



# **MERLEX ENGINEERING LTD.**

CONSULTING GEOTECHNICAL ENGINEERS

**FINAL  
FOUNDATION INVESTIGATION AND  
DESIGN REPORT  
SITE A**

**CULVERT STATION 22+335 – TWP. OF FRANKLIN  
GWP 5553-04-00**

**Highway 60, From 0.3 km West of Highway 35  
Easterly 9.4 km To 0.6 km West of  
the Oxtongue Lake Narrows Bridge**

MEL Ref. No.: 08/07/08085A

July 20, 2009

Submitted to:

AECOM Canada Ltd.  
189 Wyld Street  
North Bay, Ontario  
P1B 1Z2

**Geocres No.: 31E-288**



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## **1.0 INTRODUCTION**

Merlex Engineering Ltd. (MEL) has been retained by AECOM Canada Ltd., on behalf of the Ministry of Transportation of Ontario (MTO), to carry out a foundation investigation at a culvert located at Station 22+335, Township of Franklin. The GWP 5553-04-00 on Highway 60 runs from 0.3 km west of Highway 35 easterly 9.4 km to 0.6 km west of the Oxtongue Lake Narrows Bridge (see Figure No. 1, Key Plan, in Appendix A). This project involves the replacement of a single 1.0 m diameter CSP culvert in a 5 m high embankment.

The foundation investigation location was specified by the MTO in the RFP/TPM documentation Agreement No. 5006-E-0037. The terms of reference for the scope of work are outlined in MEL's proposal P-05-029, dated April 2, 2008. The purpose of the investigation was to determine the subsurface conditions in the area of the culvert and along a possible detour route to the north of the embankment. MEL investigated the foundation area by the drilling of boreholes, carrying out in-situ tests, and performing laboratory testing on select samples.

## **2.0 SITE DESCRIPTION**

The CSP culvert is located on Highway 60, approximately 4.1 km east of Highway 35. The topography at the site is generally of moderate relief and the direction of flow in the culvert is from north to south. The existing highway embankment supports three undivided lanes of highway, running in an east west direction (ie. two main lanes with an east bound passing lane). The existing road embankment is some 5.0 m higher than the grade level to the north and south sides of the road. The culvert discharges into a low lying area along the south toe of embankment (see Photo No. 1, Appendix C).



## **2.1 Site Physiography and Surficial Geology**

This Highway 60 project falls within the limits of the geomorphic sub-province known as the Algonquin Uplands. The topography at the site is generally rolling. There is exposed bedrock ridges present at many locations throughout the project and specifically within the area of the culvert under investigation. At other locations, significant layers of earth overlay the bedrock. Within the project area overburden conditions consist primarily of earth containing varying amounts of silt and sand. Organic terrain is also present.

Bedrock in the area is highly metamorphosed rocks of the Grenville Province of the Precambrian Shield. The high degree of metamorphism has changed the initial rock (siltstone, greywacke, arkoses, calcareous sandstone) mass fabric and structure resulting in a blocky, medium grained and very strong rock mass (OGS Map 2441).

## **3.0 INVESTIGATION PROCEDURES**

The field work for this investigation was carried out during the period of October 29 to 31, 2008 and February 25, 2009 and consisted of a total of eight (8) sampled boreholes.

The field investigation was carried out using a Bombardier mounted CME 45B drilling rig equipped with hollow stem augers, standard augers, and routine geotechnical sampling equipment. Soil samples were obtained at regular intervals of depth using the standard 50 mm O.D. split spoon sampler advanced in accordance with the Standard Penetration Test (SPT) procedures at the borehole locations. The investigation, at Borehole No. A2, was postponed until February 25, 2009 when sufficient ice had developed over the low lying wet area to the south of the embankment to allow access to the borehole location.



Groundwater conditions in the open boreholes were observed during and immediately following completion of the individual boreholes. All open boreholes were backfilled upon completion with compacted auger cuttings, in the general order they were removed and, where necessary, additional granular backfill was added to the boreholes to bring them up to grade. At the borehole through the embankment, the upper portion of the hole was backfilled with a cold patch to seal the existing asphalt surface.

The field work for this investigation was under the full time direction of a senior member of our engineering staff, who was responsible for locating the boreholes, clearing the borehole locations of underground services, in-situ sampling and testing operations, logging of the boreholes, labeling and preparation of samples for transport to our North Bay laboratory, plus overall drill supervision. All samples received a visual confirmatory inspection in our laboratory. Laboratory testing of select samples included routine testing for natural moisture content determination and particle size analysis. The results of the laboratory testing are presented on the individual Record of Borehole Sheets (Appendix B), with a summary of results presented on the laboratory sheets in Appendix C (Figures L-1 to L-3).

The location of the individual boreholes were determined in the field using highway chainage (established by others) and offset relative to highway centerline. Elevations contained in this report are referenced to a geodetic datum.

#### **4.0 SUBSURFACE CONDITIONS**

Details of the subsurface conditions revealed by the investigation program are presented on the enclosed Record of Borehole Logs (Appendix B) and on Figure No. A-1 (Appendix C). Please note that stratigraphic delineation presented on the borehole logs and soil strata plot are the



results of non-continuous sampling, response to drilling progress, the results of SPT and Dynamic Cone Penetration Test (DCPT) plus field observations. Typically such boundaries represent transitions from one zone to another and are not an exact demarcation of specific geological unit. Additional consideration should be given to the fact that subsurface conditions may vary markedly between adjacent boreholes and beyond any specific boring location, and are shown on the drawings for design purposes only.

#### **4.1 Culvert, Station 22+335, Township of Franklin - SITE A**

A plan and profile showing the borehole locations and stratigraphic sequences is shown on Figure No. A-1, Appendix C. During the course of the exploration program, eight (8) sampled boreholes were put down at this site, with Borehole No. A1 advanced from the surface of the existing highway embankment. Borehole Nos. A2 and A3 were advanced at the south and north ends of the existing culvert respectively. Borehole Nos. A4, A5, A6, A7, and A8, along with Borehole Nos. B4 and B5 (from Site B investigation covered under separate report) were advanced north of the existing embankment for a possible detour.

At the location of Borehole No. A1, embankment fill consisting of sands with some gravel and some fines (predominately silt) was encountered to a depth of 1.5 m. Below this depth the concentration of cobble/boulder/rock fill sizes increased with refusal on the DCPT met at a depth of 2.3 m and auger refusal encountered at the borehole at a depth of 3.2 m. A second boring was advanced, at a location 5 m east of the original boring, and auger refusal was met at a depth of 2.8 m. Based on drill response and site topography and geology it is considered that this refusal is due to the presence of rock fill/boulders in the embankment fill, generally below a depth of  $\pm 1.5$  m. Typical gradation curves of the portion of embankment fill which was retained in the spilt spoon sampler (37 mm inside diameter) are found on Figures L-1 and indicate 8 to



23% gravel size particles, 70 to 86% sand size particles and 6 to 13% silt and clay size particles. Based on the SPT values, which ranged from 45 to greater than 100 blows per 300 mm penetration, the compactness of the embankment fill was described as dense to very dense. The SPT values are probably on the high side due to the influence of cobbles and boulder sizes in the embankment fill, below a 1.5 m depth.

Borehole No. A2 was advanced at the outlet to the existing 1.0 m CSP culvert at the south toe of embankment. The culvert discharges into a wet land area (see Photo 1, Appendix C). At the location of Borehole No. A2 a dark brown fine sand with some silt, trace of gravel and fine roots was penetrated to a depth of 1.5 m below grade. The lower portion of this fine sand had a higher content of silt and organics. A typical gradation curve is shown on Figure L-2 and indicates 0% gravel size particles, 59% sand size particles, 37% silt size particles, and 4% clay size particles. Based on the SPT values, which was 0 blows (weight of hammer) per 300 mm penetration, the compactness of the deposit was described as very loose. Below a depth of 1.5 m (elevation 363.4 m) a coarser sand deposit was penetrated and has been described as fine and medium sand with a trace gravel and silt. A typical gradation curve is shown on Figure L-3 and indicates 4% gravel size particles, 86% sand size particles, and 12% silt and clay size particles. Based on the SPT values, which returned a value of greater than 100 blows per 300 mm penetration and the DCPT data, the compactness of the deposit was described as dense to very dense. At a depth of 2.3 m (elevation 362.3 m), refusal to further advance of the hollow stem augers and split spoon sampler was met at Borehole No. A2. Two additional borings were advanced, at distances of 3 m east and 4 m west of the original borehole, and auger refusal was met at a depth of 2.2 m at both locations. Based on drill response and bedrock topography in the area it is our opinion that refusal is likely due to bedrock.



Borehole No. A3 was advanced at the inlet to the culvert along with Borehole Nos. A4, A5, A6, A7, and A8 which were advanced along the north side of the embankment, for a possible detour, at generally an offset of 14 m from centerline. These boreholes indicated a very thin layer of overburden consisting of generally fine to medium sands with a trace of gravel and silt. A typical gradation curve is shown on Figure L-4 and indicates 17% gravel size particles, 76% sand size particles, and 7% silt size particles. This overburden layer varied between 1.5 and 0.8 m in thickness at Borehole Nos. A3, A4, and A6, and was shallower at the other boreholes with bedrock exposed at grade at Borehole Nos. A5, A7, and A8 (see Photos 3, 4, 5, 6, and 7, Appendix C). Due to the lack of depth in this deposit only one SPT value was recorded in Borehole No. A6. This SPT returned a value of 26 blows per 300 mm penetration indicating a compactness of compact.

#### **4.2 Groundwater Conditions**

Groundwater and cave-in levels in the open boreholes were taken during the advance of the individual borings and upon completion. These levels were recorded on the individual Record of Borehole Log Sheets (Appendix B). The area to the south of Borehole No. A2 is a wet land and the water level in this borehole was elevated and recorded at a depth of 0.25 m below grade, at the borehole location. The boreholes to the north of the embankment, which penetrated through minimal overburden were dry except for Borehole No. A6, where a water level depth of 1.1 m was recorded upon completion. These groundwater levels will fluctuate seasonally.

#### **MERLEX ENGINEERING LTD.**

M. A. Merleau, P. Eng.  
Principal

J. R. Berghamer, P. Eng.  
Project Engineer





## **5.0 DESIGN COMMENTS AND RECOMMENDATIONS**

### **5.1 General**

The existing culvert, which is a 1000 mm diameter CSP, located at Station 22+335 in the Township of Franklin, was identified as requiring replacement in the RFP. The embankment at this location is some 5 m in height. Based on data from this foundation investigation and the geotechnical investigation, the embankment has been constructed of gravelly sands with occasional to frequent boulder size rock and rock fill fragments generally encountered below a depth of  $\pm 1.5$  m. The flow in the culvert is from north to south, where it discharges into a low lying swampy area with high water table.

It is understood that the existing CSP culvert is being considered for replacement, with either a flexible or rigid pipe of similar section, or possibly relined. The existing culvert has not deformed or distorted along its length and, as such, we understand that the decision to reline the culvert was made following our completion of this foundation investigation. The alignment will be similar to the existing. If the culvert was to be replaced, we understand the preferred method is to detour the traffic to the north of the existing alignment. However, considering the 3 lane width and limited embankment height, a single lane with traffic control on a 24 hour basis would probably be adequate and more cost effective. A grade raise is not required.

### **5.2 Foundation Consideration**

The existing embankment is constructed of sands with gravel and with occasional to frequent cobble and boulder size and/or rock fill inclusions present below a depth of some 1.5 m. At the culvert location a visual review indicated no signs of embankment slope instability and there were no signs of settlement of the pavement structure at the culvert location. The founding native soils, at the culvert ends/toe of slope, comprise of a thin fluvial deposit of loose fine sands



with some silt and organic pockets/seams over a medium and fine sand deposit generally in a dense state of compactness, underlain by bedrock at a relatively shallow depth of some 1 to 2 m below original grade. These founding materials are considered good for a conventional highway embankment of this height, and bearing resistance and/or embankment stability will not be an issue provided groundwater is controlled during construction, as discussed in the following.

### **5.3 Founding Soils and Subgrade Preparation**

The results of this investigation indicate that the native subgrade soils, below elevation  $\pm 363$  m at Borehole No. A2 (outlet), consists of sands generally in a dense state of compactness. However, they are overlain by a thin deposit of fluvial fine sands and silts with trace of organics, which were in a very loose state of compactness, at the culvert ends. This deposit has been preloaded, under the centerline of the culvert, with a 5 m height of embankment fill. This thin layer was measured at 1.2 m and 0.6 m in thickness at the outlet (Borehole No. A2) and inlet (Borehole No. A3), respectively. A review of the condition of the pavement surface, at the culvert location, did not reveal any transverse cracking, past patching, or settlement which indicates the embankment fill has performed well in this area .

The embankment fill is composed of gravelly sands with occasional to frequent boulder size rock (boulders and/or rock fill), below a depth of  $\pm 1.5$  m and the groundwater level was measured at or slightly above (+300 mm) the culvert outlet, at the time of this investigation. Groundwater control, in accordance with OPSS 517, will be required to maintain a stable subgrade during culvert installation. Local temporary sandbagging, at the inlet and outlet, combined with installation of filtered sumps and pumping from the base of the excavation will, at a minimum, be required to maintain the excavation in a dewatered condition during subgrade



preparation. Ultimately, the method of dewatering will be the choice of the contractor; however the importance of maintaining the subgrade in an unwatered stable condition during excavation and foundation construction cannot be stressed enough.

Embedment material, for flexible pipe, can consist of Granular B Type I placed in accordance with OPSD 802.010. If a rigid pipe is used provide Class B bedding as per OPSD 802.031 with a bedding depth of 150 mm using Granular A. Alternately, a bedding of 19 mm clear stone can be used to aid in groundwater control. Cover material can consist of a Granular B Type I placed as per OPSD 802.031. Since the discussion on the type of pipe to be used will be made at the construction stage, not at detailed design, specifying Granular A material for cover and embedment material would be prudent considering the minimal quantities involved. The existing embankment material can be used as trench backfill provided a Class II non-woven geotextile is placed as a separator at the underside of granular sub-base similar to the treatment shown on the attached Figure SK-2, Appendix D.

Based on the above noted soil conditions, the presence of the existing embankment and a founding elevation similar to that of the existing culvert, we have determined a factored bearing resistance value at ULS of 450 kPa. A SLS bearing resistance of 145 kPa reflects settlement considerations of the preloaded zone of soil below the existing culvert, a settlement estimate of 25 mm, and the assumption that a stable subgrade is maintained with proper groundwater control during excavation and construction.

#### **5.4 Excavation and Embankment Reconstruction**

All excavations greater than 1.2 m in depth must be sloped or shored in accordance with the Occupational Health and Safety Act Regulations for Construction Projects. Excavation for



installation of a replacement culvert will penetrate the existing embankment fills, which consist of sands with some gravels and some fines (predominately silt) to a depth of approximately 1.5 m, after which the concentration boulder/rockfill sizes increased. The embankment fill overlies native fine sands some silt with bedrock at a shallow depth. Temporary open excavations will be stable above the groundwater table at an angle of 1H:1V. Below the prevailing groundwater table, the slopes of open excavations will have to be flattened to 2H:1V or possibly shallower depending upon the method of dewatering employed. As previously discussed, excavations must be maintained in an unwatered condition during excavation and foundation construction and every reasonable effort must be made to prevent disturbing the founding subgrade. The cohesionless soils, below the groundwater table, are classified as a Type 4 soil if the groundwater is not adequately controlled. For this condition, side slopes should be cut at a 3H:1V.

## **5.5 Embankment Stability**

The embankment platform is three lanes in width (including the east bound passing lane) and the existing fore slopes (at the culvert location) are at 2.5H:1V and 1.9H:1V, right and left respectively. If temporary embankment widening is required at this location for staged construction it will be necessary to strip the existing fill slopes in accordance to the appropriate OPSS and OPSD. An average stripping depth of 150 mm can be used and from a design point of view no topsoil will be available from the stripped material. Widening to the north will be restricted by the presence of bedrock outcrops at the ditch back slope, unless rock removal will be considered for temporary widening. Temporary widening for staged construction will likely spill over into the existing ditch and as such, temporary drainage may have to be considered depending upon the duration of the staged construction. The embankment is constructed



predominately with granulars, as such the new side slopes should be reconstructed to the standard of 2H:1V.

To confirm the future stability of the proposed cross-sections, a slope stability analysis was carried out on the embankment cross-sections at Station 22+335 on the left (north side) with the widening constructed of granular fill, using the commercially available program SLOPE/W Version 4.23 produced by Geo-slope International, which uses Bishop's Modified Method. The factor of safety is defined as the ratio of the forces tending to resist failure to the driving forces tending to cause failure. Surficial stability of the granular fill slope, as expected, resulted in lower factors of safety, however, granular fill will be stable if constructed at the slopes proposed. The factor of safety for a surficial failure arc through the underlying native soils was calculated as 2.1, which is well above the design minimum factor of safety of 1.3 (see Figure S-1, Appendix D).



## **6.0 CLOSURE**

Information provided in this report is valid only at the locations described above. Any assumptions of continuity of soil stratigraphy between boreholes, as shown on the enclosed cross-sections, is intended as an aid for design purposes only and does not constitute a statement of existing conditions for contractual or construction purposes. Field investigation was carried out using a CME drill rig mounted on a Bombardier carrier owned by a sister company of Merlex Engineering Ltd. The report was prepared by Mr. J.R. Berghamer, P. Eng and reviewed by the firm's principal and MTO designate Mr. M. A. Merleau, P. Eng.

Details of the investigation, the material analysis and recommendation in this report are considered to be complete. However, should any questions arise, please do not hesitate to contact the undersigned.

### **MERLEX ENGINEERING LTD.**

M. A. Merleau, P. Eng.  
Principal

J. R. Berghamer, P. Eng.

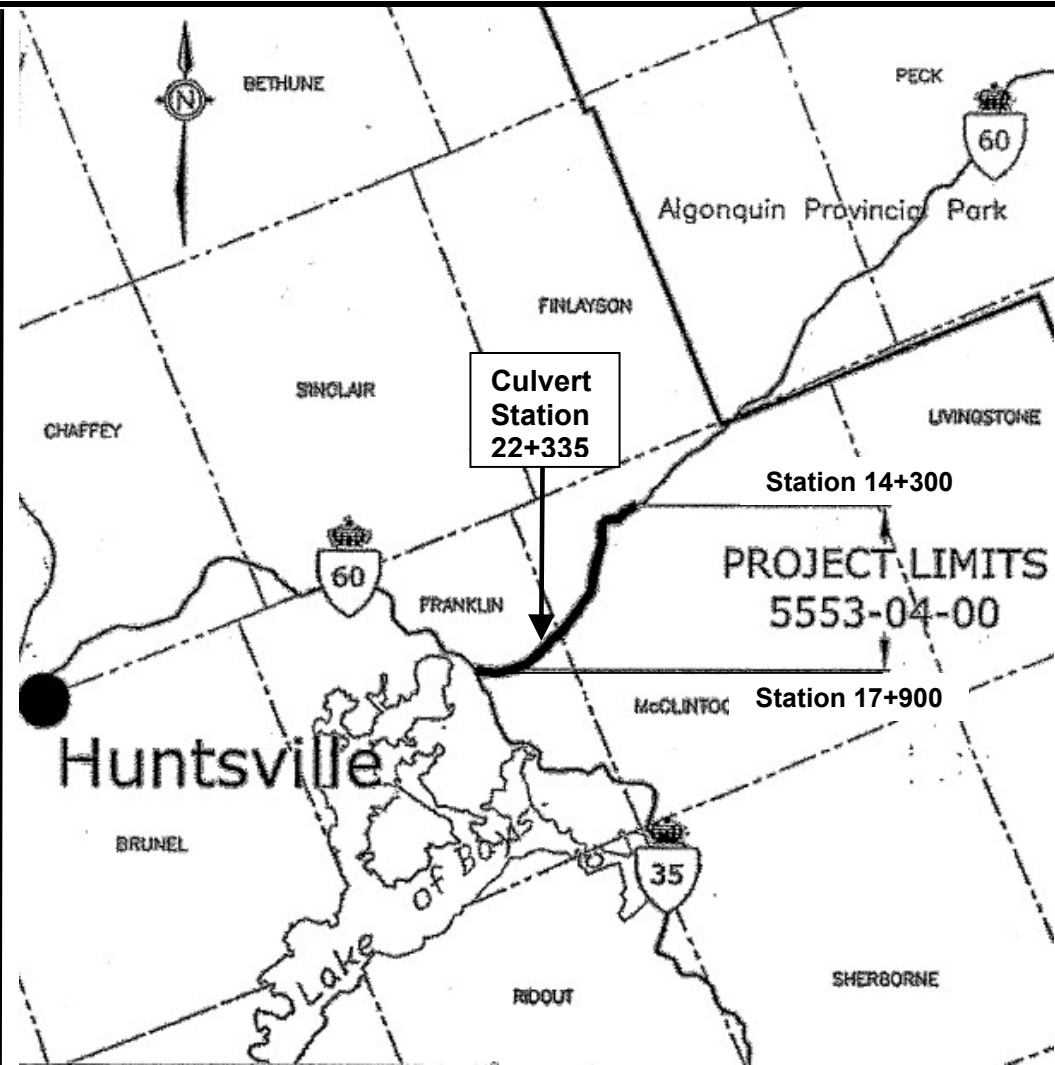
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## **APPENDIX A**

Figure No. 1      Key Plan

# KEY PLAN

NOT TO SCALE



**FINAL  
FOUNDATION INVESTIGATION AND  
DESIGN REPORT  
SITE A – CULVERT STATION 22+335  
GWP 5553-04-00**

Highway 60, From Highway 35  
Easterly 9.1 km To 0.6 km West Of  
The Oxtongue Lake Narrows Bridge

MEL Ref. No.: 08/07/08085A

July 2009



**MERLEX ENGINEERING LTD.**

CONSULTING GEOTECHNICAL ENGINEERS



## **APPENDIX B**

Enclosure No. 1	List of Abbreviations and Symbols
Enclosure Nos. 2 to 9	Record of Borehole Sheets



## LIST OF ABBREVIATIONS & DESCRIPTION OF TERMS

The abbreviations and terms, used to describe retrieved samples and commonly employed on the borehole logs, on the figures and in the report are as follows:

### 1. ABBREVIATIONS

AS	Auger Sample
CS	Chunk Sample
DS	Denison type sample
FS	Foil Sample
HB	Hammer Bouncing
NFP	No Further Progress
PH	Sampler advanced by hydraulic pressure
PM	Sampler advanced by manual pressure
RC	Rock core with size & percentage of recovery
SS	Split Spoon
ST	Slotted Tube
TO	Thin-walled, open
TP	Thin-walled, piston
WH	Sampler Advanced by static weight (weight of hammer and/or rods)
WS	Wash Sample

### 2. PENETRATION RESISTANCE/"N"

*Dynamic Cone Penetration Test (DCPT):*

A continuous profile showing the number of blows for each 300 mm of penetration of a 50 mm diameter 90° point cone driven by a 63 kg hammer falling 760 mm.

Plotted as

*Standard Penetration Test (SPT) or "N" Values*

The number of blows of a 63 kg hammer falling 760 mm required to advance a 50 mm O.D. drive open sampler 300 mm.

### 3. SOIL DESCRIPTION

a) *Cohesionless Soils:*

"N" (blows/0.3 m)	Relative Density
0 to 4	very loose
4 to 10	loose
10 to 30	compact
30 to 50	dense
over 50	very dense

### 3. SOIL DESCRIPTION (Cont'd)

b) *Cohesive Soils:*

Undrained Shear Strength (kPa)	Consistency
Less than 12	very soft
12 to 25	soft
25 to 50	firm
50 to 100	stiff
100 to 200	very stiff
over 200	hard

c) *Method of Determination of Undrained Shear Strength of Cohesive Soils:*

+ 3.2 - Field Vane test in borehole.  
The number denotes the sensitivity to remoulding.

D - Laboratory Vane Test

.. - Compression test in laboratory

For a saturated cohesive soil the undrained shear strength is taken as one-half of the undrained compressive strength.

### 4. TERMINOLOGY

Terminology used for describing soil strata is based on the proportion of individual particle sizes present in the samples (please note that, with the exception of those samples subject to a grain-size analysis, all samples were classified visually and the accuracy of visual examination is not sufficient to determine exact grain sizing):

Trace, or occasional	Less than 10%
Some	10 to 20%
With	20 to 30%
Adjective (i.e. silty or sandy)	30 to 40%
And (i.e. sand and gravel)	40 to 60%

### 5. LABORATORY TESTS

P	Standard Proctor Test
A	Atterberg Limit Test
GS	Grain Size Analysis
H	Hydrometer Analysis
C	Consolidation



**SAMPLE DESCRIPTION NOTES:**

1. **FILL:** The term fill is used to designate all man-made deposits of natural soil and/or waste materials. The reader is cautioned that fill materials can be very heterogeneous in nature and variable in depth, density and degree of compaction. Fill materials can be expected to contain organics, waste materials, construction materials, shot rock, rip-rap, and/or larger obstructions such as boulders, concrete foundations, slabs, abandoned tanks, etc.; none of which may have been encountered in the borehole. The description of the material penetrated in the borehole therefore may not be applicable as a general description of the fill material on the site as boreholes cannot accurately define the nature of fill material. During the boring and sampling process, retrieved samples may have certain characteristics that identify them as 'fill'. Fill materials (or possible fill materials) will be designated on the Borehole Logs. If fill material is identified on the site, it is highly recommended that testpits be put down to delineate the nature of the fill material. However, even through the use of testpits defining the true nature and composition of the fill material cannot be guaranteed. Fill deposits often contain pockets or seams of organics, organically contaminated soils or other deleterious material that can cause settlement or result in the production of methane gas. It should be noted that the origins and history of fill material is frequently very vague or non-existent. Often fill material may be contaminated beyond environmental guidelines and the material will have to be disposed of at a designated site (i.e. registered landfill). Unless requested or stated otherwise in this report, fill material on this site has not been tested for contaminants however, environmental testing of the fill material can be carried out at your request. Detection of underground storage tanks cannot be determined with conventional geotechnical procedures.
2. **TILL:** The term till indicates a material that is an unstratified, glacial deposit, heterogeneous in nature and, as such, may consist of mixtures and pockets of clay, silt, sand, gravel, cobbles and/or boulders. These heterogeneous deposits originate from a geological process associated with glaciation. It must be noted that due to the highly heterogeneous nature of till deposits, the description of the deposit on the borehole log may only be applicable to a very limited area and therefore, caution must be exercised when dealing with a till deposit. When excavating in till, contractors may encounter cobbles/boulders or possibly bedrock even if they are not indicated on the borehole logs. It must be appreciated that conventional geotechnical sampling equipment does not identify the nature or size of any obstruction.
3. **BEDROCK:** Auger refusal may be due to the presence of bedrock, but possibly could also be due to the presence of very dense underlying deposits, boulders or other large obstructions. Auger refusal is defined as the point at which an auger can no longer be practically advanced. It must be appreciated that conventional geotechnical sampling equipment does not differentiate between nature and size of obstructions that prevent further penetration of the boring below grade. Bedrock indicated on the borehole logs will be labeled 'possibly' or 'probable' etc. based on the response of the boring and sampling equipment, surrounding topography, etc. Bedrock can be proven at individual borehole locations, at your request, by diamond core drilling operations or, possibly, by testpits. It must also be appreciated that bedrock surfaces can be, and most times are, very erratic in nature (i.e. sheer drops, isolated rock knobs, etc.) and caution must be used when interpreting subsurface conditions between boreholes. A bedrock profile can be more accurately estimated, at the clients' request, through a series of closely positioned unsampled auger probes combined with core drilling.
4. **GROUNDWATER:** Although the groundwater table may have been encountered during this investigation and the elevation noted in the report and/or on the record of boreholes, it must be appreciated that the elevation of the groundwater table will fluctuate based upon seasonal conditions, localized changes, erratic changes in the underlying soil profile between boreholes, underlying soil layers with highly variable permeabilities, etc. These conditions may affect the design and type and nature of dewatering procedures. Cave-in levels recorded in borings give a general indication of the groundwater level in cohesionless soils however, it must be noted that cave-in levels may also be due to the relative density of the deposit, drilling operations etc.

## METRIC

## RECORD OF BOREHOLE NO. A1



REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE CME 45B - Hollow Stem Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 29, 2008 TIME 10:30:00 AM CHECKED BY MAM  
 DATE (Completed) October 29, 2008

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w <sub>p</sub>	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" VALUES								
370.4	Granular Shoulder												
0.0	EMBANKMENT FILL												
	Brown Medium Fine Sand trace Gravel Trace Silt		1	AS									8 86 (6)
	(Dense, Very Dense)		2	SS	43								17 70 (13)
368.9													
1.5	Increase in Cobble/Boulder content below 1.5 m depth		3	SS	50/75mm								
			4	SS	80								23 70 (7)
367.2			5	SS	50/75mm								
3.2	Auger Refusal, Probably Boulders End of Borehole												
	Auger Probe 5 m East Auger Refusal @ 2.8 m												
COMMENTS								+ <sup>3</sup> , X <sup>3</sup> : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 120 kPa ○ 3% STRAIN AT FAILURE					
								WATER LEVEL RECORDS Date (yy/mm/dd)/Time      Water Depth (m)      Cave In (m) 1) 10/29/08 10:30:00 AM      DRY      3 2)      -      - 3)      -      -					

The stratification lines represent approximate boundaries. The transition may be gradual.

MERLEX ENGINEERING LTD.

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MEL-GEO 08085 - BOREHOLE LOGS.GPJ MEL-GEO.GDT 7/2/09



## METRIC

## RECORD OF BOREHOLE NO. A3



REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE Standard Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 31, 2008 TIME 11:50:00 AM CHECKED BY MAM  
 DATE (Completed) October 31, 2008

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT  SHEAR STRENGTH kPa ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL × LAB VANE 20 40 60 80 100	PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" VALUES								
365.6	Ground Surface												
0.0	SAND												
	Brown Fine to Medium Sand Trace Silt Trace Gravel (Loose)		1	AS									
364.8			2	SS	25/0mm								
0.8	DCPT Refusal Auger Refusal, probably Bedrock End of Borehole												

COMMENTS		WATER LEVEL RECORDS			
+ <sup>3</sup> , × <sup>3</sup> : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 120 kPa ○ 3% STRAIN AT FAILURE The stratification lines represent approximate boundaries. The transition may be gradual.		Date (yy/mm/dd)Time	Water Depth (m)	Cave In (m)	
		1)	DRY	▽	-
		2)	-	▽	-
		3)	-	▽	-

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**METRIC****RECORD OF BOREHOLE NO. A4**

REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE Standard Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 31, 2008 TIME \_\_\_\_\_ CHECKED BY MAM  
 DATE (Completed) October 31, 2008

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT w <sub>p</sub>	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" VALUES								
366.4	Ground Surface												
0.0	150 mm Black Organics												
	SAND												
	Brown Medium Fine Sand Trace Silt Trace Gravel and Cobbles		1	AS									
365.6													
0.8	DCPT Refusal Auger Refusal, probably Bedrock End of Borehole												
	Auger Probe 1.0 m East Auger Refusal @ 0.8 m												

COMMENTS		+ <sup>3</sup> , X <sup>3</sup> : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 120 kPa		WATER LEVEL RECORDS	
Borehole in ditch, 6" of surface water		○ 3% STRAIN AT FAILURE		Date (yy/mm/dd)/Time	Water Depth (m)
				1)	DRY
				2)	-
				3)	-

The stratification lines represent approximate boundaries. The transition may be gradual.

MEL-GEO 08085 - BOREHOLE LOGS.GPJ MEL-GEO.GDT 7/2/09

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## METRIC

## RECORD OF BOREHOLE NO. A5



REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE Standard Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 31, 2008 TIME \_\_\_\_\_ CHECKED BY MAM  
 DATE (Completed) October 31, 2008

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE			"N" VALUES	20					
371.4	Ground Surface												
370.9	50 mm Grass and Organics												
0.1	Auger Refusal - Bedrock Exposed End of Borehole												
	Auger Probe 2.0 m West Auger Refusal @ 50 mm					371							
COMMENTS							+ <sup>3</sup> , X <sup>3</sup> : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 120 kPa ○ 3% STRAIN AT FAILURE						
							WATER LEVEL RECORDS Date (yy/mm/dd) Time Water Depth (m) Cave In (m) 1) DRY - - 2) - - 3) - -						

The stratification lines represent approximate boundaries. The transition may be gradual.

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MEL-GEO 08085 - BOREHOLE LOGS.GPJ MEL-GEO.GDT 7/2/09



## METRIC

## RECORD OF BOREHOLE NO. A6



REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE Standard Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 30, 2008 TIME 2:50:00 PM CHECKED BY MAM  
 DATE (Completed) October 30, 2008

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT	PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)												
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" VALUES																				
366.0 0.0	Ground Surface SAND Brown Medium Fine Sand Trace Silt Trace Gravel (Compact)		1	SS								17 76 (7)													
364.5 1.5	DCPT Refusal Auger Refusal, probably Bedrock End of Borehole  Auger Probe 2.0 m West Auger Refusal @ 1.5 m		2	SS	26																				
<p>COMMENTS</p> <p>+<sup>3</sup>, X<sup>3</sup>: Numbers on right refer to Sensitivity            Numbers on left refer to values greater than 120 kPa            ○ 3% STRAIN AT FAILURE</p> <p>WATER LEVEL RECORDS</p> <table border="1"> <thead> <tr> <th>Date (yy/mm/dd)/Time</th> <th>Water Depth (m)</th> <th>Cave In (m)</th> </tr> </thead> <tbody> <tr> <td>1) 10/30/08 3:10:00 PM</td> <td>1.1</td> <td>1.1</td> </tr> <tr> <td>2)</td> <td>-</td> <td>-</td> </tr> <tr> <td>3)</td> <td>-</td> <td>-</td> </tr> </tbody> </table>														Date (yy/mm/dd)/Time	Water Depth (m)	Cave In (m)	1) 10/30/08 3:10:00 PM	1.1	1.1	2)	-	-	3)	-	-
Date (yy/mm/dd)/Time	Water Depth (m)	Cave In (m)																							
1) 10/30/08 3:10:00 PM	1.1	1.1																							
2)	-	-																							
3)	-	-																							

The stratification lines represent approximate boundaries. The transition may be gradual.

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SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT			UNIT WEIGHT $\gamma$	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE			"N" VALUES	20 40 60 80 100	20 40 60 80 100	W <sub>p</sub> W W <sub>L</sub>	WATER CONTENT (%)		
372.7	Ground Surface												
0.0	Auger Refusal - Bedrock exposed at grade End of Borehole												
COMMENTS							+ <sup>3</sup> , × <sup>3</sup> : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 120 kPa ○ 3% STRAIN AT FAILURE		WATER LEVEL RECORDS				
									Date (yy/mm/dd)Time		Water Depth (m)		Cave In (m)
							1) 2) 3)		DRY - -		- - -		

**METRIC****RECORD OF BOREHOLE NO. A8**

REFERENCE 08/07/08085 DATUM Geodetic LOCATION Site A - Culvert Station 22+335, Franklin Twp. (See Plan) ORIGINATED BY JL  
 PROJECT GI & FDN - Highway 60, GWP 5553-04-00 BOREHOLE TYPE CME 45B - Hollow Stem Augers COMPILED BY MCM/RG  
 CLIENT AECOM Canada Ltd. DATE (Started) October 31, 2008 TIME \_\_\_\_\_ CHECKED BY MAM  
 DATE (Completed) October 31, 2008

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT  SHEAR STRENGTH kPa ○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL x LAB VANE 20 40 60 80 100	PLASTIC LIMIT W <sub>p</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" VALUES								
373.1	Ground Surface												
0.0	Auger Refusal - Bedrock exposed at grade End of Borehole												
COMMENTS  The stratification lines represent approximate boundaries. The transition may be gradual.								WATER LEVEL RECORDS Date (yy/mm/dd)/Time    Water Depth (m)    Cave In (m)					
								1) _____ 2) _____ 3) _____					

MEL-GEO 08085 - BOREHOLE LOGS.GPJ MEL-GEO.GDT 7/2/09

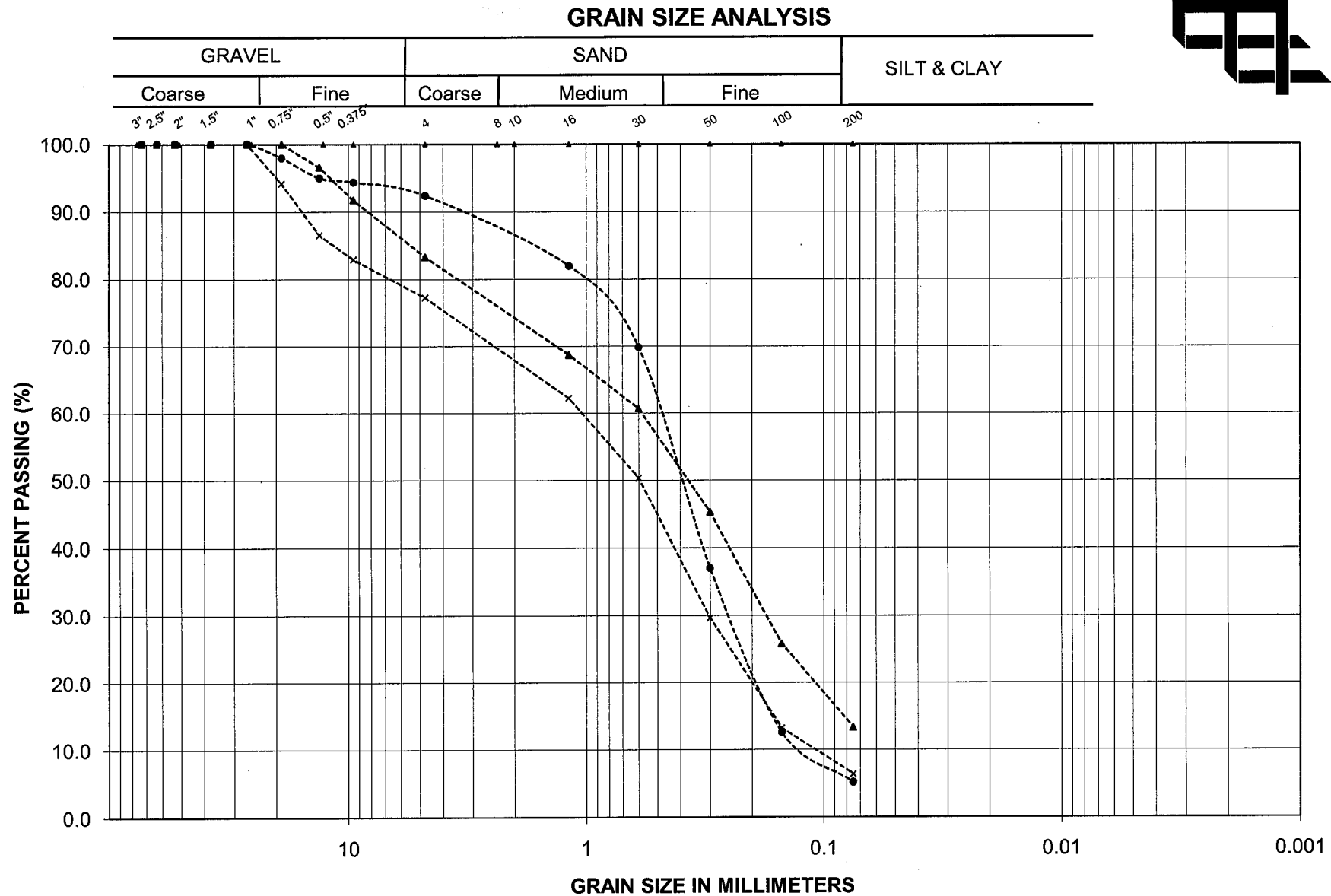
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## **APPENDIX C**

Figure A-1	Borehole Locations & Soil Strata
Figures L-1 to L-4	Summary Grain Size Analysis Graph
Enclosure No. 10	Photo Essay





-----●----- BH No.: A1 Sa. No.: 1    Depth: 0 - 2.5 m   
 -----▲----- BH No.: A1 Sa. No.: 2    Depth: 760 - 1.2 m   
 -----x----- BH No.: A1 Sa. No.: 4    Depth: 7.5 - 9 m

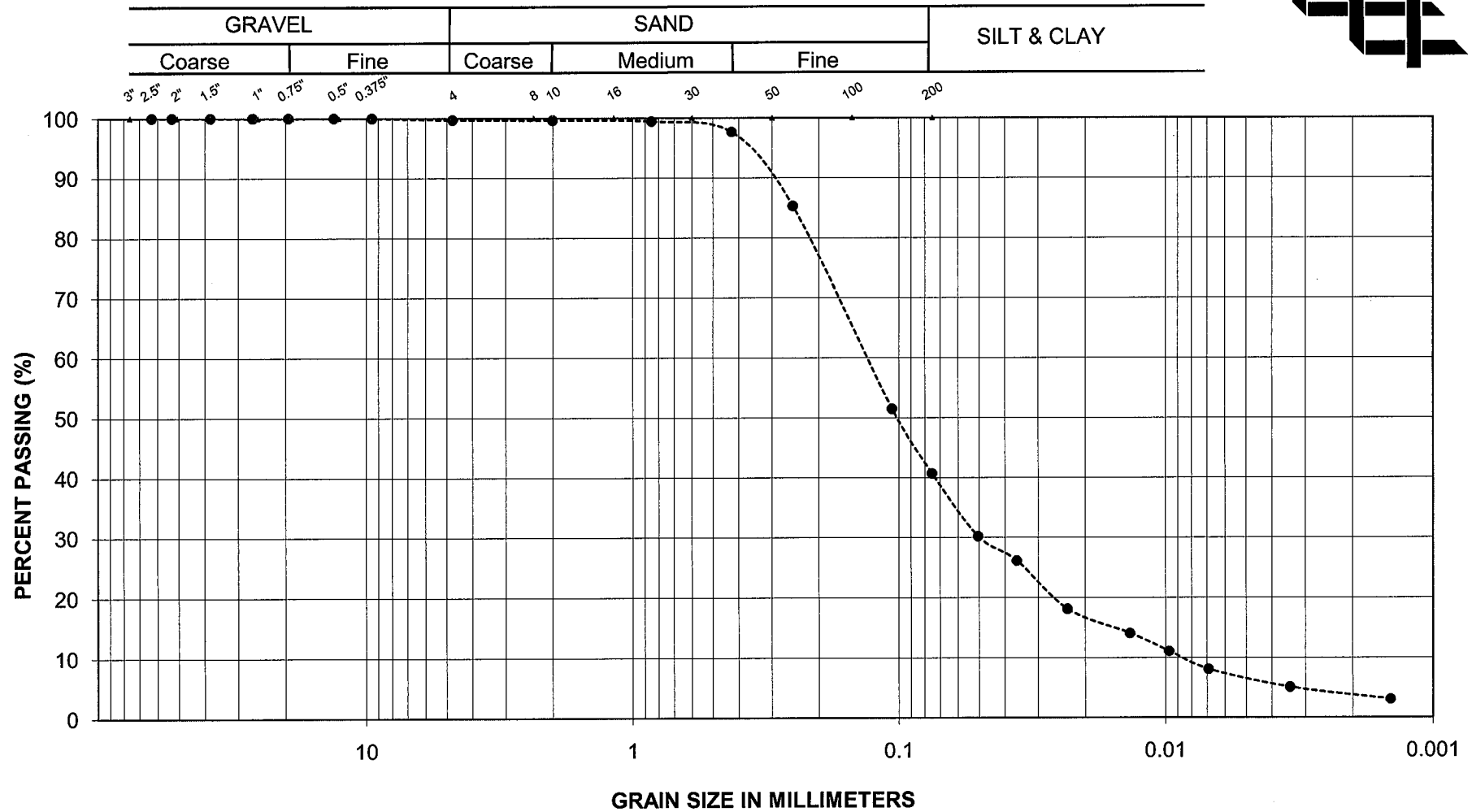
PROJECT: Hwy 60 - Culvert Station 22+335  
LOCATION: Franklin Twp

EMBANKMENT FILL  
SANDS, fine and medium sands, with gravel trace silt

Reference No.: 08085-A

Date: Mar 19, 2009

# GRAIN SIZE ANALYSIS



---●--- BH No.: A2 Sa No.: 2 Depth: 0.8 - 1.2 m

PROJECT: Hwy 60 - Culvert Station 22+335  
LOCATION: Franklin Twp

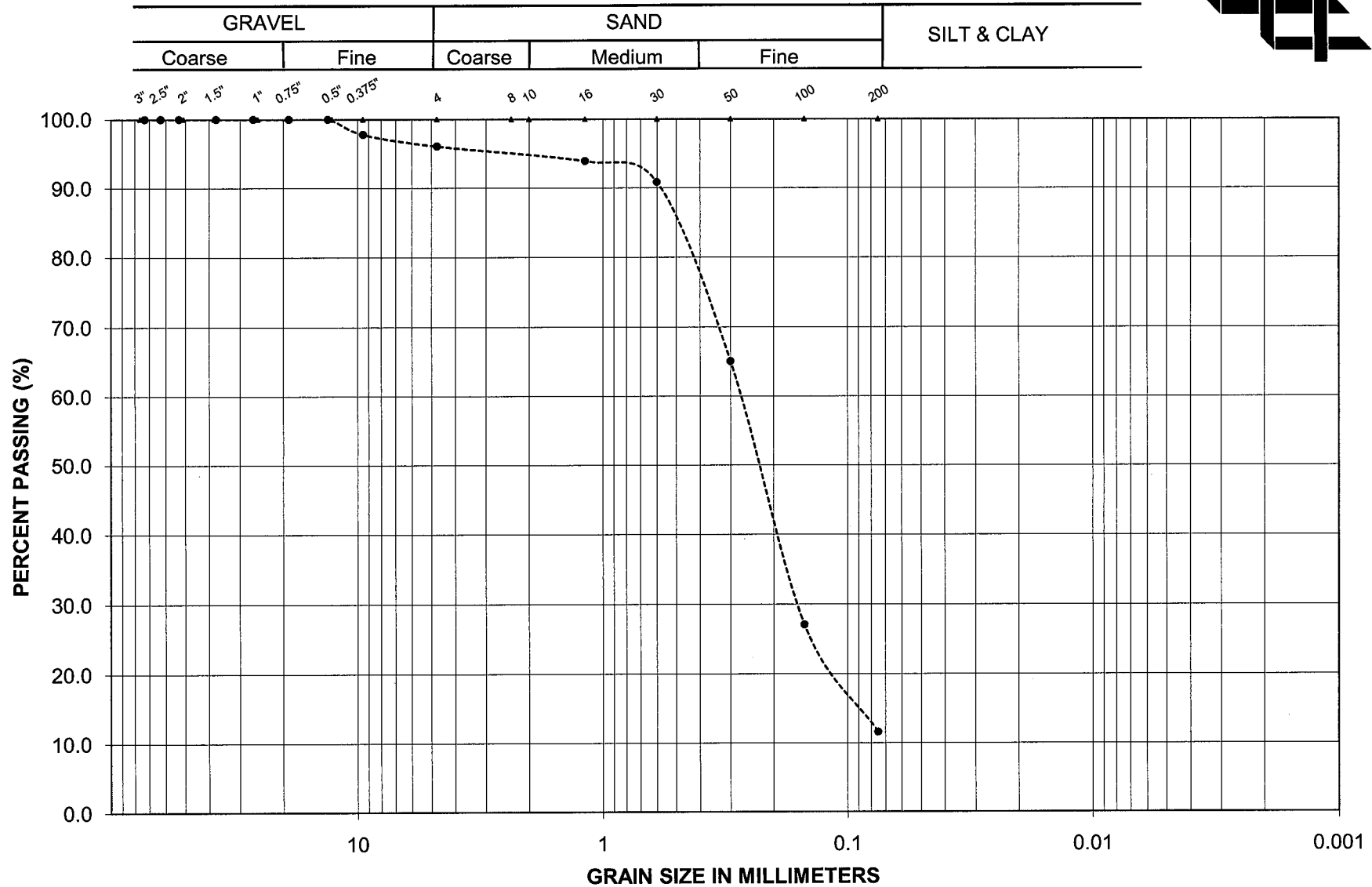
SANDS, with silt trace of clay  
MERLEX ENGINEERING LTD.

FIG L-2

Reference No.: 08085-A  
Date: Mar 19, 2009



## GRAIN SIZE ANALYSIS



---●--- BH No.: A2 Sa. No.: 3 Depth: 5 - 6.5 m

PROJECT: Hwy 60 - Culvert Station 22+335  
LOCATION: Franklin Twp

SANDS, fine and medium sands, trace of gravel and silt

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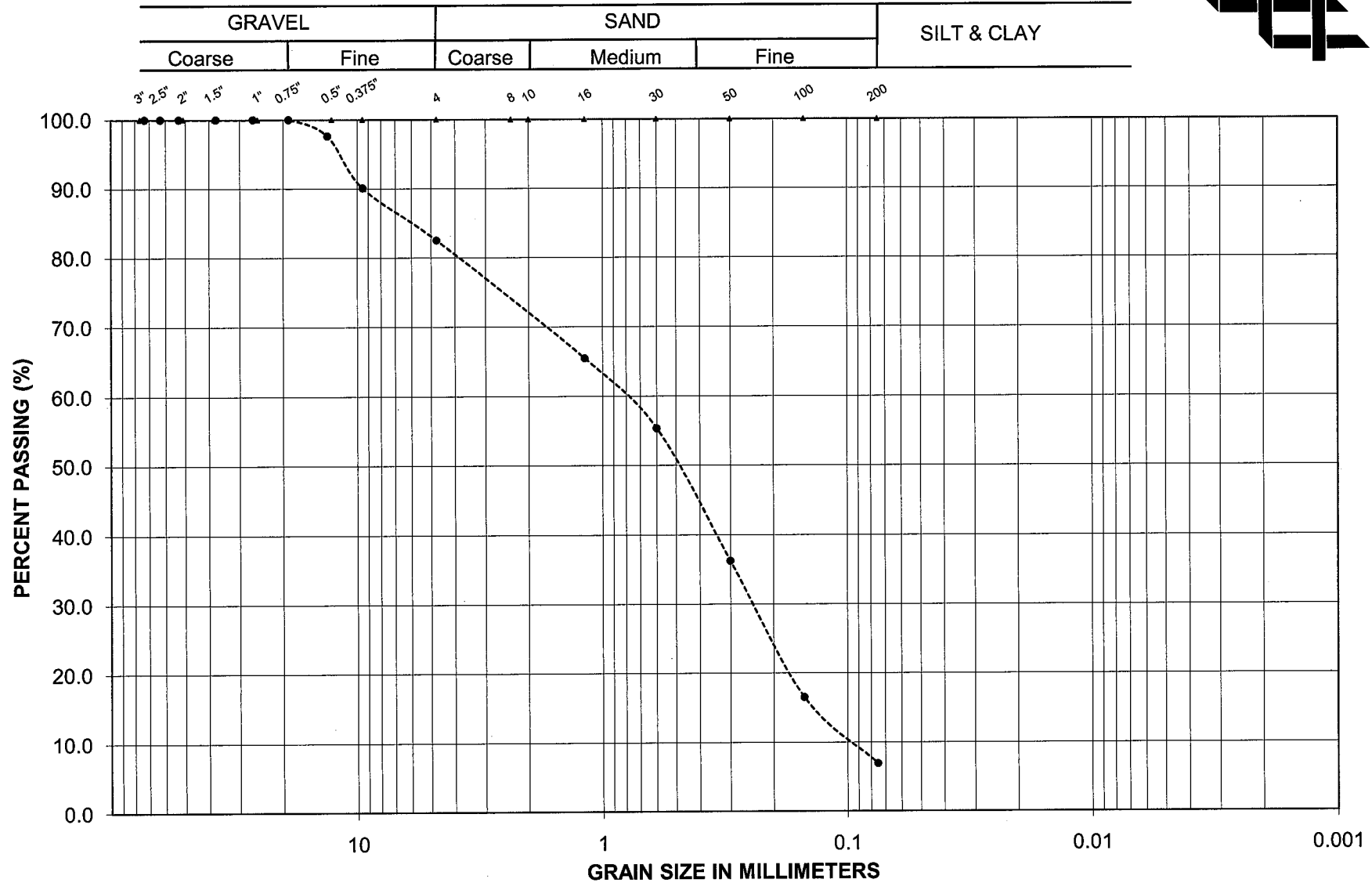
FIG L-3



Reference No.: 08085 -A  
Date: Mar 19, 2009



# GRAIN SIZE ANALYSIS



---●--- BH No.: A6 Sa. No.: 2 Depth: 2.5 - 4 m

PROJECT: Hwy 60 - Culvert Station 22+335  
LOCATION: Franklin Twp

SANDS, some gravel, trace of silt

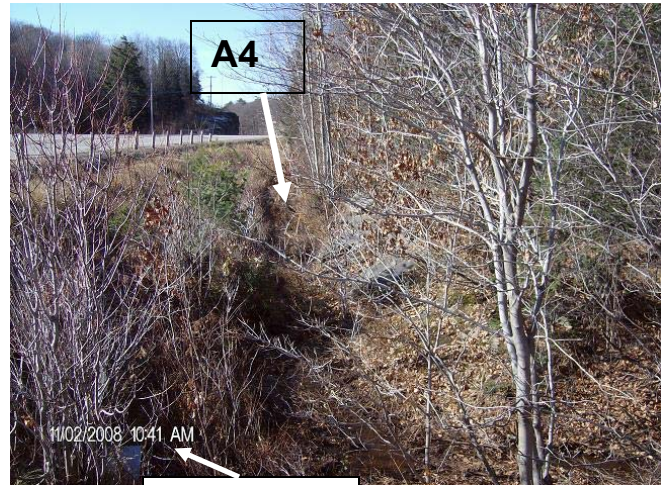
MERLEX ENGINEERING LTD.

FIG L-4



Left: Low level area south of culvert outlet, Station 22+335 Rt.  
Right: Looking west along proposed detour alignment from inlet to culvert at Station 22+335 (foreground to left) and at Borehole No. A4 (top of 1 x 2 stake, fluorescent, in ditch).

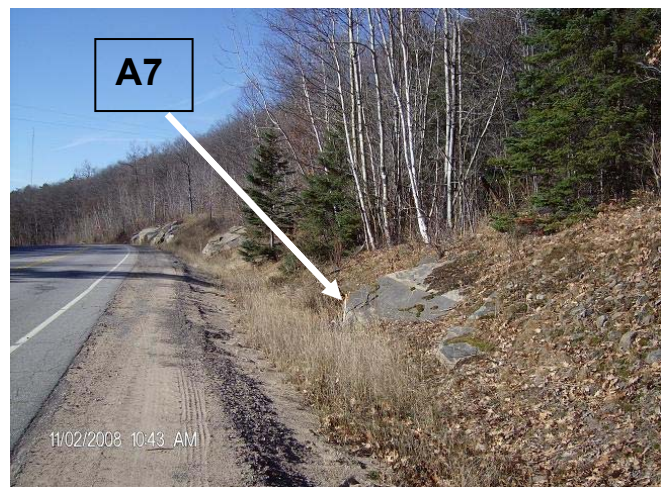
Photos: 1 - 2



Culvert Inlet

Left: Exposed bedrock at Borehole No. A8 (Station 22+165, 14 m Lt).  
Right: Exposed bedrock at Borehole No. A7 (Station 22+215, 14 m Lt).

Photos: 3 - 4



Reference No.: 08/07/08085A

Project: Foundation Investigation and Design Report, Highway 60, From Highway 35  
Easterly 9.1 km To 0.6 km West Of The Oxtongue Lake Narrows Bridge,  
GWP 5553-04-00

Provided By: MEL

Date: November 2008





Left: Exposed bedrock at Borehole No. A5 (Station 22+265, 14 m Lt).  
Right: Exposed bedrock to east of Borehole No. A5.

Photos: 5 - 6



Left: Exposed bedrock at Borehole No. A4.  
Right: Looking east from Angle Lake Road at Borehole No. A6.

Photos: 7 - 8



Reference No.: 08/07/08085A

Project: Foundation Investigation and Design Report, Highway 60, From Highway 35  
Easterly 9.1 km To 0.6 km West Of The Oxtongue Lake Narrows Bridge,  
GWP 5553-04-00

Provided By: MEL

Date: November 2008

## **APPENDIX D**

Figure SK-1      Culvert Replacement in Rock Fills

Figure S-1      Stability Analysis

DIMENSIONS ARE IN METRES  
AND/OR MILLIMETRES UNLESS  
OTHERWISE INDICATED.

# CULVERT REPLACEMENT IN HIGH ROCKFILLS

FIGURE: SK-1

NOT TO SCALE

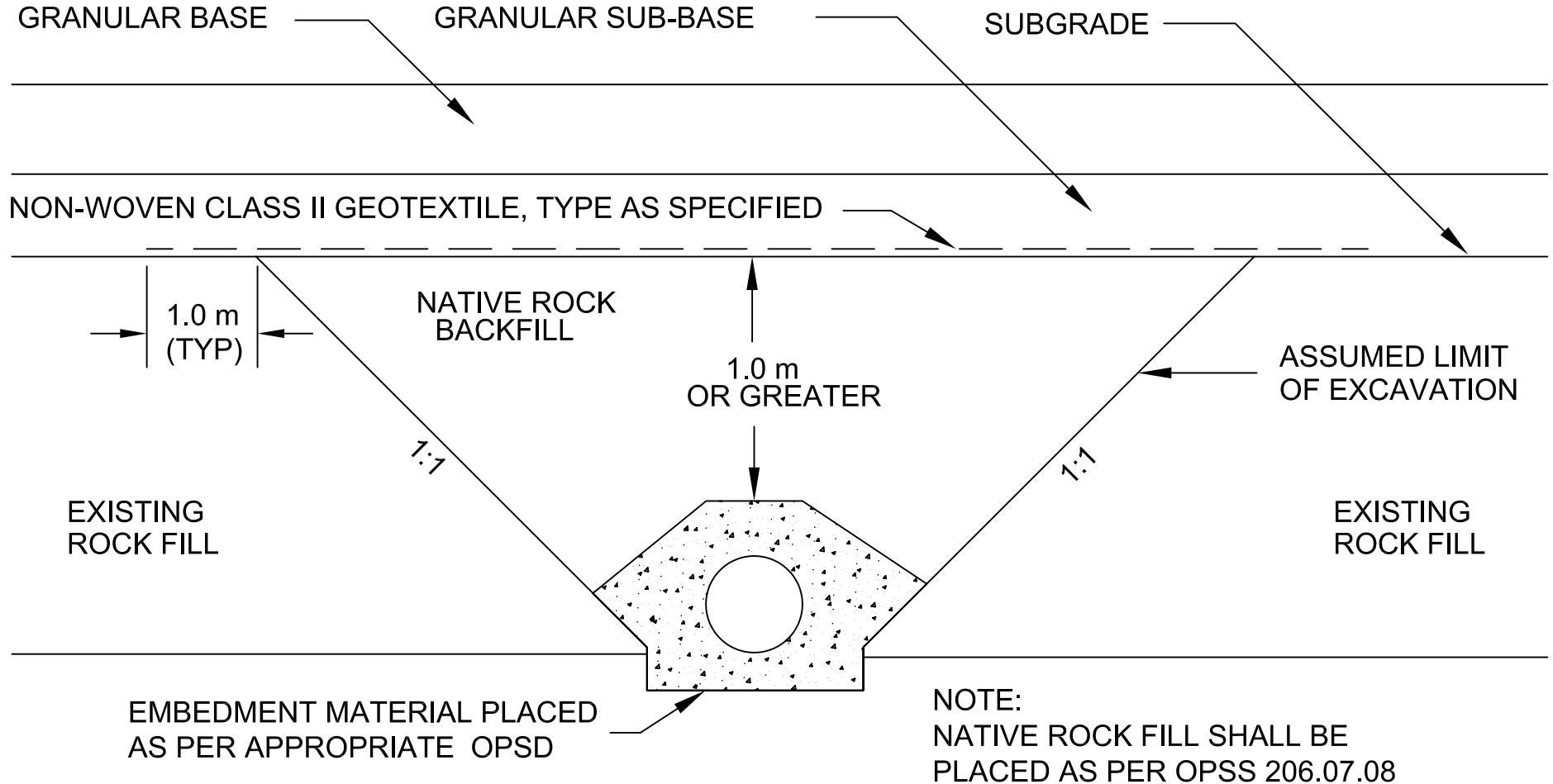




FIGURE S1

Site A - Culvert Station 22+335 - Township of Franklin

