



March 12, 2015

FOUNDATION INVESTIGATION REPORT

**SHAWANAGA RIVER NBL BRIDGE STRUCTURE, SITE NO. 44-443/1
HIGHWAY 69 FOUR-LANING FROM 1.0 KM NORTH OF THE NEW HIGHWAY
559 INTERCHANGE NORTHERLY TO 1.5 KM NORTH OF HIGHWAY 7182
(SHEBESHEKONG ROAD) FOR 17 KM
MINISTRY OF TRANSPORTATION, ONTARIO
G.W.P. 5111-07-00; W.P. 5187-06-01 (Phase 2 of G.W.P. 5402-05-00)**

Submitted to:
McCormick Rankin, a member of MMM Group Limited
2655 North Sheridan Way
Mississauga, Ontario
L5K 2P8



GEOCREs No: 41H-145

Report Number: 07-1111-0029-9

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REPORT



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PART A

FOUNDATION INVESTIGATION REPORT

SHAWANAGA RIVER NBL BRIDGE STRUCTURE, SITE NO. 44-443/1

HIGHWAY 69 FOUR-LANING FROM 1.0 KM NORTH OF THE NEW

HIGHWAY 559 INTERCHANGE NORTHERLY TO 1.5 KM NORTH OF

HIGHWAY 7182 (SHEBESHEKONG ROAD) FOR 17 KM

MINISTRY OF TRANSPORTATION, ONTARIO

G.W.P. 5111-07-00; W.P. 5187-06-01



1.0 INTRODUCTION

Golder Associates Ltd. (Golder) has been retained by McCormick Rankin (MRC), a member of MMM Group Limited on behalf of Ministry of Transportation, Ontario (MTO) to provide detail foundation engineering services for the proposed three-span Shawanaga River Northbound Lanes (NBL) Bridge structure over the Shawanaga River (Site No. 44-443/1). The proposed work is part of the four-laning of Highway 69 from 1.0 km north of the new Highway 559 Interchange northerly to 1.5 km north of Highway 7182 (Shebeshekong Road), which involves high fill embankments and embankments over swamps, the New Woods Road and Shebeshekong Road interchanges and structures, the Shawanaga River and Site 9 Road structures, as well as culvert crossings. The general location of this bridge structure along the new Highway 69 four-laning alignment is shown on Drawing 1.

The Terms of Reference and the Scope of Work for the foundation engineering services are outlined in MTO's Request for Proposal, dated July 2006. Golder's proposal for foundation engineering services associated with the Shawanaga River NBL Bridge is contained in Section 6.8 of MRC's Technical Proposal for this assignment. The work has been carried out in accordance with Golder's Supplementary Specialty Plan for this project, dated July 4, 2007. The General Arrangement (GA) Drawing for the proposed Shawanaga River NBL Bridge structure was provided to Golder by MRC on January 15, 2009.

This report addresses the investigation carried out for the Shawanaga River NBL Bridge and the associated approach embankments only. Separate reports address the foundation investigations for the swamp crossings, high fill areas associated with interchange ramps and roadways, culverts and other bridge structures for the project.

The purpose of this investigation is to establish the subsurface conditions at the proposed structure, including the associated approach embankments, by borehole drilling, rock coring, test pitting and geological mapping of bedrock outcrops and laboratory testing on selected samples. The bridge structure centerline and the foundation units/limits for this investigation were located in the field prior to drilling by Callon Dietz Inc. (Callon Dietz), a professional surveying company retained by MRC. The investigation area is shown in plan on Drawing 2.

2.0 SITE DESCRIPTION

The proposed Shawanaga River NBL Bridge is located approximately 450 m north of the intersection of the existing Shebeshekong Road and Highway 69 and is approximately 20.5 km northwest of Nobel, Ontario. The proposed new Highway 69 alignment runs generally in a southeast-northwest direction on the west side of the existing Highway 69, which will become part of the future Shawanaga River Service Road (Site No. 9) in this area.

In general, the topography in the area of the overall project limits consists of rolling terrain including densely treed areas and numerous bedrock outcrops separated by low-lying swamps and rivers. The proposed bridge structure and associated approach embankments are to be situated on a bedrock outcrop on the south side of the Shawanaga River and on the side slope of the existing Highway 69 rock fill embankment on the north side of the river. On the south side of the river, the ground surface within the limits of the proposed structure ranges from about Elevation 212.9 m at the south approach embankment, to about Elevation 207.5 m at the south abutment and to as low as about Elevation 202.0 m at the south pier. On the north side, the ground surface ranges from about Elevation 201.0 m at the north pier to about Elevations 202 m to 205.4 m at the north abutment and about Elevations 200.0 m to 202.5 m at the north approach. All elevations are referenced to Geodetic datum.



3.0 INVESTIGATION PROCEDURES

3.1 Foundation Investigation

The field work for the Shawanaga River NBL Bridge subsurface investigation was carried out between February 6 and 9, 2009, between March 21 and 26, 2009, on April 2, 2009 and between July 20 and 26, 2009 during which time a total of twenty-one (21) boreholes and three (3) test pits were advanced: five (5) boreholes at the south abutment; five (5) boreholes at the south pier (Pier 3); four (4) boreholes and three (3) test pits at the north pier (Pier 4); five (5) boreholes at the north abutment; and one (1) borehole at each approach embankment. The boreholes, designated as Boreholes B4-01 to B4-21 and the test pits, designated as B4-TP1 to B4-TP3, were advanced at the locations shown in plan on Drawing 2.

The boreholes were drilled using a track-mounted Diedrich D-25 drill rig supplied and operated by Walker Drilling Co. Ltd. of Utopia, Ontario and portable equipment supplied and operated by OGS Inc. of Almonte, Ontario. The test pits were excavated using a 240 John Deere excavator operated by Weeks Construction Inc. of Parry Sound, Ontario. Hand excavation methods were used as appropriate depending on the terrain. The boreholes were advanced through the overburden using 165 mm O.D. solid-stem augers and/or 'BW' and 'NW' casing. Soil samples were obtained at intervals of depth of about 0.75 m using a 50 mm outside diameter (O.D.) split-spoon sampler driven by an automatic hammer in accordance with Standard Penetration Test (SPT) procedures (ASTM D1586 Standard Test Method for Standard Penetration Tests and Split Barrel Sampling of the Soil). Boreholes advanced by portable equipment employed one-third ($\frac{1}{3}$) weight hammers lifted manually. Chunk samples were obtained in five (5) boreholes at locations of thin overburden over bedrock outcrops. Samples of the bedrock were obtained using an 'NQ' or 'BQ' size rock core barrel.

The boreholes at the foundation elements were typically advanced to casing and/or sampler refusal (i.e. inferred bedrock) and bedrock was confirmed by coring in selected boreholes. The boreholes at the approaches were advanced to the bedrock surface / sampler refusal and extended into bedrock by coring at the north approach. The boreholes were drilled to depths of up to about 10.2 m below existing ground surface, including coring of bedrock for core lengths between about 3.0 m and 10.2 m in Boreholes B4-01 to B4-11, B4-15 and B4-18.

The test pits at the proposed north pier were excavated to bucket refusal to depths between 0.8 m to 2.1 m below existing ground surface and the bedrock surface was confirmed by exposure at the base of the test pits.

The groundwater conditions in the open boreholes and test pit excavations were observed during the drilling and on completion of test pitting operations and a total of four (4) piezometers were installed in Boreholes B4-01, B4-07, B4-15 and B4-18 to permit monitoring of the water level at these locations. The piezometers consist of 32 mm diameter PVC pipe, with a slotted screen sealed at a select depth within the boreholes. The boreholes and annulus surrounding the piezometer pipe above the screen sand pack were backfilled to the surface with bentonite pellets/grout. Piezometer installation details and water level readings are described on the Record of Borehole sheets presented in Appendix A. All boreholes in which standpipe piezometers were not installed were backfilled with bentonite upon completion in accordance with Ontario Regulation 903 Wells (as amended). The test pit excavations were backfilled with the excavated soil which was nominally compacted by the backhoe bucket and the ground surface was graded to the surrounding ground surface.

The field work was observed by members of our engineering and technical staff who located the boreholes and test pits, arranged for the clearance of underground services, observed the drilling, sampling and in situ testing operations, logged the boreholes and test pits, and examined and cared for the soil and rock samples. The samples were identified in the field, placed in appropriate containers, labelled and transported to our Mississauga geotechnical laboratory where the samples underwent further visual examination and laboratory



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testing. All of the laboratory tests were carried out to MTO and/or ASTM Standards, as appropriate. Classification testing (water content, Atterberg limits and grain size distribution) was carried out on selected samples. Strength tests such as unconfined compression and point load index, were carried out on specimen of the rock core. The results of the laboratory testing are included in Appendix B.

The as-drilled borehole and test pits locations and ground surface elevations were surveyed by a member of our technical staff, referenced to the survey stakes put down by Callon Dietz. The borehole / test pit locations given in the Record of Borehole/Drillhole sheets and Field Test Pit Logs and shown on Drawings 2 and 3 are positioned relative to MTM NAD 83 northing and easting coordinates and the ground surface elevations are referenced to Geodetic datum and are summarized below.

Borehole / Test Pit	Location (MTM Nad 83)		Ground Surface Elevation (m)	Depth Drilled / Excavated (m)
	Northing	Easting		
B4-TP1	5045843.4	243508.3	199.3	1.1
B4-TP2	5045846.6	243510.3	199.6	2.1
B4-TP3	5045847.0	243506.5	201.4	0.8
B4-01	5045857.8	243498.8	203.5	6.0
B4-02	5045853.5	243493.5	201.7	4.3
B4-03	5045856.8	243490.0	201.2	5.3
B4-04	5045871.6	243474.4	200.2	3.8
B4-05	5045846.6	243510.3	201.8	6.0
B4-06	5045847.0	243506.5	201.5	5.5
B4-07	5045850.8	243512.1	202.4	6.9
B4-08	5045846.8	243514.4	201.8	8.1
B4-09	5045866.1	243505.4	205.3	7.0
B4-10	5045860.9	243510.8	205.5	6.2
B4-11	5045815.2	243536.2	207.6	10.2
B4-12	5045818.8	243534.4	204.5	0.0
B4-13	5045818.8	243539.1	205.0	0.1
B4-14	5045819.1	243544.2	202.3	0.2
B4-15	5045823.7	243541.0	202.0	6.5
B4-16	5045802.5	243546.3	208.3	1.1



Borehole / Test Pit	Location (MTM Nad 83)		Ground Surface Elevation (m)	Depth Drilled / Excavated (m)
	Northing	Easting		
B4-17	5045805.3	243543.4	207.5	0.3
B4-18	5045807.7	243550.6	207.2	6.4
B4-19	5045809.0	243559.7	208.2	0.1
B4-20	5045811.8	243556.8	206.5	0.1
B4-21	5045793.8	243565.0	212.9	0.1

Additional field work was completed on August 26, 2009 for geological mapping of the bedrock outcrops to supplement the foundation investigation for the Shawanaga River NBL Bridge structure. The location of rock mapping is shown in plan on Figure C1 found in Appendix C.

4.0 SITE GEOLOGY AND SUBSURFACE CONDITIONS

4.1 Regional Geology

As delineated in *The Physiography of Southern Ontario*¹, this section of Highway 69 lies within the physiographic region known as the Georgian Bay Fringe, which extends along the east side of Georgian Bay through the Parry Sound and Muskoka areas, then eastward from Muskoka in patches into the area north of the Kawartha Lakes.

This part of the Georgian Bay Fringe physiographic region was never submerged during periods of glacial recession. As a result, the surficial soils in this area consist of very shallow deposits of sand, silt and clay underlain by metamorphic bedrock; numerous bare knobs and ridges of bedrock are present throughout the area. Localized low lying swampy areas, containing peat and/or organic soils underlain by soft/loose native soils, are present in valleys between the bedrock knobs and ridges.

The bedrock in the area consists typically of gneisses of the Britt Domain of the Central Gneiss Belt, a subdivision of the Grenville Structural Province, as described in *Geology of Ontario, OGS Special Volume 4*². Deposition of Palaeozoic strata initially covered, and later erosion during glaciation subsequently exposed, these Precambrian rocks.

4.2 Subsurface Conditions

The detailed subsurface soil and groundwater conditions encountered in the boreholes and test pits advanced for this investigation, together with the results of the laboratory tests carried out on selected soil and bedrock

¹ Chapman, L.J. and Putnam, D.F.. *The Physiography of Southern Ontario*, Ontario Geological Survey Special Volume 2, Third Edition, 1984. Accompanied by Map P.2715, Scale 1:600,000.

² Ontario Geological Society, 1991. *Geology of Ontario*, Special Volume 4, Part 2. Ministry of Northern Development and Mines, Ontario.



core samples, are presented in the record of Boreholes sheets provided in Appendix A. The results of the laboratory tests are also provided in Appendix B. The results of the in situ field tests (i.e. SPT 'N'-values and undrained shear strengths from the field vanes) as presented on the Record of Borehole sheets and in Section 4.3 to 4.6 are uncorrected. The stratigraphic boundaries shown on the Record of Borehole and Drillhole sheets are inferred from non-continuous sampling, observations of drilling progress and the results of Standard Penetration Tests. These boundaries, therefore, represent transitions between soil types rather than exact planes of geological change. Further, subsurface conditions will vary between and beyond the borehole locations. It should be noted that the interpreted stratigraphy shown on Drawings 2 and 3 is a simplification of the subsurface conditions.

In general, the subsurface conditions in the area of the of the south abutment and south pier consist of bedrock outcrops and surficial layers of topsoil over bedrock, while the areas of the north abutment and north pier consist of sand fill and rock fill over bedrock.

A detailed description of the subsurface conditions encountered in the boreholes and test pits advanced at the abutments, piers and approach areas is provided in the following sections.

4.3 South Abutment and Approach Area

A total of five (5) boreholes (Boreholes B4-16 to B4-20) were advanced at the location of the south abutment and one (1) borehole (Borehole B4-21) was advanced on the centerline at the south approach. In general, the subsurface conditions consist of topsoil, underlain by silty sand at some locations over bedrock.

4.3.1 Topsoil

Up to about 0.1 m of topsoil was encountered at the ground surface at all borehole locations.

4.3.2 Silty Sand

A deposit of silty sand containing trace to some gravel, trace to some clay, rootlets and rock fragments was encountered below the topsoil in Boreholes B4-16 and B4-17. The top of this deposit was encountered at about Elevations 208.2 m and 207.4 m and its thickness is about 1.0 m and 0.2 m at the respective boreholes.

The Standard Penetration Test (SPT) 'N'-values measured within this deposit range from about 1 blow to 23 blows per 0.3 m of penetration, indicating a very loose to compact relative density.

The natural water content measured on two (2) samples of this deposit is about 14 percent and 64 percent with the higher water content value indicating the presence of organics.

4.3.3 Bedrock

Bedrock was encountered and core samples were recovered below the topsoil in Borehole B4-18. The presence of bedrock was inferred by refusal to split-spoon advancement in Boreholes B4-16 and B4-17 and by exposure in hand excavations at B4-19 to B4-21.

The depth to bedrock below ground surface and corresponding bedrock surface elevation is summarized below.



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Foundation Element / Approach Area	Borehole No.	Depth to Bedrock Surface (m)	Bedrock Surface Elevation (m)	Refusal Type
South Approach	B4-21	0.1	212.8	Hand Shovel
	B4-16	1.1	207.2	Split-Spoon
South Abutment	B4-17	0.3	207.2	Split-Spoon
	B4-18	0.1	207.1	Bedrock Cored
	B4-19	0.1	208.1	Hand Shovel
	B4-20	0.1	206.4	Hand Shovel

Across the east edge of the south abutment footprint (a distance of approximately 4.0 m between borehole locations), the bedrock surface elevation varies by about 1.7 m corresponding to an approximately 2.4H:1V slope or a dip of approximately 23° from the horizontal.

Based on the bedrock core samples, the bedrock consists of granite gneiss. In general the bedrock samples are described as slightly weathered, coarse grained, highly foliated, slightly to moderately porous, dark to light grey and containing quartz veins. The Rock Quality Designation (RQD) measured on the core samples is between 83 percent to 100 percent, indicating a rock mass of good to excellent quality, according to Table 3.10 in CFEM (2006)³. The Total Core Recovery (TCR) and Solid Core Recovery (SCR) of the core samples are 100 percent and between 58 percent and 100 percent, respectively.

Point load strength tests were performed on selected samples of the rock core. The diametral point load strength index values are shown on the Record of Drillhole sheets in Appendix A and are presented in Table B1 in Appendix B. The diametral tests carried out on four (4) core samples of the granite gneiss bedrock from this location measured I_{s50} values ranging from about 1.1 MPa to 4.9 MPa.

Also presented in Table B1 are the estimated Unconfined Compressive Strength (UCS) values for each sample tested for point load strength based on a relationship between I_{s50} and UCS which is given by a correlation factor (K) in accordance with ASTM D5731 Standard Test Method for Determination of the Point Load Strength Index of Rock and Application to Rock Strength Classification, which may vary depending on the size of the core sample and the strength of the rock. For this site, the UCS values are based on an estimated average correlation factor (K) of 11, which was calculated based on a comparison of the two UC test results and the average of nine (9) of the corresponding point load strength test results. These values have been given for comparison only and should be interpreted together with the results of the UCS tests.

Based on the point load testing results, according to Table 3.5 in CFEM (2006)³ the granite gneiss bedrock at this location is classified as weak (R2, 5 MPa < UCS < 25 MPa) to medium strong (R3, 25 MPa < UCS < 50 MPa) at this location but is noted to be classified as strong (R4, 50 MPa < UCS < 100 MPa) in immediately adjacent areas.

³ Canadian Geotechnical Society. 2006. *Canadian Foundation Engineering Manual*, 4th Edition. The Canadian Geotechnical Society c/o BiTech Publisher Ltd, British Columbia.



4.3.4 Rock Mapping

Geological mapping of the bedrock outcrops within the vicinity of the existing south approach and abutment was carried out to collect information on the geological conditions of the general area of the south abutment of the Shawanaga River NBL Bridge structure and the Service Road (Site No. 9) bridge structure. The data collected includes estimates of rock strength, fracture characteristics and orientation, and groundwater conditions, which, combined with the geotechnical borehole investigation described in Section 4.3.3, addresses specific rock foundations engineering considerations.

The inspected areas are shown on Figure C1, in Appendix C. The data mapped from the rock outcrops is shown in stereonet format on Figure C2, in Appendix C. Joint condition data is presented below and refers to major joint sets defined on Figure C2.

Joint Set	Spacing (m)*	Surface condition
J1	0.2-3 (2.0)	Planar to Undulating, Slightly Rough to Rough
J2	0.5-2 (1.5)	Planar to Undulating, Smooth to Slightly Rough
J3	0.4-1.5 (1)	Planar to Undulating, Smooth to Slightly Rough

*Average spacing values given in parentheses.

Based on the site mapping (observations), the bedrock outcrops are described as consisting of dark orangeish to reddish grey, slightly weathered, coarse-grained, non-porous, strong to very strong igneous intrusive (granite). The bedrock is faintly foliated parallel to the J2 set and weathering on joint surfaces comprises of orange iron oxide staining or slight alteration. The rock mass mapped was drained above the water line of the adjacent Shawanaga River.

In general the bedrock exhibits strong rock characteristics. The stability of the rock cut slopes (faces) excavated for the bridge abutment will be controlled by the orientation, spacing and persistency of the discontinuities that exist within the rock mass.

The south abutment area bedrock mapping indicates two sub-vertical sets (J1 and J2) and a moderately dipping set J3, as shown on Figure C2. These joints appear to be moderately to widely spaced (0.5 m to greater than 1 m). For a cut face excavated with a strike angle of 245°, the outcrop data presented on Figure C2 suggests that the cut slope will be kinematically favourable with respect to wedge, planar and toppling failure. Should the cut face be oriented to align with the J3 set, there would be increased potential for planar failure along a joint surface with that orientation.

Based on the above observations, pre-supporting the rock face with dowels does not appear necessary for the possible cut orientation shown on Figure C2. Once excavated, the cut face and bench surface should be inspected by the geotechnical engineer to verify rock conditions.

4.3.5 Groundwater Conditions

A standpipe piezometer was installed in Borehole B4-18 to allow monitoring of the groundwater level at this location. Details of the piezometer installations are shown on the Record of Borehole and Drillhole sheets in Appendix A. In general, the overburden samples taken in the boreholes advanced in this area were moist. The groundwater levels measured in the piezometer installation are summarized below.



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Foundation Element	Borehole No.	Ground Surface Elevation (m)	Groundwater Elevation (m)	Date of Measurement
South Abutment	B4-18	207.2	203.3	July 28, 2009
			202.5	August 26, 2009

It should be noted that the groundwater level in the area is subject to seasonal fluctuations due to snow melt and precipitation events. The water level in the adjacent Shawanaga River is also affected by run-off during parts of the year which can influence the groundwater conditions on the adjacent banks. Groundwater levels should be expected to be higher during wet periods of the year.

4.4 South Pier (Pier 3)

A total of five (5) boreholes (Borehole B4-11 to B4-15) were advanced at the location of proposed south pier (Pier 3). In general, the subsurface conditions consist of bedrock (exposed at ground surface) or topsoil over bedrock.

4.4.1 Topsoil

A layer of topsoil up to about 0.2 m thick was encountered at the ground surface in Boreholes B4-13 and B4-14.

4.4.2 Cobbles/Rock Fragments and Bedrock

An approximately 0.5 m thick layer of cobbles/rock fragments was encountered at ground surface in Borehole B4-15. In general, the size of the recovered cobbles/rock fragments ranges from about 38 mm to 102 mm.

Bedrock was encountered and core samples were recovered in Boreholes B4-11 and B4-15, and bedrock was also observed to outcrop at Borehole B4-12. At Boreholes B4-13 and B4-14, the bedrock surface was identified upon being exposed by shovel excavation.

The depth to bedrock below ground surface and corresponding bedrock surface elevation is summarized below.

Foundation Element	Borehole No.	Depth to Bedrock Surface (m)	Bedrock Surface Elevation (m)	Refusal Type
South Pier (Pier 3)	B4-11	0.0	207.6	Bedrock Cored
	B4-12	0.0	204.5	Bedrock Outcrop
	B4-13	0.1	204.9	Hand Shovel
	B4-14	0.2	202.1	Hand Shovel
	B4-15	0.5	201.5	Bedrock Cored

Across the south pier from the southwest corner to the northeast corner of the pier footprint (a distance of approximately 9.5 m between borehole locations), the bedrock surface elevation varies by about 6.1 m corresponding to an approximately 1.6H:1V slope or a dip angle of approximately 32° from the horizontal.



Based on the bedrock core samples, the bedrock consists of granite gneiss. In general the bedrock samples are described as slightly weathered, coarse grained, highly foliated, slightly to moderately porous, dark to light grey. The Rock Quality Designation (RQD) measured on the cores samples is between 65 percent to 100 percent, indicating a rock mass of fair to excellent quality, according to Table 3.10 in CFEM (2006)³, except for the near-surface core sample in Borehole B4-15 where a RQD of 45 percent was recorded, indicating a rock mass of poor quality. The Total Core Recovery (TCR) and Solid Core Recovery (SCR) of core samples are 100 percent and between 33 percent and 100 percent, respectively.

Point load strength tests were performed on selected samples of the rock core. The diametral point load strength index values are shown on the Record of Drillhole sheets in Appendix A and are presented in Table B1 in Appendix B. The diametral tests carried out on nine (9) core samples of the granite gneiss bedrock measured I_{s50} values ranging from about 2.9 MPa to 8.8 MPa.

Also presented in Table B1 are the estimated Uniaxial Compressive Strength (UCS) values for each sample tested for point load strength based on a relationship between I_{s50} and UCS and an estimated average correlation factor (K) of 11, which was calculated based on a comparison of the two UC test results and the average of nine (9) of the corresponding point load strength test results. These values have been given for comparison only and should be interpreted together with the results of the UCS test.

Based on the point load testing results, according to Table 3.5 in CFEM (2006)³ the granite gneiss bedrock at this location is classified as medium strong (R3, 25 MPa < UCS < 50 MPa) to strong (R4, 50 MPa < UCS < 100 MPa).

4.4.3 Groundwater Conditions

A standpipe piezometer was installed in Borehole B4-15 to allow monitoring of the groundwater level at the site. Details of the piezometer installation are shown on the Record of Borehole and Drillhole sheets in Appendix A. In general the overburden samples taken in the boreholes advanced in this area were moist. The groundwater levels measured in the piezometer installation are summarized below.

Foundation Element	Borehole No.	Ground Surface Elevation (m)	Groundwater Elevation (m)	Date of Measurement
South Pier (Pier 3)	B4-15	202.0	202.6*	July 28, 2009
			201.7	August 26, 2009

*Water level above ground surface due to heavy rain.

It should be noted that groundwater level in the area is subject to seasonal fluctuations, snow melt and precipitation events. The water level in the adjacent Shawanaga River is also affected by run-off during parts of the year which can influence the groundwater conditions on the adjacent banks. Groundwater levels should be expected to be higher during wet periods of the year.

4.5 North Pier (Pier 4)

A total of four (4) boreholes (Boreholes B4-05 to B4-08) were drilled and a total of three (3) test pits (Test Pits B4-TP1 to B4-TP3) were excavated at the location of the proposed north pier (Pier 4). In general, the subsurface conditions consist of sand fill and rock fill over bedrock.



4.5.1 Sand Fill

A deposit of fill comprised of sand containing some gravel, trace to some silt, trace clay and occasional cobbles was encountered at the ground surface in all borehole at this location. The top of the sand fill varies between about Elevations 202.4 m and 201.5 m and the thickness ranges from about 1.1 m to 2.2 m. A 0.1 m thick layer of sand and gravel fill containing some silt, organics and wood fragments was encountered in Borehole B4-06 immediately over the bedrock surface.

The SPT 'N'-values measured within the fill deposit range from 5 blows to 20 blows per 0.3 m of penetration, indicating a loose to compact relative density. A SPT 'N'-value of 23 blows per 0.2 m of penetration was recorded in Borehole B4-08 immediately above the rock fill deposit.

The natural water content measured on samples of this fill deposit ranges from about 5 percent to 12 percent.

A grain size distribution of a sample from this deposit is shown on Figure B1-1 in Appendix B.

4.5.2 Rock Fill

Rock fill containing sand, topsoil and rootlets was encountered at the ground surface in Test Pits B4-TP1 to B4-TP3 and below the sand fill in all boreholes except at Borehole B4-06. The top of the rock fill deposit varies between about Elevations 201.4 m and 199.3 m and the thickness of the deposit ranges from about 0.4 m to 2.1 m.

4.5.3 Bedrock

Bedrock was encountered below the rock fill or the sand to sand and gravel fill and core samples were recovered at all borehole locations. The bedrock surface was exposed in all test pits locations.

The depth to bedrock below ground surface and corresponding bedrock surface elevation is summarized below.

Foundation Element	Borehole / Test Pit No.	Depth to Bedrock Surface (m)	Bedrock Surface Elevation (m)	Refusal Type
North Pier (Pier 4)	B4-05	2.6	199.2	Bedrock Cored
	B4-06	1.9	199.6	Bedrock Cored
	B4-07	2.5	199.9	Bedrock Cored
	B4-08	3.1	198.7	Bedrock Cored
	B4-TP1	1.1	198.2	Bucket Refusal
	B4-TP2	2.1	197.5	Bucket Refusal
	B4-TP3	0.8	200.6	Bucket Refusal

Across the north pier from about the centre of the pier to the northwest corner of the pier footprint (a distance of approximately 4 m between test pit locations) the bedrock surface elevation varies by about 3.1 m corresponding to an approximately 1.3H:1V slope or a dip angle of approximately 38° from the horizontal.

Based on the bedrock core samples, the bedrock consists of granite gneiss. In general the bedrock samples are described as slightly weathered to fresh, fine to medium grained with feldspar banding, highly foliated, black, pink and grey colour. The Rock Quality Designation (RQD) measured on the core samples are typically between 30 percent and 80 percent, indicating a rock mass of poor to good quality according to Table 3.10 in



CFEM (2006)³. RQD values between 0 percent and 13 percent were measured on the cores samples from near the bedrock surface, indicating an upper rock mass of poor quality. The Total Core Recovery (TCR) and Solid Core Recovery (SCR) of samples recovered are typically between 74 percent and 100 percent and 11 percent and 80 percent, respectively.

Point load strength tests were performed on selected samples of the rock core. The diametral point load strength index values are shown on the Record of Drillhole sheets in Appendix A and are presented in Table B1 in Appendix B. The diametral tests carried out on six (6) samples of the granite gneiss bedrock measured I_{s50} values ranging from about 6.9 MPa to 11.1 MPa.

One (1) Unconfined Compression (UC) test was carried out in accordance to ASTM D7102 Standard Test Method for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens on a selected core sample of granite gneiss bedrock from Borehole B4-06 and measured a compressive strength of about 78 MPa, as summarized in Table B2-1 and detailed in Table B2-2 in Appendix B.

Also presented in Table B1 are the estimated Uniaxial Compressive Strength (UCS) values for each sample tested for point load strength based on a relationship between I_{s50} and UCS and an estimated average correlation factor (K) of 11, which was calculated based on a comparison of the two UC test results and the average of nine (9) of the corresponding point load strength test results. These values have been given for comparison only and should be interpreted together with the results of the UCS test.

Based on the laboratory UC test and the point load test results, according to Table 3.5 in CFEM (2006)³ the granite gneiss bedrock at this location is classified as strong (R4, 50 MPa < UCS < 100 MPa) to very strong (R5, 100 MPa < UCS < 250 MPa).

4.5.4 Groundwater Conditions

A standpipe piezometer was installed in Borehole B4-07 to allow monitoring the groundwater level at the site. Details of the piezometer installation are shown on the Record of the Borehole and Drillhole sheets in Appendix A. In general the overburden samples taken in the boreholes advanced in this area were moist. The groundwater levels measured in the piezometer installation are summarized below.

Foundation Element	Borehole No.	Ground Surface Elevation (m)	Groundwater Elevation (m)	Date of Measurement
North Pier (Pier 4)	B4-07	202.4	199.2	March 26, 2009
			199.2	April 16, 2009
			198.5	August 26, 2009

It should be noted that groundwater level in the area is subject to seasonal fluctuations, snow melt and precipitation events. The water level in the adjacent Shawanaga River is also affected by run-off during parts of the year which can influence the groundwater conditions on the adjacent banks. Groundwater levels should be expected to be higher during wet periods of the year.

4.6 North Abutment and Approach Embankment

A total of five (5) boreholes (Boreholes B4-01 to B4-03, B4-09 and B4-10) were advanced at the location of the proposed north abutment and one (1) borehole (Borehole B4-04) was advanced on the west side of the north



approach. In general, the subsurface conditions consist of sand fill and rock fill underlain in places by layers of sand to sand and silt or clayey silt to silt, over bedrock.

4.6.1 Asphalt

An approximately 0.2 m thick layer of asphalt was encountered at the ground surface in Boreholes B4-09 and B4-10.

4.6.2 Topsoil

An approximately 0.1 m thick layer of topsoil was encountered at the ground surface in Borehole B4-03.

4.6.3 Sand to Sand and Gravel Fill

A deposit of fill comprised of sand containing trace gravel, trace silt, cobbles, boulders and organics to sand and gravel containing trace silt, organics and wood fragments was encountered at the ground surface in Borehole B4-01 and underlying the asphalt layer in Boreholes B4-09 and B4-10. The top of this fill deposit varies between about Elevations 205.3 m and 203.5 m and the thickness of this deposit ranges from about 0.7 m to 2.1 m.

A 0.5 m thick deposit of fill comprised of silt, some sand, trace to some clay, containing organics was encountered in Borehole B4-04 at ground surface.

The SPT 'N'-values measured within the sand to sand and gravel fill typically range from 20 blows to 98 blows per 0.3 m of penetration indicating a compact to very dense relative density. The higher blow counts measured are attributed to the presence of cobbles or boulders. A SPT 'N'-value of 5 blows per 0.3 m of penetration was recorded with the silt fill, indicating a loose relative density.

The natural water content measured on samples of the sand fill ranges from about 4 percent to 15 percent while the natural water content of about 22 percent was measured on a sample of the silt fill.

4.6.4 Rock Fill

Rock fill was encountered underlying the sand fill deposit in Borehole B4-01. The top of rock fill deposit was encountered at about Elevation 202.8 m and its thickness is about 2.0 m.

4.6.5 Sand to Sand and Silt

A cohesionless deposit comprised of sand containing trace gravel, organics and rootlets to sand and silt containing trace to some gravel and trace clay was encountered at the ground surface and below the sand and gravel fill in Boreholes B4-02 and B4-09, respectively. The top of this deposit was encountered at about Elevations 201.7 m and 203.0 m and its thickness is about 0.7 m and 0.6 m in the respective boreholes.

Two (2) SPT 'N'-values measured within this deposit are 4 blows and 5 blows per 0.3 m of penetration, indicating a loose relative density.



The natural water contents measured on two (2) samples within the sand and silt to sand deposit are about 16 percent and 21 percent.

The grain size distribution of a sample of this deposit is shown on Figure B1-2 in Appendix B.

4.6.6 Clayey Silt

A cohesive stratum comprised of clayey silt containing trace sand, organics and rootlets was encountered underlying the topsoil in Borehole B4-03. The top of the clayey silt stratum is at about Elevation 201.1 m and its thickness is about 0.6 m.

An SPT 'N'-value measured within clayey silt stratum is 4 blows per 0.3 m of penetration, suggesting a soft to firm consistency.

The natural water content and organic content measured on a sample of clayey silt is about 30 percent and 4 percent, respectively.

An Atterberg limits test was carried out on a sample of the clayey silt and yielded liquid and plastic limit of about 24 percent and 18 percent, respectively, corresponding a plasticity index of about 6 percent. The result of the Atterberg limits test is shown on the plasticity chart on Figure B1-3 in Appendix B and indicates that this material is a clayey silt of low plasticity.

4.6.7 Silt

A 0.7 m thick deposit of silt, trace sand and trace clay was encountered underlying the cohesive layer in Borehole B4-03, at about Elevation 200.5 m.

An SPT 'N'-value measured within the silt deposit is 26 blows per 0.3 m of penetration, indicating a compact relative density.

4.6.8 Cobbles and Boulders

A deposit of cobbles and boulders was encountered below the silt deposit in Borehole B4-03. The cobbles and boulders were encountered at about Elevation 199.8 m and the deposit is about 0.9 m thick.

4.6.9 Bedrock

Bedrock was encountered and core samples were recovered all boreholes at this location. The depth to the bedrock surface from the ground surface and the corresponding bedrock elevation is summarized below.



FOUNDATION REPORT – SHAWANAGA RIVER NBL BRIDGE STRUCTURE – HIGHWAY 69 G.W.P. 5111-07-00

Foundation Element / Approach Embankment	Borehole No.	Depth to Bedrock Surface (m)	Bedrock Surface Elevation (m)	Refusal Type
North Abutment	B4-01	2.7	200.8	Bedrock Cored
	B4-02	0.7	201.0	Bedrock Cored
	B4-03	2.3	198.9	Bedrock Cored
	B4-09	2.9	202.4	Bedrock Cored
	B4-10	2.3	203.2	Bedrock Cored
North Approach Embankment	B4-04	0.5	199.7	Bedrock Cored

Across the north abutment from the northwest corner to the southwest corner of the abutment footprint (a distance of approximately 4.7 m between borehole locations) the bedrock surface varies in elevation by about 2.1 m corresponding to an approximately 2.2H:1V slope or a dip angle of about approximately 24° from the horizontal.

Based on the cored samples of bedrock, the bedrock consists of granite gneiss bedrock. In general, the bedrock samples are described as fresh to slightly weathered, fine to medium grained with feldspar banding, faintly to moderately porous, weakly to moderately foliated, black, pink and grey colour. In general, the Rock Quality Designation (RQD) measured on the core samples ranges between about 54 percent and 100 percent, with values typically increasing with depth, indicating a rock mass of fair quality to excellent quality, according to Table 3.10 in CFEM (2006)³. RQD values ranging between about 0 percent and 39 percent were encountered on the upper portions of the bedrock in some boreholes, indicating an upper rock mass of very poor to poor quality at some locations. The Total Core Recovery (TCR) and Solid Core Recovery (SCR) of the core samples are typically between about 75 percent and 100 percent and 30 percent and 100 percent, respectively. TCR and SCR values ranging between 40 percent and 58 percent and between 0 percent and 17 percent, respectively, were measured on some of the cored samples generally corresponding to the zones of lower RQD.

Point load strength tests were performed on selected sample of the rock core. The axial and diametral point load strength index values are shown on the Record of the Drillhole sheets in Appendix A and in Table B1 in Appendix B. The axial point load index (Is_{50}) results from the laboratory tests carried out on three (3) core samples of the granite gneiss bedrock range from approximately 10.3 MPa to 10.8 MPa. The diametral tests carried out on fifteen (15) core samples of granite gneiss bedrock measured Is_{50} values ranging from about 4.6 MPa to 8.6 MPa.

One (1) Unconfined Compression (UC) test was carried out in accordance to ASTM D7102 Standard Test Method for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens, on a selected core sample of the granite gneiss bedrock from Borehole B4-09 and measured a compressive strength of about 86 MPa, as summarized in Table B2-1 and detailed in Table B2-3 in Appendix B.

Also presented in Table B1 are the estimated Uniaxial Compressive Strength (UCS) values for each sample tested for point load strength based on a relationship between Is_{50} and UCS and an estimated average correlation factor (K) of 11, which was calculated based on a comparison of the two UC test results and the average of nine (9) of the corresponding point load strength test results. These values have been given for comparison only and should be interpreted together with the results of the UCS test.



Based on the laboratory UC test and the point load test results, according to Table 3.5 in CFEM (2006)³ the granite gneiss bedrock is classified as strong (R4, 50 MPa < UCS < 100 MPa) to very strong (R5, 100 MPa < UCS < 250 MPa).

4.6.10 Groundwater Conditions

A standpipe piezometer was installed in Borehole B4-01 to allow monitoring of the groundwater level at the site. Details of the piezometer installations are shown the Record of Borehole and Drillhole sheets in Appendix A. In general, the overburden samples taken in the boreholes advanced in this area were moist to wet. The groundwater levels measured in the piezometer installation are summarized below.

Foundation Element	Borehole No.	Ground Surface Elevation (m)	Groundwater Elevation (m)	Date of Measurement
North Abutment	B4-01	203.5	202.0 201.6	April 16, 2009 August 26, 2009

It should be noted that groundwater level in the area is subject to seasonal fluctuations, snow melt and precipitation events. The water level in the adjacent Shawanaga River is also affected by run-off during parts of the year which can influence the groundwater conditions on the adjacent banks. Groundwater levels should be expected to be higher during wet periods of the year.

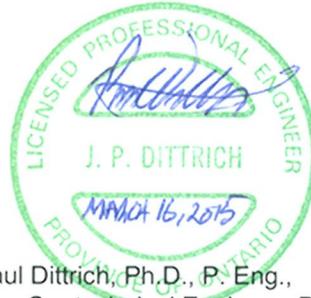
5.0 CLOSURE

Messrs. Matt Rhody, Chris Radway and Indulis Dumpis, senior technicians with Golder, directed the drilling program. Messrs. Marc Rougier, David Chesser and Adam Horwitz completed the field work for rock mapping. This report was prepared by Ms. T. Veronica Ayetan, P.Eng. and Mr. Christopher Ng, P.Eng., and was reviewed by Mr. J. Paul Dittrich, Ph.D., P.Eng., a senior geotechnical engineer and Principal with Golder. Mr. Jorge M. A. Costa, P.Eng., Golder's Designated MTO Contact for this project and Principal with Golder, conducted an independent quality control review of the report.



Report Signature Page

Christopher Ng, P. Eng.
Geotechnical Engineer, Associate



J. Paul Dittrich, Ph.D., P. Eng.,
Senior Geotechnical Engineer, Principal



Jorge M. A. Costa, P. Eng.
Designated MTO Contact, Principal

TVA/CN/JPD/JMAC/J

\\golder.gds\gal\mississauga\active\2007\1111\07-1111-0029 - mrc - hwy 69 four-laning -report\final\9 - shawanaga river nbl bridge\07-1111-0029-9 fidr
15mar12 highway 69 shawanaga river nbl bridge structure.docx



DRAWINGS

METRIC
 DIMENSIONS ARE IN METRES AND/OR
 MILLIMETRES UNLESS OTHERWISE SHOWN.
 STATIONS IN KILOMETRES + METRES.

CONT No.
 WP No. 5187-06-01



HIGHWAY 69
 SITE LOCATION PLAN

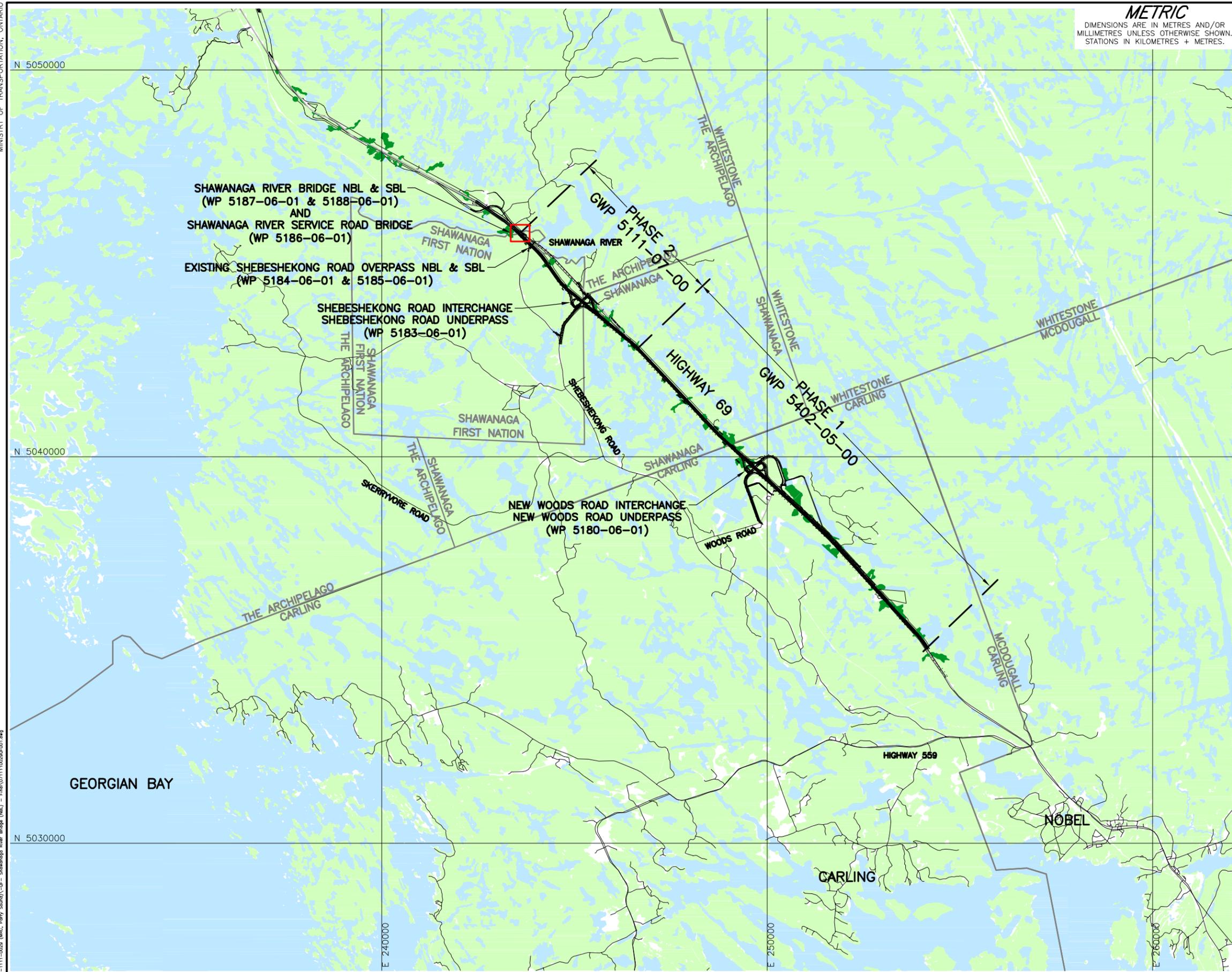
SHEET



Golder Associates Ltd.
 MISSISSAUGA, ONTARIO, CANADA



KEY PLAN
 NOT TO SCALE



PLAN



P:\GIS\NET\work\1_2015\PROJECTS\5187-06-01\1111-0029 (MTRC_Parry_Survey)_06- Shawanaga River Bridge (NBL) - Final\0711102029001.dwg

REFERENCE
 Base Data - MNR NRVIS, obtained 2004, CANMAP v2006.4
 Produced by Golder Associates Ltd under licence from
 Ontario Ministry of Natural Resources, © Queens Printer 2008
 Datum : NAD 83 Projection : MTM Zone 10

NO.	DATE	BY	REVISION
Geocres No. 41H-145			
HWY. 69			PROJECT NO. 07-1111-0029 DIST.
SUBM'D. VA	CHKD. VA	DATE: Mar. 2015	SITE: 44-443/1
DRAWN: DD/CD	CHKD. CN	APPD. JPD/JMAC	DWG. 1

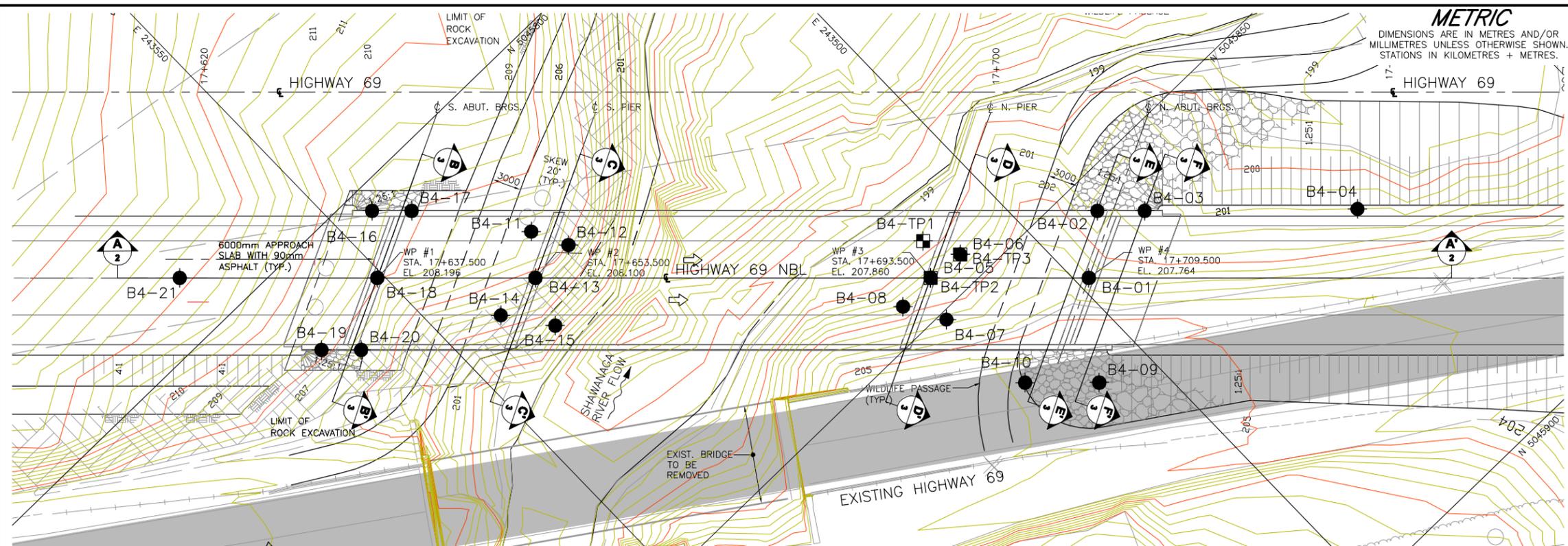
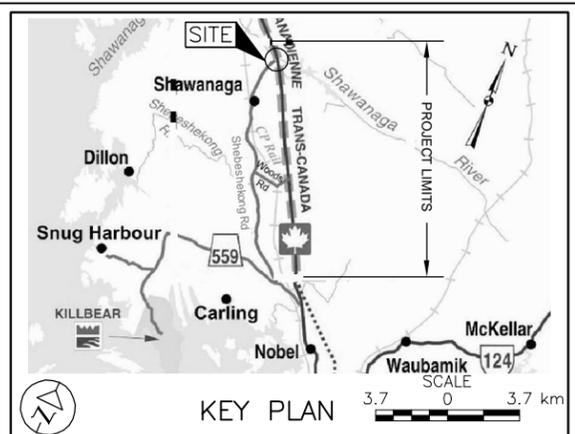
METRIC
DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN. STATIONS IN KILOMETRES + METRES.

CONT No. 5187-06-01
WP No. 5187-06-01



HIGHWAY 69
SHAWANAGA RIVER BRIDGE (NBL)
BOREHOLE LOCATIONS
AND SOIL STRATA

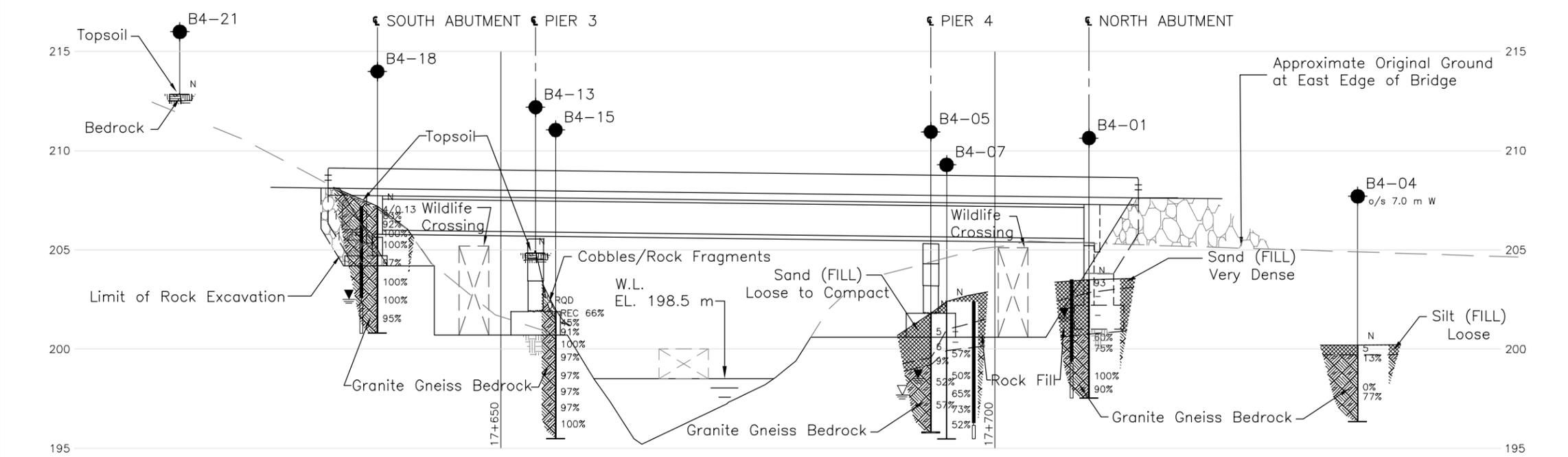
SHEET



PLAN
SCALE
5 0 5 10 m

LEGEND

- Borehole - Current Investigation
- ⊠ Test Pit - Approximate Location
- ▬ Seal
- ▬ Piezometer
- N Standard Penetration Test Value
- 16 Blows/0.3m unless otherwise stated (Std. Pen. Test, 475 j/blow)
- 100% Rock Quality Designation (RQD)
- REC Recovery
- ▬ WL in piezometer, measured on 26/08/2009
- ▬ WL upon completion of drilling
- R Refusal



A-A' CENTRELINE PROFILE
SCALE
HORIZONTAL SCALE: 5 0 5 10 m
VERTICAL SCALE: 2.5 0 2.5 5 m

NOTES

This drawing is for subsurface information only. The proposed structure details/works are shown for illustration purposes only and may not be consistent with the final design configuration as shown elsewhere in the Contracts Documents.

The boundaries between soil strata have been established only at borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

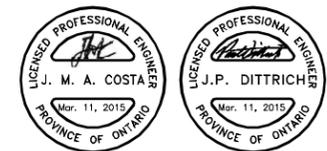
The complete foundation investigation and design report for this project and other related documents may be examined at the Materials Engineering and Research Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with Section GC 2.01 of OPS General Conditions.

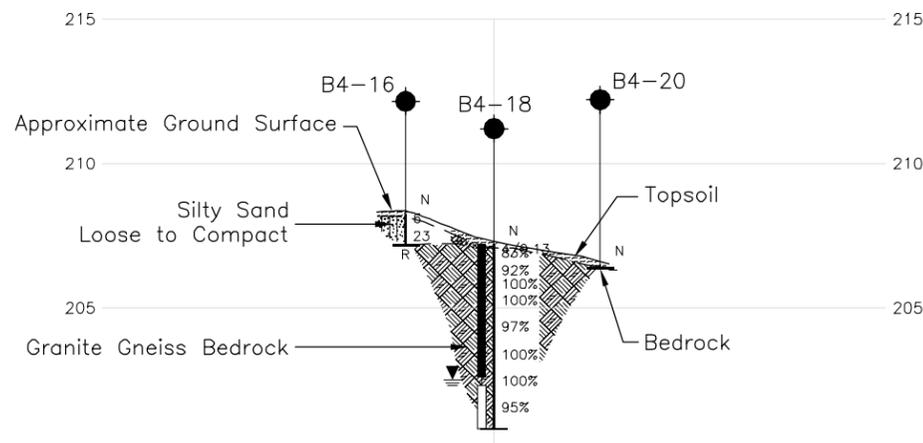
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		NORTHING	EASTING
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B4-TP3	201.4	5045847.0	243506.5
B4-01	203.5	5045857.8	243498.8
B4-02	201.7	5045853.5	243493.5
B4-03	201.2	5045856.8	243490.0
B4-04	200.2	5045871.6	243474.4
B4-05	201.8	5045846.6	243510.3
B4-06	201.5	5045847.0	243506.5
B4-07	202.4	5045850.8	243512.1
B4-08	201.8	5045846.8	243514.4
B4-09	205.3	5045866.1	243505.4
B4-10	205.5	5045860.9	243510.8
B4-11	207.6	5045815.2	243536.2
B4-12	204.5	5045818.8	243534.4
B4-13	205.0	5045818.8	243539.1
B4-14	202.3	5045819.1	243544.2
B4-15	202.0	5045823.7	243541.0
B4-16	208.3	5045802.5	243546.3
B4-17	207.5	5045805.3	243543.4
B4-18	207.2	5045807.7	243550.6
B4-19	208.2	5045809.0	243559.7
B4-20	206.5	5045811.8	243556.8
B4-21	212.9	5045793.8	243565.0

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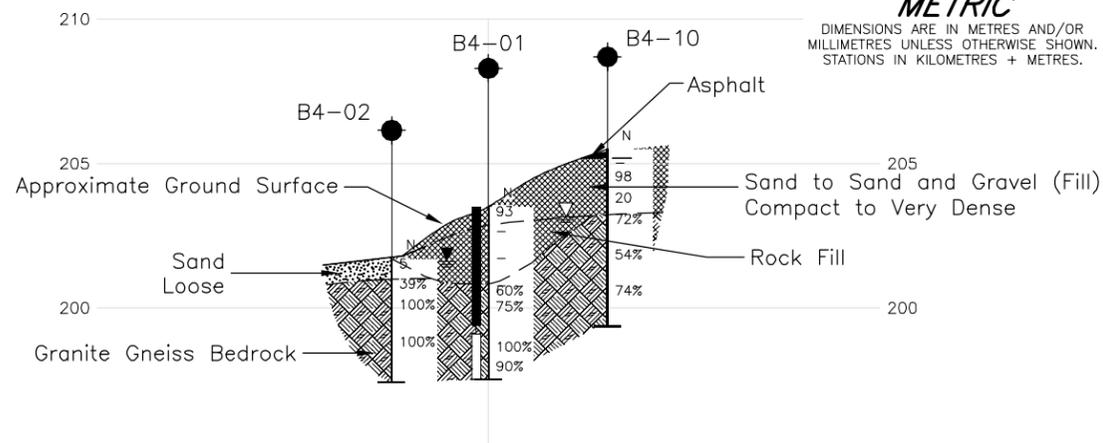
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NO.	DATE	BY	REVISION
Geocres No. 41H-145			
HWY. 69		PROJECT NO. 07-1111-0029	DIST.
SUBM'D. VA	CHKD. VA/OK	DATE: Mar. 2015	SITE: 44-443/1
DRAWN: RJ/JS	CHKD. CN	APPD. JPD/JMAC	DWG. 2

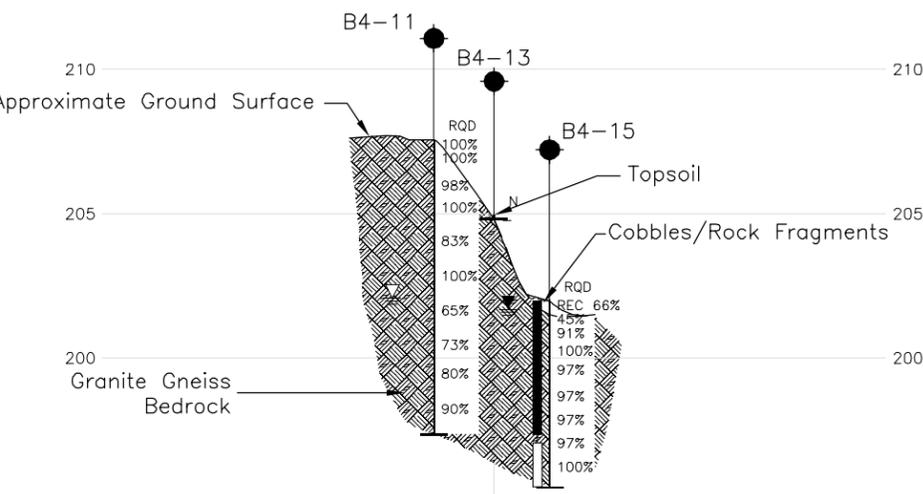




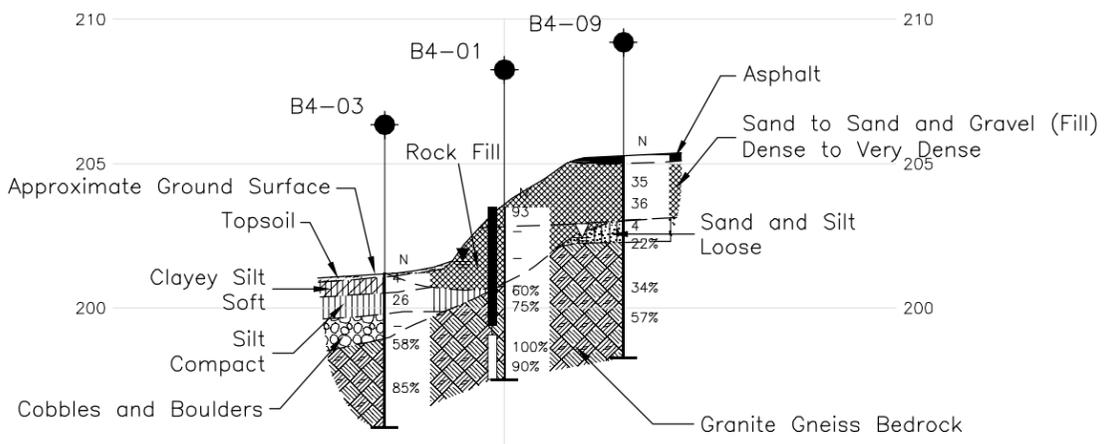
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 SOUTH ABUTMENT
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 VERTICAL SCALE: 1:100



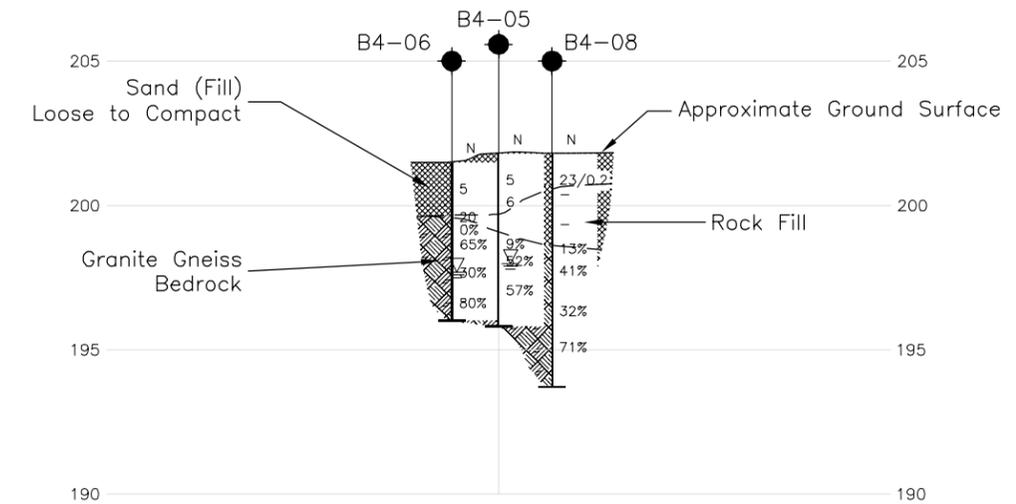
E-E' 2
 NORTH ABUTMENT (FRONT)
 HORIZONTAL SCALE: 1:1000
 VERTICAL SCALE: 1:100



C-C' 2
 PIER 3
 HORIZONTAL SCALE: 1:1000
 VERTICAL SCALE: 1:100



F-F' 2
 NORTH ABUTMENT (BACK)
 HORIZONTAL SCALE: 1:1000
 VERTICAL SCALE: 1:100



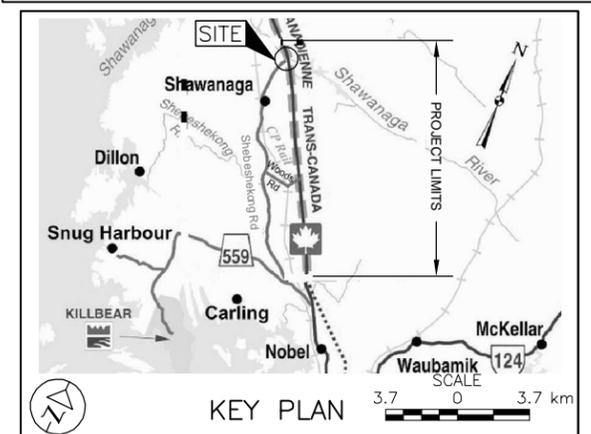
D-D' 2
 PIER 4
 HORIZONTAL SCALE: 1:1000
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METRIC
 DIMENSIONS ARE IN METRES AND/OR MILLIMETRES UNLESS OTHERWISE SHOWN. STATIONS IN KILOMETRES + METRES.

CONT No.
WP No. 5187-06-01

HIGHWAY 69
SHAWANAGA RIVER BRIDGE (NBL)
SOIL STRATA

SHEET



LEGEND

- Borehole - Current Investigation
- ⊥ Seal
- ⊥ Piezometer
- N Standard Penetration Test Value
- 16 Blows/0.3m unless otherwise stated (Std. Pen. Test, 475 j/blow)
- 100% Rock Quality Designation (RQD)
- REC Recovery
- ⊥ WL in piezometer, measured on 26/08/2009
- ⊥ WL upon completion of drilling
- R Refusal

No.	ELEVATION	CO-ORDINATES	
		NORTHING	EASTING
B4-01	203.5	5045857.8	243498.8
B4-02	201.7	5045853.5	243493.5
B4-03	201.2	5045856.8	243490.0
B4-05	201.8	5045846.6	243510.3
B4-06	201.5	5045847.0	243506.5
B4-08	201.8	5045846.8	243514.4
B4-09	205.3	5045866.1	243505.4
B4-10	205.5	5045860.9	243510.8
B4-11	207.6	5045815.2	243536.2
B4-13	205.0	5045818.8	243539.1
B4-15	202.0	5045823.7	243541.0
B4-16	208.3	5045802.5	243546.3
B4-17	207.5	5045805.3	243543.4
B4-18	207.2	5045807.7	243550.6
B4-19	208.2	5045809.0	243559.7
B4-20	206.5	5045811.8	243556.8

NOTES

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The complete foundation investigation and design report for this project and other related documents may be examined at the Materials Engineering and Research Office, Downsview. Information contained in this report and related documents is specifically excluded in accordance with Section GC 2.01 of OPS General Conditions.



REFERENCE

Base plans provided in digital format by MRC, drawing files 271XB01.DWG, 5271-XPD-SHAWANAGA.dwg, PR # 5377-02-00-PR-1.dwg, received October 1, 2007, and S6878-305-001GA.dwg, received January 15, 2009.

NO.	DATE	BY	REVISION
Geocres No. 41H-145			
HWY. 69			PROJECT NO. 07-1111-0029 DIST.
SUBM'D. VA	CHKD. VA/OK	DATE: Mar. 2015	SITE: 44-443/1
DRAWN: RJ/JS	CHKD. CN	APPD. JPD/JMAC	DWG. 3



APPENDIX A

RECORD OF BOREHOLES, DRILLHOLES and FIELD TEST PIT LOGS



LIST OF SYMBOLS

Unless otherwise stated, the symbols employed in the report are as follows:

I.	GENERAL	(a)	Index Properties (continued)
π	3.1416	w	water content
$\ln x$,	natural logarithm of x	w_l or LL	liquid limit
\log_{10}	x or log x, logarithm of x to base 10	w_p or PL	plastic limit
g	acceleration due to gravity	I_p or PI	plasticity index = $(w_l - w_p)$
t	time	w_s	shrinkage limit
FoS	factor of safety	I_L	liquidity index = $(w - w_p) / I_p$
		I_C	consistency index = $(w_l - w) / I_p$
		e_{max}	void ratio in loosest state
		e_{min}	void ratio in densest state
		I_D	density index = $(e_{max} - e) / (e_{max} - e_{min})$ (formerly relative density)
II.	STRESS AND STRAIN	(b)	Hydraulic Properties
γ	shear strain	h	hydraulic head or potential
Δ	change in, e.g. in stress: $\Delta \sigma$	q	rate of flow
ε	linear strain	v	velocity of flow
ε_v	volumetric strain	i	hydraulic gradient
η	coefficient of viscosity	k	hydraulic conductivity (coefficient of permeability)
ν	Poisson's ratio	j	seepage force per unit volume
σ	total stress	(c)	Consolidation (one-dimensional)
σ'	effective stress ($\sigma' = \sigma - u$)	C_c	compression index (normally consolidated range)
σ'_{vo}	initial effective overburden stress	C_r	recompression index (over-consolidated range)
$\sigma_1, \sigma_2, \sigma_3$	principal stress (major, intermediate, minor)	C_s	swelling index
σ_{oct}	mean stress or octahedral stress = $(\sigma_1 + \sigma_2 + \sigma_3)/3$	C_α	secondary compression index
τ	shear stress	m_v	coefficient of volume change
u	porewater pressure	c_v	coefficient of consolidation (vertical direction)
E	modulus of deformation	C_h	coefficient of consolidation (horizontal direction)
G	shear modulus of deformation	T_v	time factor (vertical direction)
K	bulk modulus of compressibility	U	degree of consolidation
III.	SOIL PROPERTIES	σ'_p	pre-consolidation stress
(a)	Index Properties	OCR	over-consolidation ratio = σ'_p / σ'_{vo}
$\rho(\gamma)$	bulk density (bulk unit weight)*	(d)	Shear Strength
$\rho_d(\gamma_d)$	dry density (dry unit weight)	τ_p, τ_r	peak and residual shear strength
$\rho_w(\gamma_w)$	density (unit weight) of water	ϕ'	effective angle of internal friction
$\rho_s(\gamma_s)$	density (unit weight) of solid particles	δ	angle of interface friction
γ'	unit weight of submerged soil ($\gamma' = \gamma - \gamma_w$)	μ	coefficient of friction = $\tan \delta$
D_R	relative density (specific gravity) of solid particles ($D_R = \rho_s / \rho_w$) (formerly G_s)	c'	effective cohesion
e	void ratio	c_u, s_u	undrained shear strength ($\phi = 0$ analysis)
n	porosity	p	mean total stress $(\sigma_1 + \sigma_3)/2$
S	degree of saturation	p'	mean effective stress $(\sigma'_1 + \sigma'_3)/2$
		q	$(\sigma_1 - \sigma_3)/2$ or $(\sigma'_1 - \sigma'_3)/2$
		q_u	compressive strength $(\sigma_1 - \sigma_3)$
		S_t	sensitivity
		Notes: 1	$\tau = c' + \sigma' \tan \phi'$
		2	shear strength = (compressive strength)/2



LIST OF ABBREVIATIONS

The abbreviations commonly employed on Records of Boreholes, on figures and in the text of the report are as follows:

I. SAMPLE TYPE

AS	Auger sample
BS	Block sample
CS	Chunk sample
DS	Denison type sample
FS	Foil sample
RC	Rock core
SC	Soil core
SS	Split-spoon
ST	Slotted tube
TO	Thin-walled, open
TP	Thin-walled, piston
WS	Wash sample

II. PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg. (140 lb.) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) drive open sampler for a distance of 300 mm (12 in.)

Dynamic Cone Penetration Resistance; N_d :

The number of blows by a 63.5 kg (140 lb.) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

PH: Sampler advanced by hydraulic pressure

PM: Sampler advanced by manual pressure

WH: Sampler advanced by static weight of hammer

WR: Sampler advanced by weight of sampler and rod

Piezo-Cone Penetration Test (CPT)

A electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (Q_t), porewater pressure (PWP) and friction along a sleeve are recorded electronically at 25 mm penetration intervals.

III. SOIL DESCRIPTION

(a) Cohesionless Soils

Density Index	N
Relative Density	Blows/300 mm or Blows/ft
Very loose	0 to 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very dense	over 50

(b) Cohesive Soils Consistency

	kPa	C_u, S_u	psf
Very soft	0 to 12		0 to 250
Soft	12 to 25		250 to 500
Firm	25 to 50		500 to 1,000
Stiff	50 to 100		1,000 to 2,000
Very stiff	100 to 200		2,000 to 4,000
Hard	over 200		over 4,000

IV. SOIL TESTS

w	water content
w _p	plastic limit
w _l	liquid limit
C	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test ¹
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
D _R	relative density (specific gravity, G _s)
DS	direct shear test
M	sieve analysis for particle size
MH	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO ₄	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V	field vane (LV-laboratory vane test)
γ	unit weight

Note: 1 Tests which are anisotropically consolidated prior to shear are shown as CAD, CAU.

V. MINOR SOIL CONSTITUENTS

Percent by Weight	Modifier	Example
0 to 5	Trace	Trace sand
5 to 12	Trace to Some (or Little)	Trace to some sand
12 to 20	Some	Some sand
20 to 30	(ey) or (y)	Sandy
over 30	And (cohesionless) or With (cohesive)	Sand and Gravel Silty Clay with sand / Clayey Silt with sand



LITHOLOGICAL AND GEOTECHNICAL ROCK DESCRIPTION TERMINOLOGY

WEATHERING STATE

Fresh: no visible sign of weathering

Faintly weathered: weathering limited to the surface of major discontinuities.

Slightly weathered: penetrative weathering developed on open discontinuity surfaces but only slight weathering of rock material.

Moderately weathered: weathering extends throughout the rock mass but the rock material is not friable.

Highly weathered: weathering extends throughout rock mass and the rock material is partly friable.

Completely weathered: rock is wholly decomposed and in a friable condition but the rock and structure are preserved.

BEDDING THICKNESS

<u>Description</u>	<u>Bedding Plane Spacing</u>
Very thickly bedded	Greater than 2 m
Thickly bedded	0.6 m to 2 m
Medium bedded	0.2 m to 0.6 m
Thinly bedded	60 mm to 0.2 m
Very thinly bedded	20 mm to 60 mm
Laminated	6 mm to 20 mm
Thinly laminated	Less than 6 mm

JOINT OR FOLIATION SPACING

<u>Description</u>	<u>Spacing</u>
Very wide	Greater than 3 m
Wide	1 m to 3 m
Moderately close	0.3 m to 1 m
Close	50 mm to 300 mm
Very close	Less than 50 mm

GRAIN SIZE

<u>Term</u>	<u>Size*</u>
Very Coarse Grained	Greater than 60 mm
Coarse Grained	2 mm to 60 mm
Medium Grained	60 microns to 2 mm
Fine Grained	2 microns to 60 microns
Very Fine Grained	Less than 2 microns

Note: * Grains greater than 60 microns diameter are visible to the naked eye.

CORE CONDITION

Total Core Recovery (TCR)

The percentage of solid drill core recovered regardless of quality or length, measured relative to the length of the total core run.

Solid Core Recovery (SCR)

The percentage of solid drill core, regardless of length, recovered at full diameter, measured relative to the length of the total core run.

Rock Quality Designation (RQD)

The percentage of solid drill core, greater than 100 mm length, recovered at full diameter, measured relative to the length of the total core run. RQD varied from 0% for completely broken core to 100% for core in solid sticks.

DISCONTINUITY DATA

Fracture Index

A count of the number of discontinuities (physical separations) in the rock core, including both naturally occurring fractures and mechanically induced breaks caused by drilling.

Dip with Respect to Core Axis

The angle of the discontinuity relative to the axis (length) of the core. In a vertical borehole a discontinuity with a 90° angle is horizontal.

Description and Notes

An abbreviation description of the discontinuities, whether naturally occurring separations such as fractures, bedding planes and foliation planes or mechanically induced features caused by drilling such as ground or shattered core and mechanically separated bedding or foliation surfaces. Additional information concerning the nature of fracture surfaces and infillings are also noted.

Abbreviations

JN Joint	PL Planar
FLT Fault	CU Curved
SH Shear	UN Undulating
VN Vein	IR Irregular
FR Fracture	K Slickensided
SY Stylolite	PO Polished
BD Bedding	SM Smooth
FO Foliation	SR Slightly Rough
CO Contact	RO Rough
AXJ Axial Joint	VR Very Rough
KV Karstic Void	
MB Mechanical Break	

FIELD TEST PIT LOG

JOB NUMBER:	07-1111-0029	JOB NAME:	MRC / Highway 69 Four-Laning / Shawanaga NBL Structure	DATE:	Mar 21, 2009
TEST PIT NUMBER:	B4-TP1	APPROXIMATE LOCATION:	N 5045843.4; E 243508.3	APPROXIMATE ELEVATION:	199.3 m
MACHINE TYPE:	240 John Deere Excavator	TEST PIT SIZE:	N/A	DATUM:	Geodetic
TEMP/WEATHER:	Sunny, 2.5°C	CONTRACTOR:	Weeks Construction Inc.		

Elev.		Soil Description	Samples		Remarks
Depth			No.	Depth (m)	
From (m)	To (m)				
0.0	1.1	Rock fill, containing topsoil and rootlets	--	--	Exposed bedrock at a depth of 1.1 m
198.2					
1.1		End of Test Pit Bedrock			

Comments:

Water Conditions in Test Pit: Dry



JOB No.	07-1111-0029
TEST PIT No.:	B4-TP1
TECHNICIAN:	MR
COMPILED BY:	VA

FIELD TEST PIT LOG

JOB NUMBER:	07-1111-0029	JOB NAME:	MRC / Highway 69 Four-Laning / Shawanaga NBL Structure	DATE:	Mar 21, 2009
TEST PIT NUMBER:	B4-TP2	APPROXIMATE LOCATION:	N 5045846.6; E 243510.3	APPROXIMATE ELEVATION:	199.6 m
MACHINE TYPE:	240 John Deere Excavator	TEST PIT SIZE:	N/A	DATUM:	Geodetic
TEMP/WEATHER:	Sunny, 2.5°C	CONTRACTOR:	Weeks Construction Inc.		

Elev.		Soil Description	Samples		Remarks
Depth			No.	Depth (m)	
From (m)	To (m)				
0.0	2.1	Rock fill, containing sand, topsoil and rootlets	--	--	Exposed bedrock at a depth of 2.1 m
197.5					
2.1		End of Test Pit Bedrock			

Comments:

Water Conditions in Test Pit: Dry

For additional soil and bedrock details, see Record of Borehole B4-05



JOB No.	07-1111-0029
TEST PIT No.:	B4-TP2
TECHNICIAN:	MR
COMPILED BY:	VA

FIELD TEST PIT LOG

JOB NUMBER:	07-1111-0029	JOB NAME:	MRC / Highway 69 Four-Laning / Shawanaga NBL Structure	DATE:	Mar 21, 2009
TEST PIT NUMBER:	B4-TP3	APPROXIMATE LOCATION:	N 5045847.0; E 243506.5	APPROXIMATE ELEVATION:	201.4 m
MACHINE TYPE:	240 John Deere Excavator	TEST PIT SIZE:	N/A	DATUM:	Geodetic
TEMP/WEATHER:	Sunny, 2.5°c	CONTRACTOR:	Weeks Construction Inc.		

Elev.		Soil Description	Samples		Remarks
Depth			No.	Depth (m)	
From (m)	To (m)				
0.0	0.8	Rock fill, containing topsoil and rootlets	--	--	Exposed bedrock at a depth of 0.8 m
200.6					
0.8		End of Test Pit Bedrock			

Comments:

Water Conditions in Test Pit: Dry

For additional soil and bedrock details, see Record of Borehole B4-06



JOB No.	07-1111-0029
TEST PIT No.:	B4-TP3
TECHNICIAN:	MR
COMPILED BY:	VA

PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-01** **SHEET 1 OF 1** **METRIC**
W.P. 5187-06-01 **LOCATION** N 5045857.8 ; E 243498.8 **ORIGINATED BY** MR
DIST HWY 69 **BOREHOLE TYPE** 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring **COMPILED BY** VA
DATUM Geodetic **DATE** February 6 & 7, 2009 **CHECKED BY** OK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)										
ELEV. DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	SHEAR STRENGTH kPa								
											○ UNCONFINED	+ FIELD VANE	WATER CONTENT (%)													
											● QUICK TRIAXIAL	× REMOULDED	20	40	60	80	100	10	20	30		GR	SA	SI	CL	
203.5	GROUND SURFACE																									
0.0	Sand, trace gravel, trace silt, containing organics and cobbles (FILL)		1	SS	93																					
202.8	Very dense Brown Wet ROCK FILL		2	RC	-																					
0.7			3	RC	-																					
200.8			4	RC	-																					
2.7	Granite Gneiss (BEDROCK)		1	RC	REC 95%																				RQD = 60%	
	Bedrock cored from depths of 2.7 m to 6.0 m		2	RC	REC 100%																				RQD = 75%	
	For bedrock coring details, refer to Record of Drillhole B4-01		3	RC	REC 100%																				RQD = 100%	
			4	RC	REC 96%																				RQD = 90%	
197.5	END OF BOREHOLE																									
6.0	NOTE: 1. Water level measurements in Piezometer. Date Depth (m) Elev. (m) 16/04/09 1.5 202.0 26/08/09 1.9 201.6																									

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PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-01

SHEET 1 OF 1

LOCATION: N 5045857.8 ;E 243498.8

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	B Angle	DIP w.r.t. CORE AXIS	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY K, cm/sec	Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES
							TOTAL CORE %	SOLID CORE %					TYPE AND SURFACE DESCRIPTION						
							FLUSH	Jr					Ja	Jun					
		Continued from Record of Borehole B4-01		200.76															
3	NO RC February 7, 2009	GRANITE GNEISS Fresh to slightly weathered, medium grained, faintly to moderately porous, strong, weakly to moderately foliated, grey with black stippling, minor chloritic alteration and oxidation		2.74	1								FO,UN,RO JN,IR,RO FO,UN,RO JN,UN,SM FO,UN,SM JN,CU,RO FO,PL,SM JN,CU,SM FO,UN,RO JN,CU,SM FO,PL,SM JN,ST,RO						
4				2									JN,CU,SM JN,UN,RO FO,CU,RO						
5				198.75 4.75	3									JN,UN,RO JN,ST,RO JN,PL,RO FO,PL,SM JN,UN,SM JN,PL,RO					
6				197.53 5.97	4									FO,PL,RO FO,UN,RO				8.6 MPa	
6		END OF DRILLHOLE																	

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PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-02** SHEET 1 OF 1 **METRIC**
 W.P. 5187-06-01 LOCATION N 5045853.5 ; E 243493.5 ORIGINATED BY MR
 DIST HWY 69 BOREHOLE TYPE 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring COMPILED BY VA
 DATUM Geodetic DATE February 7, 2009 CHECKED BY OK

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100					
201.7	GROUND SURFACE																
0.0	SAND, trace gravel, containing organics and rootlets Loose Brown Wet		1	SS	5												
201.0	Granite Gneiss (BEDROCK)		1	RC	REC 89%		201										RQD = 39%
0.7	Bedrock cored from depths of 0.7 m to 4.3 m For bedrock coring details, refer to Record of Drillhole B4-02		2	RC	REC 100%		200										RQD = 100%
			3	RC	REC 100%		199										RQD = 100%
197.4	END OF BOREHOLE						198										
4.3	NOTE: 1. Open borehole dry upon completion of overburden drilling.																

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA_GDT_03/12/15 SAC

PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-02

SHEET 1 OF 1

LOCATION: N 5045853.5 ;E 243493.5

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY				FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA	HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES				
							TOTAL CORE %	SOLID CORE %	R.Q.D. %	FLUSH			Jr	Ja	Un				K, cm/sec	10	10	10
							80	80	80	80			80	80	80				80	80	80	80
		Continued from Record of Borehole B4-02		201.00																		
1	NW casing February 7, 2009	GRANITE GNEISS Slightly weathered to fresh, fine to medium grained with feldspar banding, foliated, black, pink and grey		0.70	1						JN,UN,RO JN,UN,RO JN,UN,RO JN,UN,RO JN,UN,RO JN,UN,RO											
2	NQ RC February 7, 2009				2						JN,UN,RO JN,UN,VR											
3					3						JN,UN,RO FO,UN,RO											
4		END OF DRILLHOLE		197.43 4.27																		
5																						
6																						
7																						
8																						
9																						
10																						

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PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-04	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045871.6 ; E 243474.4</u>	ORIGINATED BY <u>MR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>February 9, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100						20
200.2	GROUND SURFACE																	
0.0	Silt, some sand, trace to some clay, containing organics (FILL)		1	SS	5		200											
199.7	Loose Brown Moist																	
0.5	Granite Gneiss (BEDROCK)		1	RC	REC 75%		199											RQD = 13%
	Bedrock cored from depths of 0.5 m to 3.8 m		2	RC	REC 76%		198											RQD = 0%
	For bedrock coring details, refer to Record of Drillhole B4-04		3	RC	REC 98%		197											RQD = 77%
196.4	END OF BOREHOLE																	
3.8	NOTE: 1. Open borehole dry upon completion of overburden drilling.																	

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PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-04

SHEET 1 OF 1

LOCATION: N 5045871.6 ;E 243474.4

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	B Angle	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES	
							TOTAL CORE %	SOLID CORE %				TYPE AND SURFACE DESCRIPTION			K, cm/sec	φ	ψ				τ
							FLUSH					Jr	Ja	Jun	10 ⁰	10 ¹	10 ²				
		Continued from Record of Borehole B4-04		199.71																	
1	NW/Casing February 8, 2009	GRANITE GNEISS Slightly weathered to fresh, fine to coarse grained with feldspar banding, foliated, black, pink and grey		0.49	1							JN,IR,RO JN,IR,RO JN,UN,RO JN,PL,RO JN,PL,RO JN,UN,RO JN,PL,RO JN,UN,VR JN,UN,RO JN,UN,RO FO,UN,RO FO,UN,VR									
2	NO RC February 9, 2009				2							JN,UN,RO									
3					3							JN,UN,VR JN,UN,RO JN,UN,RO JN,UN,RO JN,UN,VR JN,UN,RO									
4		END OF DRILLHOLE		196.36 3.84								JN,IR,VR JN,IR,VR									
5																					
6																					
7																					
8																					
9																					
10																					

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC



PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-06	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045847.0 ; E 243506.5</u>	ORIGINATED BY <u>MR</u>	
DIST <u> </u> HWY <u>69</u>	BOREHOLE TYPE <u>165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring</u>		COMPILED BY <u>TZ</u>
DATUM <u>Geodetic</u>	DATE <u>March 24, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa									
								20	40	60	80	100					
								○ UNCONFINED + FIELD VANE									
								● QUICK TRIAXIAL × REMOULDED									
								20	40	60	80	100	10	20	30		
201.5	GROUND SURFACE																
0.0	Sand, some gravel, trace to some silt, trace clay (FILL) Loose to compact Brown Moist																
			1	SS	5												
199.7			2A	SS	20												20 67 11 2
1.9	Sand and gravel, some silt, containing organics and wood fragments (FILL) Compact Brown Moist		2B	RC	REC 100%												RQD = 0%
	Granite Gneiss (BEDROCK)		2	RC	REC 96%												RQD = 65%
	Bedrock cored from depths of 1.9 m to 5.5 m																
	For bedrock coring details, refer to Record of Drillhole B4-05		3	RC	REC 95%												RQD = 30%
			4	RC	REC 100%												RQD = 80%
196.0	END OF BOREHOLE																
5.5	NOTES: 1. Open borehole dry upon completion of drilling. 2. Sand fill placed as backfill to Test Pit B4-TP3 to allow for bedrock coring.																

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

PROJECT: 07-1111-0029
 LOCATION: N 5045847.0 ;E 243506.5
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: B4-06

SHEET 1 OF 1
 DATUM: Geodetic

DRILLING DATE:
 DRILL RIG: D25
 DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	B Angle	DIP w.r.t. CORE AXIS	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY K, cm/sec	Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES
							TOTAL CORE %	SOLID CORE %					TYPE AND SURFACE DESCRIPTION						
							FLUSH						Jr	Ja	Jun				
		Continued from Record of Borehole B4-06		199.61															
2	NW casing March 24, 2009	GRANITE GNEISS Slightly weathered to fresh, fine to medium grained with feldspar banding, foliated, black, pink and grey		1.89	1								JN,UN,RO JN,UN,RO						
														FO,UN,RO					
														JN,UN,RO JN,UN,RO					
3						2								JN,UN,VR JN,UN,RO					8.1 MPa 9.3 MPa
4	NQ RC March 24, 2009												JN,UN,VR JN,PL,RO JN,UN,RO JN,UN,VR JN,UN,VR					UC=78.1 MPa	
5					3								JN,PL,RO JN,IR,VR JN,IR,VR						
													JN,IR,VR JN,UN,VR						
6		END OF DRILLHOLE		196.01									JN,IR,RO						
				5.49															

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DEPTH SCALE
 1 : 50



LOGGED: MR
 CHECKED: OK

PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-07** **SHEET 1 OF 1** **METRIC**
W.P. 5187-06-01 **LOCATION** N 5045850.8 ; E 243512.1 **ORIGINATED BY** MR
DIST HWY 69 **BOREHOLE TYPE** 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring **COMPILED BY** TZ
DATUM Geodetic **DATE** March 24, 2009 **CHECKED BY** OK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)																			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	SHEAR STRENGTH kPa																	
											○ UNCONFINED	+ FIELD VANE	WATER CONTENT (%)																						
											● QUICK TRIAXIAL	× REMOULDED	20	40	60	80	100	10	20	30		GR SA SI CL													
202.4	GROUND SURFACE																																		
0.0	Sand, some gravel, containing cobbles (FILL) Brown Moist																																		
201.1	ROCK FILL																																		
1.3			1	RC	-																														
			2	RC	-																														
199.9	Granite Gneiss (BEDROCK)																																		
2.5	Bedrock cored from depths of 2.5 m to 6.9 m For bedrock coring details, refer to Record of Drillhole B4-07		1	RC	REC 92%																		RQD = 57%												
			2	RC	REC 100%																		RQD = 50%												
			3	RC	REC 100%																		RQD = 65%												
			4	RC	REC 100%																		RQD = 73%												
			5	RC	REC 94%																		RQD = 52%												
195.5	END OF BOREHOLE																																		
6.9	NOTES: 1. Water level in open borehole at a depth of 3.1 m below ground surface (Elev. 199.3 m) upon completion of drilling. 2. Water level measurements in piezometer: <table border="1"> <tr> <th>Date</th> <th>Depth (m)</th> <th>Elev. (m)</th> </tr> <tr> <td>26/03/09</td> <td>3.2</td> <td>199.2</td> </tr> <tr> <td>16/04/09</td> <td>3.2</td> <td>199.2</td> </tr> <tr> <td>26/08/09</td> <td>3.9</td> <td>198.5</td> </tr> </table>	Date	Depth (m)	Elev. (m)	26/03/09	3.2	199.2	16/04/09	3.2	199.2	26/08/09	3.9	198.5																						
Date	Depth (m)	Elev. (m)																																	
26/03/09	3.2	199.2																																	
16/04/09	3.2	199.2																																	
26/08/09	3.9	198.5																																	

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PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-07

SHEET 1 OF 1

LOCATION: N 5045850.8 ;E 243512.1

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY				FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA	HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES	
							TOTAL CORE %	SOLID CORE %	R.Q.D. %	FLUSH			TYPE AND SURFACE DESCRIPTION	Jr	Ja				Jn
							888888	888888	888888	888888									
		Continued from Record of Borehole B4-07		199.93															
3		GRANITE GNEISS Slightly weathered to fresh, fine to medium grained with feldspar banding, laminated, dark grey and pink		2.47	1							JN,IR,RO JN,UN,RO							
4				2									JN,UN,RO FO,UN,RO JN,UN,RO JN,UN,RO						
5				3									JN,UN,RO FO,UN,RO JN,UN,RO						
6				4									JN,UN,RO JN,UN,RO JN,PL,RO JN,PL,RO JN,UN,RO						
7				5									JN,UN,RO FO,UN,RO JN,UN,RO						
7		END OF DRILLHOLE		195.48 6.92															

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC



PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-08** **SHEET 1 OF 1** **METRIC**
W.P. 5187-06-01 **LOCATION** N 5045846.8 ; E 243514.4 **ORIGINATED BY** MR
DIST HWY 69 **BOREHOLE TYPE** 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring **COMPILED BY** TZ
DATUM Geodetic **DATE** March 25 & 26, 2009 **CHECKED BY** OK

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100						20
201.8	GROUND SURFACE																	
0.0	Sand, some gravel, trace silt (FILL) Compact Brown Moist		1	SS	23/0.2		201											
200.7	ROCK FILL		2	RC	-		200											
198.7	ROCK FILL		3	RC	-		199											
198.7	Granite Gneiss (BEDROCK) Bedrock cored from depths of 3.1 m to 8.1 m For bedrock coring details, refer to Record of Drillhole B4-08		1	RC	REC 100%	▽	198											RQD = 13%
3.1			2	RC	REC 98%		197											RQD = 41%
			3	RC	REC 100%		196											RQD = 32%
			4	RC	REC 98%		195											RQD = 71%
193.7	END OF BOREHOLE						194											
8.1	NOTE: 1. Water level in open borehole at a depth of 3.8 m below ground surface (Elev. 198.0 m) upon completion of drilling.																	

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-08

SHEET 1 OF 1

LOCATION: N 5045846.8 ;E 243514.4

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES		
							TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION	Jr	Ja	Jn	K, cm/sec	10 ⁰				10 ¹	10 ²
							FLUSH	FLUSH													
		Continued from Record of Borehole B4-08		198.66																	
		GRANITE GNEISS Slightly weathered to fresh, fine to coarse grained with feldspar banding, foliated, black, pink and grey		3.14	1						JN,UN,RO JN,PL,RO JN,UN,RO JN,UN,RO JN,UN,RO JN,UN,VR JN,UN,RO JN,IR,RO										
					2						JN,UN,RO JN,UN,RO JN,UN,RO										
					3						JN,IR,RO JN,IR,RO JN,UN,RO JN,UN,RO FO,UN,RO FO,UN,RO										
					4						JN,UN,VR JN,PL,RO										
		END OF DRILLHOLE		193.72							JN,IR,VR JN,UN,RO JN,UN,RO							8.6 MPa 11.1 MPa			
				8.08																	

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DEPTH SCALE

1 : 50



LOGGED: MR
CHECKED: OK

PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-09** SHEET 1 OF 1 **METRIC**
 W.P. 5187-06-01 LOCATION N 5045866.1 ; E 243505.4 ORIGINATED BY ID
 DIST HWY 69 BOREHOLE TYPE 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring COMPILED BY TZ
 DATUM Geodetic DATE April 2, 2009 CHECKED BY OK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	SHEAR STRENGTH kPa								
						20	40	60	80	100						
205.3	GROUND SURFACE															
0.0	ASPHALT															
0.2	Sand, trace gravel, trace silt (FILL) Dense Brown Moist															
204.0			1A	SS	35											
1.3	Sand and gravel, trace silt, containing wood fragments (FILL) Dense Brown Moist		1B													
203.0			2	SS	36											
2.3	SAND and SILT, trace to some gravel, trace clay Loose Brown Moist		3	SS	4											
202.4																
2.9	Granite Gneiss (BEDROCK)															
	Bedrock cored from depths of 2.9 m to 7 m		1	RC	REC 56%											9 39 51 1
	For bedrock coring details, refer to Record of Drillhole B4-09		2	RC	REC 40%											RQD = 22%
			3	RC	REC 63%											RQD = 34%
198.3																RQD = 57%
7.0	END OF BOREHOLE															
	NOTE: 1. Water level in open borehole at a depth of 2.9 m below ground surface (Elev. 202.4 m) upon completion of drilling.															

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PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-09

SHEET 1 OF 1

LOCATION: N 5045866.1 ;E 243505.4

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: D25

DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY			FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES				
							TOTAL CORE %	SOLID CORE %	R.Q.D. %		B Angle	DIP w.r.t. CORE AXIS	TYPE AND SURFACE DESCRIPTION	Jr	Ja	Jn				K, cm/sec	10 ⁰	10 ¹	10 ²
3	NQ RC April 2, 2009	Continued from Record of Borehole B4-09		202.40																			
4		GRANITE GNEISS Slightly weathered to fresh, fine to medium grained with feldspar banding, foliated, black, pink and grey		2.90																10.3MPa(Axial) 10.5MPa(Axial) 10.8MPa(Axial)			
5																				UC=85.5 MPa 8.3 MPa			
6																							
7		END OF DRILLHOLE		198.29 7.01																			
8																							
9																							
10																							
11																							
12																							

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC



PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-10** SHEET 1 OF 1 **METRIC**
 W.P. 5187-06-01 LOCATION N 5045860.9; E 243510.8 ORIGINATED BY ID
 DIST HWY 69 BOREHOLE TYPE 165 mm O.D. Continuous Flight Solid Stem Augers and NW Casing, Wash Boring COMPILED BY TZ
 DATUM Geodetic DATE April 2, 2009 CHECKED BY OK

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)					
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100						20	40	60	80	100
205.5	GROUND SURFACE																					
0.0	ASPHALT																					
0.2	Sand, some gravel, trace silt (FILL) Brown Moist		1	AS	-		205															
204.6			2	SS	98																	
0.9	Sand and gravel, trace silt, containing organics and cobbles (FILL) Compact to very dense Brown Moist		3	SS	20		204															
203.2																						
2.3	Granite Gneiss (BEDROCK)						203															
	Bedrock cored from depths of 2.3 m to 6.2 m For bedrock coring details, refer to Record of Drillhole B4-10		1	RC	REC 98%		202														RQD = 72%	
			2	RC	REC 98%		201															RQD = 54%
			3	RC	REC 99%		200															RQD = 74%
199.3	END OF BOREHOLE																					
6.2	NOTE: 1. Water level in open borehole at a depth of 2.3 m below ground surface (Elev. 203.2 m) upon completion of drilling.																					

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

PROJECT: 07-1111-0029
 LOCATION: N 5045860.9 ;E 243510.8
 INCLINATION: -90° AZIMUTH: —

RECORD OF DRILLHOLE: B4-10

SHEET 1 OF 1
 DATUM: Geodetic

DRILLING DATE:
 DRILL RIG: D25
 DRILLING CONTRACTOR: Walker Drilling

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR	FLUSH	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	B Angle	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY K, cm/sec	Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES		
								TOTAL CORE %	SOLID CORE %				Jr	Ja	Jun						
								JN - Joint FLT - Fault SH - Shear VN - Vein CJ - Conjugate	BD - Bedding FO - Foliation CO - Contact OR - Orthogonal CL - Cleavage				PL - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular	PO - Polished K - Slickensided SM - Smooth RO - Rough VR - Very Rough	MB - Mechanical Break BR - Broken Rock						
		Continued from Record of Borehole B4-10		203.20																	
3	NO RC April 2, 2009	GRANITE GNEISS Slightly weathered to fresh, fine to medium grained, with feldspar banding, laminated, dark grey and pink		2.30	1																
4				2																	
5				3																	
6		END OF DRILLHOLE		199.34 6.16																	

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC

DEPTH SCALE
1 : 50



LOGGED: ID
CHECKED: OK

PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-11** SHEET 1 OF 1 **METRIC**
 W.P. 5187-06-01 LOCATION N 5045815.2 ; E 243536.2 ORIGINATED BY CR
 DIST HWY 69 BOREHOLE TYPE Portable Equipment, BW Casing, Wash Boring COMPILED BY VA
 DATUM Geodetic DATE July 22 & 23, 2009 CHECKED BY OK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80					
											○ UNCONFINED	+ FIELD VANE				GR SA SI CL
207.6	GROUND SURFACE															
0.0	Granite Gneiss (BEDROCK)		1	RC	REC 100%											RQD = 100%
	Bedrock cored from ground surface to a depth of 10.2 m		2	RC	REC 100%											RQD = 100%
	For bedrock coring details, refer to Record of Drillhole B4-11		3	RC	REC 100%											RQD = 98%
			4	RC	REC 100%											RQD = 100%
			5	RC	REC 100%											RQD = 83%
			6	RC	REC 100%											RQD = 100%
			7	RC	REC 100%											RQD = 65%
			8	RC	REC 100%											RQD = 73%
			9	RC	REC 100%											RQD = 80%
			10	RC	REC 100%											RQD = 90%
197.4	END OF BOREHOLE															
10.2	NOTE: 1. Water level in open borehole at a depth of 5.5 m below ground surface (Elev. 202.1 m) upon completion of drilling.															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-11

SHEET 1 OF 2

LOCATION: N 5045815.2 ; E 243536.2

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: Portable Equipment

DRILLING CONTRACTOR: OGS

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC - Q AVG.	NOTES			
							TOTAL CORE %	SOLID CORE %			B Angle	DIP w.r.t. CORE AXIS	TYPE AND SURFACE DESCRIPTION	Jr	Ja	Jn				K _v cm/sec	K _h cm/sec	K _z cm/sec
							FLUSH															
0		GROUND SURFACE		207.57																		
0		GRANITE GNEISS Grey to light grey, highly foliated, coarse grained, slightly to moderately porous		0.00	1																	
1					2																	
2					3																	
3					4																	
4					5																	
5					6																	
6					7																	
7					8																	
8					9																	
9					10																	
10																						

CONTINUED NEXT PAGE

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DEPTH SCALE

1 : 50



LOGGED: CR

CHECKED: OK

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-12	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045818.8 ; E 243534.4</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Hand Excavation</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 20, 2009</u>	CHECKED BY <u>OK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80					
204.5 0.0	GROUND SURFACE BEDROCK OUTCROP															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-13	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045818.8 ; E 243539.1</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Hand Excavation</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 20, 2009</u>	CHECKED BY <u>OK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC NATURAL LIQUID LIMIT MOISTURE LIMIT CONTENT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80	100	W _p	W		
205.0	GROUND SURFACE															
0.0	TOPSOIL	////	1	CS	-											
0.1	END OF EXCAVATION BEDROCK															
	NOTE: 1. Hand digging carried out at proposed borehole location to expose bedrock (Dry).															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-14	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045819.1 ; E 243544.2</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Hand Excavation</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 21, 2009</u>	CHECKED BY <u>OK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC NATURAL LIQUID LIMIT MOISTURE LIMIT CONTENT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80	100	W _p	W		
202.3	GROUND SURFACE															
0.0	TOPSOIL		1	CS	-											
0.2	END OF EXCAVATION BEDROCK															
	NOTE: 1. Hand digging carried out at proposed borehole location to expose bedrock (Dry).															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-15	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045823.7 ; E 243541.0</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Portable Equipment, BW Casing, Wash Boring</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 21 & 22, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100					
202.0	GROUND SURFACE																
0.0	Cobbles/Rock Fragments		1	RC	REC 66%												
201.5	Granite Gneiss (BEDROCK)		2	RC	REC 100%												RQD = 45%
0.5	Bedrock cored from depths of 0.5 m to 6.5 m For bedrock coring details, refer to Record of Drillhole B4-15		3	RC	REC 100%		201										RQD = 91%
			4	RC	REC 100%		200										RQD = 100%
			5	RC	REC 100%		199										RQD = 97%
			6	RC	REC 100%		198										RQD = 97%
			7	RC	REC 100%		197										RQD = 97%
			8	RC	REC 100%		196										RQD = 97%
			9	RC	REC 100%												RQD = 100%
195.5 6.5	END OF BOREHOLE																
	NOTE: 1. Water level measurements in piezometer: Date Depth (m) Elev. (m) 28/07/09 -0.6* 202.6 26/08/09 0.3 201.7 *Water level above ground surface due to heavy rain.																

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-15

SHEET 1 OF 1

LOCATION: N 5045823.7 ;E 243541.0

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: Portable Equipment

DRILLING CONTRACTOR: OGS

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY			FRACT. INDEX PER 0.3 m	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY			Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES	
							TOTAL CORE %	SOLID CORE %	R.Q.D. %		B Angle	DIP w.r.t. CORE AXIS	TYPE AND SURFACE DESCRIPTION	Ur	Ja	Un				K _v cm/sec
							FLUSH	FLUSH	FLUSH		°	°		°	°	°				°
0		GROUND SURFACE		201.98																
0	B/C Casing July 21, 2009	COBBLES		0.00																
0.48		Moderately weathered, dark grey, highly foliated, coarse grained, slightly to moderately porous		201.50	2						JN,PL,RO JN,UN,RO JN,UN,RO	1.5 3 0.5 2								
1		GRANITE GNEISS Slightly weathered, dark to light grey, highly foliated, coarse grained, slightly to moderately porous			3						JN,UN,RO HUN, JN,UN,RO JN,CU,RO	3 1 0.75 3 3 3 1								
2					4						JN,UN,RO HUN, JN,UN,RO	3 1 0.75 3 1								
3					5						JN,UN,RO HFO, JN,UN,RO	1.5 3 0.75 3 2								
4	BORC July 21 & 22, 2009				6						HUN, JN,UN,RO HUN, HUN, HUN, JN,UN,RO JN,UN,RO JN,PL,RO JN,UN,RO JN,UN,RO	0.75 3 2 0.75 0.75 1.5 2 1.5 1 3 3 3 3								
5					7						JN,UN,RO JN,UN,RO JN,UN,RO	1.5 1 1.5 1 3 3						8.8 MPa		
6					8						JN,PL,RO JN,UN,RO	1.5 1 3 1						8.6 MPa		
6					9						JN,UN,RO JN,PL,RO JN,UN,RO	3 1 1.5 1 3 3								
6.48		END OF DRILLHOLE		195.50							JN,PL,RO JN,UN,RO	1.5 1 3 2								
7																				
8																				
9																				
10																				

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC

DEPTH SCALE

1 : 50



LOGGED: CR

CHECKED: OK

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-16	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045802.5 ; E 243546.3</u>	ORIGINATED BY <u>CR</u>	
DIST <u> </u> HWY <u>69</u>	BOREHOLE TYPE <u>Portable Equipment</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 20, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100					
208.3	GROUND SURFACE																
0.0	TOPSOIL		1A				208										
0.1	Silty SAND, some gravel, rock fragments, containing rootlets Loose to compact Brown Moist		1B	SS	6												
207.2			2	SS	23												
1.1	END OF BOREHOLE SPOON REFUSAL																
	NOTES: 1. Borehole advanced using portable drilling equipment with one-third weight hammer. SPT 'N' values shown have been adjusted to infer values that would be obtained with standard weight hammer. 2. Open borehole dry upon completion of drilling.																

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-17	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045805.3 ; E 243543.4</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Portable Equipment</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 20, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
			NUMBER	TYPE	"N" VALUES			20	40	60	80	100					
207.5	GROUND SURFACE																
0.0	TOPSOIL		1A	SS	1												
0.3	Silty SAND, trace to some clay, trace gravel, containing rootlets Very loose Brown Moist END OF BOREHOLE SPOON REFUSAL NOTES: 1. Borehole advanced using portable drilling equipment with one-third weight hammer. SPT 'N' value shown has been adjusted to infer value that was obtained with standard weight hammer. 2. Open borehole dry upon completion of drilling.																

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-18	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045807.7 ; E 243550.6</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Portable Equipment, BW Casing, Wash Boring</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 20 & 21, 2009</u>	CHECKED BY <u>OK</u>	

ELEV DEPTH	SOIL PROFILE DESCRIPTION	STRAT PLOT	SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)						
			NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa												
							20	40	60	80	100	PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	GR	SA	SI	CL		
207.2	GROUND SURFACE																			
0.0	TOPSOIL		1	SS	4/0.13															
0.1	Granite Gneiss (BEDROCK)		1	RC	REC 100%														RQD = 83%	
	Bedrock cored from depths of 0.1 m to 6.4 m		2	RC	REC 100%														RQD = 92%	
	For bedrock coring details refer to Record of Drillhole B4-18		3	RC	REC 100%														RQD = 100%	
			4	RC	REC 100%														RQD = 100%	
			5	RC	REC 100%														RQD = 97%	
			6	RC	REC 100%														RQD = 100%	
			7	RC	REC 100%														RQD = 100%	
			8	RC	REC 100%														RQD = 95%	
200.8	END OF BOREHOLE																			
6.4	NOTES: 1. Borehole advanced using portable drilling equipment with one-third weight hammer. SPT 'N' value shown has been adjusted to infer value that was obtained with standard weight hammer. 2. Water level measurements in piezometer: Date Depth (m) Elev. (m) 28/07/09 3.9 203.3 26/08/09 4.7 202.5																			

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT: 07-1111-0029

RECORD OF DRILLHOLE: B4-18

SHEET 1 OF 1

LOCATION: N 5045807.7 ;E 243550.6

DRILLING DATE:

DATUM: Geodetic

INCLINATION: -90° AZIMUTH: —

DRILL RIG: Portable Equipment

DRILLING CONTRACTOR: OGS

DEPTH SCALE METRES	DRILLING RECORD	DESCRIPTION	SYMBOLIC LOG	ELEV. DEPTH (m)	RUN No.	COLOUR % RETURN	RECOVERY		R.Q.D. %	FRACT. INDEX PER 0.3 m	B Angle	DIP w.r.t. CORE AXIS	DISCONTINUITY DATA			HYDRALLIC CONDUCTIVITY K, cm/sec	Diametral Point Load Index (MPa)	RMC -Q AVG.	NOTES
							TOTAL CORE %	SOLID CORE %					TYPE AND SURFACE DESCRIPTION						
							FLUSH						Jr	Ja	Un				
		Continued from Record of Borehole B4-18		207.07															
		GRANITE GNEISS Slightly weathered, dark to light grey, highly foliated, coarse grained, slightly to moderately porous		0.13 0.28 208.74 0.46	1							JN,PL,RO HUN, JN,IR,RO JN,PL,RO	1.5 3 3 1 1.5 2						
		QUARTZ VEIN										HUN, 70	0.75 1.5 3 3 1 1.5 2						
		GRANITE GNEISS Slightly weathered, dark to light grey, highly foliated, coarse grained, slightly to moderately porous			2							JN,PL,RO JN,UN,RO HUN,	1.5 3 3 1 1.5 2						
		QUARTZ VEIN		205.83 1.37 205.63 1.57	3							JN,UN,RO	3 1						
		GRANITE GNEISS Slightly weathered, dark to light grey, highly foliated, coarse grained, slightly to moderately porous			4							JN,PL,RO FO,PL,RO	1.5 3 1.5 3						
					5							JN,PL,RO JN,UN,RO	1.5 3 3 1						
					6							HUN, JN,PL,RO JN,PL,RO	0.75 1.5 1 1.5 1						
					7							JN,UN,RO	3 1						
					8							JN,UN,RO HUN, HUN, JN,PL,RO JN,PL,RO HUN,	3 1 0.75 0.75 1.5 2 1.5 3 0.75						
		END OF DRILLHOLE		200.80 6.40								JN,UN,RO	3 1						

GTA-RCK 018 T:\PROJECTS\2007\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-MISS.GDT 03/12/15 SAC July 20 & 21, 2009

DEPTH SCALE

1 : 50



LOGGED: CR

CHECKED: OK

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-19	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045809.0 ; E 243559.7</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Hand Excavation</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 21, 2009</u>	CHECKED BY <u>OK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC NATURAL LIQUID LIMIT MOISTURE LIMIT CONTENT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80	100	W _p	W		
208.2	GROUND SURFACE															
8.0	TOPSOIL		1	CS												
	END OF EXCAVATION BEDROCK															
	NOTE: 1. Hand digging carried out at proposed borehole location to expose bedrock (DRY).															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT <u>07-1111-0029</u>	RECORD OF BOREHOLE No B4-20	SHEET 1 OF 1	METRIC
W.P. <u>5187-06-01</u>	LOCATION <u>N 5045811.8 ; E 243556.8</u>	ORIGINATED BY <u>CR</u>	
DIST <u>HWY 69</u>	BOREHOLE TYPE <u>Hand Excavation</u>	COMPILED BY <u>VA</u>	
DATUM <u>Geodetic</u>	DATE <u>July 21, 2009</u>	CHECKED BY <u>OK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC NATURAL LIQUID LIMIT MOISTURE LIMIT CONTENT			UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80	100	W _p	W		
206.5	GROUND SURFACE	//	1	CS	-											
0.0	TOPSOIL	//														
0.1	END OF EXCAVATION BEDROCK															
	NOTE: 1. Hand digging carried out at proposed borehole location to expose bedrock (DRY).															

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC



PROJECT 07-1111-0029 **RECORD OF BOREHOLE No B4-21** SHEET 1 OF 1 **METRIC**

W.P. 5187-06-01 LOCATION N 5045793.8 ; E 243565.0 ORIGINATED BY CR

DIST HWY 69 BOREHOLE TYPE Hand Excavation COMPILED BY VA

DATUM Geodetic DATE July 24, 2009 CHECKED BY OK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)								
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE			"N" VALUES	20	40	60	80						100	20	40	60	80	100	10	20
212.9	GROUND SURFACE																							
8.9	TOPSOIL																							
	END OF EXCAVATION																							
	BEDROCK																							
	NOTE:																							
	1. Hand digging carried out at proposed borehole location to expose bedrock (DRY).																							

GTA-MTO 001 T:\PROJECTS\2007\07-1111-0029 (MRC, PARRY SOUND)\LOG\07-1111-0029-SHAWANAGA NBL & SBL-PHASE III.GPJ GAL-GTA.GDT 03/12/15 SAC

+³, ×³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE



APPENDIX B

LABORATORY TEST RESULTS

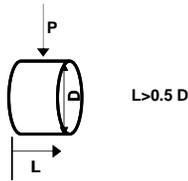
TABLE B1
POINT LOAD TEST RESULTS ON ROCK SAMPLES

Borehole Number	Run Number	Sample Depth (m)	Sample Elevation (m)	Bedrock Description	Test Type	Is (50mm) (MPa)	Approx. ⁽¹⁾ UCS (MPa)
B4-01	3	5.31	198.2	Granite Gneiss	Diametral	6.252	69.0
B4-01	4	5.47	198.0	Granite Gneiss	Diametral	7.504	83.0
B4-01	4	5.54	195.7	Granite Gneiss	Diametral	8.573	94.0
B4-02	1	1.0	200.7	Granite Gneiss	Diametral	6.185	68.0
B4-02	1	1.0	200.7	Granite Gneiss	Diametral	6.394	70.0
B4-02	1	1.2	200.5	Granite Gneiss	Diametral	5.175	57.0
B4-02	2	2.0	199.7	Granite Gneiss	Diametral	5.501	61.0
B4-02	2	2.1	199.6	Granite Gneiss	Diametral	6.052	67.0
B4-02	2	2.2	199.5	Granite Gneiss	Diametral	4.591	51.0
B4-06	2	2.6	198.9	Granite Gneiss	Diametral	6.995	77.0
B4-06	2	2.8	198.7	Granite Gneiss	Diametral	8.072	89.0
B4-06	2	2.9	198.6	Granite Gneiss	Diametral	9.291	102.0
B4-08	2	5.0	196.8	Granite Gneiss	Diametral	8.640	95.0
B4-08	2	5.1	196.7	Granite Gneiss	Diametral	11.102	122.0
B4-08	2	5.2	196.6	Granite Gneiss	Diametral	6.862	75.0
B4-09	1	3.1	202.2	Granite Gneiss	Axial	10.322	114.0
B4-09	1	3.2	202.1	Granite Gneiss	Diametral	5.543	61.0
B4-09	1	3.3	202.0	Granite Gneiss	Axial	10.509	116.0
B4-09	1	3.3	202.0	Granite Gneiss	Axial	10.828	119.0
B4-09	1	3.3	202.0	Granite Gneiss	Diametral	5.735	63.0
B4-09	1	3.4	201.9	Granite Gneiss	Diametral	6.386	70.0
B4-09	2	4.8	200.5	Granite Gneiss	Diametral	7.830	86.0
B4-09	2	4.9	200.4	Granite Gneiss	Diametral	8.272	91.0
B4-09	2	5.0	200.3	Granite Gneiss	Diametral	6.928	76.0
B4-11	6	4.6	203.0	Granite Gneiss	Diametral	6.331	70.0
B4-11	7	6.3	201.3	Granite Gneiss	Diametral	4.853	53.0
B4-11	9	8.6	199.0	Granite Gneiss	Diametral	2.940	32.0
B4-11	10	9.3	198.3	Granite Gneiss	Diametral	4.878	54.0
B4-11	10	9.8	197.8	Granite Gneiss	Diametral	5.797	64.0
B4-15	5	3.1	198.9	Granite Gneiss	Diametral	6.717	74.0
B4-15	7	4.1	197.9	Granite Gneiss	Diametral	8.827	97.0
B4-15	8	5.1	196.9	Granite Gneiss	Diametral	8.573	94.0
B4-15	9	6.3	195.8	Granite Gneiss	Diametral	4.073	45.0
B4-18	6	3.9	203.3	Granite Gneiss	Diametral	3.375	37.0
B4-18	6	4.6	202.7	Granite Gneiss	Diametral	2.110	23.0
B4-18	7	5.1	202.1	Granite Gneiss	Diametral	1.059	12.0
B4-18	9	5.9	201.3	Granite Gneiss	Diametral	4.927	54.0

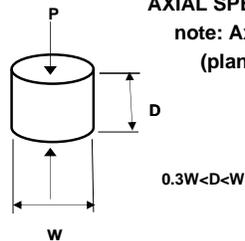
**TABLE B1
POINT LOAD TEST RESULTS ON ROCK SAMPLES**

Borehole Number	Run Number	Sample Depth (m)	Sample Elevation (m)	Bedrock Description	Test Type	Is (50mm) (MPa)	Approx. ⁽¹⁾ UCS (MPa)
<p>⁽¹⁾ $I_{s50} \times K$ (actual value could be confirmed by UCS testing), from ISRM. This range has been given based on $K = 11$, calculated from I_{s50} Average (9 tests) equal to 7.2 MPa on Diametral orientation on samples from BH B4-06 and B4-09, and UCS Average (2 tests) equal to 81.5 MPa on a sample from end of BH B4-06 and B4-09.</p> <p>"Suggested Methods for Determining Point Load Strength", International Society for Rock Mechanics Commission on Testing Methods, Int. J. Rock. Mech. Min. Sci. and Geomechanical Abstr., Vol 22, No. 2 1985, pp. 51-60.</p>							

DIAMETRAL SPECIMEN SHAPE REQUIREMENTS
note: Diametral tests are perpendicular to core axis (planes of weakness)



AXIAL SPECIMEN SHAPE REQUIREMENTS
note: Axial tests are parallel to core axis (planes of weakness)



Compiled By: OK
 Checked By: CN
 Reviewed By: JPD/JMAC

**TABLE B2-1
SUMMARY OF UNCONFINED COMPRESSION TEST RESULTS
SHAWANAGA RIVER NBL BRIDGE STRUCTURE
HIGHWAY 69, TOWNSHIP OF PARRY SOUND
GWP 5111-07-00, W.P 5187-06-01**

Borehole Number (Core Run)	Sample Depth (m)	Sample Elevation (m)	Rock Type	Core Diameter (mm)	Unconfined Compressive Strength (MPa)
B4-06 (2)	3.3	198.2	Granite Gneiss	47.4	78.1
B4-09 (2)	4.7	200.6	Granite Gneiss	47.5	85.5

Compiled By: OKChecked By: CNReviewed By: JPD/JMAC

TABLE B2-2
UNCONFINED COMPRESSION (UC) TEST
ASTM D 7012-07

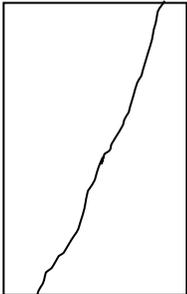
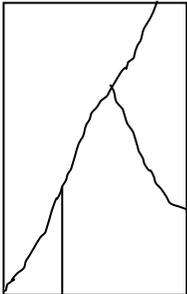
SAMPLE IDENTIFICATION			
PROJECT NUMBER	07-1111-0029	RUN NUMBER	2
BOREHOLE NUMBER	B4-06	SAMPLE DEPTH, m	3.1-3.4
TEST CONDITIONS			
MACHINE SPEED, mm/min	-	TYPE OF SPECIMEN	Rock Core
DURATION OF TEST, min	>2 <15	L/D	2.11
SPECIMEN INFORMATION			
SAMPLE HEIGHT, cm	10.00	WATER CONTENT, (specimen) %	0.13
SAMPLE DIAMETER, cm	4.74	UNIT WEIGHT, kN/m ³	26.10
SAMPLE AREA, cm ²	17.65	DRY UNIT WT., kN/m ³	26.07
SAMPLE VOLUME, cm ³	176.46	SPECIFIC GRAVITY, assumed	2.70
WET WEIGHT, g	469.90	VOID RATIO	0.02
DRY WEIGHT, g	469.29		
VISUAL INSPECTION	FAILURE SKETCH		
			
TEST RESULTS			
STRAIN AT FAILURE, %	-	COMPRESSIVE STRESS, MPa	78.1
REMARKS:	N/A	DATE:	6/16/2009
CHECKED BY:	CN	REVIEWED BY:	JPD/JMAC

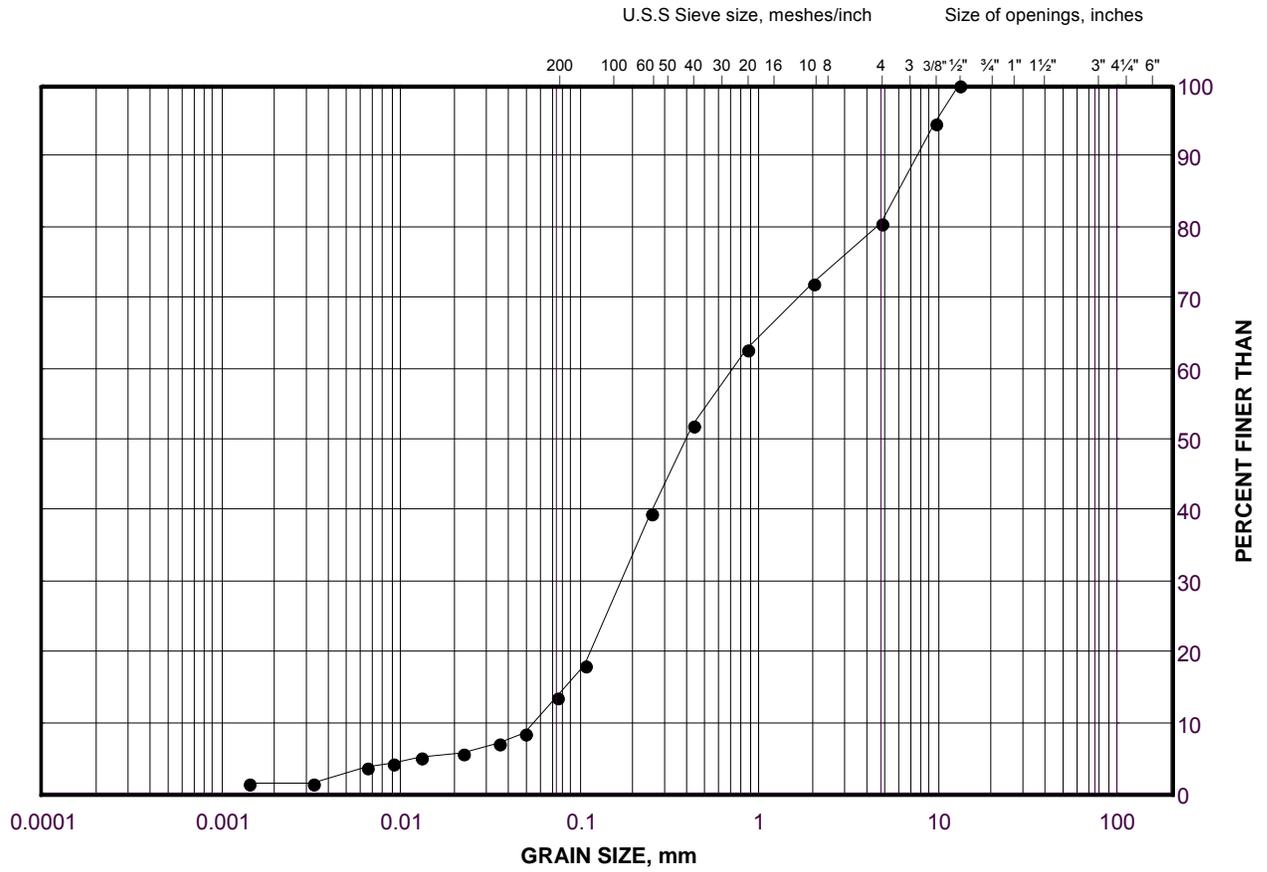
TABLE B2-3
UNCONFINED COMPRESSION (UC) TEST
ASTM D 7012-04

SAMPLE IDENTIFICATION			
PROJECT NUMBER	07-1111-0029	RUN NUMBER	2
BOREHOLE NUMBER	B4-09	SAMPLE DEPTH, m	4.5-4.8
TEST CONDITIONS			
MACHINE SPEED, mm/min	-	TYPE OF SPECIMEN	Rock Core
DURATION OF TEST, min	>2 <15	L/D	2.16
SPECIMEN INFORMATION			
SAMPLE HEIGHT, cm	10.25	WATER CONTENT, (specimen) %	0.12
SAMPLE DIAMETER, cm	4.75	UNIT WEIGHT, kN/m ³	26.91
SAMPLE AREA, cm ²	17.68	DRY UNIT WT., kN/m ³	26.88
SAMPLE VOLUME, cm ³	181.25	SPECIFIC GRAVITY, assumed	2.70
WET WEIGHT, g	497.61	VOID RATIO	-0.02
DRY WEIGHT, g	497.01		
VISUAL INSPECTION	FAILURE SKETCH		
			
TEST RESULTS			
STRAIN AT FAILURE, %	-	COMPRESSIVE STRESS, MPa	85.5
REMARKS:	N/A	DATE:	6/16/2009
CHECKED BY:	CN	REVIEWED BY:	JPD/JMAC

GRAIN SIZE DISTRIBUTION

Sand (Fill)
North Pier (Pier 4)

FIGURE B1-1



SILT AND CLAY SIZES	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE
FINE GRAINED	SAND SIZE			GRAVEL SIZE		SIZE

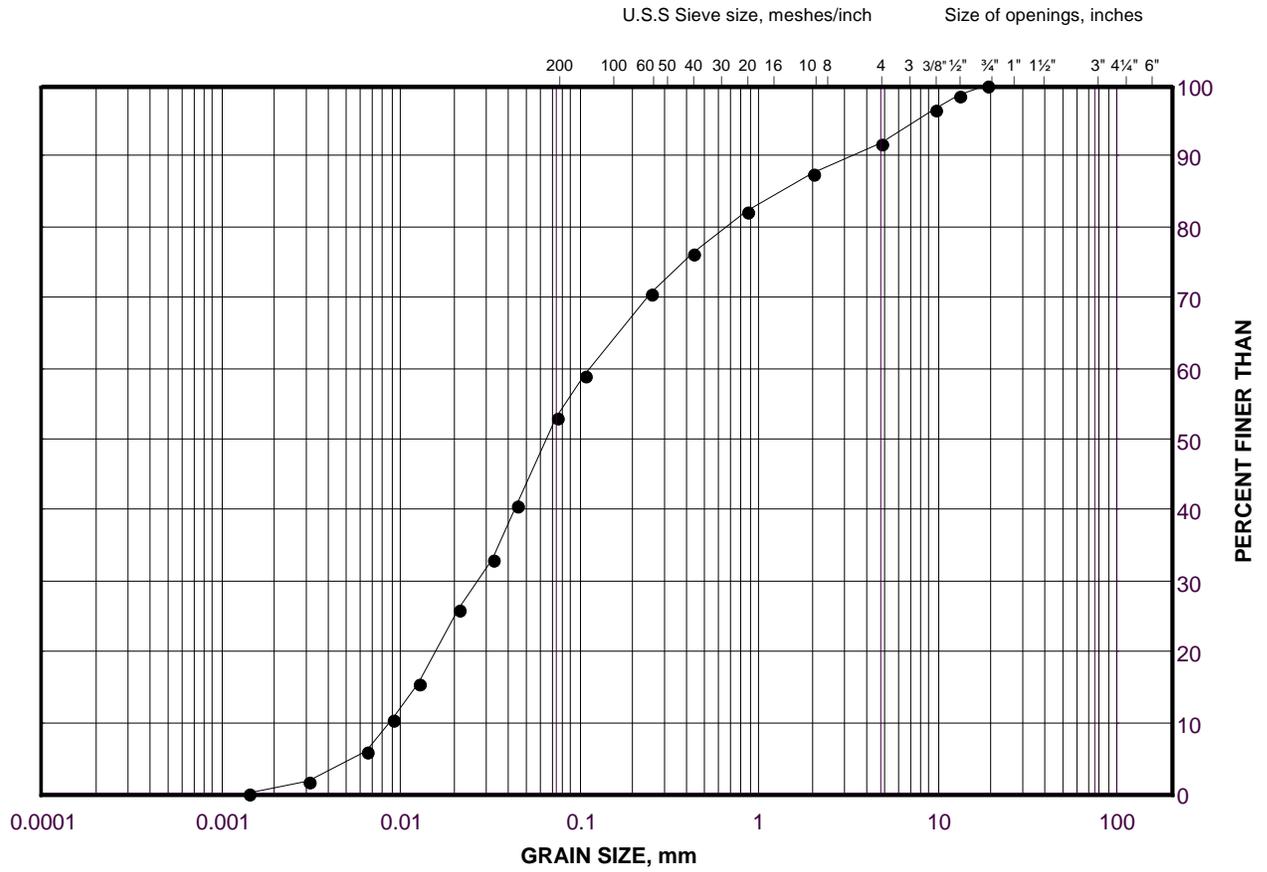
LEGEND

SYMBOL	BOREHOLE	SAMPLE	ELEVATION(m)
●	B4-06	2A	199.9

GRAIN SIZE DISTRIBUTION

Sand and Silt
North Abutment

FIGURE B1-2



SILT AND CLAY SIZES		FINE	MEDIUM	COARSE	FINE	COARSE	COBBLE
FINE GRAINED		SAND SIZE			GRAVEL SIZE		SIZE

LEGEND

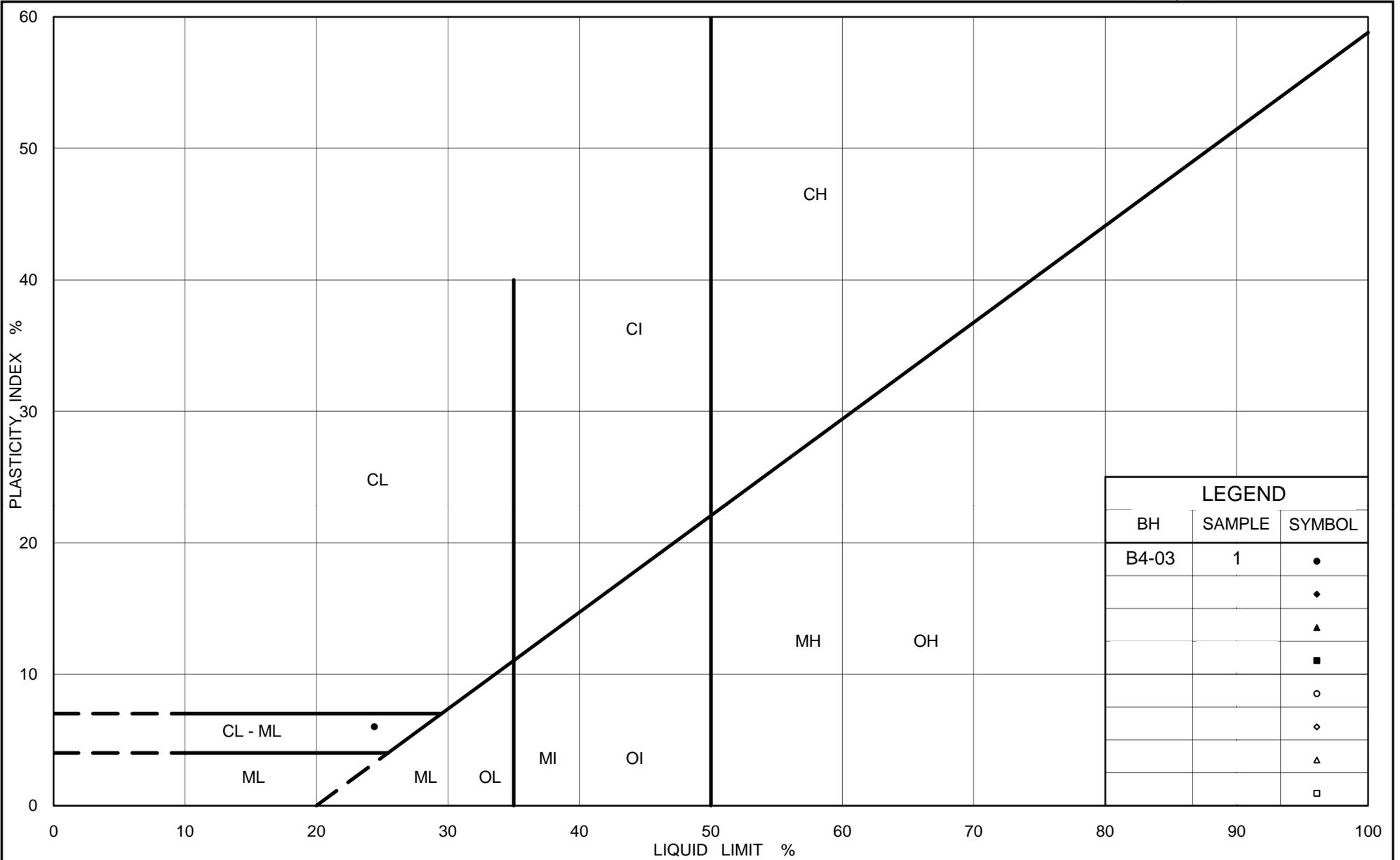
SYMBOL	BOREHOLE	SAMPLE	ELEVATION(m)
•	B4-09	3	202.7

Project Number: 07-1111-0029

Checked By: CN

Golder Associates

Date: 12-Aug-10



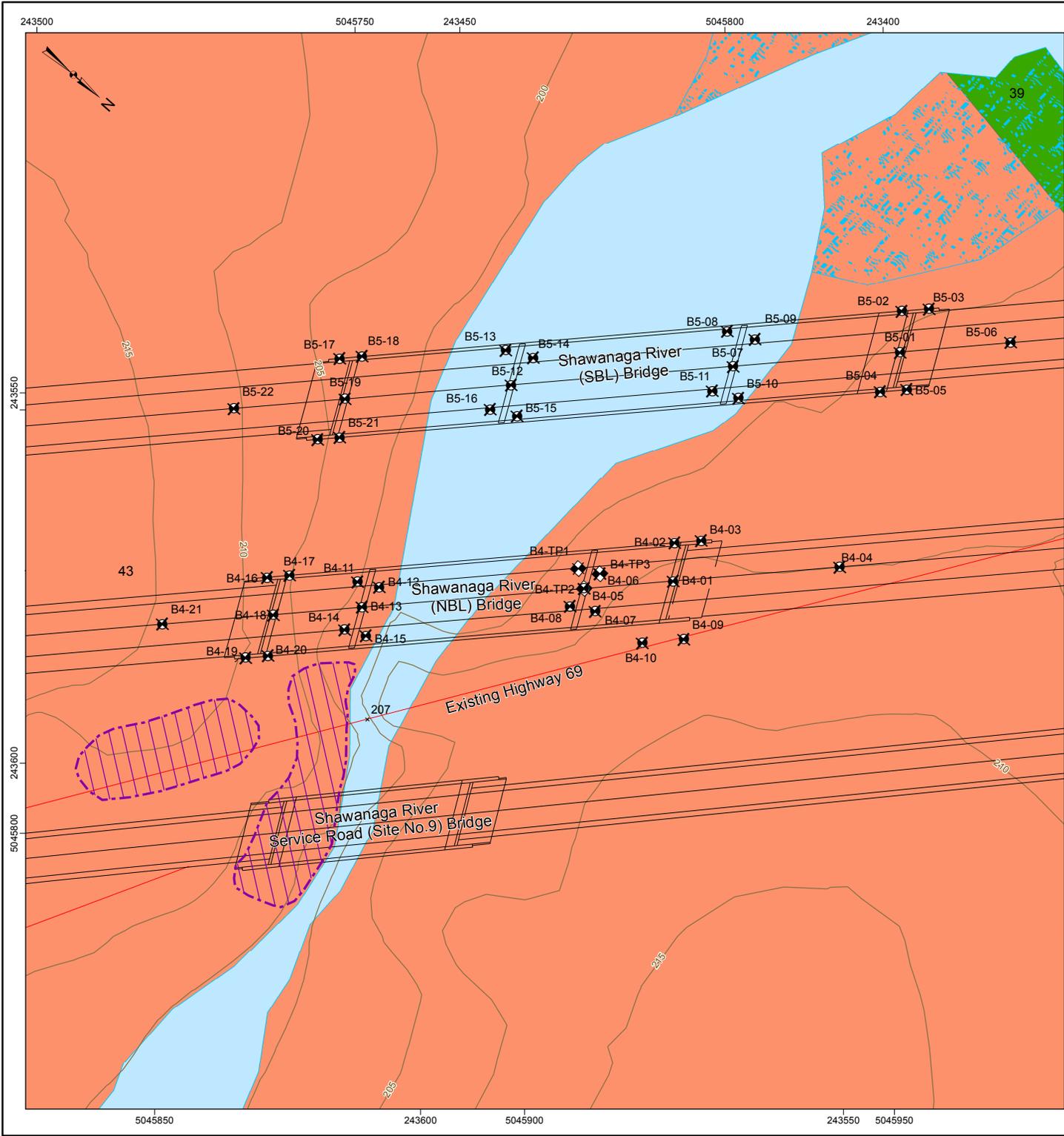
LEGEND		
BH	SAMPLE	SYMBOL
B4-03	1	•
		◊
		▲
		■
		○
		◇
		△
		□



APPENDIX C

ROCK MAPPING

G:\Projects\2007\07-1111-0029_MRC_ParrySound\GIS\MXDs\Draft\Bedrock-Geology_8.5x11_Rotated.mxd



LEGEND

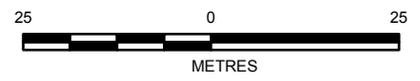
- + Spot Height
- Building
- ⊕ Borehole
- ⊕ Test Pit
- ▭ August 2009 Field Mapping Areas
- Proposed Highway 69
- Existing Road
- Rivers
- UtilityLine
- Contours - 5m Interval
- Water Area, Permanent
- Wetland, Permanent

GEOLOGICAL LEGEND

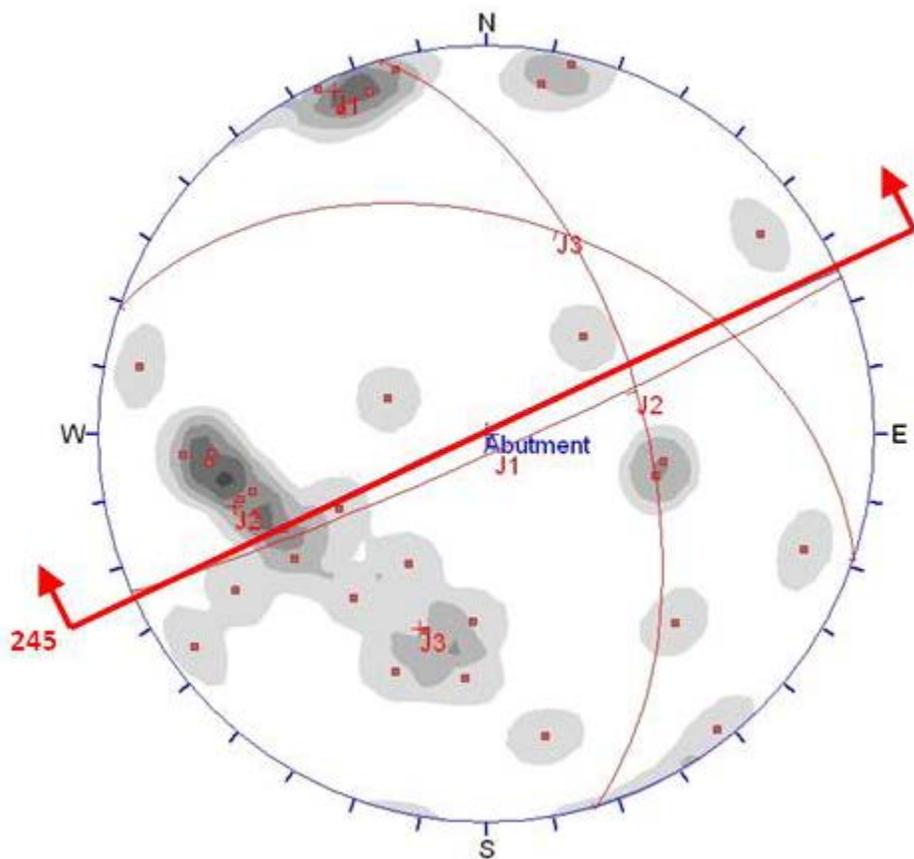
- 39 Gneisses of metasedimentary origin
- 43 Felsic igneous rocks

REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v2006.4
 Bedrock Geology of Ontario - The Ontario Geological Survey, issued 2003
 Produced by Golder Associates Ltd under licence from
 Ontario Ministry of Natural Resources, © Queens Printer 2008
 Projection: Modified Transverse Mercator Datum: NAD 83 Projection: MTM Zone 10



PROJECT		HIGHWAY 69 FOUR-LANING GWP 5402-05-00	
TITLE		BEDROCK GEOLOGY MAP SHAWANAGA RIVER BRIDGES	
 Golder Associates Mississauga, Ontario	PROJECT No. 07-1111-0029	SCALE AS SHOWN	REV. 0
	DESIGN CGE 24 Apr. 2008		
	GIS SO 12 Mar. 2015		
	CHECK VA 12 Mar. 2015		
REVIEW VA 12 Mar. 2015			FIGURE: C1



Fisher Concentrations
% of total per 1.0 % area



0.00 ~ 2.00 %
2.00 ~ 4.00 %
4.00 ~ 6.00 %
6.00 ~ 8.00 %
8.00 ~ 10.00 %
10.00 ~ 12.00 %
12.00 ~ 14.00 %

Orientations

ID	Dip	Direction
J1	m	86 / 156
J2	m	57 / 074
J3	m	44 / 019

No Bias Correction
Max. Conc. = 10.7163%

Equal Area
Lower Hemisphere
32 Poles
32 Entries

PROJECT Highway 69 Shawanaga River Bridge (NBL) Structure

TITLE **Shawanaga River Bridge (NBL) South Abutment Bedrock Outcrop Data**



PROJECT No. 07-1111-0029			FILE No. ----	
DESIGN	DAC	APR 2011	SCALE	AS SHOWN
CADD	DAC	APR 2011	REV.	
CHECK	JPD	APR 2011		
REVIEW	JMAC	APR 2011		

Figure C2

Old Highway 69 South Abutment (pre-1957)



Existing Highway 69 South Abutment



PROJECT		Highway 69 Shawanaga River Bridge (NBL) Structure			
TITLE		Rock Mapping Photos			
PROJECT No. 07-1111-0029		FILE No. ----			
DESIGN	DAC	APR 2011	SCALE	AS SHOWN	REV.
CADD	DAC	APR 2011	Figure C3		
CHECK	JPD	APR 2011			
REVIEW	JMAC	APR 2011			



Proposed Shawanaga River Bridge (NBL) South Abutment



East Side of Proposed Highway 69 NBL between about STA 17+ 620 and 17+650



PROJECT		Highway 69 Shawanaga River Bridge (NBL) Structure			
TITLE		Rock Mapping Photos			
PROJECT No. 07-1111-0029		FILE No. ----			
DESIGN	DAC	APR 2011	SCALE		AS SHOWN
CADD	DAC	APR 2011			REV.
CHECK	JPD	APR 2011			
REVIEW	JMAC	APR 2011			



Figure C4

East Side of Proposed Shawanaga River Bridge (NBL) South Abutment



PROJECT		Highway 69 Shawanaga River Bridge (NBL) Structure			
TITLE		Rock Mapping Photo			
PROJECT No. 07-1111-0029		FILE No. ----			
DESIGN	DAC	APR 2011	SCALE	AS SHOWN	REV.
CADD	DAC	APR 2011	Figure C5		
CHECK	JPD	APR 2011			
REVIEW	JMAC	APR 2011			



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solutions@golder.com
www.golder.com



Golder Associates Ltd.
6925 Century Avenue, Suite #100
Mississauga, Ontario, L5N 7K2
Canada
T: +1 (905) 567 4444

