 16.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC NATURAL LIQUID LIMIT MOISTURE CONTENT LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				GR	SA	SI	CL
326.7								20	40	60	80	100							
0.0																			
0.1	ASPHALT (100 mm)																		
	SAND with gravel to GRAVEL with sand very dense brown to grey-brown FILL		1	SS	65														
325.7			2	SS	100/														
1.0	ROCKFILL 0.9 m boulder at 1 m				230 mm														
			3	NQ	-														
			4	SS	100/														
					100 mm														
			5	NQ	-														
322.5																			
4.2	SAND with silt and gravel occasional cobbles and boulders very loose red-brown FILL		6	SS	3														
322.0																			
4.7	SAND (SP-SM) with silt very dense red-brown																		
			7	SS	75														
			8	SS	74														
319.1																			
7.6	SAND (SP) compact grey to grey-brown		9	SS	12														
			10	SS	16														
			</																

Continued Next Page

+³, ×³: Numbers refer to Sensitivity 20 15 10 5 0 (%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-2

2 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004157°, Long: -79.109093°
St. 11+494 N 4 984 982.2 E 335 616.5 ORIGINATED BY AC
HWY 118 BOREHOLE TYPE NW Washboring COMPILED BY AC
DATUM Geodetic DATE 16.09.2018 - 16.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL				
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa							WATER CONTENT (%)			
								20 40 60 80 100										
								<div><div></div><div></div><div></div><div></div><div></div></div> <div>20 40 60 80 100</div>					<div><div></div><div></div><div></div><div></div><div></div></div> <div>20 40 60</div>					
Continued From Previous Page																		
316.0	SAND (SP) compact grey to grey-brown						316								42 47 11 (SI+CL)			
10.7	SANDY SILT (ML) trace organics very dense		11	SS	64													
315.7	grey																	
11.0	SAND (SP-SM) with silt and gravel TILL very dense						315								RUN #1 TCR=100% SCR=87% RQD=83%			
314.9	grey																	
11.8	BEDROCK GRANITE fresh medium grained strong grey with black and pink		1	RUN			314											
			2	RUN			313								RUN #2 TCR=100% SCR=100% RQD=100%			
			3	RUN			312								RUN #3 TCR=96% SCR=96% RQD=96%			
311.5																		
15.2	End of Borehole																	

+³, ×³: Numbers refer to
Sensitivity 20
15 10 5
(%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-3

1 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004081°, Long: -79.108992°
St. 11+494 N 4 984 973.6 E 335 624.5 ORIGINATED BY AC
HWY 118 BOREHOLE TYPE NW Washboring COMPILED BY AC
DATUM Geodetic DATE 15.09.2018 - 15.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)				
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				GR	SA	SI	CL	
								20 40 60 80 100				w _p w w _L								
								○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE												
327.2																				
0.0																				
0.1		ASPHALT (100 mm)																		
		SAND with gravel to GRAVEL with sand very dense grey-brown FILL		1	SS	79														
				2	SS	55														
325.8																				
1.4		ROCKFILL 0.8 m boulder at 1.4 m		3	NQ	-														
				4	SS	100/														
		0.3 m boulder at 2.3 m		5	NQ	200 mm														
				6	SS	11														
323.9																				
3.3		SAND with silt and gravel frequent cobbles and boulders loose to dense brown FILL		7	SS	38											26	67	7 (SI+CL)	
				8	SS	9														
				9	SS	24														
321.9																				
5.3		SAND (SP-SM) with silt compact to dense brown		10	SS	25														
				11	SS	27												0	95	5 (SI+CL)
				12	SS	32														
				13	SS	29														

Continued Next Page

+³, ×³: Numbers refer to
Sensitivity



20
15
10
(%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-3

2 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.004081°, Long: -79.108992°
St. 11+494 N 4 984 973.6 E 335 624.5 ORIGINATED BY AC
HWY 118 BOREHOLE TYPE NW Washboring COMPILED BY AC
DATUM Geodetic DATE 15.09.2018 - 15.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)						
								20 40 60 80 100				w _P w w _L						
Continued From Previous Page							○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE											
315.0	SAND (SP-SM) with silt compact to dense brown to grey		14	SS	15		317									11 73 16 (SI+CL)		
12.2			SILTY SAND (SM) some gravel TILL compact grey	15	SS		25	316										
314.2								315										
13.0	BEDROCK GRANITE fresh to slightly weathered medium to coarse grained strong grey with black and pink		1	RUN			314									RUN #1 TCR=100% SCR=89% RQD=74%		
			2	RUN			313											
							312											
311.0																RUN #2 TCR=100% SCR=90% RQD=90%		
16.2	End of Borehole																	



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RECORD OF BOREHOLE No 18-4

1 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.003971°, Long: -79.109048°
St. 11+494 N 4 984 961.5 E 335 620.1 ORIGINATED BY SOB
HWY 118 BOREHOLE TYPE Portable NWT Washboring COMPILED BY AC
DATUM Geodetic DATE 28.09.2018 - 29.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				
								20	40	60	80	100	W _P	W		
321.3																
0.0	STAND															
321.0																
0.3	SILTY SAND (SM) trace organics very loose brown		1	SS	4										0 69 25 6 non-plastic	
320.4																
0.9	SAND (SP-SM) with silt compact to dense brown		2	SS	26										0 91 9 (SI+CL)	

Continued Next Page

+³, ×³: Numbers refer to Sensitivity 20 15 10 5 (%) STRAIN AT FAILURE

RECORD OF BOREHOLE No 18-4

2 OF 2

METRIC

GWP# 5287-14-00 LOCATION Lat: 45.003971°, Long: -79.109048° ORIGINATED BY SOB
 HWY 118 BOREHOLE TYPE Portable NWT Washboring COMPILED BY AC
 DATUM Geodetic DATE 28.09.2018 - 29.09.2018 CHECKED BY KE

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT	NATURAL MOISTURE CONTENT	LIQUID LIMIT	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa	WATER CONTENT (%)					
	Continued From Previous Page							20 40 60 80 100 ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE	20 40 60 W P W W L					
	Water level in 19 mm diameter standpipe: 29/09/2018 at 1.2 mbgs (el. 319.8 m)													

DOUBLE LINE ST 11+490.GPJ 2012TEMPLATE(MTO).GDT 21/8/19



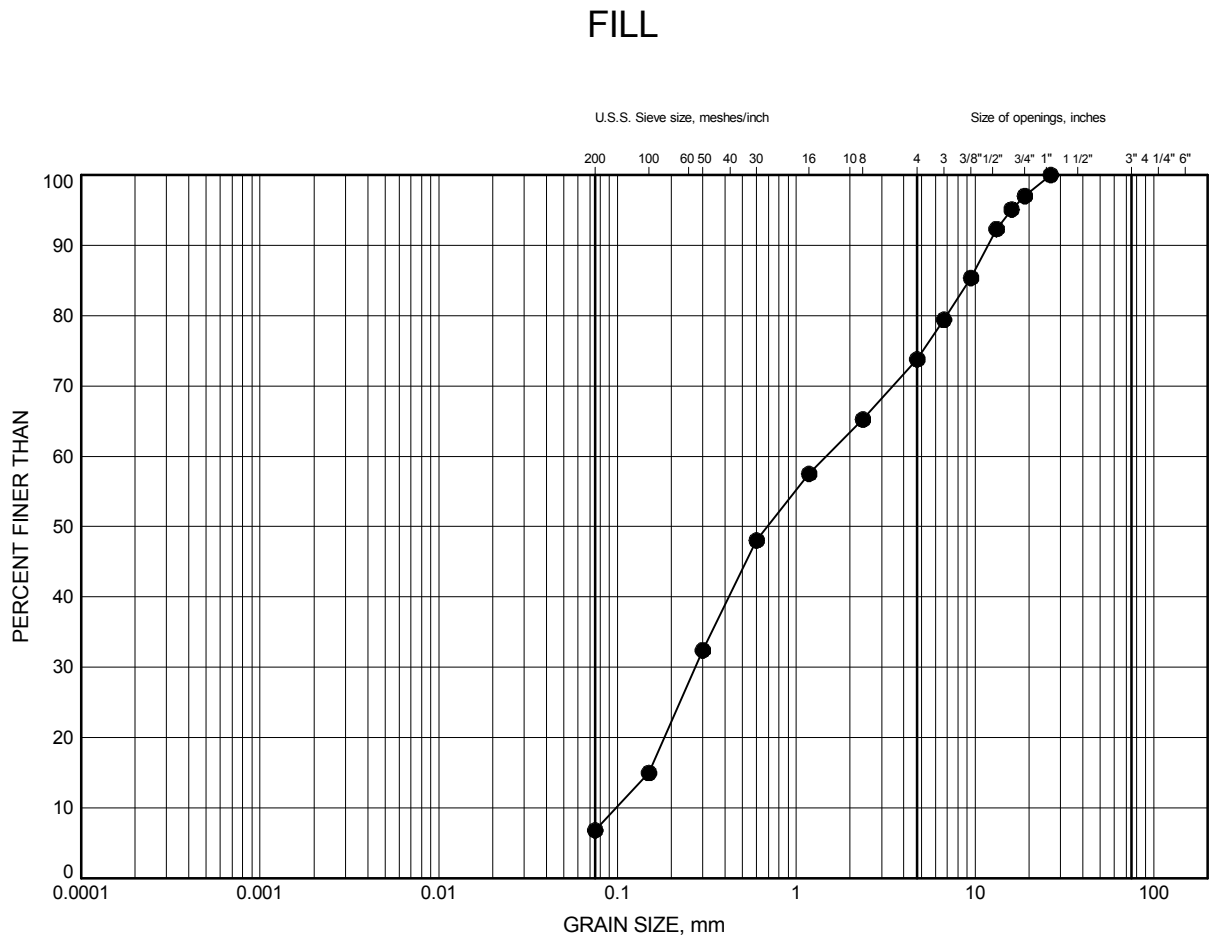
Appendix C.
Laboratory Testing



Appendix C.1
Particle Size Analysis Figures

HWY 118 Culverts Station 11+494 GRAIN SIZE DISTRIBUTION

FIGURE C1



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-3	3.5	323.7

Date ..October 2018.....
 GWP# ..5287-14-00.....

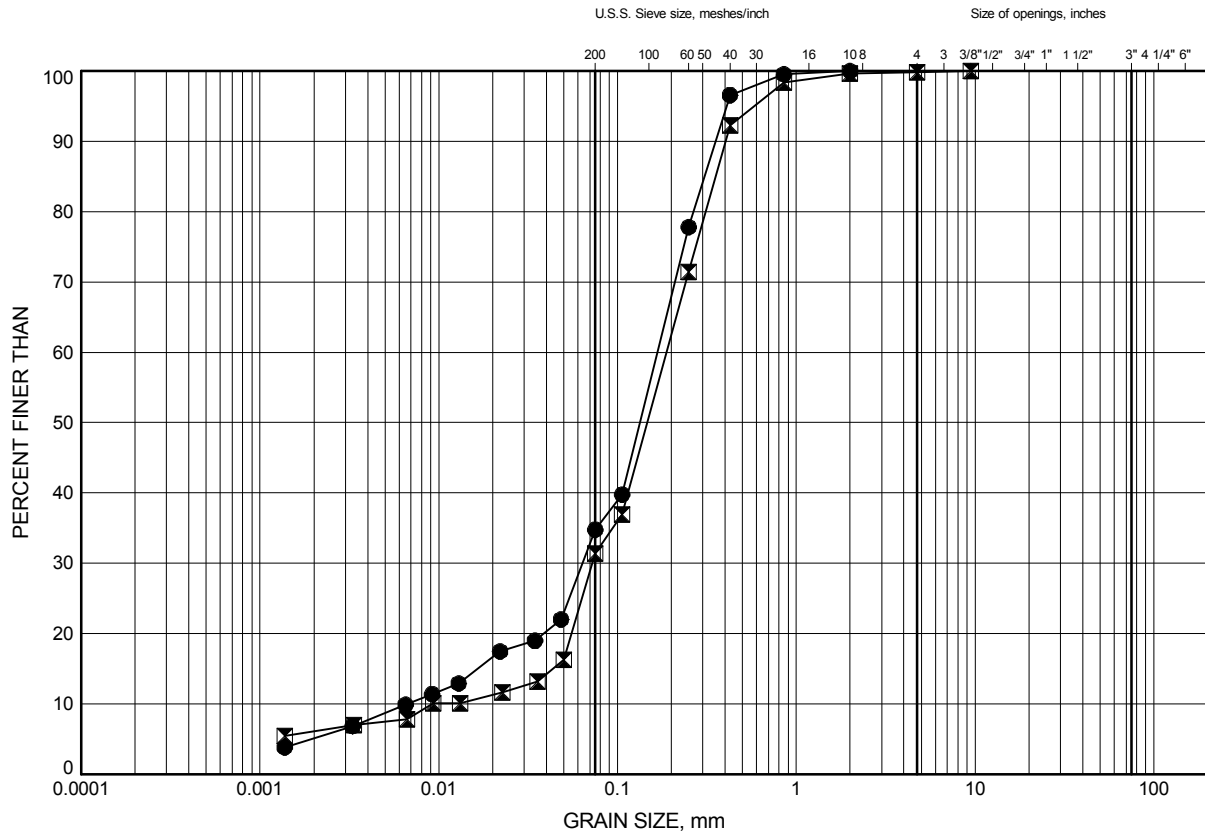


Prep'dAC.....
 Chkd.KE.....

HWY 118 Culverts Station 11+494 GRAIN SIZE DISTRIBUTION

FIGURE C2

SILTY SAND (SM) with organics



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	0.9	321.4
⊠	18-4	0.3	320.7

Date ..October 2018.....
GWP# ..5287-14-00.....

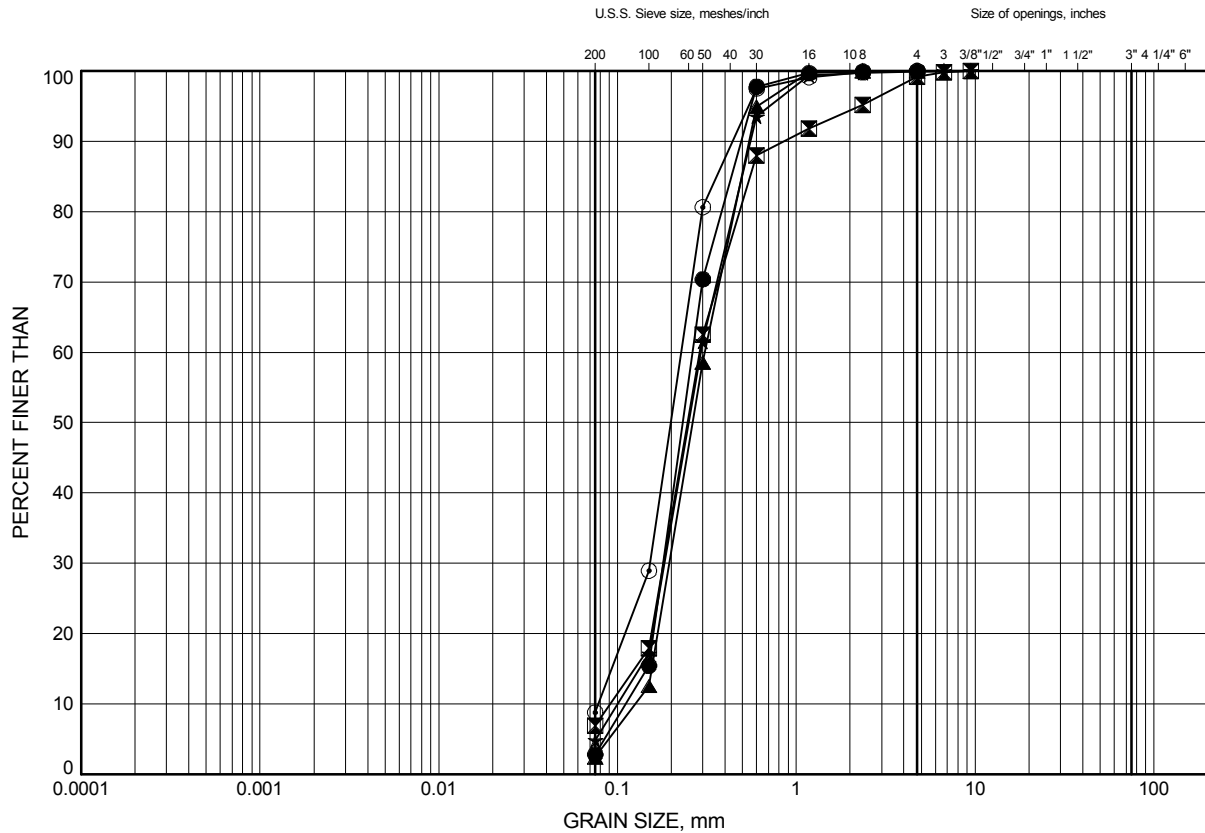


Prep'dAC.....
Chkd.KE.....

HWY 118 Culverts Station 11+494 GRAIN SIZE DISTRIBUTION

FIGURE C3

SAND (SP to SP-SM)



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-1	2.9	319.4
⊠	18-2	5.6	321.1
▲	18-2	9.4	317.3
★	18-3	6.4	320.8
⊙	18-4	0.9	320.1

Date ..October 2018.....

GWP# ..5287-14-00.....



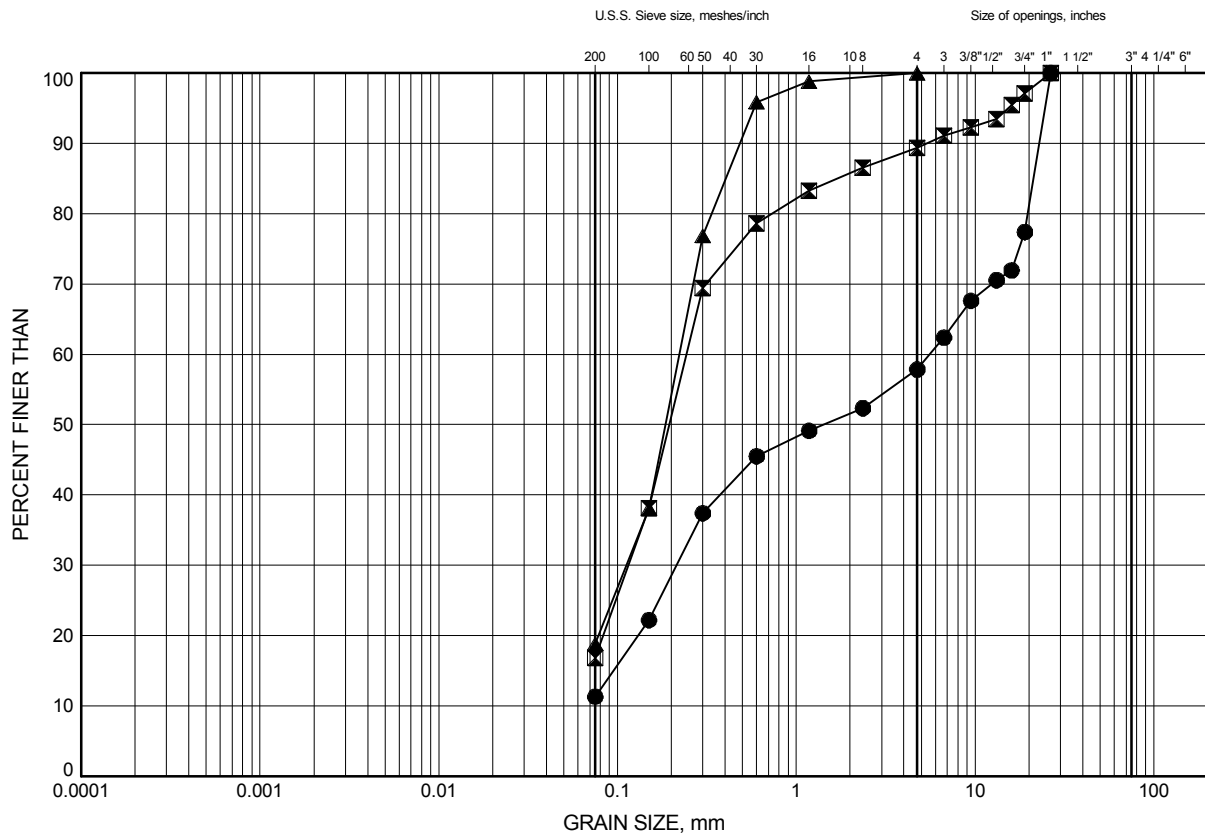
Prep'dAC.....

Chkd.KE.....

HWY 118 Culverts Station 11+494 GRAIN SIZE DISTRIBUTION

FIGURE C4

SAND (SP-SM) to SILTY SAND (SM) (TILL)



SILT and CLAY	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE SIZE
FINE GRAINED	SAND			GRAVEL		

LEGEND

SYMBOL	BOREHOLE	DEPTH (m)	ELEV. (m)
●	18-2	11.2	315.5
⊠	18-3	12.5	314.7
▲	18-4	4.9	316.1

Date October 2018.....
GWP# 5287-14-00.....



Prep'd AC.....
Chkd. KE.....



Appendix C.2
Rock Core Photos
Rock Core Testing Results

Borehole 18-1
Run 1 to 3 (of 3)
Elevation 316.6 m to 313.2 m



THURBER ENGINEERING LTD.

Foundation Investigation
Hwy 118 Culverts St. 11+494
Foundations

GWP: 5287-14-00

Project No.: 20244

Borehole 18-2
Run 1 to 3 (of 3)
Elevation 314.9 m to 311.5 m



THURBER ENGINEERING LTD.

Foundation Investigation
Hwy 118 Culverts St. 11+494
Foundations

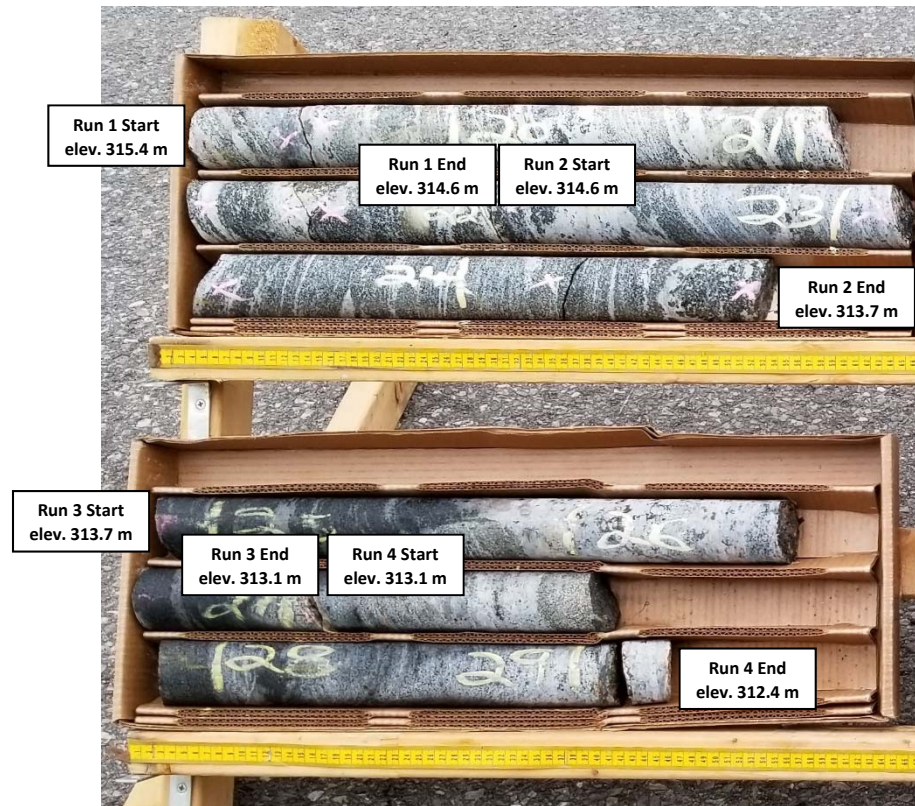
GWP: 5287-14-00

Project No.: 20244

Borehole 18-3
Run 1 to 2 (of 2)
Elevation 314.2 m to 311.0 m



Borehole 18-4
Run 1 to 4 (of 4)
Elevation 315.4 m to 312.4 m





Stantec

Stantec Consulting Ltd
2781 Lancaster Rd, Suite 100 A&B
Ottawa, ON K1B 1A7
Tel: (613) 738-6075
Fax: (613) 722-2799

October 17, 2018
File: 122410864

Attention: Thurber Engineering Ltd., File #20244

Reference: ASTM D7012, Method C, Unconfined Compressive Strength of Intact Rock Core

The table below summarizes five (5) rock core unconfined compressive strength results.

Location	Sample Depth	Compressive Strength (MPa)	Description of Break
18+550, 18-2 Run-1	26'1"-27'1"	115.0	Diagonal Fracture with no cracking through ends
18+550, 18-4 Run-1	11'6"-12'1"	141.6	Well-formed cone on one end. Vertical crack, no well-defined cone on the other end
18+875, 18-1 Run-2	7'7"-8'1"	127.8	Well-formed cone on one end. Vertical crack, no well-defined cone on the other end
18+875, 18-4 Run-1	7'2"-7'9"	76.2	Columnar vertical crack through both ends, no well-defined cones
11+490, 18-1 Run-2	23';7"-24'3"	88.4	Columnar vertical crack through both ends, no well-defined cones

Sincerely,

Stantec Consulting Ltd

Brian Prevost

Brian Prevost
Laboratory Supervisor
Tel: 613-738-6075
brian.prevost@stantec.com



Appendix C.3

Analytical Testing Results

Certificate of Analysis

Thurber Engineering Ltd.

2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya Edney

Client PO: 20244
Project: Hwy 11+118
Custody: 39862

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018

Order #: 1839096

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1839096-01	18+250 18-1 SS3 6-8'
1839096-02	18+250 18-4 SS3 5'6"-7-6"
1839096-03	18+875 18-2 SS8 17'6"-19'6"
1839096-04	11+490 18-01 SS3B 7-8'

Depths provided in results are measured from the top of the drilling platform not shown in the Record of Borehole Sheets. Platform height measured 0.6 and 0.3 m for Boreholes 18-1 and 18-4, respectively.

Approved By:



Mark Foto, M.Sc.
Lab Supervisor

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018
Project Description: Hwy 11+118

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC, water extraction	26-Sep-18	26-Sep-18
Conductivity	MOE E3138 - probe @25 °C, water ext	27-Sep-18	27-Sep-18
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	27-Sep-18	27-Sep-18
Resistivity	EPA 120.1 - probe, water extraction	27-Sep-18	27-Sep-18
Solids, %	Gravimetric, calculation	27-Sep-18	27-Sep-18

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018

Order Date: 24-Sep-2018

Project Description: Hwy 11+118

Client ID:	18+250 18-1 SS3 6-8'	18+250 18-4 SS3 5'6"-7-6"	18+875 18-2 SS8 17'6"-19'6"	11+490 18-01 SS3B 7-8'
Sample Date:	09/12/2018 09:00	09/14/2018 09:00	09/19/2018 09:00	09/16/2018 09:00
Sample ID:	1839096-01	1839096-02	1839096-03	1839096-04
MDL/Units	Soil	Soil	Soil	Soil

Physical Characteristics

% Solids	0.1 % by Wt.	79.4	79.6	80.3	82.1
----------	--------------	------	------	------	------

General Inorganics

Conductivity	5 uS/cm	227	243	1340	383
pH	0.05 pH Units	5.86	5.11	4.82	5.32
Resistivity	0.10 Ohm.m	44.1	41.1	7.45	26.1

Anions

Chloride	5 ug/g dry	40	104	1260	236
Sulphate	5 ug/g dry	129	62	70	24

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018
Project Description: Hwy 11+118

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	5	ug/g						
Sulphate	ND	5	ug/g						
General Inorganics									
Conductivity	ND	5	uS/cm						
Resistivity	ND	0.10	Ohm.m						

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018
Project Description: Hwy 11+118

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	22.3	5	ug/g dry	23.2			4.1	20	
Sulphate	15.1	5	ug/g dry	15.6			3.3	20	
General Inorganics									
Conductivity	211	5	uS/cm	204			3.1	6.2	
pH	7.90	0.05	pH Units	7.93			0.4	10	
Resistivity	47.5	0.10	Ohm.m	48.9			3.1	20	
Physical Characteristics									
% Solids	79.2	0.1	% by Wt.	79.5			0.4	25	

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018
Project Description: Hwy 11+118

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	117	5	ug/g	23.2	93.8	78-113			
Sulphate	120	5	ug/g	15.6	104	78-111			

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 28-Sep-2018
Order Date: 24-Sep-2018
Project Description: Hwy 11+118

Qualifier Notes:

None

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable
ND: Not Detected
MDL: Method Detection Limit
Source Result: Data used as source for matrix and duplicate samples
%REC: Percent recovery.
RPD: Relative percent difference.

Soil results are reported on a dry weight basis when the units are denoted with 'dry'.
Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

Subcontracted Analysis

Thurber Engineering Ltd.2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya EdneyTel: (613) 247-2121
Fax: (613) 247-2185Paracel Report No **1839096**Client Project(s): **Hwy 11+118**Client PO: **20244**Reference: **Standing Offer**CoC Number: **39862**Order Date: 24-Sep-18
Report Date: 27-Sep-18

Sample(s) from this project were subcontracted for the listed parameters. A copy of the subcontractor's report is attached

Paracel ID	Client ID	Analysis
1839096-01	18+250 18-1 SS3 6-8'	Sulphide, solid
1839096-02	18+250 18-4 SS3 5'6"-7-6"	Sulphide, solid
1839096-03	18+875 18-2 SS8 17'6"-19'6"	Sulphide, solid
1839096-04	11+490 18-01 SS3B 7-8'	Sulphide, solid

**SGS Canada Inc.**

P.O. Box 4300 - 185 Concession St.
Lakefield - Ontario - K0L 2H0
Phone: 705-652-2000 FAX: 705-652-6365

Paracel Laboratories

Attn : Dale Robertson

300-2319 St.Laurent Blvd.
Ottawa, ON
K1G 4K6, Canada

Phone: 613-731-9577
Fax:613-731-9064

27-September-2018

Date Rec. : 25 September 2018
LR Report: CA13421-SEP18
Reference: Project#: 1839096

Copy: #1

CERTIFICATE OF ANALYSIS

Final Report

Sample ID	Sample Date & Time	Sulphide %
1: Analysis Start Date		27-Sep-18
2: Analysis Start Time		12:40
3: Analysis Completed Date		27-Sep-18
4: Analysis Completed Time		13:39
5: QC - Blank		< 0.02
6: QC - STD % Recovery		83%
7: QC - DUP % RPD		ND
8: RL		0.02
9: 18+250 18-1 SS3 6-8'	12-Sep-18	< 0.02
10: 18+250 18-4 SS3 5'6"-7-6"	14-Sep-18	< 0.02
11: 18+875 18-2 SS8 17'6-19'6"	19-Sep-18	< 0.02
12: 11+490 18-01 SS3B 7-8'	16-Sep-18	< 0.02

RL - SGS Reporting Limit
ND - Not Detected

Kimberley Didsbury
Project Specialist
Environmental Services, Analytical

Certificate of Analysis

Thurber Engineering Ltd.

2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya Edney

Client PO: 20244
Project: HWY11+118
Custody: 39863

Report Date: 9-Oct-2018
Order Date: 2-Oct-2018

Order #: 1840220

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1840220-01	18+550 18-1 SS3 5'6"-6'2"
1840220-02	18+550 18-4 SS2 3-5
1840220-03	18+875 18-4 SS1 2'6"-4'6"
1840220-04	11+490 18-4 SS3 5-7
1840220-05	22+590 18-1 SS2 4-6
1840220-06	22+590 18-4 SS3 6-8'

Depths provided in results are measured from the top of the drilling platform not shown in the Record of Borehole Sheets. Platform height measured 0.6 and 0.3 m for Boreholes 18-1 and 18-4, respectively.

Approved By:



Mark Foto, M.Sc.
Lab Supervisor

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 09-Oct-2018

Order Date: 2-Oct-2018

Project Description: HWY11+118

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC, water extraction	5-Oct-18	5-Oct-18
Conductivity	MOE E3138 - probe @25 °C, water ext	4-Oct-18	5-Oct-18
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	5-Oct-18	5-Oct-18
Resistivity	EPA 120.1 - probe, water extraction	4-Oct-18	5-Oct-18
Solids, %	Gravimetric, calculation	3-Oct-18	3-Oct-18

Certificate of Analysis
 Client: Thurber Engineering Ltd.
 Client PO: 20244

Report Date: 09-Oct-2018

Order Date: 2-Oct-2018

Project Description: HWY11+118

Client ID:	18+550 18-1 SS3 5'6"-6'2"	18+550 18-4 SS2 3-5	18+875 18-4 SS1 2'6"-4'6"	11+490 18-4 SS3 5-7
Sample Date:	09/23/2018 09:00	09/22/2018 09:00	09/20/2018 09:00	09/28/2018 09:00
Sample ID:	1840220-01	1840220-02	1840220-03	1840220-04
MDL/Units	Soil	Soil	Soil	Soil

Physical Characteristics

% Solids	0.1 % by Wt.	85.4	79.7	90.5	82.8
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General Inorganics

Conductivity	5 uS/cm	347	117	124	225
pH	0.05 pH Units	7.47	5.65	6.26	6.22
Resistivity	0.10 Ohm.m	28.8	85.1	80.9	44.5

Anions

Chloride	5 ug/g dry	211	55	19	124
Sulphate	5 ug/g dry	10	21	6	7

Client ID:	22+590 18-1 SS2 4-6	22+590 18-4 SS3 6-8'	-	-
Sample Date:	09/25/2018 09:00	09/26/2018 09:00	-	-
Sample ID:	1840220-05	1840220-06	-	-
MDL/Units	Soil	Soil	-	-

Physical Characteristics

% Solids	0.1 % by Wt.	86.5	85.5	-	-
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General Inorganics

Conductivity	5 uS/cm	302	15	-	-
pH	0.05 pH Units	6.44	5.59	-	-
Resistivity	0.10 Ohm.m	33.1	653	-	-

Anions

Chloride	5 ug/g dry	168	<5	-	-
Sulphate	5 ug/g dry	11	<5	-	-

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 09-Oct-2018

Order Date: 2-Oct-2018

Project Description: HWY11+118

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	5	ug/g						
Sulphate	ND	5	ug/g						
General Inorganics									
Conductivity	ND	5	uS/cm						
Resistivity	ND	0.10	Ohm.m						

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 09-Oct-2018

Order Date: 2-Oct-2018

Project Description: HWY11+118

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	205	5	ug/g dry	211			2.7	20	
Sulphate	9.29	5	ug/g dry	9.98			7.2	20	
General Inorganics									
Conductivity	364	5	uS/cm	347			4.6	6.2	
pH	11.69	0.05	pH Units	11.61			0.7	10	
Resistivity	27.5	0.10	Ohm.m	28.8			4.6	20	
Physical Characteristics									
% Solids	90.9	0.1	% by Wt.	94.3			3.8	25	

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 09-Oct-2018

Order Date: 2-Oct-2018

Project Description: HWY11+118

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	308	5	ug/g	211	97.2	78-113			
Sulphate	110	5	ug/g	9.98	100	78-111			

Certificate of Analysis
Client: Thurber Engineering Ltd.
Client PO: 20244

Report Date: 09-Oct-2018
Order Date: 2-Oct-2018
Project Description: HWY11+118

Qualifier Notes:

None

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable
ND: Not Detected
MDL: Method Detection Limit
Source Result: Data used as source for matrix and duplicate samples
%REC: Percent recovery.
RPD: Relative percent difference.

Soil results are reported on a dry weight basis when the units are denoted with 'dry'.
Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

Subcontracted Analysis

Thurber Engineering Ltd.2460 Lancaster Rd, Suite 104
Ottawa, ON K1B 4S5
Attn: Katya EdneyTel: (613) 247-2121
Fax: (613) 247-2185Paracel Report No **1840220**Client Project(s): **HWY11+118**Client PO: **20244**Reference: **Standing Offer**CoC Number: **39863**Order Date: 02-Oct-18
Report Date: 9-Oct-18

Sample(s) from this project were subcontracted for the listed parameters. A copy of the subcontractor's report is attached

Parcel ID	Client ID	Analysis
1840220-01	18+550 18-1 SS3 5'6"-6'2"	Sulphide, solid
1840220-02	18+550 18-4 SS2 3-5	Sulphide, solid
1840220-03	18+875 18-4 SS1 2'6"-4'6"	Sulphide, solid
1840220-04	11+490 18-4 SS3 5-7	Sulphide, solid
1840220-05	22+590 18-1 SS2 4-6	Sulphide, solid
1840220-06	22+590 18-4 SS3 6-8'	Sulphide, solid

**SGS Canada Inc.**

P.O. Box 4300 - 185 Concession St.
Lakefield - Ontario - K0L 2H0
Phone: 705-652-2000 FAX: 705-652-6365

Paracel Laboratories

Attn : Dale Robertson

300-2319 St.Laurent Blvd.
Ottawa, ON
K1G 4K6, Canada

Phone: 613-731-9577
Fax: 613-731-9064

10-October-2018

Date Rec. : 04 October 2018
LR Report: CA12131-OCT18
Reference: Project#:1840220

Copy: #1

CERTIFICATE OF ANALYSIS

Final Report

Sample ID	Sample Date & Time	Sulphide %
1: Analysis Start Date		05-Oct-18
2: Analysis Start Time		13:35
3: Analysis Completed Date		05-Oct-18
4: Analysis Completed Time		14:36
5: QC - Blank		< 0.02
6: QC - STD % Recovery		99%
7: QC - DUP % RPD		1%
8: RL		0.02
9: 18+550 18-1 SS3 5'6"-6'2"	23-Sep-18	< 0.02
10: 18+550 18-4 SS2 3-5	22-Sep-18	< 0.02
11: 18+875 18-4 SS1 2'6"-4'6"	20-Sep-18	< 0.02
12: 11+490 18-4 SS3 5-7	28-Sep-18	< 0.02
13: 22+590 18-1 SS2 4-6	25-Sep-18	< 0.02
14: 22+590 18-4 SS3 6-8'	26-Sep-18	< 0.02

RL - SGS Reporting Limit

Kimberley Didsbury
Project Specialist
Environmental Services, Analytical



Appendix D.

Site Photographs



Photo 1. Looking West along HWY 118 at St. 11+494 (2018/09/15)



Photo 2. Looking East along HWY 118 at St. 11+494 (2018/09/15)



Photo 3. St. 11+494 - Looking Southwest at Culvert Inlet (2018/09/11)



Photo 4. Looking East along northern embankment of HWY 118 (2018/09/11)



Photo 5. Drilling Platform Set Up for Borehole 18-1 at St. 11+494 (2018/09/16)



Photo 6. Looking North at Culvert Outlet at St. 11+494 (2018/09/11)



Appendix E.

Foundation Comparison

COMPARISON OF ALTERNATIVE FOUNDATION TYPES

	<i>Circular Pipe or Closed Box Culvert</i>	<i>Circular Pipe Culvert (Trenchless Installation)</i>	<i>Open Bottom Culvert</i>	<i>Precast Concrete Slab on Steel Sheet Piles</i>
<i>Advantages</i>	Relatively expedient installation if precast units are used. Smaller magnitude of settlement than open footing culvert due to lower bearing stress on subgrade.	Can tolerate larger magnitude of settlement than concrete (rigid frame) culverts). Avoids large open cuts Allows two lanes of traffic to be maintained throughout construction	Relatively expedient installation if precast units are used. Possibility to maintain work zone to span the existing waterway.	Potentially minimized volume of excavation and roadway protection Maintains water flow during construction Could allow for winter construction
<i>Disadvantages</i>	Requires large excavation and roadway protection. Requires water flow realignment or installation of a temporary by-pass culvert to maintain existing water flow alignment	Requires construction of entry and exit pits and access to toes of slope. Requires specialised construction equipment. Feasibility also depends on flow capacity and other hydraulic properties. Not suitable for mixed soil faces.	Requires deeper excavation increasing excavation volume and dewatering concern. Requires roadway protection. Potential for post construction settlement.	Quantity and cost of sheet piles
<i>Risks/Consequences</i>	Disruption to traffic	High risk of encountering obstructions and mixed soils Risk of encountering groundwater in a loose soil	Disruption to traffic	High risk of obstructions within the fill. Obstructions expected in till. Shallow bedrock
<i>Relative Cost</i>	Low to Medium	Medium to High	Medium	Medium to High
<i>Recommendation</i>	Recommended	Not Recommended	Not Recommended	Not Feasible



Appendix F.

GSC Seismic Hazard Calculation

2015 National Building Code Seismic Hazard Calculation

INFORMATION: Eastern Canada English (613) 995-5548 français (613) 995-0600 Facsimile (613) 992-8836
Western Canada English (250) 363-6500 Facsimile (250) 363-6565

October 18, 2018

Site: 45.0041 N, 79.1089 W User File Reference: Site 11 490

Requested by: , Thurber Engineering Ltd.

National Building Code ground motions: 2% probability of exceedance in 50 years (0.000404 per annum)

Sa(0.05)	Sa(0.1)	Sa(0.2)	Sa(0.3)	Sa(0.5)	Sa(1.0)	Sa(2.0)	Sa(5.0)	Sa(10.0)	PGA (g)	PGV (m/s)
0.085	0.119	0.118	0.103	0.086	0.052	0.027	0.0069	0.0031	0.069	0.072

Notes. Spectral ($S_a(T)$, where T is the period in seconds) and peak ground acceleration (PGA) values are given in units of g (9.81 m/s^2). Peak ground velocity is given in m/s . Values are for "firm ground" (NBCC 2015 Site Class C, average shear wave velocity 450 m/s). NBCC2015 and CSAS6-14 values are specified in **bold** font. Three additional periods are provided - their use is discussed in the NBCC2015 Commentary. Only 2 significant figures are to be used. *These values have been interpolated from a 10-km-spaced grid of points. Depending on the gradient of the nearby points, values at this location calculated directly from the hazard program may vary. More than 95 percent of interpolated values are within 2 percent of the directly calculated values.*

Ground motions for other probabilities:

Probability of exceedance per annum	0.010	0.0021	0.001
Probability of exceedance in 50 years	40%	10%	5%
Sa(0.05)	0.013	0.036	0.054
Sa(0.1)	0.020	0.053	0.078
Sa(0.2)	0.022	0.055	0.080
Sa(0.3)	0.019	0.049	0.070
Sa(0.5)	0.015	0.040	0.058
Sa(1.0)	0.0073	0.023	0.035
Sa(2.0)	0.0031	0.011	0.017
Sa(5.0)	0.0007	0.0025	0.0041
Sa(10.0)	0.0004	0.0011	0.0018
PGA	0.011	0.030	0.045
PGV	0.0090	0.029	0.045

References

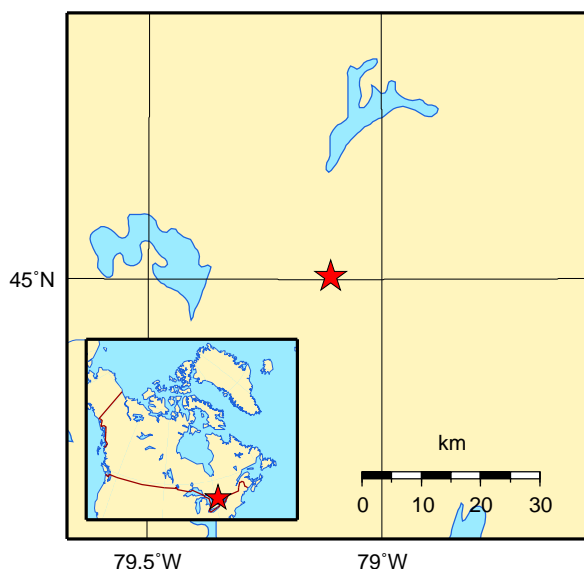
National Building Code of Canada 2015 NRCC no. 56190;
Appendix C: Table C-3, Seismic Design Data for Selected Locations in Canada

User's Guide - NBC 2015, Structural Commentaries NRCC no. xxxxxx (in preparation)
Commentary J: Design for Seismic Effects

Geological Survey of Canada Open File 7893 Fifth Generation Seismic Hazard Model for Canada: Grid values of mean hazard to be used with the 2015 National Building Code of Canada

See the websites www.EarthquakesCanada.ca and www.nationalcodes.ca for more information

Aussi disponible en français



Natural Resources
Canada

Ressources naturelles
Canada

Canada



Appendix G.

List of Special Provisions and OPSS Documents Referenced in this Report



1. The following Special Provisions and OPSS Documents are referenced in this report:

OPSS.PROV 206	Construction Specification for Grading
OPSS.PROV 209	Construction Specification for Embankments over Swamps and Compressible Soils
OPSS.PROV 401	Construction Specification for Trenching, Backfilling, and Compacting
OPSS 421	Construction Specification for Pipe Culvert Installation in Open Cut
OPSS 422	Construction Specification for Precast Reinforced Concrete Box Culverts in Open Cuts
OPSS.PROV 501	Construction Specification for Compacting
OPSS.PROV 517	Construction Specification for Dewatering
OPSS.PROV 539	Construction Specification for Temporary Protection Systems
OPSS.PROV 804	Construction Specification for Seed and Cover
OPSS 902	Construction Specification for Excavating and Backfilling Structures
OPSS.PROV 1010	Material Specification for Aggregates Base, Subbase, Select Subgrade, and Backfill Material
OPSS.PROV 1205	Material Specification for Clay Seal
OPSS 1860	Material Specification for Geotextile
OPSD 208.010	Benching of Earth Slopes
OPSD 3090.101	Foundation Frost Penetration Depths for Southern Ontario
OPSD 802.010	Flexible Pipe Embedment and Backfill Earth Excavation
OPSD 803.010	Backfill and Cover for Concrete Culverts with Span Less than or Equal to 3.0 m
OPSD 810.010	General Rip-Rap Layout for Sewer and Culvert Outlets
SP 517F01	Design Storm Return Period and Preconstruction Survey
NSSP FOUN0003	Dewatering Structure Excavations

2. Suggested text for a NSSP on “Obstructions”

Obstructions such as rock fill, cobbles and boulders may be encountered in the embankment and native till during excavation, installation of roadway protection systems



and/or sheet pile coffer dams. The Contractor shall design the temporary works accordingly and/or be prepared to remove, drill through and/or penetrate these obstructions and extend the work to the design depths.