



Englobe

Soils Materials Environment

**Submitted to Planmac Engineering Inc.
80 North Queen Street, Suite 302, Toronto, Ontario M8Z 2C9
For the Ontario Ministry of Transportation**

**Bridge Rehabilitation
Highway 11
Oro Line 7 UP (Simcoe Road 27)
Site No 30-471
GWP 2087-15-00**

FINAL FOUNDATION INVESTIGATION REPORT

Date: February 20, 2018
Ref. Nº: P-0014012-0-00-100-01-F2

Geocres No. 31D-687



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Final Foundation Investigation Report

Prepared by:


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Project Engineer

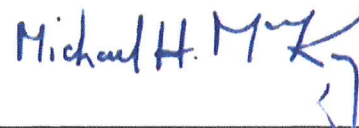




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2018-02-20

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Test results mentioned herein are only valid for the sample(s) stated in this report.

Englobe's subcontractors who may have accomplished work either on site or in laboratory are duly qualified as stated in our Quality Manual's procurement procedure. Should you require any further information, please contact your Project Manager."

Client:

Planmac Engineering Inc.

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Toronto, Ontario

M8Z 2C9

Attention: **Mr. Mike Neumann, President, P. Eng.**

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1 INTRODUCTION

Englobe Corp. (Englobe), has been retained by Planmac Engineering Inc. (Planmac), on behalf of the Ministry of Transportation of Ontario (MTO), to carry out a foundation investigation at an existing bridge site. The site has been described as Oro Line 7 UP (Simcoe Road 27) – Site No. 30-471, and crosses over Highway 11 in the Township of Oro-Medonte, Ontario (see Drawing No. 1, Appendix 1).

The foundation investigation location was specified by the MTO in the Terms of Reference for the work under Assignment No. 2017-E-0004: GWP Nos. 2087-15-00 and 2003-16-00 and its Clarification 3. The terms of reference for the scope of work are outlined in Englobe's Proposal 2017-P152-084, dated May 30, 2017. The purpose of this investigation was to determine the subsurface conditions in the area of the bridge approaches to provide the factual information at the bridge site for the Detailed Design for bridge rehabilitation. Englobe investigated the foundation area by the drilling of boreholes, carrying out in-situ tests, and performing laboratory testing on select samples.

2 SITE DESCRIPTION

The existing bridge underpass is located approximately between Stations 0+017 and 0+058 on Oro Line 7 and crosses over Highway 11, in the Township of Oro-Medonte. The existing Oro Line 7 Underpass embankment currently supports two undivided lanes, locally running in a south-north direction. A visual review of the roadway to the north and the south of the bridge approaches indicates that, in general, the approaches are in fair to good condition (see Photo Essay in Appendix 4).

The existing underpass approached have been constructed on an embankment generally consisting of sand fills. At the bridge location, the existing highway centerline is at Elevation 290.0 m at the north and south expansion joints of the bridge.

There is no known infrastructure within the area of investigation.

2.1 SITE PHYSIOGRAPHY AND SURFICIAL GEOLOGY

The topography in the area of this site is generally flat to slightly rolling. Layers of earth overlay bedrock. Within the project area, the native overburden consists primarily of sands.

Bedrock in the area, based on Ontario Geologic Survey (OGS) Map MRD-126, consists of limestone, dolostone, shale, arkose, and/or sandstone.

3 INVESTIGATION PROCEDURES

The fieldwork for this investigation was carried out on September 21, 2017, during which time two (2) sampled boreholes were advanced. One (1) borehole was advanced at the north approach and one (1) borehole was advanced at the south approach.

The field investigation was carried out using a truck mounted CME drilling rig equipped with hollow stem augers, standard augers, casing equipment and routine geotechnical sampling equipment. Soil samples were obtained at the borehole locations at regular intervals of depth using the standard 50 mm O.D. split spoon sampler advanced in accordance with the Standard Penetration Test (SPT) procedures (ASTM D-1586). The SPT method involves advancing a 50 mm O.D. split spoon sampler with the force of a 63.5 kg hammer freely dropping 760 mm. All samples taken during this investigation were stored in labeled containers for transport to our North Bay laboratory for visual examination and select laboratory testing.

Groundwater conditions in the open boreholes were observed during the advancement of, and immediately following, completion of the individual boreholes. A 19 mm diameter standpipe was installed in Borehole No. 1 prior to backfilling to allow for further monitoring of the shallow groundwater levels. The remaining open borehole was backfilled upon completion with compacted auger cuttings in the general order they were removed, and where necessary, bentonite pellet backfill was added to the boreholes to bring them up to grade in accordance with requirements of Ontario Regulation 903. At the boreholes through the embankment, the upper portion of the hole, where necessary, was backfilled with an asphalt cold patch to seal the existing asphalt surface.

The fieldwork for this investigation was under the full time direction of a senior member of the Englobe engineering staff, who was responsible for locating the boreholes, clearing the borehole locations of underground services, in-situ sampling and testing operations, logging of the boreholes, labeling and preparation of samples for transport to our North Bay laboratory, plus overall drill supervision. All samples received a visual confirmatory inspection in our laboratory. Laboratory testing of select samples included routine geotechnical testing for natural moisture content determination and particle size analyses. The results of the laboratory testing are presented on the individual Record of Borehole Sheets (Appendix 2), with a summary of testing results presented on the laboratory sheets in Appendix 3 (Figures Nos. L-1 to L-4 and Table No. L-5).

The location of the individual boreholes was determined in the field based on the existing underpass bridge approaches and offsets relative to highway centreline. The MTO co-ordinates, northing and easting, were then established for the boring locations using coordinates from MTM Zone 10, NAD 83 CSRS. The borehole elevations are based on coordinating the borehole locations with the highway survey carried out by IBW Surveyors. Elevations contained in this report are referenced to geodetic datum.

4 SUBSURFACE CONDITIONS

Details of the subsurface conditions revealed by the investigation program are presented on the enclosed Records of Borehole Logs (Enclosure Nos. 2 and 3, Appendix 2) and on Drawing No. 2 (Appendix 3). Please note that stratigraphic delineation presented on the borehole logs and soil strata plot are the results of non-continuous sampling, response to drilling progress, the results of SPT, plus field observations. Typically such boundaries represent transitions from one zone to another and are not an exact demarcation of specific geological unit. Additional consideration should be given to the fact that subsurface conditions may vary markedly between adjacent boreholes and beyond any specific boring location, and are shown on the drawings for illustration purposes only.

4.1 ORO LINE 7 UNDERPASS BRIDGE

A plan and profile illustrating the borehole locations and stratigraphic sequences is shown on Drawing No. 2, Appendix 3. During the course of the exploration program, two (2) sampled boreholes were put down at this site, with Borehole Nos. 1 advanced through the north approach and Borehole No. 2 advanced through the south approach. At the time of the subsurface investigation, the ground surface elevations at Boreholes Nos. 1 and 2 were recorded at Elevations 290.0 m.

4.1.1 Pavement Structure

Borehole Nos. 1 and 2 was advanced through the embankment where a pavement structure consisting of 70 to 80 mm of asphalt was penetrated, underlain by a base layer of concrete approach slab, 250 to 300 mm in thickness. The concrete approach slab was underlain by a layer of crushed gravel approximately 100 to 140 mm in thickness.

4.1.2 Embankment Fill

4.1.2.1 Sand and Gravel Fill

Underlying the pavement structure at Borehole No. 1, a layer of embankment fill described as sand and gravel, some silt was penetrated. The natural moisture content measured on samples retrieved from this layer was generally in the order of 4 to 5%. A gradation (sieve) analysis was carried out on one (1) sample of this deposit and the testing results indicated 40% gravel size particles, 48% sand size particles, and 12% silt and clay size particles (Figure No. L-1, Appendix 3). Based on SPT 'N' values of 59 to 67 blows per 300 mm penetration, the compactness of this layer was described as very dense. This layer was encountered to a depth of 1.4 m below grade at Borehole No. 1 (Elevation 288.6 m).

4.1.2.2 Sand Fill (1)

Underlying the pavement structure at Borehole No. 2, a layer of embankment fill described as sand, trace gravel, trace silt was penetrated. The natural moisture content measured on samples retrieved from this layer was generally in the order of 4 to 8%. Based on SPT 'N'

values of 13 to 22 blows per 300 mm penetration, the compactness of this layer was described as compact. This layer was encountered to a depth of 1.3 m below grade at Borehole No. 2 (Elevations 288.7 m).

4.1.2.3 **Gravel Fill**

Underlying the sand fill at Borehole No. 2, a layer of embankment fill described as gravel, with sand, trace silt was penetrated. The natural moisture content measured on samples retrieved from this layer was generally in the order of 3 to 5%. A gradation (sieve) analysis was carried out on one (1) sample of this deposit and the testing results indicated 68% gravel size particles, 24% sand size particles, and 8% silt and clay size particles (Figure No. L-2, Appendix 3). Based on SPT 'N' values of 30 to 6 blows per 300 mm penetration, the compactness of this layer was described as compact to loose. This layer was encountered to a depth of 2.6 m below grade at Borehole No. 2 (Elevations 287.4 m).

4.1.2.4 **Sand Fill (2)**

Underlying the sand and gravel fill at Borehole No. 1, and underlying the gravel fill at Borehole No. 2, a layer of embankment fill described as sand, with to some gravel, some to trace silt, trace clay was penetrated. Layers of silty sands approximately 50 mm in thickness and pockets of silt were encountered within this fill layer at Borehole No. 1. The natural moisture content measured on a sample of this deposit was in the order of 4 to 15%. A gradation (sieve) analysis was carried out on one (1) sample of this layer and the testing results indicated 28% gravel size particles, 58% sand size particles, and 14% silt and clay size particles (Figure No. L-3, Appendix 3). In addition, gradation (hydrometer) analyses were carried out on two samples of this layer, and the testing results indicated 19 to 30% gravel size particles, 54 to 58% sand size particles, 11 to 17% silt size particles and 5 to 6% clay size particles (Figure No. L-3, Appendix 3). Based on SPT 'N' values of 6 to 38 blows per 300 mm penetration, the compactness of this layer was described as loose to dense, generally compact on average. This fill layer was encountered to depths of 7.8 and 5.9 m below grade at Borehole Nos. 1 and 2, respectively (Elevations 282.2 and 284.1 m, respectively).

4.1.3 **Organic Soils**

Underlying the layer of sand fill (2) at Borehole No. 1, a layer of compressed organic soils was penetrated. The natural moisture content measured on samples of this deposit was in the order of 38%. The organic soils were encountered to a depth of 8.2 m below grade (Elevation 281.8 m).

4.1.4 **Sand**

Underlying the organic soils at Borehole No. 1, and the sand fill (2) at Borehole No. 2, a deposit of grey sand, some gravel, with silt, trace clay was penetrated. The natural moisture content measured on samples of this deposit was in the order of 7 to 12%. A gradation (hydrometer) analysis was carried out on one (1) sample of this deposit, and the testing results indicated 19%

gravel size particles, 51% sand size particles, 23% silt size particles, and 7% clay size particles (Figure No. L-4, Appendix 3). Based on SPT 'N' values of 10 to 86 blows per 300 mm penetration, compactness of this deposit was described as compact to very dense, generally dense on average. Auger refusal was encountered within this deposit at a depth of 9.4 m below grade at Borehole No. 1 (Elevation 280.6 m). Sampling was terminated in this deposit at a depth of 12.7 m below grade at Borehole No. 2 (Elevation 277.3 m).

4.2 GROUNDWATER DATA

Measurements of the groundwater table and cave-in levels were undertaken, where possible, in the open boreholes during the advance of the individual borings and upon completion. A 19 mm diameter standpipe piezometer was installed in Borehole No. 1 to obtain post borehole completion water levels. These levels are recorded on the individual Record of Borehole Log Sheets (Appendix 2).

The groundwater levels were measured at Elevations 281.9 and 280.0 m at Borehole Nos. 1 and 2, respectively.

The groundwater and surface water levels will fluctuate seasonally/yearly.

4.3 CHEMICAL TESTING

One (1) soil sample recovered at Borehole No. 2 during the foundation investigation was submitted to AGAT analytical laboratory and tested for corrosivity potential to determine the potential for degradation of concrete in the presence of soluble sulphates used in foundations and buried infrastructure. The results of chemical testing (including PH, water soluble sulphate, chloride, resistivity and electrical conductivity analyses) are tabulated below and included in Appendix 3.

SAMPLE LOCATION	SAMPLE NO.	DEPTH BELOW GRADE (m)	pH	Soluble Sulphate (ppm)	Chloride (ppm)	Resistivity (Ohm.cm)	Electrical Conductivity (mS/cm)
BH 2	2	0.8	9.06	21	55	3800	0.263

5 MISCELLANEOUS

The field investigation was carried out using a truck mounted CME drilling rig owned by Chrisdanat Management Inc. and operated by Englobe Corp. Laboratory testing of select soil samples was undertaken at the North Bay Englobe Laboratory. The fieldwork for this investigation was under the full time supervision of Ed Sullivan, a senior member of the Englobe engineering staff. The report was written by Alexander Tepylo, P.Eng., and reviewed by Sen Hu, P.Eng.

6 STATEMENT OF LIMITATIONS

Proper subgrade preparation, groundwater control, compaction, etc. are all critical aspects of the bearing capacity of native soils. It must be noted that different aspects of the geotechnical design are based on the assumption that Englobe will be retained during site preparation and construction of the proposed works to ensure that both the geotechnical site characteristics and the construction operations/techniques are consistent with our recommendations. Should Englobe not be involved during the full construction phase, our liability is strictly limited to the factual information contained herein only.

The comments in this report are intended solely for the guidance of the design engineer and address the geotechnical conditions only. The number of boreholes required to determine the localized conditions between boreholes directly affecting construction costs, equipment, scheduling, etc. would in fact be greater than what has been carried out for design purposes. Therefore, contractors bidding on this project or undertaking this work should make their own interpretations of the factual borehole results and carry out further work as they deem necessary to assess the scope of the project.

If, during construction, conditions in the field vary from those assumed at the design stage, an engineer from this office must be notified immediately.

Appendix 1 Key Plan

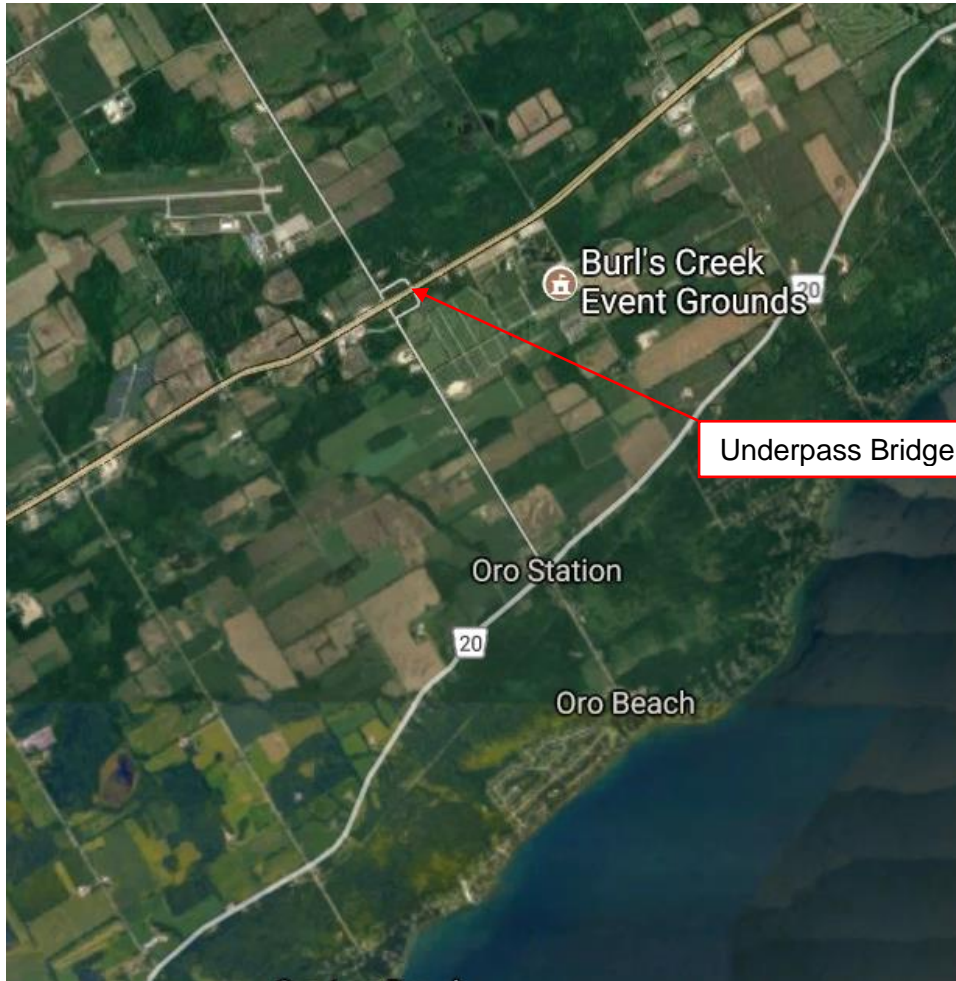
Drawing No. 1

Key Plan

KEY PLAN

Drawing No. 1

NOT TO SCALE



FINAL FOUNDATION INVESTIGATION REPORT

GWP 2087-15-00

Highway 11

Oro Line 7 UP (Simcoe Road 27)

Reference No: P-0014012-0-00-100-01-F2

January 2018



Appendix 2 Subsurface Data

Enclosure No. 1	List of Abbreviations and Symbols
Enclosure Nos. 2 and 3	Record of Borehole Sheet

LIST OF ABBREVIATIONS & DESCRIPTION OF TERMS

The abbreviations and terms, used to describe retrieved samples and commonly employed on the borehole logs, on the figures and in the report are as follows:

1. ABBREVIATIONS

AS	Auger Sample
CS	Chunk Sample
DS	Denison type sample
FS	Foil Sample
NFP	No Further Progress
PH	Sampler advanced by hydraulic pressure
PM	Sampler advanced by manual pressure
RC	Rock core with size & percentage of recovery
SS	Split Spoon
ST	Slotted Tube
TO	Thin-walled, open
TP	Thin-walled, piston
WS	Wash Sample
WH	Sampler advanced by static weight of hammer and/or rods
Rec	% recovery from individual run of rock core
RQD	Rock quality designation (%)

2. PENETRATION RESISTANCE/"N"

Dynamic Cone Penetration Test (DCPT):

A continuous profile showing the number of blows for each 300 mm of penetration of a 50 mm diameter 60° cone attached to AW rod driven by a 63 kg hammer falling 760 mm.

Plotted as —●—●—●—●—

Standard Penetration Test (SPT) or "N" Values

The number of blows of a 63 kg hammer falling 760 mm required to advance a 50 mm O.D. drive open sampler 300 mm.

3. SOIL DESCRIPTION

a) *Cohesionless Soils:*

"N" (blows/0.3 m)	Compactness Condition
0 to 4	very loose
4 to 10	loose
10 to 30	compact
30 to 50	dense
over 50	very dense

b) *Cohesive Soils:*

Undrained Shear Strength (kPa)	Consistency
Less than 12	very soft
12 to 25	soft
25 to 50	firm
50 to 100	stiff
100 to 200	very stiff
over 200	hard

3. SOIL DESCRIPTION (Cont'd)

c) *Bedrock:*

RQD (%)	Classification
Less than 25	Very poor quality
25 to 50	Poor quality
50 to 75	Fair quality
75 to 90	Good quality
90 to 100	Excellent quality

d) *Method of Determination of Undrained Shear Strength of Cohesive Soils:*

+ 3.2 - Field Vane test in borehole.
The number denotes the sensitivity to remoulding.

D - Laboratory Vane Test

" - Compression test in laboratory

For a saturated cohesive soil the undrained shear strength is taken as one-half of the undrained compressive strength.

e) *Soil Moisture:*

Moisture	Described as
Dry	Below optimum moisture content
Moist	Near optimum moisture content
Wet	Above optimum moisture content

4. TERMINOLOGY

Terminology used for describing soil strata is based on the proportion of individual particle sizes present in the samples (please note that, with the exception of those samples subject to a grain-size analysis, all samples were classified visually and the accuracy of visual examination is not sufficient to determine exact grain sizing):

Trace, or occasional	Less than 10%
Some	10 to 20%
With	20 to 30%
Adjective (i.e. silty or sandy)	30 to 40%
And (i.e. sand and gravel)	40 to 60%

Terminology for cobbles and boulders is based on auger response and field observations:

Occasional	Obstructions encountered in borehole, however advance is not impeded
Numerous	Obstructions are essentially continuous over drilled length

SAMPLE DESCRIPTION NOTES:

1. **FILL:** The term fill is used to designate all man-made deposits of natural soil and/or waste materials. The reader is cautioned that fill materials can be very heterogeneous in nature and variable in depth, density and degree of compaction. Fill materials can be expected to contain organics, waste materials, construction materials, shot rock, rip-rap, and/or larger obstructions such as boulders, concrete foundations, slabs, abandoned tanks, etc.; none of which may have been encountered in the borehole. The description of the material penetrated in the borehole therefore may not be applicable as a general description of the fill material on the site as boreholes cannot accurately define the nature of fill material. During the boring and sampling process, retrieved samples may have certain characteristics that identify them as 'fill'. Fill materials (or possible fill materials) will be designated on the Borehole Logs. If fill material is identified on the site, it is highly recommended that testpits be put down to delineate the nature of the fill material. However, even through the use of testpits defining the true nature and composition of the fill material cannot be guaranteed. Fill deposits often contain pockets or seams of organics, organically contaminated soils or other deleterious material that can cause settlement or result in the production of methane gas. It should be noted that the origins and history of fill material is frequently very vague or non-existent. Often fill material may be contaminated beyond environmental guidelines and the material will have to be disposed of at a designated site (i.e. registered landfill). Unless requested or stated otherwise in this report, fill material on this site has not been tested for contaminants however, environmental testing of the fill material can be carried out at your request. Detection of underground storage tanks cannot be determined with conventional geotechnical procedures.
2. **TILL:** The term till indicates a material that is an unstratified, glacial deposit, heterogeneous in nature and, as such, may consist of mixtures and pockets of clay, silt, sand, gravel, cobbles and/or boulders. These heterogeneous deposits originate from a geological process associated with glaciation. It must be noted that due to the highly heterogeneous nature of till deposits, the description of the deposit on the borehole log may only be applicable to a very limited area and therefore, caution must be exercised when dealing with a till deposit. When excavating in till, contractors may encounter cobbles/boulders or possibly bedrock even if they are not indicated on the borehole logs. It must be appreciated that conventional geotechnical sampling equipment does not identify the nature or size of any obstruction.
3. **BEDROCK:** Auger refusal may be due to the presence of bedrock, but possibly could also be due to the presence of very dense underlying deposits, boulders or other large obstructions. Auger refusal is defined as the point at which an auger can no longer be practically advanced. It must be appreciated that conventional geotechnical sampling equipment does not differentiate between nature and size of obstructions that prevent further penetration of the boring below grade. Bedrock indicated on the borehole logs will be labeled 'possibly' or 'probable' etc. based on the response of the boring and sampling equipment, surrounding topography, etc. Bedrock can be proven at individual borehole locations, at your request, by diamond core drilling operations or, possibly, by testpits. It must also be appreciated that bedrock surfaces can be, and most times are, very erratic in nature (i.e. sheer drops, isolated rock knobs, etc.) and caution must be used when interpreting subsurface conditions between boreholes. A bedrock profile can be more accurately estimated, at the clients' request, through a series of closely positioned unsampled auger probes combined with core drilling.
4. **GROUNDWATER:** Although the groundwater table may have been encountered during this investigation and the elevation noted in the report and/or on the record of boreholes, it must be appreciated that the elevation of the groundwater table will fluctuate based upon seasonal conditions, localized changes, erratic changes in the underlying soil profile between boreholes, underlying soil layers with highly variable permeabilities, etc. These conditions may affect the design and type and nature of dewatering procedures. Cave-in levels recorded in borings give a general indication of the groundwater level in cohesionless soils however, it must be noted that cave-in levels may also be due to the relative density of the deposit, drilling operations etc.

METRIC

RECORD OF BOREHOLE NO. 1



REFERENCE P-0014012-0-00-100-01-F2 DATUM Geodetic LOCATION N 4926628.3 E 302303.0, Twp. of Oro-Medonte ORIGINATED BY ELS
 PROJECT GWP 2087-15-00, Hwy 11 - Oro Line 7 UP BOREHOLE TYPE Truck Mounted CME 45 - Hollow Stem Augers COMPILED BY DM
 CLIENT Planmac Engineering Inc. DATE (Started) 21 September 2017 TIME
 DATE (Completed) 21 September 2017 (Completed) CHECKED BY AT

SOIL PROFILE		STRATA PLOT	SAMPLES		GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION (see Enclosure No. 1)		NUMBER	TYPE			"N" VALUES	20						40	60
290.0	Ground Surface														
0.0	70 mm asphalt 250 mm concrete 140 mm crushed gravel														
	EMBANKMENT FILL - sand and gravel, some silt (very dense)		1	SS	59										
			2	SS	67								40 48 (12)		
288.6	EMBANKMENT FILL - sand, with to some gravel, some to trace silt, trace clay (dense/loose)		3	SS	36										
1.4			4	SS	38								28 58 (14)		
	50 mm black silty sand, some gravel		6A 6B	SS	28										
			7A 7B	SS	8								19 58 17 6		
	50 mm grey silty sand, trace gravel		8A 8B	SS	16										
	pockets of silt encountered		9	SS	14										
282.2	ORGANIC SOIL - compressed organic soils black (loose)		10A 10B	SS	9										
281.8	SAND - some gravel, with silt, trace clay														
8.2	grey (very dense)		11	SS	27/100 mm								19 51 23 7		
280.6	Auger Refusal End of Borehole														
9.4															
COMMENTS						+ 3, × 3 : Numbers on right refer to Sensitivity Numbers on left refer to values greater than 100 kPa ○ 3% STRAIN AT FAILURE		WATER LEVEL RECORDS Date (dd/mm/yy)/Time 1) 21/9/17 11:35:00 AM 2) 3)						Water Depth (m) 8.1 - -	Cave In (m) - - -
The stratification lines represent approximate boundaries. The transition may be gradual.															

MEL-GEO P-0014012 - BOREHOLE LOGS - F2.GPJ MEL-GEO.GDT 15/2/18

Englobe Corp.

120 Progress Court, North Bay, On P1A 0C2 Phone: (705)476-2550 Fax: (705)476-8882 Email: northbay@englobecorp.com

METRIC

RECORD OF BOREHOLE NO. 2



REFERENCE	<u>P-0014012-0-00-100-01-F2</u>	DATUM	<u>Geodetic</u>	LOCATION	<u>N 4926587.6 E 302331.7, Twp. of Oro-Medonte</u>	ORIGINATED BY	<u>ELS</u>
PROJECT	<u>GWP 2087-15-00, Hwy 11 - Oro Line 7 UP</u>	BOREHOLE TYPE	<u>Truck Mounted CME 45 - Hollow Stem Augers</u>			COMPILED BY	<u>DM</u>
CLIENT	<u>Planmac Engineering Inc.</u>	DATE (Started)	<u>21 September 2017</u>	TIME		CHECKED BY	<u>AT</u>
		DATE (Completed)	<u>21 September 2017</u>	(Completed)			

[illegible]

MEL-GEO P-0014012 - BOREHOLE LOGS - F2.GPJ MEL-GEO.GDT 15/2/18

METRIC

RECORD OF BOREHOLE NO. 2



REFERENCE	<u>P-0014012-0-00-100-01-F2</u>	DATUM	<u>Geodetic</u>	LOCATION	<u>N 4926587.6 E 302331.7, Twp. of Oro-Medonte</u>	ORIGINATED BY	<u>ELS</u>
PROJECT	<u>GWP 2087-15-00, Hwy 11 - Oro Line 7 UP</u>	BOREHOLE TYPE	<u>Truck Mounted CME 45 - Hollow Stem Augers</u>			COMPILED BY	<u>DM</u>
CLIENT	<u>Planmac Engineering Inc.</u>	DATE (Started)	<u>21 September 2017</u>	TIME		CHECKED BY	<u>AT</u>
		DATE (Completed)	<u>21 September 2017</u>	(Completed)			

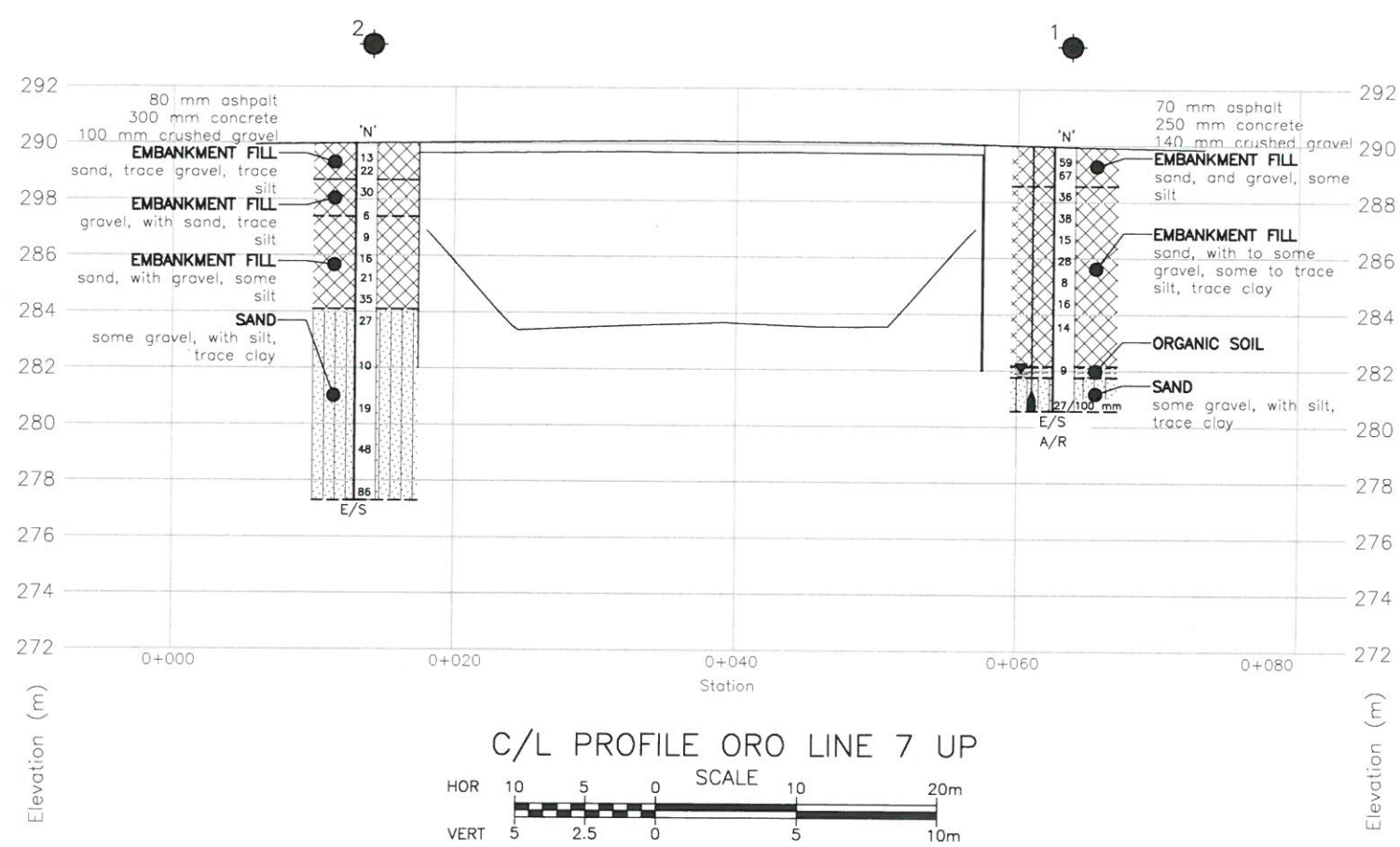
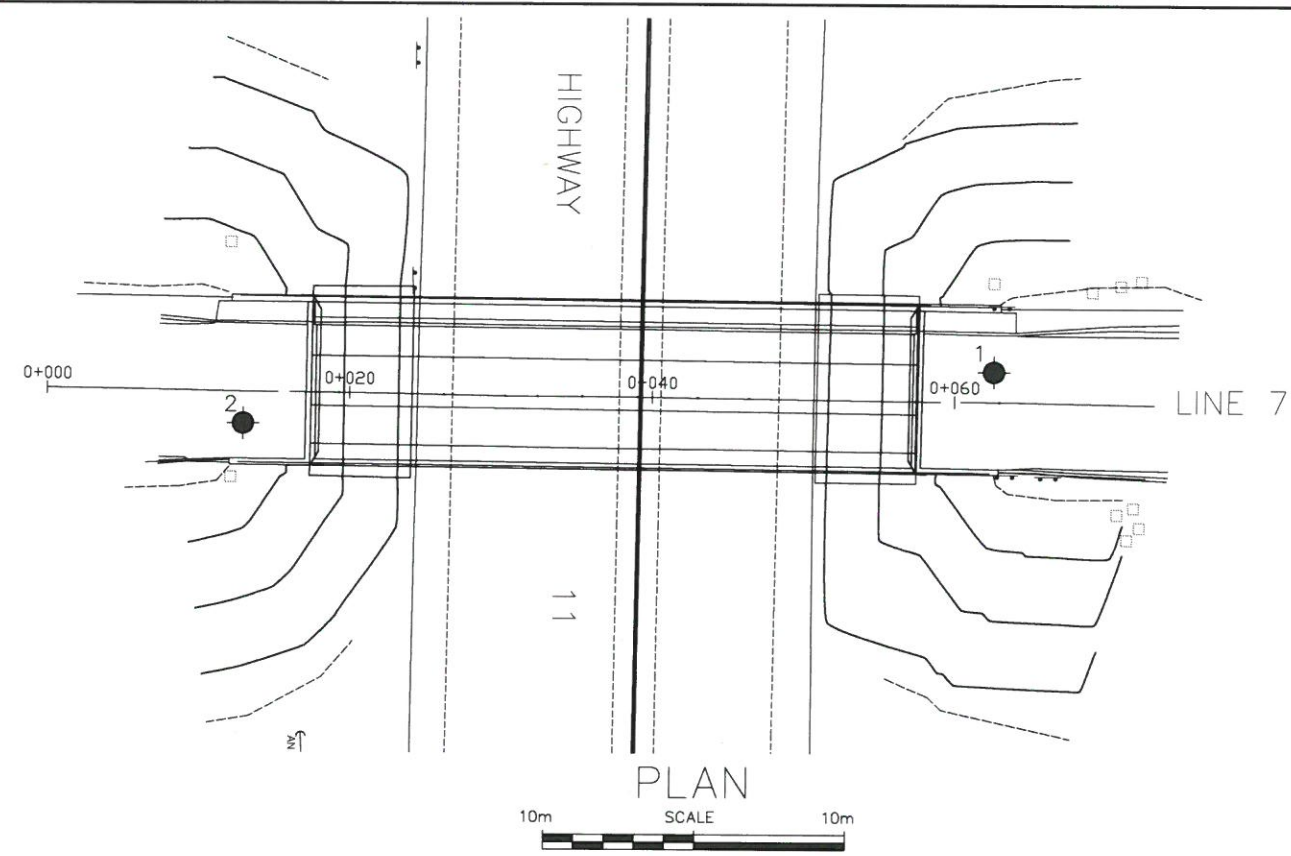
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT NATURAL MOISTURE LIQUID CONTENT LIMIT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA (SI CL)
ELEV DEPTH	DESCRIPTION (see Enclosure No. 1) Continued from Previous Page	STRATA PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa				WATER CONTENT (%)				
								○ UNCONFINED + FIELD VANE		● QUICK TRIAXIAL × LAB VANE						

Appendix 3 Borehole Plan and Laboratory Data

Drawing No. 2: Borehole Location and Soil Strata
Figure Nos. L-1 to L-4: Grain Size Distribution Curves
Table No. L-5: Lab Test Summary Sheet
 Analytical Test Results

CAD FILE LOCATION AND NAME: \\S:\215-001\012 - PAV & FDN, Hwy 11 Drills (Planmac)\Foundations\1_CAD\F215-001\012-F2 - Borehole Location Plan.dwg
MODIFIED: 2/15/2018 9:35:22 AM BY: MTCU
DATE PLOTTED: 2/15/2018 10:45:10 AM BY:

PR-D-107 BR-05
MINISTRY OF TRANSPORTATION, ONTARIO



This drawing is for subsurface information only. Surface details and features are for conceptual illustration. The proposed structure location is shown for illustration purposes only and may not be consistent with the final design configuration as shown elsewhere in the Contract Documents.



2018-02-20

DISTRICT
CONT. No.
GWP No. 2087-15-00

HWY 11
ORO LINE 7 UP (SIMCOE ROAD 27)
SITE NO. 30-471

BOREHOLE LOCATIONS
AND SOIL STRATIGRAPHY

SHEET
2

N

DCPT

A/R

E/S

Borehole w/ DCPT

Borehole

Blows/0.3 m (Std Pen Test, 475 J/blow)

Blows/0.3 m (60' Cone, 475 J/blow)

Water Level at Time of Investigation

Auger Refusal at Elevation

End of Sampling

Piezometer

BOREHOLE No.	ELEVATION	O/S	NORTHING	EASTING
1	290.0	2.0 Lt	4926628.3	302303.0
2	290.0	2.1 Rt	4926587.6	302331.7

NOTES:

The boundaries between soil strata have been established at the borehole locations only. The boundaries illustrated and stratigraphy between boreholes on this drawing are assumed based on borehole data and may vary. They are intended for design only.

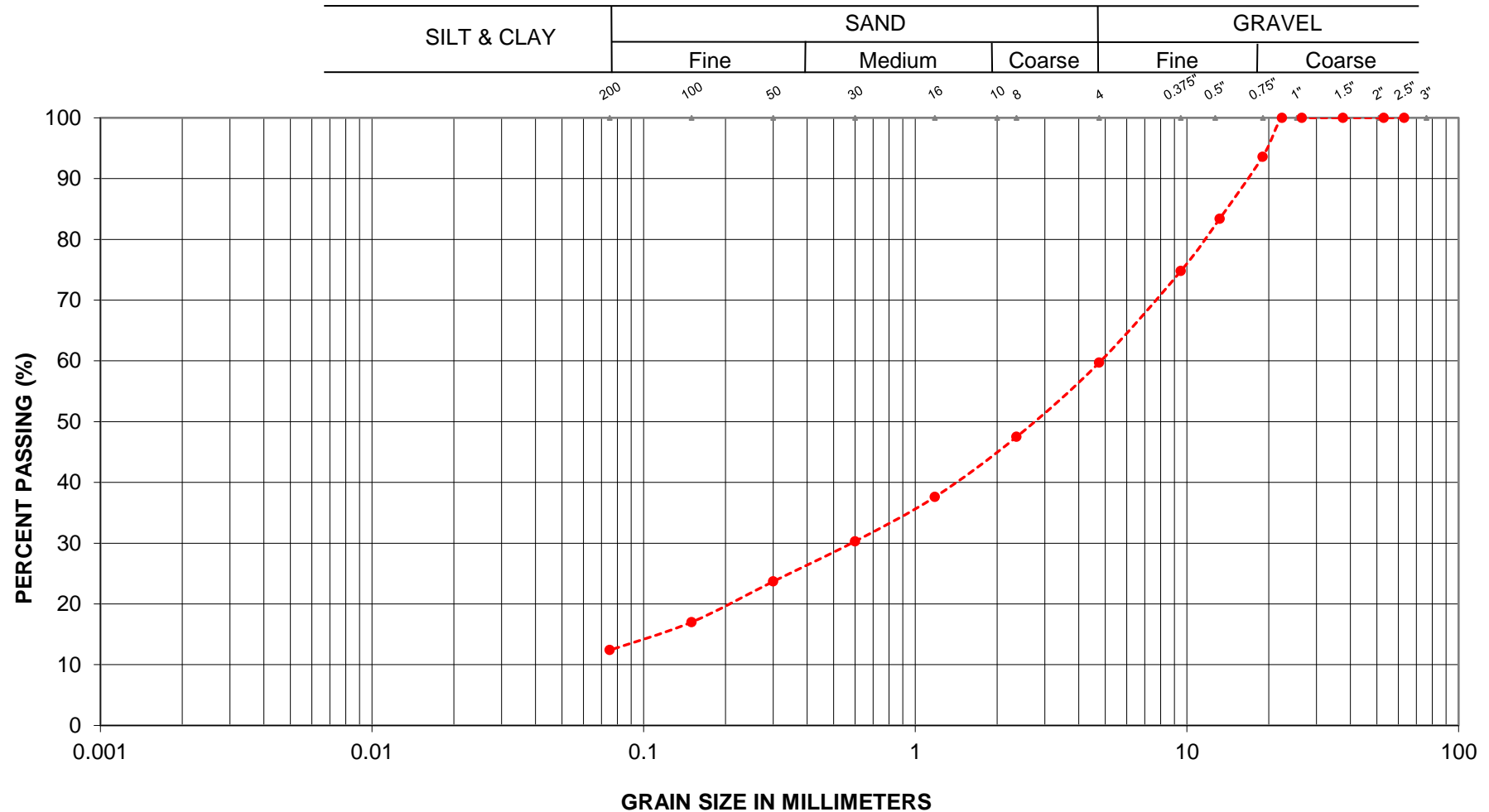
Base plan and alignment provided in digital format by Planmac Engineering Inc. on December 21, 2017

Coordinates based on MTM Zone 10 NAD83 CSRS

GEOCREs No. 31D-687

REVISIONS	DATE	BY	DESCRIPTION
1	JAN/18	DM	DRAFT
2	FEB/18	DM	FINAL

DESIGN	CHK	CODE	LOAD	DATE
DRAWN	DM	CHK	AT	FEB/18

GRAIN SIZE ANALYSIS

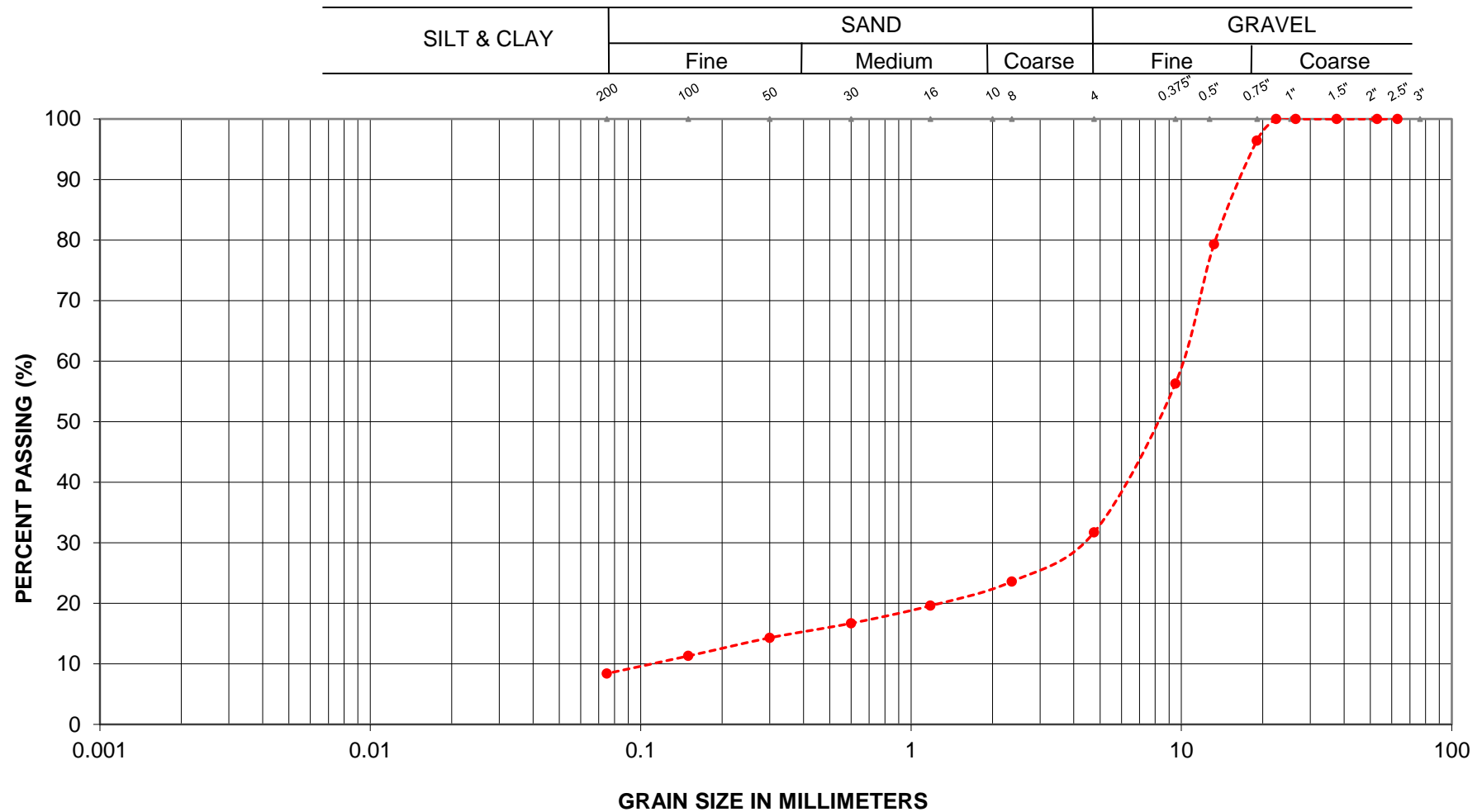
---●--- BH No.: 1 Sa No.: 2 Depth: 0.8 - 1.2 m

EMBANKMENT FILL, sand and gravel

LOCATION: Hwy 11
Oro Line 7 UP

Englobe Corp.

FIGURE L-1

GRAIN SIZE ANALYSIS

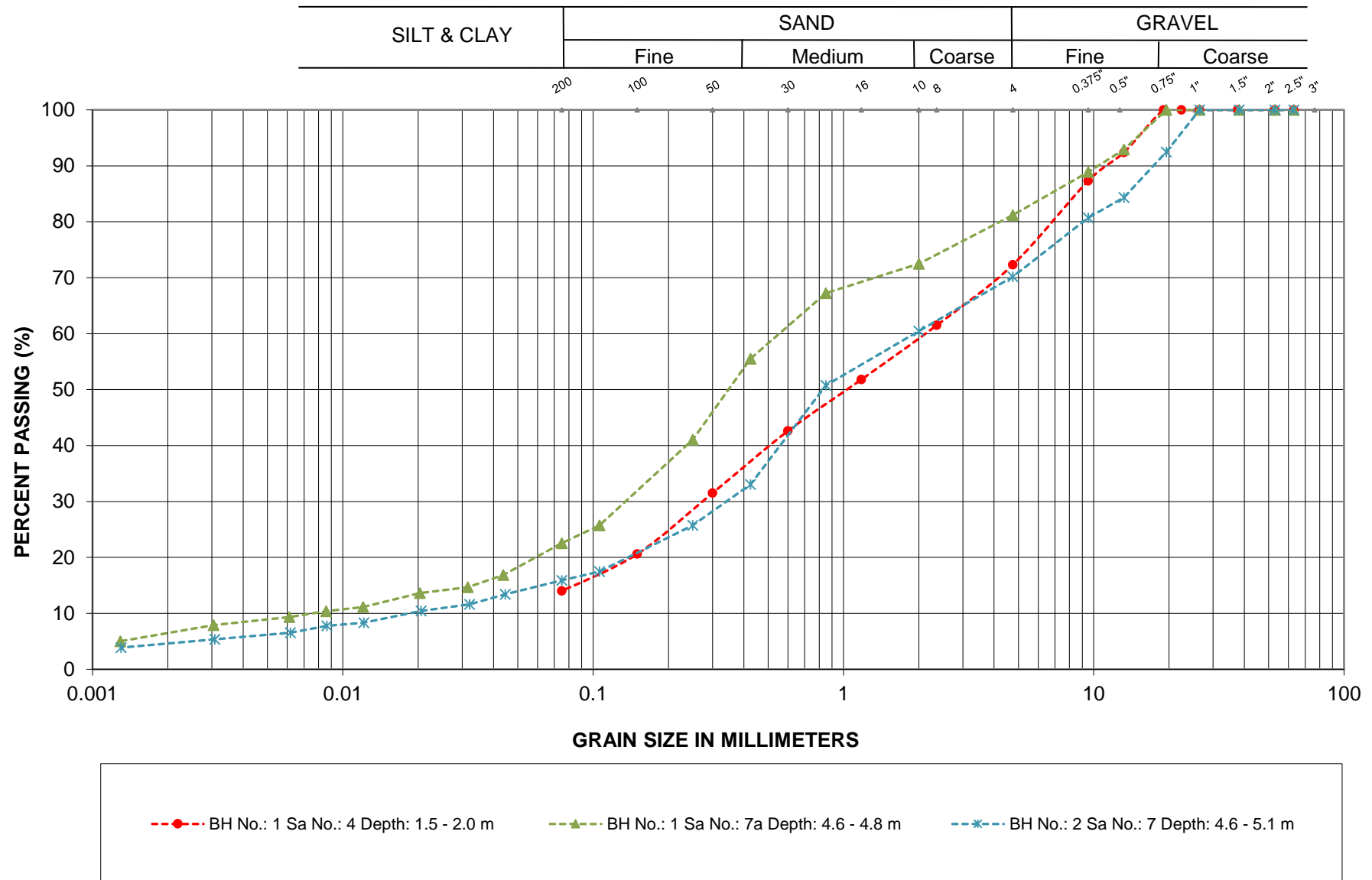
---●--- BH No.: 2 Sa No.: 3 Depth: 1.5 - 2.0 m

EMBANKMENT FILL, gravel with sand

LOCATION: Hwy 11
Oro Line 7 UP

Englobe Corp.

FIGURE L-2

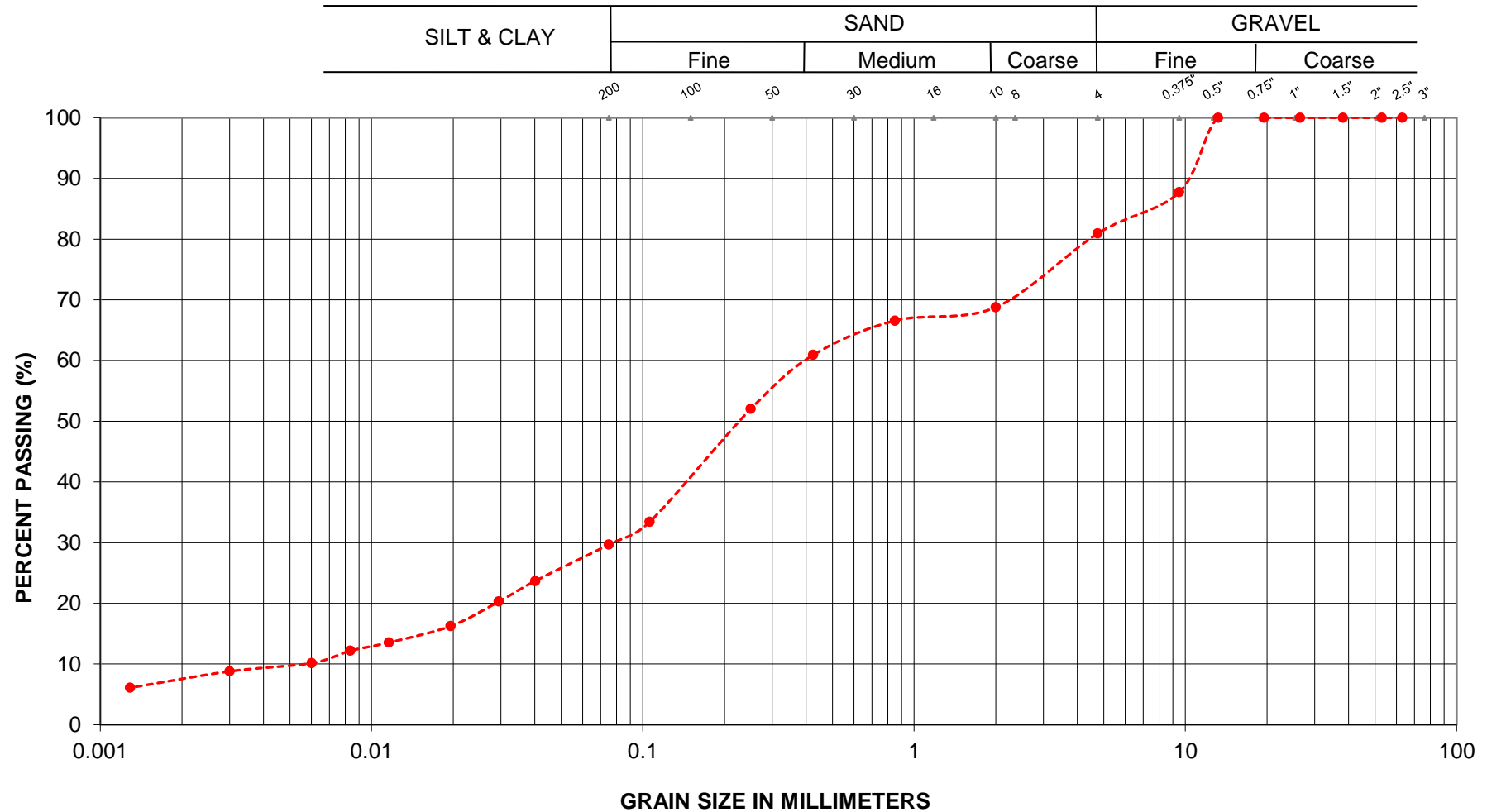
GRAIN SIZE ANALYSIS

EMBANKMENT FILL, sand with to some gravel

LOCATION: Hwy 11
Oro Line 7 UP

Englobe Corp.

FIGURE L-3

GRAIN SIZE ANALYSIS

---●--- BH No.: 1 Sa No.: 11 Depth: 9.1 - 9.3 m

SAND

LOCATION: Hwy 11
Oro Line 7 UP

Englobe Corp.

FIGURE L-4

Laboratory Tests - Summary Sheet



Borehole No.	Sample No.	Depth	Grain Size Analysis				NMC	Atterberg Limits			SPT 'N'	USCS	Unit Weight (kN/m3)	Remarks
			Gravel Size (%)	Sand Size (%)	Silt Size (%)	Clay Size (%)		LL (%)	PL (%)	IP (%)				
1	1	0.3					4.6				59			
	2	0.8	40	48	12		3.7				67			
	3	1.5					4.2				36			
	4	2.3	28	58	14		5.5				38			
	5	3.1					6.5				15			
	6a	3.8					7.0				28			
	6b	4.0					11.0							
	7a	4.6	19	58	17	6	8.1				8			
	7b	4.9					7.6							
	8a	5.3					13.0				16			
	8b	5.7					15.2							
	9	6.1					9.7				14			
	10a	7.6					11.4				9			
	10b	7.9					37.6							
	11	9.1	19	51	23	7	11.9				27/100 mm			
2	1	0.3					8.1				13			
	2	0.8					4.5				22			
	3	1.5	68	24	8		2.5				30			
	4a	2.3					4.8				6			
	4b	2.6					11.2							
	5	3.1					11.4				9			
	6	3.8					9.4				16			
	7	4.6	30	54	11	5	6.7				21			
	8	5.3					11.9				35			
	9	6.1					11.3				27			
	10	7.6					10.7				10			
	11	9.1					7.4				19			
	12	10.7					7.53				48			
	13	12.2					11.72				86			

CLIENT NAME: ENGLOBE CORP
120 PROGRESS CRT.
NORTH BAY , ON P1A0C2
(705) 476-2550

ATTENTION TO: Victoria Steuernol

PROJECT: P-0014012-0-00-100-01

AGAT WORK ORDER: 17T286462

SOIL ANALYSIS REVIEWED BY: Amanjot Bhela, Inorganic Coordinator

DATE REPORTED: Nov 30, 2017

PAGES (INCLUDING COVER): 6

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.



Certificate of Analysis

AGAT WORK ORDER: 17T286462

PROJECT: P-0014012-0-00-100-01

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

CLIENT NAME: ENGLOBE CORP

SAMPLING SITE:

ATTENTION TO: Victoria Steuernol

SAMPLED BY:

Corrosivity Package

DATE RECEIVED: 2017-11-21

DATE REPORTED: 2017-11-30

		SAMPLE DESCRIPTION:		F1-BH1-Sa2	F2-BH2-Sa2	F3-BH1-Sa3	F4-BH2-Sa2	F5-BH2-Sa3	F6-BH1-Sa2	F7-BH1-Sa8	F8-BH1-Sa3
		SAMPLE TYPE:		Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil
		DATE SAMPLED:									
Parameter	Unit	G / S	RDL	8924749	8924752	8924753	8924754	8924755	8924756	8924757	8924758
Sulfide (S2-)	%		0.05	<0.05	0.06	<0.05	<0.05	<0.05	<0.05	<0.05	0.08
Chloride (2:1)	µg/g		2	8	55	190	17	25	12	5	345
Sulphate (2:1)	µg/g		2	4	21	21	7	4	10	7	44
pH (2:1)	pH Units		NA	9.81	9.06	9.80	9.14	8.87	8.91	9.05	9.44
Electrical Conductivity (2:1)	mS/cm		0.005	0.218	0.263	0.557	0.137	0.118	0.104	0.066	0.710
Resistivity (2:1)	ohm.cm		1	4590	3800	1800	7300	8470	9620	15200	1410
Redox Potential (2:1)	mV		5	143	156	118	148	156	158	151	127
		SAMPLE DESCRIPTION:		F9-BH1-Sa2		F10-BH1-Sa5		F11-BH1-Sa3		F12-BH1-Sa4	
		SAMPLE TYPE:		Soil		Soil		Soil		Soil	
		DATE SAMPLED:									
Parameter	Unit	G / S	RDL	8924759	RDL	8924760	RDL	8924761	8924762		
Sulfide (S2-)	%		0.05	<0.05	0.05	0.09	0.05	<0.05	<0.05		
Chloride (2:1)	µg/g		2	166	4	1210	2	175	334		
Sulphate (2:1)	µg/g		2	14	4	19	2	66	30		
pH (2:1)	pH Units		NA	9.62	NA	8.95	NA	9.22	9.33		
Electrical Conductivity (2:1)	mS/cm		0.005	0.435	0.005	1.79	0.005	0.622	0.773		
Resistivity (2:1)	ohm.cm		1	2300	1	559	1	1610	1290		
Redox Potential (2:1)	mV		5	121	5	138	5	123	117		

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard

8924749-8924759 EC/Resistivity, pH, Chloride, Sulphate and Redox Potential were determined on the extract obtained from the 2:1 leaching procedure (2 parts DI water: 1 part soil).

*Sulphide analyzed at AGAT 5623 McAdam

Sampling dates were not mentioned on COC.

8924760 EC/Resistivity, pH, Chloride, Sulphate and Redox Potential were determined on the extract obtained from the 2:1 leaching procedure (2 parts DI water: 1 part soil).

*Sulphide analyzed at AGAT 5623 McAdam

Sampling date was not mentioned on COC.

Elevated RDL indicates the degree of sample dilution prior to the analysis for Anions in order to keep analytes within the calibration range of the instrument and to reduce matrix interference.

8924761-8924762 EC/Resistivity, pH, Chloride, Sulphate and Redox Potential were determined on the extract obtained from the 2:1 leaching procedure (2 parts DI water: 1 part soil).

*Sulphide analyzed at AGAT 5623 McAdam

Sampling dates were not mentioned on COC.

Certified By:

Amanjot Bhela



Quality Assurance

CLIENT NAME: ENGLOBE CORP

PROJECT: P-0014012-0-00-100-01

SAMPLING SITE:

AGAT WORK ORDER: 17T286462

ATTENTION TO: Victoria Steuernal

SAMPLED BY:

Soil Analysis

RPT Date:			DUPLICATE			Method Blank	REFERENCE MATERIAL		METHOD BLANK SPIKE			MATRIX SPIKE			
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper

Corrosivity Package

Sulfide (S2-)	8924749	8924749	< 0.05	< 0.05	NA	< 0.05	98%	80%	120%						
Chloride (2:1)	8924749	8924749	8	8	NA	< 2	108%	80%	120%	106%	80%	120%	104%	70%	130%
Sulphate (2:1)	8924749	8924749	4	4	NA	< 2	95%	80%	120%	99%	80%	120%	103%	70%	130%
pH (2:1)	8924749	8924749	9.81	9.79	0.2%	NA	101%	90%	110%	NA			NA		
Electrical Conductivity (2:1)	8924749	8924749	0.218	0.209	4.2%	< 0.005	98%	90%	110%	NA			NA		
Redox Potential (2:1)	8924749	8924749	143	143	0.0%	< 5	104%	70%	130%	NA			NA		

Corrosivity Package

Sulfide (S2-)	8924753	8924753	< 0.05	< 0.05	NA	< 0.05	99%	80%	120%						
---------------	---------	---------	--------	--------	----	--------	-----	-----	------	--	--	--	--	--	--

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Certified By:

Amanjot Bhela

Method Summary

CLIENT NAME: ENGLOBE CORP

PROJECT: P-0014012-0-00-100-01

SAMPLING SITE:

AGAT WORK ORDER: 17T286462

ATTENTION TO: Victoria Steuernol

SAMPLED BY:

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Sulfide (S ²⁻)	MIN-200-12025	ASTM E1915-09	GRAVIMETRIC
Chloride (2:1)	INOR-93-6004	McKeague 4.12 & SM 4110 B	ION CHROMATOGRAPH
Sulphate (2:1)	INOR-93-6004	McKeague 4.12 & SM 4110 B	ION CHROMATOGRAPH
pH (2:1)	INOR 93-6031	MSA part 3 & SM 4500-H+ B	PH METER
Electrical Conductivity (2:1)	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Resistivity (2:1)	INOR-93-6036	McKeague 4.12, SM 2510 B, SSA #5 Part 3	CALCULATION
Redox Potential (2:1)		McKeague 4.12 & SM 2510 B	REDOX POTENTIAL ELECTRODE

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AGAT Laboratories

5835 Coopers Avenue
Mississauga, Ontario L4Z 1Y2
Ph: 905.712.5100 Fax: 905.712.5122
web@earth.agatlabs.com

Chain of Custody Record

If this is a Drinking Water sample, please use Drinking Water Chain of Custody Form (potable water consumed by humans)

Report Information:

Company: Englobe Corp
Contact: Victoria Steuermol
Address: 2-120 Progress Court
North Bay, Ontario
Phone: 705 476 2550 Fax: 705 476 8882
Reports to be sent to:
1. Email: alexander.tepylo@englobecorp.com
2. Email: victoria.steuernol@englobecorp.com

Project Information:

Project: P-0014012 0-00-100-08
Site Location: _____
Sampled By: _____
AGAT Quote #: _____ PO: A12736

Please note: If quotation number is not provided, client will be billed full price for analysis.

Invoice Information:

Bill To Same: Yes ☒ No ☐

Company: _____
Contact: _____
Address: _____
Email: _____

Regulatory Requirements:

(Please check all applicable boxes)

☐ Regulation 153/04 ☐ Sewer Use ☐ Regulation 558
☐ Table Indicate One ☐ Sanitary ☐ CCME
☐ Ind/Com ☐ Storm ☐ Prov. Water Quality
☐ Res/Park ☐ Agriculture ☐ Objectives (PWQO)
☐ Soil Texture (Check One) Region Indicate One ☐ Other
☐ Coarse ☐ MISA ☐ Fine ☐ Indicate One

Is this submission for a Record of Site Condition?

☐ Yes ☐ No

Report Guideline on Certificate of Analysis

☒ Yes ☐ No

Sample Matrix Legend

B Biota
GW Ground Water
O Oil
P Paint
S Soil
SD Sediment
SW Surface Water

Field Filtered - Metals, Hg, CrVI

0. Reg 153

Metals and Inorganics

☐ All Metals ☒ 153 Metals (excl. Hydrides)
☐ Hydride Metals ☐ 153 Metals (incl. Hydrides)

ORPs: ☐ B-HWS ☐ Cl ☐ CN
☐ Cr+ ☐ EC ☐ FOC ☐ Hg
☐ pH ☐ SAR

Full Metals Scan

Regulation/Custom Metals

Nutrients: ☐ TP ☐ NH₃ ☐ TKN
☐ NO₃ ☐ NO₂ ☐ NO₃+NO₂

Volatiles: ☐ VOC ☐ BTEX ☐ THM

PHCs F1 - F4

ABNs

PAHs

PCBs: ☐ Total ☐ Aroclors

Organochlorine Pesticides

TCLP: ☐ M&I ☐ VOCs ☐ ABNs ☐ B(a)P ☐ PCBs

Sewer Use

Corrosivity Package

Sample Identification	Date Sampled	Time Sampled	# of Containers	Sample Matrix	Comments/ Special Instructions	Y / N	Metals and Inorganics	ORPs	Full Metals Scan	Regulation/Custom Metals	Nutrients	Volatiles	PHCs F1 - F4	ABNs	PAHs	PCBs	Organochlorine Pesticides	TCLP	Sewer Use	Corrosivity Package
F1-BH1-Sa2			1	S	Corrosivity Package Test for all:															<input checked="" type="checkbox"/>
F2-BH2-Sa2			1	S	pH, water soluble sulphate,															<input checked="" type="checkbox"/>
F3-BH1-Sa3			1	S	sulphide, chloride, resistivity and															<input checked="" type="checkbox"/>
F4-BH2-Sa2			1	S	electrical conductivity															<input checked="" type="checkbox"/>
F5-BH2-Sa3			1	S																<input checked="" type="checkbox"/>
F6-BH1-Sa2			1	S																<input checked="" type="checkbox"/>
F7-BH1-Sa8			1	S																<input checked="" type="checkbox"/>
F8-BH1-Sa3			1	S																<input checked="" type="checkbox"/>
F9-BH1-Sa2			1	S																<input checked="" type="checkbox"/>
F10-BH1-Sa5			1	S																<input checked="" type="checkbox"/>
F11-BH1-Sa3			1	S																<input checked="" type="checkbox"/>

Samples Relinquished By (Print Name and Sign):

Sonia Clelland SC

Date

Nov 20/17

Time

7:00

Samples Received By (Print Name and Sign):

Poehli Patel

Date

21/11/17

Time

8:33

Samples Relinquished By (Print Name and Sign):

Samples Received By (Print Name and Sign):

Samples Relinquished By (Print Name and Sign):

Samples Received By (Print Name and Sign):

Page 1 of 2

N#:

Appendix 4 Photo Essay

Enclosure No. 4:

Photo Essay

Existing Bridge, North Approach – Looking South

Photo: 1



Project: GWP 2087-15-00 - Hwy 11 – Oro Line 7 UP

Photos Provided By: Englobe

Date: September 2017