

**FOUNDATION INVESTIGATION REPORT
FOR
HIGH FILL EMBANKMENTS
G.W.P. 60-00-00
HIGHWAY 401 WIDENING
WINDSOR, ONTARIO**

PLEASE DISREGARD ALL COMMENTS RELATED TO EMBANKMENT 3 IN THIS REPORT

Distribution:

4 cc: Ministry of Transportation
1 cc: Foundation Investigation Report, Ministry of Transportation 1
cc: PML Hamilton
1 cc: PML Toronto

TABLE OF CONTENTS

INTRODUCTION	1
PHYSIOGRAPHY AND GEOLOGY	1
INVESTIGATION PROCEDURES	2
SUMMARIZED SUBSURFACE CONDITIONS.....	4
Embankment 1	5
Fill	5
Topsoil	5
Silty Clay	6
Sandy Clayey Silt Till.....	6
Sand.....	6
Sandy Silty Clay Till.....	6
Sand and Gravel	7
Bedrock.....	7
Embankment 2	8
Fill	8
Topsoil	8
Silty Clay	9
Sand.....	9
Sandy Silty Clay Till.....	9
Sand/Silt.....	10
Silty Clay Till.....	10
Bedrock.....	11

TABLE OF CONTENTS cont'd

Embankment 3	11
Fill	11
Topsoil	12
Silty Clay Till.....	12
Silt/Sand.....	13
Silt and Sand Till	13
Silt Till	14
Groundwater	14
CLOSURE	15

APPENDIX A	Table I	– Summary of Locations and Characteristic Features Of Embankments
	Table II	– Summary of Subsoil Conditions
	Figures 1 to 3	– Plasticity Charts
	Figures 4 to 6	– Particle Size Distribution Charts
	Figures 7 to 9	– Laboratory Consolidation Test Results

APPENDIX B	Embankment 1	– Record of Borehole Sheets – Drawings 1-1 to 1-3
	Embankment 2	– Record of Borehole Sheets – Drawings 2-1 to 2-3
	Embankment 3	– Record of Borehole Sheets – Drawings 3-1 to 3-4

FOUNDATION INVESTIGATION REPORT

for

High Fill Embankments

G.W.P. 60-00-00

Highway 401 Widening

Windsor, Ontario

INTRODUCTION

This report summarizes the results of the foundation investigation carried out for construction/widening of three high fill embankments along Highway 401 in the 45 kilometer long study corridor that extends from the Highway 401/Highway 3 junction in Windsor to Tilbury, Ontario. The investigation was conducted for the Southwestern Region Structural Section of the Ontario Ministry of Transportation.

For the purposes of this report, the embankments have been identified by sequential numbers as follows:

Embankment 1 – Eastbound Lanes, North Leg at Dougall Avenue

Embankment 2 – Walker Road to Essex Road 46

Embankment 3 – 100 m west of Belle River to Belle River Road

Details concerning the location, length, height and type of each embankment (widening or new) are provided in Table I (Appendix A).

PHYSIOGRAPHY AND GEOLOGY

The study area is part of the Essex Clay Plain physiographic sub-region. It is essentially a till plain smoothed by deposits of lacustrine clay which settled in the depressions while the knobs were being lowered by wave action. In general, the deposits in the sub-region consist of silty clays and/or clayey silts.

The surrounding lands are mainly level and used for a mix of residential, commercial and industrial purposes. The area is crossed by ditches and rivers generally flowing south to north. The west bank of Belle River tapers down gradually to almost river level, while the east bank rises sharply to the general ground level in the area.

The bedrock belonging to the Dundee Formation and anticipated at depths of 35 to 40 m is largely composed of Middle Devonian limestone, dolostone and shale.

INVESTIGATION PROCEDURES

The field work for this investigation was carried out during the period from January to April 2002 and comprised 75 (seventy-five) boreholes put down to depths of 5.0 to 40.7 m below existing grade. The approximate locations of the boreholes advanced at each embankment are shown on the Borehole Location and Soil Strata Plans, Drawings 1-1 to 1-3, 2-1 to 2-3 and 3-1 to 3-4 for Embankments 1, 2 and 3 respectively (Appendix B).

A separate borehole numbering system was established for each of the three embankment locations:

- Embankment 1 includes boreholes designated by numbers 1 to 14 as well as boreholes 71-1 to 71-6 drilled for the Dougall Avenue underpass structure.
- Embankment 2 incorporates boreholes designated by numbers 101 to 112 as well as boreholes 72-1 to 72-4, 73-1 to 73-4, 74-1, 74-3 and 74-4 drilled for the Walker Road, Conrail Railway and Essex Road 46 overpass structures.
- Embankment 3 includes boreholes designated by numbers 201 to 224 and boreholes 84-1 to 84-4 and 85-1 to 85-4 drilled for the Belle River bridge and Belle River Road overpass structures.

The locations of and ground surface elevations at the boreholes were established in the field by Peto MacCallum Ltd. using the following four benchmarks for vertical reference:

BM: Top of standard iron bar designated
SP-HCP 130
Elevation 189.733 (geodetic)

BM 993: Plate on north wall of house at
1965 Provincial Road, 0.2 ft from west
wall and 1.2 ft above grade
Elevation 190.896 (geodetic)

BM: Top of round iron bar on south side of
Highway 401 west of Belle River 15.646
RT 22+793.284
Elevation 184.545 (geodetic)

BM: Top of round iron bar on south side of
Highway 401 west of Belle River Road
18.848 RT 10+435.567
Elevation 187.184 (geodetic)

The boreholes were advanced using continuous flight hollow and solid stem augers as well as mud rotary methods, powered by track-mounted CME-75 Nodwell and truck-mounted CME-75 drill rigs, supplied and operated by specialist drilling contractors, working under the full-time supervision of a member of our engineering staff.

Representative samples of the deposits were recovered at frequent depth intervals using a conventional split spoon sampler during drilling. Standard penetration tests were conducted simultaneously with the sampling operation to assess the strength characteristics of the substrata. In situ vane shear and pocket penetrometer tests were also performed to further assess the shear strength of the cohesive soils. Approximately 3.0 m of rock core was recovered from eight boreholes using NQ rock coring equipment.

The groundwater conditions in the boreholes were closely monitored during the course of the field work. Upon completion of drilling, nine piezometers each consisting of 19 mm PVC pipe slotted over the bottom 600 to 900 mm were installed in boreholes to monitor groundwater conditions.

The annular space around the pipe was filled with filter gravel or sand, a bentonite seal placed and native material used as backfill or grout poured up to the ground surface as illustrated on the relevant borehole logs. The water level in the piezometers was measured regularly until May 6, 2002.

The deep boreholes were sealed with cement-bentonite grout upon completion of drilling and coring. The remaining boreholes (without piezometers) were backfilled with auger cuttings to the ground surface.

Soils were identified visually in the field in accordance with the MTO Soil Classification procedures. All of the recovered samples were returned to our laboratory for detailed visual examination, classification and routine moisture content determinations. Consolidation, unconfined uniaxial compression and undrained triaxial compression tests were performed on samples retrieved from each embankment to determine the compressibility and strength characteristics of the cohesive soils. Atterberg Limits tests and grain size distribution analyses were carried out on selected samples, their results being presented in Figures 1 to 6 (Appendix A) and on the Record of Borehole sheets (Appendix B). The results of consolidation tests are presented in Figures 7 to 9 (Appendix A).

SUMMARIZED SUBSURFACE CONDITIONS

Reference is made to the appended Record of Borehole sheets for details of the subsurface conditions including soil classifications, inferred stratigraphy, boundary elevations, standard penetration test N-values, in situ vane shear/pocket penetrometer test results and groundwater observations. The results of laboratory Atterberg Limits tests, grain size distribution analyses and moisture content determinations are also shown on the borehole logs.

Stratigraphic soil profiles along the toe(s) of each embankment prepared from the borehole data are presented on Drawings 1-1 to 1-3, 2-1 to 2-3 and 3-1 to 3-4 for Embankments 1, 2 and 3 respectively. The boundaries between soil strata have been established at the borehole locations only. Between boreholes, the boundaries are assumed and may vary.

The subsurface stratigraphy revealed in the boreholes drilled at the site generally comprised a surficial fill or topsoil underlain by clay till containing discontinuous deposits of silt and sand. Limestone bedrock was contacted below the predominantly clay till deposit at depths of 31.9 to 40.7 m (elevation 150.3 to 161.0).

The subsoil conditions existing at each embankment are summarized in Table II (Appendix A). An embankment-specific description of the geotechnical characteristics of the major types of soil encountered is presented below.

Embankment 1

The subsurface stratigraphy revealed in the boreholes drilled at the site generally comprised a surficial fill or topsoil underlain by cohesive sandy silty clay till overlying a discontinuous sand and gravel deposit. Limestone bedrock was contacted below the predominantly clay till deposit at depths of 39.9 to 40.7 m (elevation 150.3 to 151.4). The strata encountered are summarized below.

Fill

Surficial fill was present in boreholes 71-2 to 71-4 and 71-6. It typically consisted of firm to stiff silty clay and was 0.7 to 1.7 m thick. In borehole 71-3 put down near a maintenance hole, 2.9 m of very loose crushed limestone was encountered. The moisture content of the fill measured in borehole 71-4 was 14%.

Topsoil

Surficial topsoil was present in boreholes 1, 2, 4 to 6, 9 to 11, 13 and 71-5. It was 100 to 200 mm thick and composed of silty clay. A 200 to 400 mm thick topsoil layer of stiff silty clay was revealed directly beneath the fill in boreholes 71-4 and 71-6 at respective depths of 1.7 and 0.7 m (elevation 189.8 and 190.9). In borehole 71-4, the topsoil had a moisture content of 30%.

Silty Clay

A 2.1 m thick layer of stiff cohesive silty clay was identified surficially in borehole 71-1. Having moisture contents of 14 and 21%, the silty clay showed a value of unconfined strength of 95 kPa in one pocket penetrometer test conducted.

Sandy Clayey Silt Till

Cohesive sandy clayey silt till was revealed below the topsoil in borehole 13. The silt till was 1.3 m thick and very stiff in consistency.

Sand

Underlying the silt till at 1.4 m depth (elevation 189.7) in borehole 13 was silty fine sand. This unit was 400 mm thick and loose in relative density.

Sandy Silty Clay Till

Cohesive sandy silty clay till was encountered either surficially or at depths of 0.1 to 2.9 m (elevation 188.6 to 191.0) in all the boreholes. The consistency of this stratum was stiff to hard. Standard penetration test 'N' values ranged from 8 to 68, being in a typical range of 10 to 25 between depths of about 4 and 25 m. The results of vane shear testing carried out in this unit at depths of 10.0 to 11.5 m indicate undisturbed and remolded shear strengths of 100 to 130 kPa and 60 to 80 kPa respectively (soil sensitivity is about 1.7). A number of pocket penetrometer tests conducted within the unit at various depths gave values of undrained shear strength varying broadly between 10 and 250 kPa, generally decreasing with depth from about 100 to 30 kPa. (Values less than about 30 kPa were likely obtained from testing of disturbed/remoulded samples). The moisture content of the clay till ranged from 11 to 34%, typically from 13 to 20%.

The results of the Atterberg Limits tests performed on the sandy silty clay till are presented in Figure 1 (Appendix A). The till plots as a clay of low plasticity with liquid limits of 26 to 32 and plastic limits of 12 to 16. The results of grain size distribution analyses conducted on the clay till are presented in Figure 4 (Appendix A).

The results of one consolidation test performed on the clay till are presented in Figure 7 (Appendix A) and indicate a preconsolidation pressure of 250 kPa, a compression index C_c of 0.16 and a recompression index C_r of 0.03. The unconfined uniaxial compression test gave a value of 158 kPa that corresponds to an estimated shear strength of about 80 kPa.

The clay till had a confirmed thickness of 33.9 to 39.8 m in boreholes 71-1, 71-4 and 71-5. The unit was not penetrated upon termination of the remaining boreholes at depths of 6.6 to 9.6 m (elevation 180.7 to 184.1).

Sand and Gravel

Underlying the clay till at depths of 36.0 to 38.1 m (elevation 152.9 to 155.5) were the following non-cohesive deposits: a 2.6 m thick layer of fine to coarse sand and gravel in borehole 71-1 and a 4.1 m thick layer of fine to coarse sand in borehole 71-4. The relative density of these deposits ranged from compact to very dense with N-values of 25 and 62.

Bedrock

Limestone bedrock was contacted below the native clay till/sand and gravel at depths of 39.9 to 40.7 m (elevation 150.3 to 151.4) in boreholes 71-1, 71-4 and 71-5.

The measured core recovery varied between 80 and 95%. The RQD determined from the rock cores was in a range of 50 to 70%, indicating a fair quality rock. Complete loss of drill water circulation was experienced immediately following the start of coring in borehole 71-5. Voids of 75 and 100 mm in size were encountered at depths of 41.0 m (elevation 150.5) and 43.9 m (elevation 147.6) in borehole 71-4. No loss of drill water was evident in the remaining cored boreholes.

The unconfined compressive strength of the rock determined on two representative samples from borehole 71-1, corresponding to depths of 41.5 and 43.3 m (elevation 149.5 and 147.7), was 70 and 50 MPa respectively.

Embankment 2

The subsurface stratigraphy revealed in the boreholes drilled at the site generally comprised a surficial fill or topsoil underlain by sandy silty clay till overlying discontinuous deposits of sand or silt. Limestone bedrock was contacted below the predominantly cohesive overburden at depths of 31.9 to 37.3 m (elevation 154.4 to 161.0). The highway embankment fill at the location of the Walker Road overpass is approximately 7 m high and will be raised by about 0.5 m. The existing approaches to the Conrail Railway overpass comprise a fill embankment with a height of approximately 9 m on the west side (the proposed increase being less than 0.5 m) and an existing multi-span structure (to be demolished) on the east side. The highway embankment fill at the location of the Essex Road 46 overpass is 8.6 m high and will be raised by about 0.5 m. The strata encountered are summarized below.

Fill

Surficial fill was present in five boreholes. In boreholes 72-2 and 72-3, the fill was composed of a 750 to 1200 mm thick layer of Granular 'A' crushed limestone over 650 to 900 mm of firm silty clay. In boreholes 73-3 and 73-4 drilled along the alignment of former Conrail railway tracks, the fill consisted of crushed limestone (ballast material) and was 1.2 m thick. In borehole 74-4 drilled through the approach embankment to the existing structure, the silty clay fill was 8.6 m thick, stiff to very stiff in consistency and had a moisture content of 13 to 17%.

Topsoil

Surficial topsoil was present in boreholes 101 to 105, 107 to 109, 111, 112, 72-1, 72-4, 73-2, 74-1 and 74-3. It was 50 to 280 mm thick and composed of silty clay.

Fine to coarse fibrous peat of 300 mm thickness was present surficially in borehole 106.

Silty Clay

A 900 mm thick layer of cohesive silty clay was encountered below the fill in borehole 73-3. In borehole 110, the silty clay was 280 mm thick and present surficially.

Sand

Directly beneath the topsoil in boreholes 102 and 104 was sand – fine to coarse, 0.9 m thick and loose in the former borehole and silty, 1.2 m thick and very loose in the latter.

Sandy Silty Clay Till

Cohesive sandy silty clay till was encountered surficially in borehole 73-1 and at depths varying between 0.1 and 8.6 m (elevation 189.0 to 190.9) in the remaining boreholes. The consistency of this stratum was generally stiff to hard. Standard penetration test 'N' values ranged from 5 to over 100, being in a typical range of 10 to 25 between depths of about 4 and 25 m. The results of one vane shear test carried out in borehole 73-4 at an approximate depth of 18.5 m (elevation 173.5) indicate undisturbed and remolded shear strengths of 100 kPa and 70 kPa respectively (soil sensitivity is 1.4). A number of pocket penetrometer tests conducted within the unit at various depths gave the values of undrained shear strength of 10 to 210 kPa, typically 35 to 110 kPa and generally decreasing with depth from 110 to 35 kPa. (Values less than about 30 kPa were likely obtained from testing of disturbed samples). The moisture content of the clay till typically varied between 13 and 18%, increasing locally to 36%.

The results of the Atterberg Limits tests performed on the clay till are presented in Figure 2 (Appendix A). The clay till plots as a clay of low plasticity with liquid limits of 20 to 32 and plastic limits of 11 to 16. The results of grain size distribution analyses conducted on the clay till are presented in Figure 5 (Appendix A).

The results of one consolidation test performed on the clay till, presented in Figure 8 (Appendix A), indicate a preconsolidation pressure of 250 kPa, a compression index C_c of 0.16 and recompression index C_r of 0.03. The quick undrained triaxial and unconfined uniaxial compression tests gave respective values of 116 and 123 kPa that correspond to an estimated shear strength of about 60 kPa.

The clay till had a confirmed thickness of 23.2 to 36.6 m in boreholes 72-1, 72-4, 73-1, 73-4, 74-1 and 74-4. A 1.4 m thick layer of dense fine sand was encountered within this unit at a depth of 21.2 m (elevation 177.1) in borehole 74-4. The clay till was not penetrated upon termination of the remaining boreholes at depths of 9.1 to 9.6 m (elevation 180.9 to 182.5).

Sand/Silt

Fine sand was revealed at an approximate depth of 25.9 m (elevation 164.9 to 166.2) in boreholes 73-1 and 73-4. It was contacted within the clay till in borehole 73-1 and had a thickness of 2.6 m. In borehole 73-4, this unit was not penetrated upon termination of drilling at 30.4 m depth (elevation 161.8) and was at least 4.5 m thick. The sand was very dense and had a moisture content of 12 to 18%.

A deposit of either fine sand or sandy silt was identified below the clay till in boreholes 74-1 and 74-4. The thickness of the sand deposit (borehole 74-1) was 6.6 m. It was dense to very dense and had a moisture content of about 13%. The silt deposit (borehole 74-4) was 5.5 m in thickness and very dense in relative density.

Silty Clay Till

A 4.3 m thick layer of cohesive silty clay till was encountered at 28.5 m depth (elevation 162.3) in borehole 73-1. The silty clay till was very stiff in consistency and had a moisture content of 25%.

The results of one Atterberg Limits test are included in the envelope shown in Figure 2 (Appendix A). The silty clay till plots as a clay of medium plasticity (liquid limit of 37, plastic limit of 19). The results of a particle size distribution analysis conducted on the silty clay till are presented in Figure 5 (Appendix A).

Bedrock

Limestone bedrock was contacted in boreholes 72-1, 72-4, 73-1, 74-1 and 74-4 below the clay till, sand or silt overburden at depths of 31.9 to 37.3 m (elevation 154.4 to 161.0) confirmed by rock coring.

The measured core recovery varied between 84 and 100%. The RQD determined from the rock cores was in a range of 76 to 85%, indicating a good quality rock. Complete loss of drill water circulation was experienced on encountering a void of 350 mm in size at about 37 m depth (elevation 154.0) in borehole 72-1 and at a depth of 34.6 m (elevation 156.0) in borehole 74-1. No loss of drill water was evident in the remaining cored boreholes.

The unconfined compressive strength of the rock determined on six representative samples from boreholes 72-4, 73-1 and 74-4, corresponding to depths of 33.1 to 39.5 m (elevation 152.8 to 160.8), was in a range of 54 to 90 MPa.

Embankment 3

The subsurface stratigraphy revealed in the boreholes drilled at the site generally comprised a surficial fill or topsoil underlain by cohesive silty clay till containing/overlying discontinuous deposits of sand, silt and sand/silt till. The existing approaches to the Belle River bridge comprise fill embankments of some 4 m in height. The highway embankment fill at the location of the Belle River Road overpass is some 3 to 5 m high and will be raised by up to 0.6 m. The strata encountered are summarized below.

Fill

Surficial fill was present in boreholes 219, 85-1 to 85-4. A 1.4 m thick layer of firm silty clay fill was identified in borehole 219. In the remaining boreholes drilled at the toe of the highway embankment fill, the fill consisted of a 150 to 900 mm thick layer of fine to coarse sand and

crushed gravel, underlain by 300 mm of gravel/cobbles over 900 mm of loose sand in borehole 85-1, by 650 mm of compact sand and silt in borehole 85-2, and by 900 mm of stiff clay in borehole 85-4. Where measured, the moisture content of the fill ranged from 13 to 22%.

Topsoil

Surficial topsoil was present in boreholes 201, 204 to 209, 211, 213, 215, 217, 221, 223, 224, 84-1 to 84-4. It was 50 to 200 mm thick and composed of silty clay. An approximate 1.3 m thick layer of alluvium/topsoil was identified surficially in borehole 202.

Silty Clay Till

Cohesive silty clay till was encountered surficially in boreholes 203, 210, 212, 214, 216, 218, 220, 222 or directly beneath the fill/topsoil at depths of 0.1 to 2.1 m (elevation 178.5 to 184.5) in the remaining boreholes. The consistency of this stratum was generally stiff to hard, locally firm. Standard penetration test 'N' values ranged from 3 to 57, being in a typical range of 10 to 25 between depths of about 4 and 25 m. The results of vane shear testing carried out in this unit at various depths indicate undisturbed and remolded shear strengths of 65 to 185 kPa and 35 to 100 kPa respectively (soil sensitivity is about 1.5 to 1.6). A number of pocket penetrometer tests conducted within the unit gave values of undrained shear strength varying broadly between 15 and 210 kPa, generally decreasing with depth. (Values less than about 30 kPa were likely obtained from testing of disturbed samples). The moisture content of the clay till typically ranged from 18 to 24%, increasing locally to 28 and 36%.

The results of the Atterberg Limits tests performed on the clay till are presented in Figure 3 (Appendix A). The clay till plots as a clay of low to medium plasticity with liquid limits of 26 to 45 and plastic limits of 13 to 28. The results of grain size distribution analyses conducted on the clay till are presented in Figure 6 (Appendix A).

The results of one consolidation test performed on the clay till and presented in Figure 9 (Appendix A) indicate a preconsolidation pressure of 125 kPa, a compression index C_c of 0.24 and recompression index C_r of 0.05. The quick undrained triaxial and unconfined uniaxial compression tests gave respective values of 69 and 97 kPa that correspond to a shear strength of 40 to 45 kPa.

This clay till had a confirmed thickness of about 25 m in borehole 85-4 (4.0 to 5.6 m in boreholes 208, 210 and 218) and was not penetrated upon termination of the remaining boreholes at depths of 5.0 to 24.8 m (elevation 155.3 to 179.7).

Silt/Sand

Within the silty clay till in boreholes 203, 204, 84-1 and 84-2 located on the west side of Belle River were discontinuous deposits of cohesionless silt and/or sand. A 300 to 700 mm thick layer of loose to very loose silt was encountered at depths of 1.4 to 1.6 m (elevation 177.3 to 178.9). The moisture content of the silt was about 18%. The silt deposit was underlain at 2.1 m depth (elevation 177.8 and 178.2) by 800 and 900 mm of loose to compact saturated sand in boreholes 84-1 and 203 respectively. Another layer of sand was contained within the clay till in boreholes 204 and 84-2. Being compact to very dense and 1.5 m thick, it was encountered at depths of 4.0 and 5.6 m (elevation 174.9 and 173.1). The moisture content of the sand was about 15%.

Underlying the clay till at a depth of 26.1 m (elevation 155.3) in borehole 85-4 was a 3.6 m thick deposit of cohesionless silt. The silt was compact in relative density (N-value of 10) and had a moisture content of 7%. It is worth noting that a cobble/boulder was encountered at the bottom of this deposit.

Silt and Sand Till

Silt and sand till of 0.9 and 2.0 m thickness was revealed at depths of 4.0 (elevation 177.1) and 29.7 m (elevation 151.7) in boreholes 214 and 85-4 respectively. This deposit was cohesionless and compact/very dense, with a moisture content of 10%.

Silt Till

Sandy/clayey silt till was identified in boreholes 208, 210, 218 and 85-4 at respective depths of 4.0 to 5.6 m (elevation 175.5 to 176.6) and 31.7 m (elevation 149.7). This unit was compact to very dense/stiff to hard in relative density/consistency and had a moisture content of 11 to 21%. The unit was not penetrated at depths of 5.0 to 6.6 m (elevation 175.2 to 175.6) in boreholes 208, 210, 218 and at 32.7 m depth (elevation 148.7) in borehole 85-4.

Groundwater

Water was observed in 24 of the 75 boreholes during the course of the field work at depths of 0.0 to 7.0 m.

At Embankment 1, groundwater was observed in five boreholes during the course of the field work at depths of 0.0 to 6.6 m. During drilling, it was detected at 1.4 m depth (elevation 189.7) in borehole 13. Upon completion of augering, water was measured at a depth of 6.6 m (elevation 183.9) in borehole 12. Perched water was present surficially in the granular material encountered in borehole 71-3 drilled near a maintenance hole.

At Embankment 2, groundwater was observed in seven boreholes during the course of the field work at depths of 0.0 to 1.7 m. In the process of augering, it was measured at depths of 0.0 to 1.2 m (elevation 189.8 to 191.0) in boreholes 102, 104, 106 and 72-3. Upon completion of drilling, groundwater was observed in borehole 112 at a depth of 0.3 m (elevation 190.1).

At Embankment 3, groundwater was observed in twelve boreholes during the course of the field work at depths of 0.0 to 7.0 m. Water was present surficially in boreholes 210, 212, 216 and perched in the surficial layer of sand and gravel in borehole 85-1. Water was detected in boreholes 84-3 and 84-4 at respective depths of 7.0 and 6.6 m (elevation 173.1 and 173.4) in the process of augering. Water was observed in borehole 85-4 at 12.7 m depth (elevation 168.7) during drilling and at 6.4 m depth (elevation 175.0) one day later. Upon completion of drilling, groundwater was measured at depths of 0.4 and 1.8 m (elevation 178.5 and 178.4) in boreholes 204 and 217 respectively.

Upon completion of drilling, piezometers were installed in nine boreholes (one piezometer was destroyed prior to reading). A number of piezometer readings subsequently taken showed water levels to be at minimum depths of 0.1 to 2.2 m (elevation 189.1 to 190.5 at Embankments 1 and 2, elevation 180.4 to 181.4 at Embankment 3). The details of depths/elevations are provided in the following table:

Date	Embankment 1				Embankment 2			
	Borehole 71-2 (Station 12+700, Dougall Ave. chainage)		Borehole 71-6 (Station 12+845, Dougall Ave. chainage)		Borehole 103 (Station 12+685, Highway 401 chainage)		Borehole 109 (Station 12+925, Highway 401 chainage)	
	Depth (m)	Elevation	Depth (m)	Elevation	Depth (m)	Elevation	Depth (m)	Elevation
February 7, 2002	6.0	185.4	8.6	183.1	-	-	-	-
February 14, 2002	3.6	187.8	7.3	184.4	-	-	-	-
February 25, 2002	-	-	-	-	8.6	182.2	5.3	185.5
March 5, 2002	1.4	190.0	2.5	189.2	-	-	-	-
March 28, 2002	1.1	190.3	2.3	189.4	1.7	189.1	0.5	190.3
May 6, 2002	0.9	190.5	2.2	189.5	-	-	-	-

Date	Embankment 2				Embankment 3			
	Borehole 112 (Station 13+095, Highway 401 chainage)		Borehole 202 (Station 22+735, Highway 401 chainage)		Borehole 219 (Station 10+390, Highway 401 chainage)		Borehole 85-3 (Station 10+485, Highway 401 chainage)	
	Depth (m)	Elevation	Depth (m)	Elevation	Depth (m)	Elevation	Depth (m)	Elevation
January 18, 2002	-	-	-	-	-	-	9.0	172.7
January 21, 2002	-	-	-	-	-	-	8.9	172.8
February 25, 2002	4.7	185.7	-	-	-	-	-	-
March 28, 2002	0.2	190.2	-	-	1.3	180.4	0.3	181.4
April 1, 2002	-	-	0.1	180.6	-	-	-	-

Groundwater levels may fluctuate subject to seasonal variations and precipitation patterns.

CLOSURE

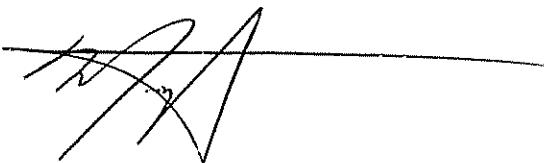
The field work was carried out under the supervision of Mr. M. Rapsey and direction of Mr. M.R. Anderson, M. Eng., P.Eng., Senior Foundation Engineer. The equipment was supplied by All-Terrain Drilling Limited.

Peto MacCallum Ltd.
CONSULTING ENGINEERS

The report was prepared by Mr. G.O. Degil, Ph.D., Senior Project Supervisor. It was reviewed by Mr. M.R. Anderson, M.Eng., P.Eng., Senior Foundation Engineer, and Mr. D.W. Kerr, M.Eng., P.Eng., Chief Foundation Engineer. Mr. B.R. Gray, M.Eng., P.Eng. carried out an independent review of the report.



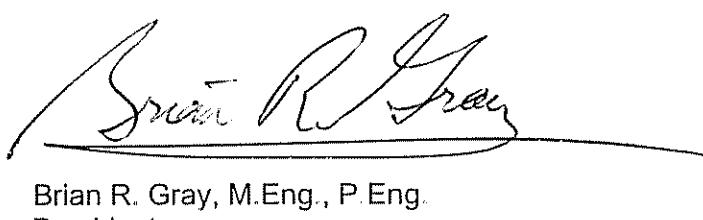
Yours very truly
Peto MacCallum Ltd.



Murray R. Anderson, M.Eng., P.Eng.
Senior Foundation Engineer



Dennis W. Kerr, M.Eng., P.Eng.
Chief Foundation Engineer



Brian R. Gray, M.Eng., P.Eng.
President

GD:lad

APPENDIX A

- | | | |
|----------------|---|---|
| TABLE I | - | SUMMARY OF LOCATIONS AND
CHARACTERISTIC FEATURES OF
EMBANKMENTS |
| TABLE II | - | SUMMARY OF SUBSOIL CONDITIONS |
| FIGURES 1 TO 3 | - | PLASTICITY CHARTS |
| FIGURES 4 TO 6 | - | PARTICLE SIZE DISTRIBUTION CHARTS |
| FIGURES 7 TO 9 | - | LABORATORY CONSOLIDATION TEST
RESULTS |

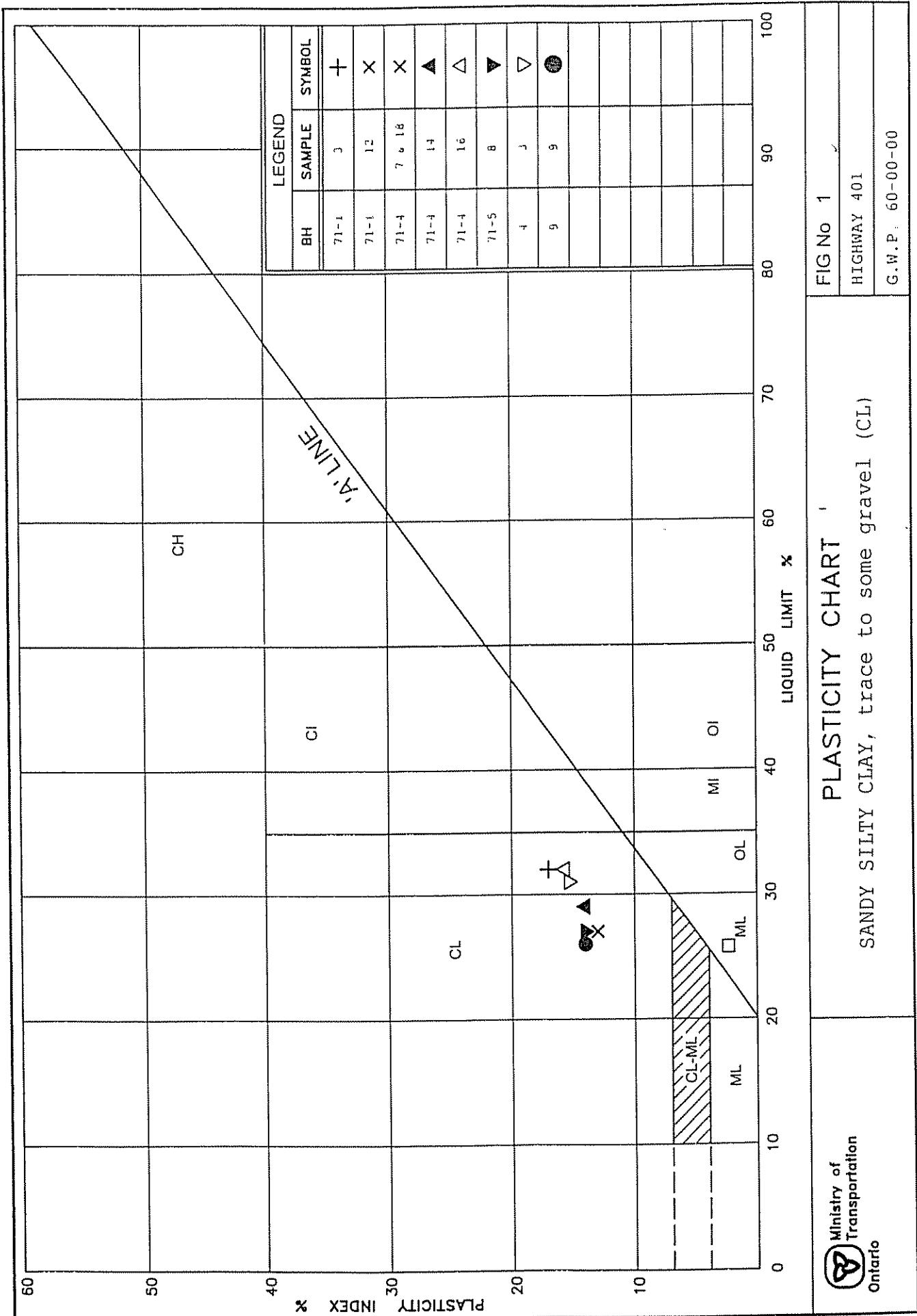
TABLE I
SUMMARY OF LOCATIONS AND
CHARACTERISTIC FEATURES OF EMBANKMENTS
G.W.P. 60-00-00
HIGHWAY 401 WIDENING
WINDSOR, ONTARIO

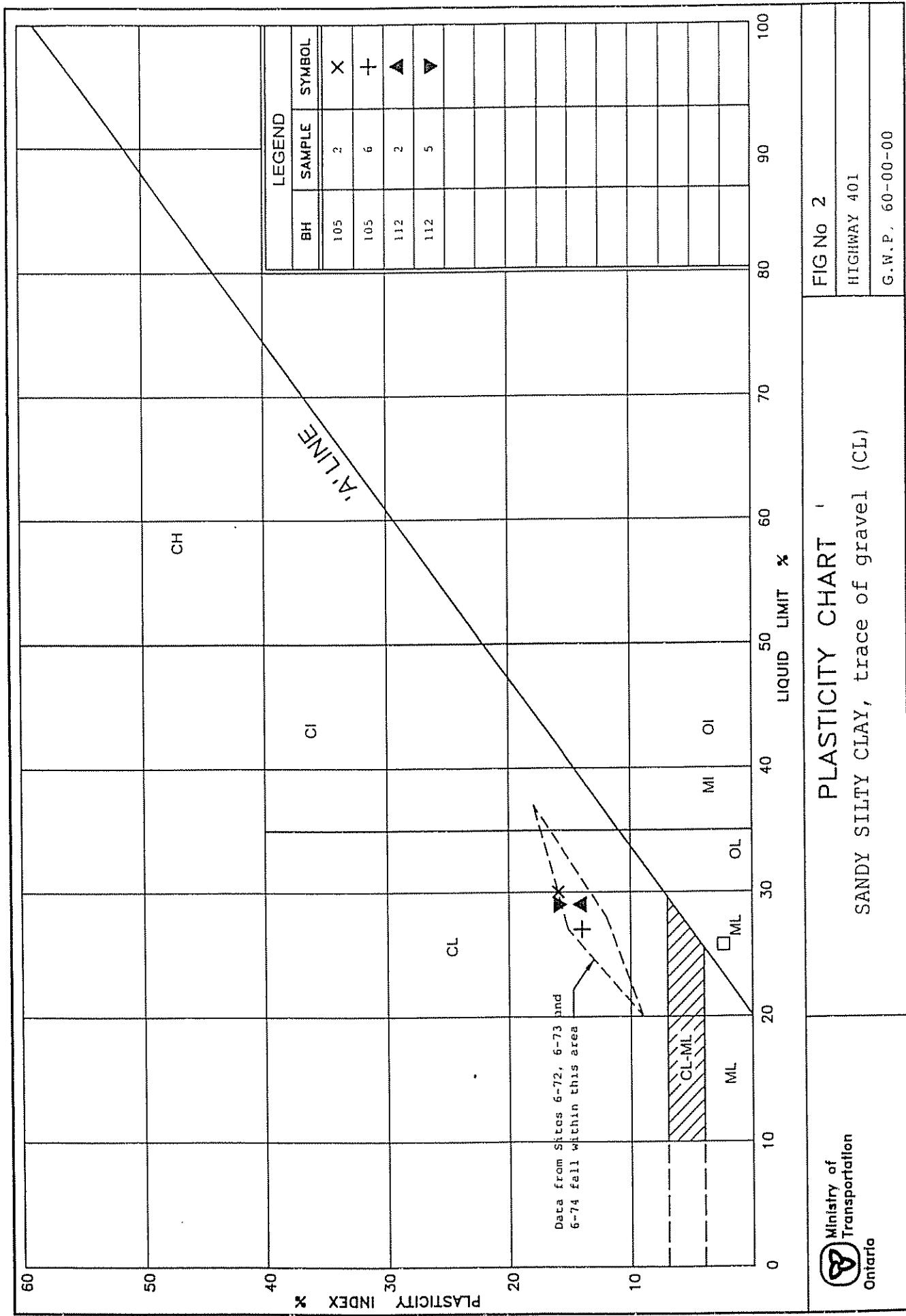
Embankment No.	Location ⁽¹⁾	Length ⁽¹⁾ (m)	Height (m)	Remarks
1	North leg at Dougall Avenue, Approx. Station 12+480 to 13+000, Dougall Avenue chainage	520	9	New embankment
2	Walker Road to Essex Road 46, Approx. Station 12+550 to 13+200, Highway 401 chainage	650	9	Widening of the existing embankment to the north. Removing an approximate 80 m long section of the Conrail Railway structure and filling in
3	100 m west of Belle River to Belle River Road, Approx. Station 22+700 (Township of Maidstone) to 10+550 (Township of Rochester), Highway 401 chainage	600	5	Widening of the existing embankment on both sides. A wetland is reported to exist on the north side of the embankment

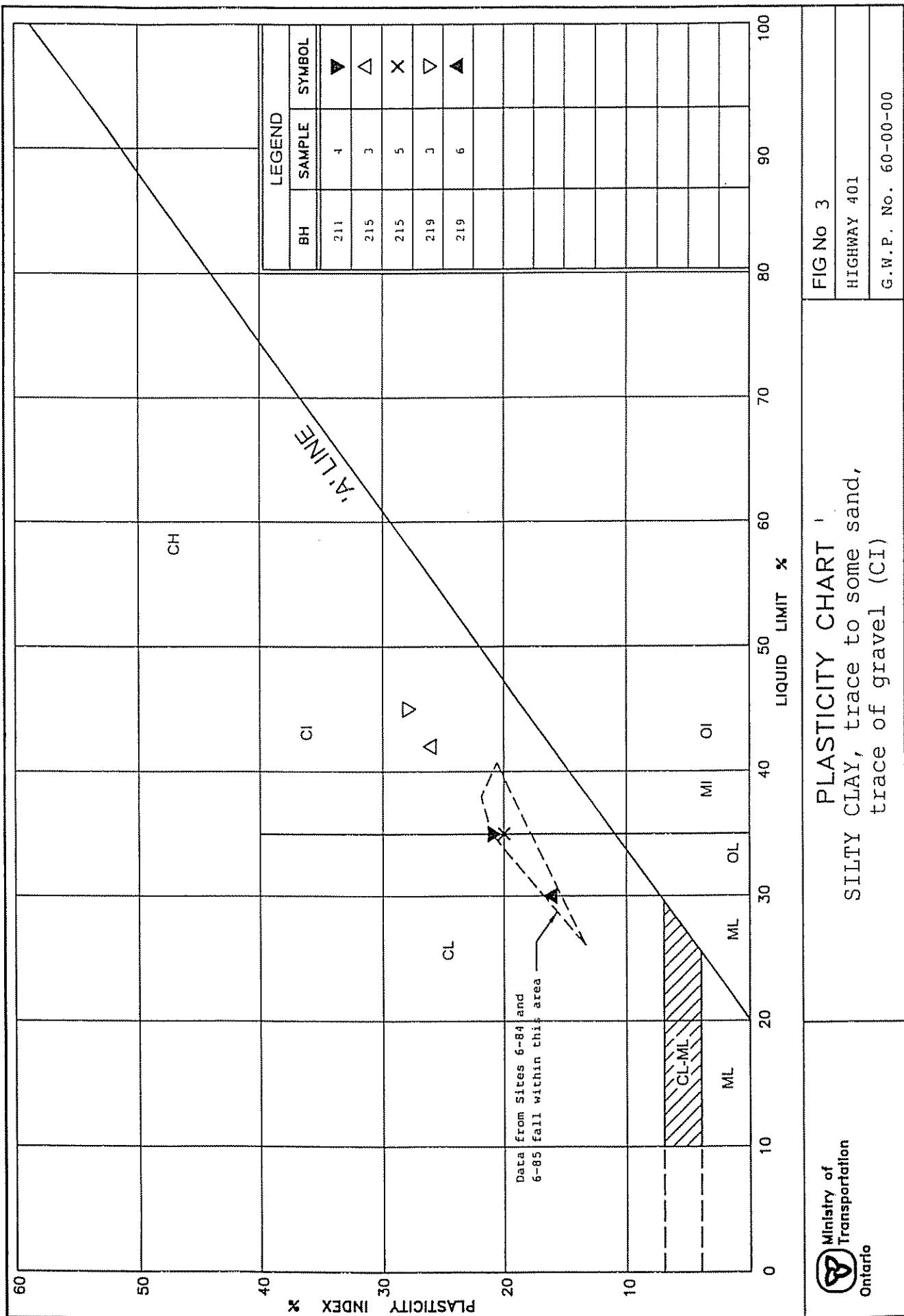
(1) Deduced from the drawings provided by MTO.

TABLE II
SUMMARY OF SUBSOIL CONDITIONS
G.W.P. 60-00-00
HIGHWAY 401 WIDENING
WINDSOR, ONTARIO

Embankment No.	Location	No. of Boreholes	Fill/Topsoil Thickness (m)	Clay Till Thickness (m)	Depth to Bedrock (m)	Notes and/or Underlying Materials
1 (Dougall Ave.)	Approx. Sta. 12+480 to 13+000, Dougall Ave. chainage	20	0.0-2.9	33.9-39.8	39.9-40.7 (El. 150.3-151.4)	Surficial fill/topsoil in 4 boreholes, surficial thin topsoil layer in 10 boreholes, and surficial native clay/clay till in 6 boreholes underlain by clayey silt till. (Predominantly sandy clayey silt) Basal sand and gravel revealed in 2 boreholes below sandy silty clay till. Bedrock contacted in 3 boreholes
2 (Walker Rd. to Essex Rd. 46)	Approx. Sta. 12+550 to 13+200, Highway 401 chainage	23	0.8-8.6	23.2-36.6	31.9-37.3 (El. 154.4-161.0)	Surficial fill in 5 boreholes, thin surficial topsoil layer in 15 boreholes, peat in 1 borehole, and native clay till at surface in 1 borehole underlain by silty clay till. (Predominantly sandy silty clay) Sand revealed below topsoil in 1 borehole. Fine sand encountered within sandy silty clay till in 2 boreholes and below till in 2 boreholes; sandy silt identified beneath sandy silty clay till in 1 borehole. Bedrock contacted in 5 boreholes
3 (Belle River)	Approx. Sta. 22+700 to 10+550, Highway 401 chainage	32	0.1-2.1	25.0	Bedrock not contacted at exploration depths of 5.0-32.7 (El. 148.7-179.7)	Surficial fill in 5 boreholes, thin surficial topsoil layer in 18 boreholes, alluvium in 1 borehole, and clay till at surface in 8 boreholes underlain by silty clay till. (Predominantly silty clay) Discontinuous deposits of silt and/or sand encountered within silty clay till in 8 boreholes. Silt over gravelly sand and silt till underlain by clayey sandy silt till revealed below silty clay till in 1 borehole

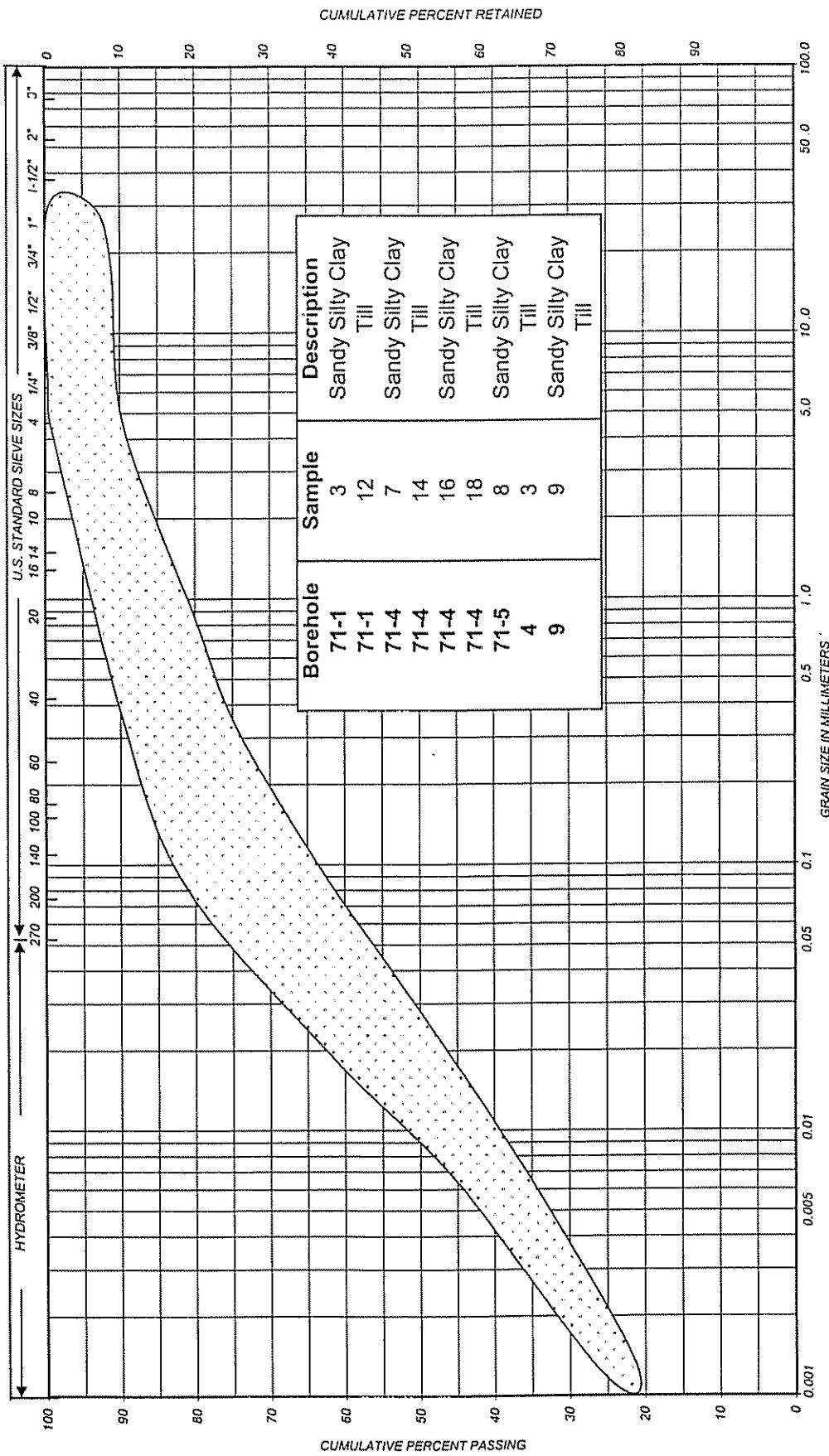






PARTICLE SIZE DISTRIBUTION CHART

FML REF
G.W.P.
FIGURE
4

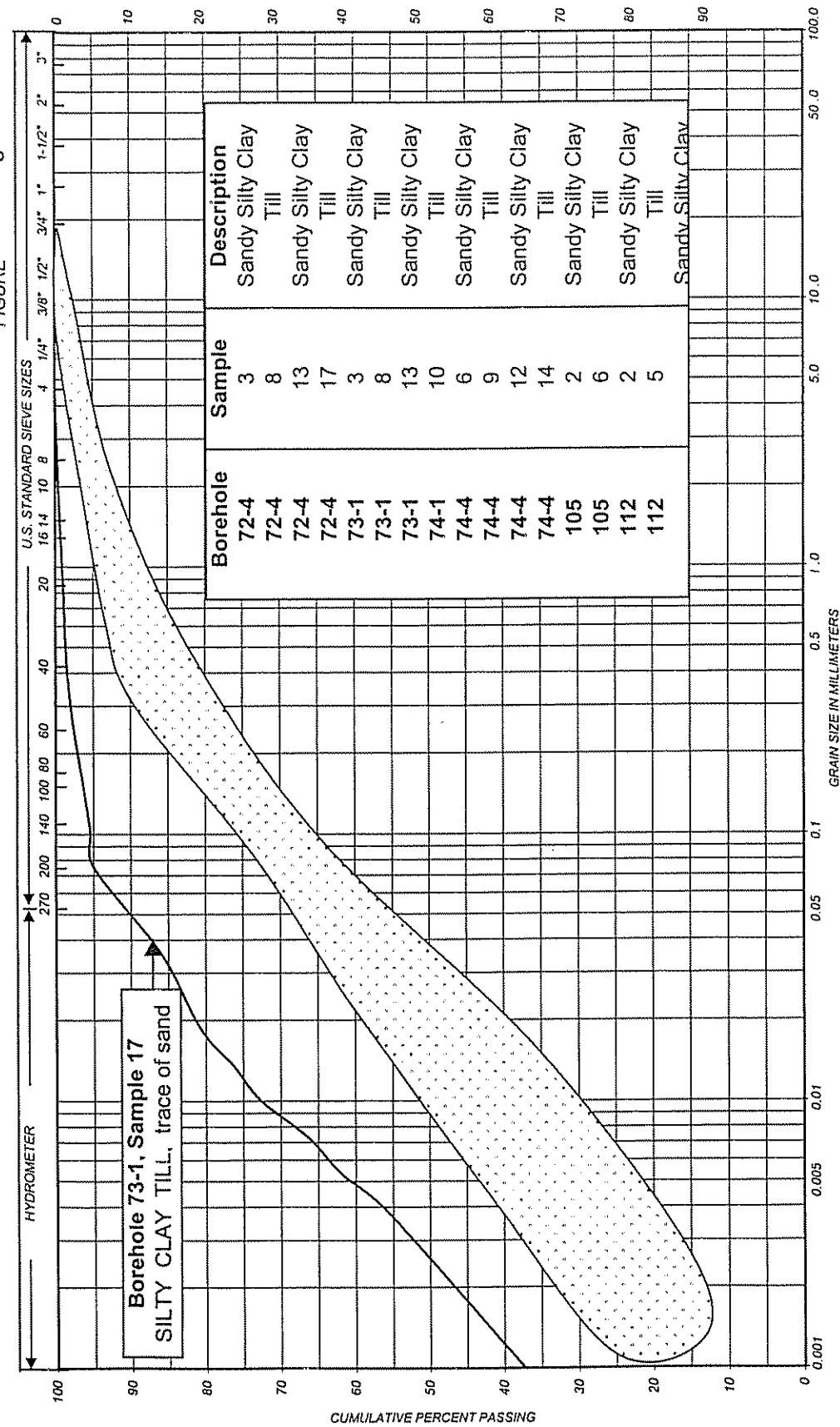


SILT & CLAY			FINE MEDIUM COARSE			GRAVEL			UNIFIED	
FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	SAND	SAND	SAND	CLAY	
CLAY	SILT	SILT	VERY FINE	FINE	MEDIUM	COARSE	COARSE	COARSE	CLAY	
									CLAY	

REMARKS SANDY SILTY CLAY TILL, trace to some gravel

PARTICLE SIZE DISTRIBUTION CHART

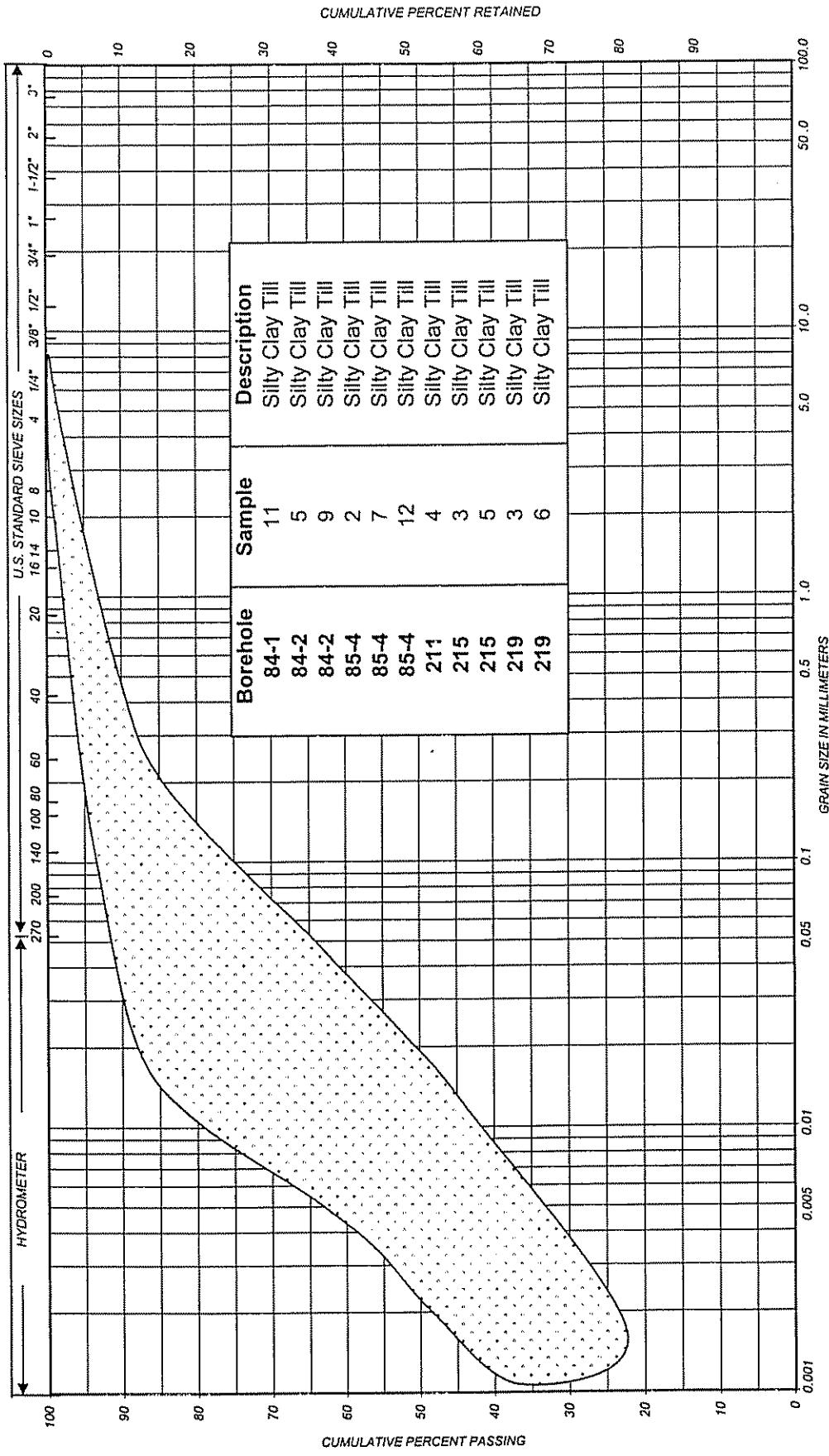
PML REF.
G.W.P.
5



SILT & CLAY		FINE		MEDIUM		COARSE		GRAVEL		UNIFIED	
CLAY	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	SAND	MEDIUM	COARSE	SILT	CLAY
	VERY FINE	FINE	SILT								COBBLES A.I.T.

REMARKS SANDY SILTY CLAY TILL, trace of gravel

PARTICLE SIZE DISTRIBUTION CHART



SILT & CLAY		FINE		MEDIUM		COARSE		GRAVEL		UNIFIED	
		FINE	MEDIUM	SAND		MEDIUM	COARSE	SAND		CLAY	MATERIAL
		SILT									MATERIAL
			VERY FINE	FINE	MEDIUM	COARSE					COCOBLES
				SAND							M.I.T.
											U.S. BUREAU

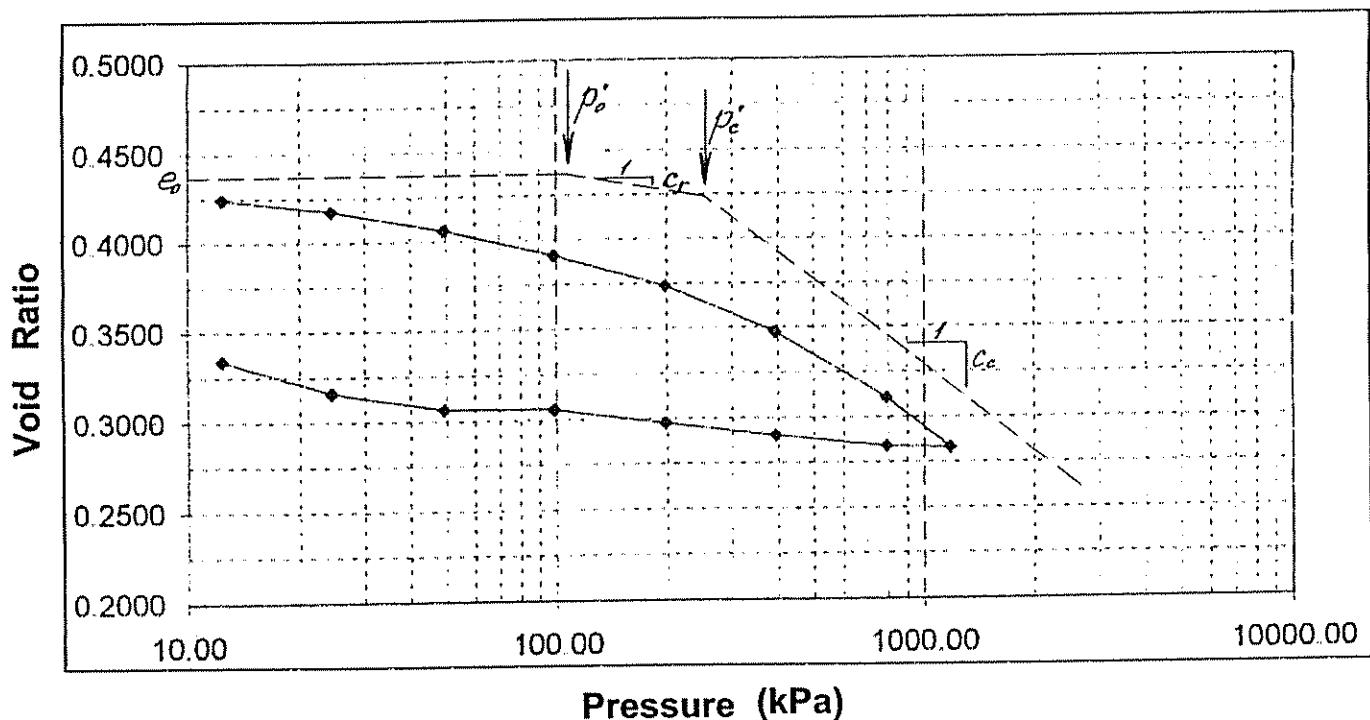
REMARKS SILTY CLAY TILL, trace to some sand, trace of gravel

Laboratory Consolidation Test Results

Highway 401
 Dougall Avenue to Belle River Road
 G.W.P. 60-00-00
 District 31, Township of Sandwich South, Windsor, Ontario

BOREHOLE 9, SAMPLE 9

Void Ratio versus Log of Pressure



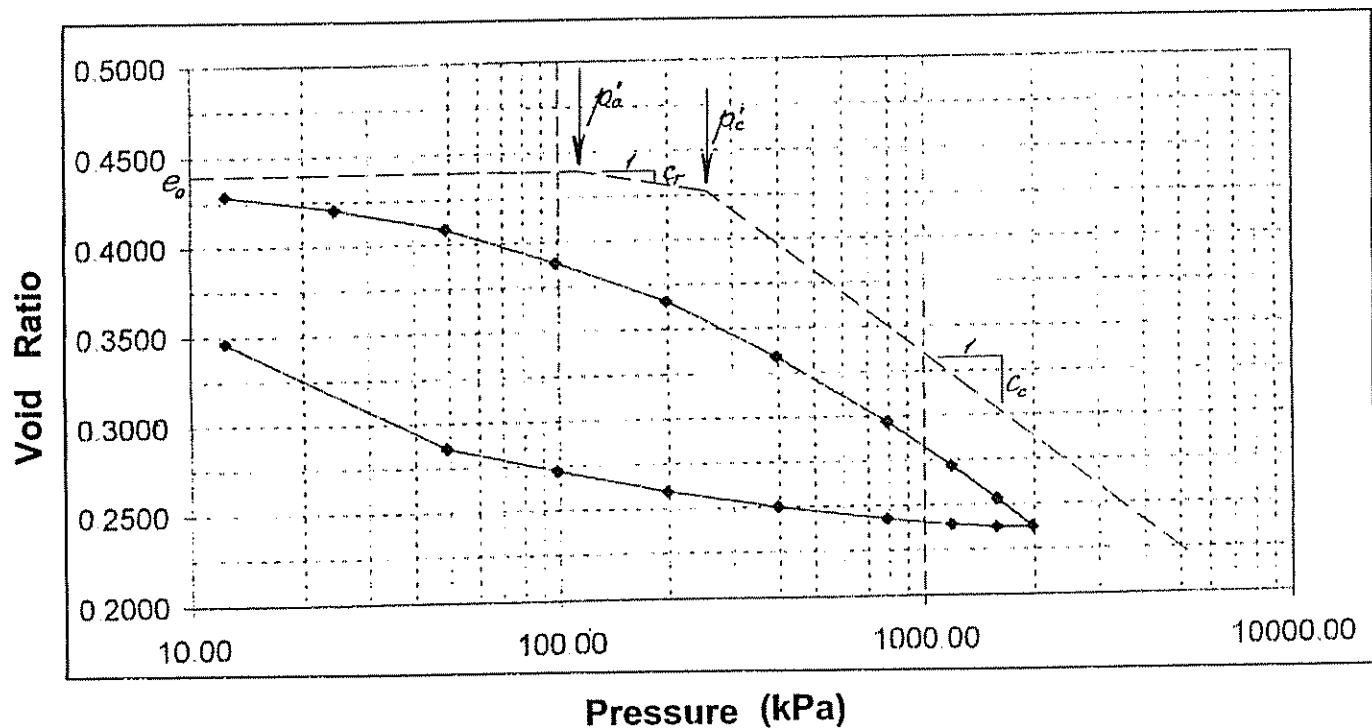
SOIL TYPE: SANDY SILTY CLAY, trace to some gravel		DEPTH: 9.2 - 9.6 m
$e_o = 0.44$	$p'_o = 105 \text{ kPa}$	FIGURE: 7
$w_o = 17\%$	$p'_c = 250 \text{ kPa}$	HIGHWAY 401 / Embankment 1
$\gamma = 22.0 \text{ kN/m}^3$	$C_c = 0.16$	TWP OF SANDWICH SOUTH, WINDSOR
$w_L = 26; w_p = 12; PI = 14$	$C_r = 0.03$	G. W. P. 60-00-00

Laboratory Consolidation Test Results

Highway 401
 Dougall Avenue to Belle River Road
 G.W.P. 60-00-00
 District 31, Township of Sandwich South, Windsor, Ontario

BOREHOLE 74-1, SAMPLE 10

Void Ratio versus Log of Pressure

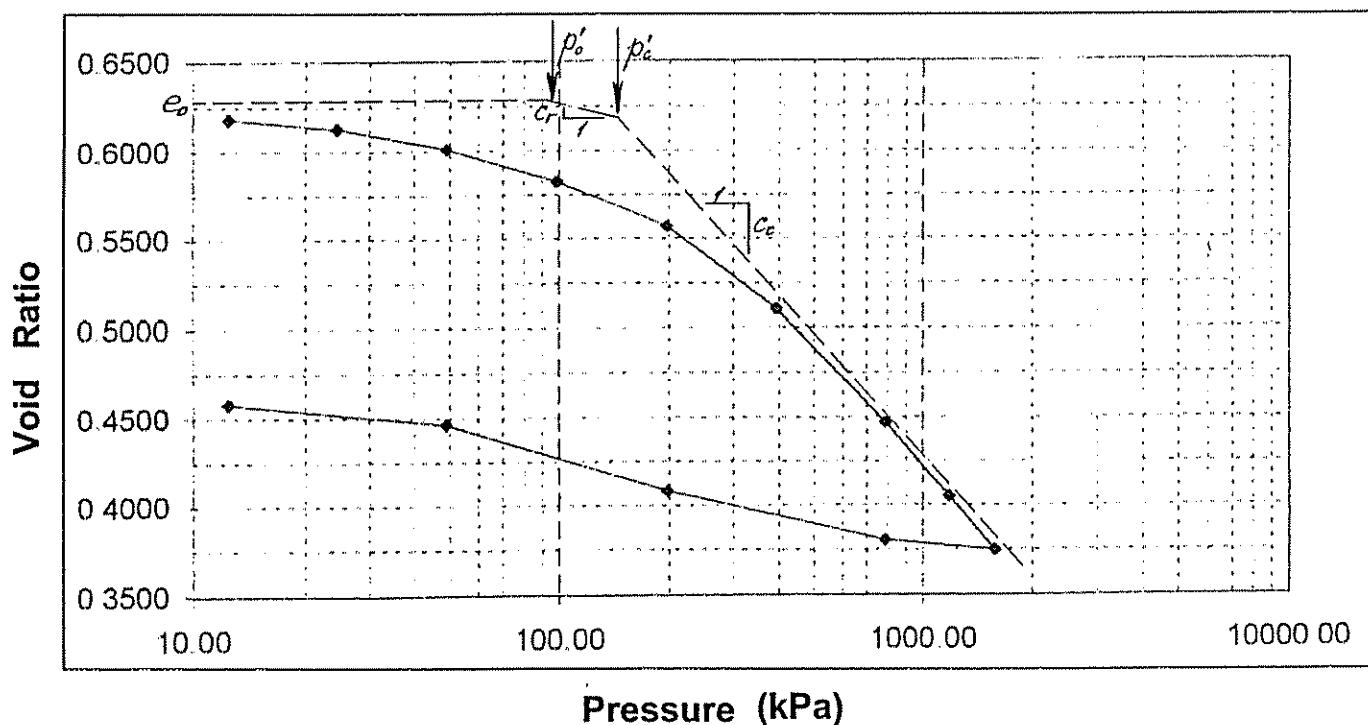


SOIL TYPE: SANDY SILTY CLAY, trace of gravel		DEPTH: 10.7 - 11.1 m
$e_0 = 0.44$	$p'_o = 110 \text{ kPa}$	FIGURE: 8
$w_o = 16\%$	$p'_c = 250 \text{ kPa}$	HIGHWAY 401 / Embankment 2
$\gamma = 21.5 \text{ kN/m}^3$	$C_c = 0.16$	TWP OF SANDWICH SOUTH, WINDSOR
$w_L = 27; w_p = 12; PI = 15$	$C_r = 0.03$	G. W. P. 60 - 00 - 00

Laboratory Consolidation Test Results

Highway 401
 Dougall Avenue to Belle River Road
 G.W.P. 60-00-00
 District 31, Townships of Maidstone and Rochester,
 North Woodslee, Ontario
 BOREHOLE 84-2, SAMPLE 9

Void Ratio versus Log of Pressure



SOIL TYPE: SILTY CLAY, trace to some sand, trace of gravel		DEPTH: 9.2 - 9.6 m
$e_o = 0.63$	$p'_o = 95 \text{ kPa}$	FIGURE: 9
$w_o = 25\%$	$p'_c = 125 \text{ kPa}$	HIGHWAY 401 / Embankment 3
$\gamma = 20.6 \text{ kN/m}^3$	$C_c = 0.24$	TWPS OF MAIDSTONE and ROCHESTER
$w_L = 35; w_p = 14; PI = 21$	$C_r = 0.05$	G. W. P. 60 - 00 - 00

APPENDIX B

- | | |
|--------------|--|
| EMBANKMENT 1 | RECORD OF BOREHOLE SHEETS
DRAWINGS 1-1 to 1-3 |
| EMBANKMENT 2 | RECORD OF BOREHOLE SHEETS
DRAWINGS 2-1 to 2-3 |
| EMBANKMENT 3 | RECORD OF BOREHOLE SHEETS
DRAWINGS 3-1 to 3-4 |

LIST OF ABBREVIATIONS

PENETRATION RESISTANCE

STANDARD PENETRATION RESISTANCE N' - THE NUMBER OF BLOWS REQUIRED TO ADVANCE A STANDARD SPLIT SPOON SAMPLER 0.3 m INTO THE SUBSOIL. DRIVEN BY MEANS OF A 63.5 kg HAMMER FALLING FREELY A DISTANCE OF 0.76 m

DYNAMIC PENETRATION RESISTANCE: - THE NUMBER OF BLOWS REQUIRED TO ADVANCE A 51 mm 60 DEGREE CONE, FITTED TO THE END OF DRILL RODS 0.3 m INTO THE SUBSOIL. THE DRIVING ENERGY BEING 475 J PER BLOW

DESCRIPTION OF SOIL

THE CONSISTENCY OF COHESIVE SOILS AND THE RELATIVE DENSITY OR DENSENESS OF COHESIONLESS SOILS ARE DESCRIBED IN THE FOLLOWING TERMS:

<u>CONSISTENCY</u>	<u>N' BLOWS/0.3 m</u>	<u>c kPa</u>	<u>DENSENESS</u>	<u>N' BLOWS/0.3 m</u>
VERY SOFT	0 - 2	0 - 12	VERY LOOSE	0 - 4
SOFT	2 - 4	12 - 25	LOOSE	4 - 10
FIRM	4 - 8	25 - 50	COMPACT	10 - 30
STIFF	8 - 15	50 - 100	DENSE	30 - 50
VERY STIFF	15 - 30	100 - 200	VERY DENSE	> 50
HARD	> 30	> 200		
W.T.P.L. WETTER THAN PLASTIC LIMIT			D.T.P.L. DRIER THAN PLASTIC LIMIT	
			A.P.L. ABOUT PLASTIC LIMIT	

TYPE OF SAMPLE

S S	SPLIT SPOON	T.W.	THINWALL OPEN
W.S.	WASHED SAMPLE	T.P.	THINWALL PISTON
S B	SCRAPER BUCKET SAMPLE	O S	OESTERBERG SAMPLE
A.S.	AUGER SAMPLE	F S.	FOIL SAMPLE
C S.	CHUNK SAMPLE	R.C.	ROCK CORE
S T.	SLOTTED TUBE SAMPLE		
P.H.	SAMPLE ADVANCED HYDRAULICALLY		
P M	SAMPLE ADVANCED MANUALLY		

SOIL TESTS

Qu	UNCONFINED COMPRESSION	L.V.	LABORATORY VANE
Q	UNDRAINED TRIAXIAL	F V	FIELD VANE
Qcu	CONSOLIDATED UNDRAINED TRIAXIAL	C	CONSOLIDATION
Qd	DRAINED TRIAXIAL		

▲ - UNDISTURBED AND REMOULDED SHEAR STRENGTH DETERMINED FROM IN SITU VANE TEST

■ - UNDRAINED SHEAR STRENGTH DETERMINED FROM POCKET PENETROMETER TEST

EMBANKMENT 1

RECORD OF BOREHOLE No 1										1 of 1	METRIC			
W.P. <u>64-00-00</u>		LOCATION <u>Dougall Parkway, EBL-N Leg Sta. 12+484, o/s 13.0m Lt. of CL</u>								ORIGINATED BY <u>MR</u>				
DIST <u>31</u>		HWY <u>401</u>		BOREHOLE TYPE <u>Continuous Flight Solid Stem Augers</u>						COMPILED BY <u>GD</u>				
DATUM <u>Geodetic</u>		DATE <u>February 01, 2002</u>								CHECKED BY <u>MRA</u>				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.70	Ground Level													
0.00	Topsoil, silty clay													
0.20	Dark Brown													
	Sandy silty clay, trace of gravel													
	Stiff Brown (Till)		1	SS	11									
			2	SS	9									
			3	SS	48									
			4	SS	50									
			5	SS	26									
			6	SS	20									
184.15	Very Stiff Grey													
6.55	End of Borehole													
	Borehole dry on completion of drilling													
	■ Penetrometer Test													

RECORD OF BOREHOLE No 2										1 of 1	METRIC					
W.P.	64-00-00	LOCATION	Dougall Parkway, EBL-N Leg Sta. 12+489, o/s 15.5m Rt. of CL						ORIGINATED BY	MR						
DIST	31	HWY	401	BOREHOLE TYPE	Continuous Flight Solid Stem Augers						COMPILED BY	GD				
DATUM	Geodetic		DATE	February 04, 2002						CHECKED BY	MRA					
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80					
189.74	Ground Level															
0.00	Topsoil, silty clay															
0.12	Dark Brown															
Sandy silty clay, trace of gravel																
Hard	Brown (Till)															
Very Stiff	Grey															
183.19	End of Borehole															
6.55	Borehole dry on completion of drilling															

RECORD OF BOREHOLE No 3										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+539, o/s 17.1m Lt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 01, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.67	Ground Level													
0.00	Sandy silty clay, trace of gravel Stiff Brown (Till)	1 2 3 4 5 6 7	SS SS SS SS SS SS SS	11 31 56 61 28 13 12			190							
	Hard						189							
	Very Stiff Grey						188							
	Stiff						187							
182.57	End of Borehole						186							
8.10	Groundwater level not established due to piezometer destruction ■ Penetrometer Test <u>Borehole Backfill Legend:</u> Native Backfill Bentonite Seal Filter Gravel Slotted Pipe						185							
							184							
							183							

RECORD OF BOREHOLE No 4										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+532, o/s 14.8m Lt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 04, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
189.92	Ground Level													
0.00	Topsoil, silty clay													
0.12	Dark Brown													
Sandy silty clay, trace of gravel														
Hard	Brown (Till)													
Very Stiff	Grey													
Stiff														
181.82	End of Borehole													
8.10	Borehole dry on completion of drilling													
	■ Penetrometer Test													

DYNAMIC CONE PENETRATION RESISTANCE PLOT

Shear Strength kPa

O UNCONFINED + FIELD VANE
 ● QUICK TRIAXIAL X LAB VANE

20 40 60 80 100

W_P W W_L

WATER CONTENT (%) 20 40 60

kN/m³ GR SA SI CL

* Bouncing on cobble

1 30 42 27

RECORD OF BOREHOLE No 5										1 of 1	METRIC		
W.P. <u>64-00-00</u>		LOCATION <u>Dougall Parkway, EBL-N Leg Sta. 12+589, o/s 23.5m Lt. of CL</u>								ORIGINATED BY <u>MR</u>			
DIST <u>31</u>		HWY <u>401</u>		BOREHOLE TYPE <u>Continuous Flight Solid Stem Augers</u>						COMPILED BY <u>GD</u>			
DATUM <u>Geodetic</u>		DATE <u>February 11, 2002</u>								CHECKED BY <u>MRA</u>			
SOIL PROFILE			SAMPLES			ELEVATION SCALE *	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20	40	60	80	100		
190.44	Ground Level												
0.00	Topsoil, silty clay												
0.20	Dark Brown												
	Sandy silty clay, trace of gravel												
	Stiff to Very Stiff Brown (Till)												
	Hard												
	Very Stiff Grey												
182.34													
8.10	End of Borehole												
	Borehole dry on completion of drilling												
	■ Penetrometer Test												

RECORD OF BOREHOLE No 6										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+589, o/s 10.5m Rt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 04, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			ELEVATION SCALE *	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20	40	60	80	100			PLASTIC LIMIT w_p
190.05	Ground Level													
0.00	Topsoil, silty clay													
0.20	Dark Brown													
	Sandy silty clay, trace of gravel													
	Hard Brown (Till)													
	Very Stiff Grey													
	Stiff													
181.95	End of Borehole													
8.10	Borehole dry on completion of drilling													
	■ Penetrometer Test													

RECORD OF BOREHOLE No 7										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+637, o/s 26.1m Lt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 02, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
SHEAR STRENGTH kPa														
○ UNCONFINED + FIELD VANE ● QUICK TRIAXIAL X LAB VANE														
20 40 60 80 100														
WATER CONTENT (%)														
20 40 60														
kN/m ³														
GR SA SI CL														
190.28	Ground Level													
0.00	Sandy silty clay, some gravel Very Stiff Brown (Till)		1	SS	16									
	Hard		2	SS	43									
	Very Stiff		3	SS	60									
	Grey		4	SS	26									
	Stiff		5	SS	25									
			6	SS	18									
			7	SS	16									
			8	SS	14									
180.68	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test		9	SS	10									
9.60														

RECORD OF BOREHOLE No 8										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+642, o/s 12.5m Rt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 04, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.73	Ground Level													
0.00	Sandy silty clay, trace of gravel Stiff to Very Stiff Brown (Till) Hard	1 2 3 4 5 6 7 8	SS SS SS SS SS SS SS SS	9 30 49 50 21 16 14 11			190							
	Very Stiff Grey						189							
	Stiff						188							
181.13	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test						187							
9.60							186							
							175							
							185							
							184							
							183							
							182							

RECORD OF BOREHOLE No 9										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+691, o/s 21.9m Lt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 01, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
SHEAR STRENGTH kPa														
190.48	Ground Level					O UNCONFINED	+ FIELD VANE	W _P	W	W _L				
0.00	Topsoil, silty clay					● QUICK TRIAXIAL	X LAB VANE							
0.20	Dark Brown					20	40	60	80	100				
	Sandy silty clay, trace of gravel		1	SS	30									
	Hard		2	SS	44									
	Brown		3	SS	40									
	(Till)		4	SS	26									
	Very Stiff		5	SS	29									
	Grey		6	SS	20									
			7	SS	19									
			8	SS	12									
	Stiff		9	TW	PH									
180.88														
9.60	End of Borehole													
	Borehole dry on completion of drilling													
	■ Penetrometer Test													

RECORD OF BOREHOLE No 10										1 of 1	METRIC			
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+877, o/s 21.6m Lt. of CL								ORIGINATED BY MR				
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD				
DATUM Geodetic		DATE February 12, 2002								CHECKED BY MRA				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.32	Ground Level													
0.00	Topsoil, silty clay Dark Brown Sandy silty clay, trace of gravel													
0.10	Stiff Brown (Till)		1	SS	13									
	Hard		2	SS	38									
	Very Stiff Grey		3	SS	55									
	Stiff		4	SS	25									
			5	SS	14									
			6	SS	12									
181.17	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test		7	SS	12									
9.15														

DYNAMIC CONE PENETRATION RESISTANCE PLOT

SHEAR STRENGTH kPa

O UNCONFINED + FIELD VANE
● QUICK TRIAXIAL X LAB VANE

W_P W W_L

WATER CONTENT (%)

PLASTIC LIMIT NATURAL MOISTURE CONTENT LIQUID LIMIT

20 40 60 80 100

20 40 60

190 189 188 187 186 185 184 183 182

15 — 5 (%) STRAIN AT FAILURE

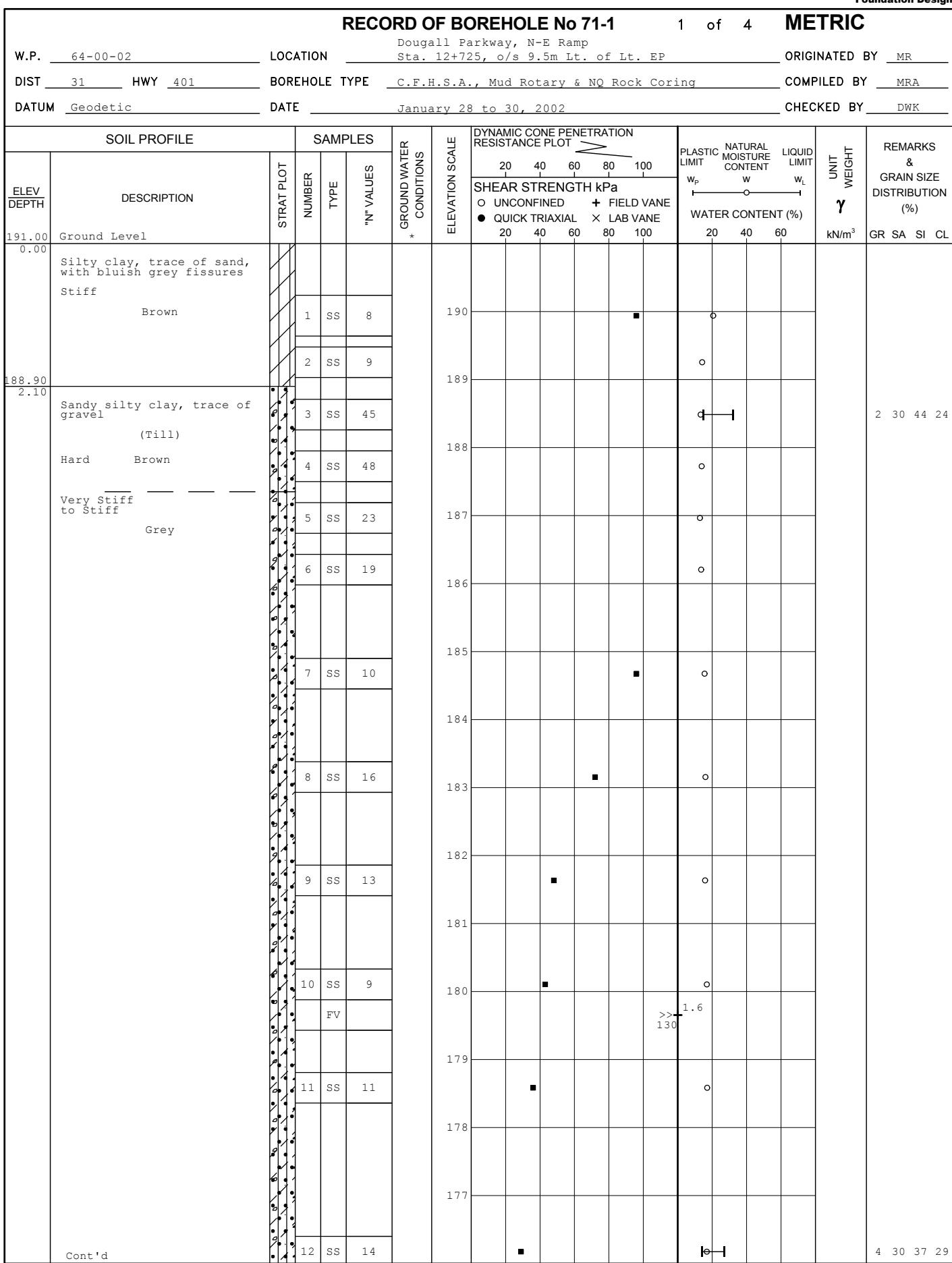
ON_MOT VER 3A 01TF072G.GPJ ON_MOT.GDT 01/06/2007 7:54:19 AM +⁷, ×⁵: Numbers refer to Sensitivity 20
15 — 5 (%) STRAIN AT FAILURE 10

RECORD OF BOREHOLE No 11										1 of 1	METRIC				
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+876, o/s 17.0m Rt. of CL								ORIGINATED BY MR					
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD					
DATUM Geodetic		DATE February 05, 2002								CHECKED BY MRA					
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100			W _P
190.56	Ground Level														
0.00	Topsoil, silty clay Dark Brown														
0.10	Sandy silty clay, trace of gravel Stiff to Hard Brown (Till)		1	SS	13										
			2	SS	34										
			3	SS	46										
			4	SS	26										
			5	SS	22										
			6	SS	17										
			7	SS	15										
181.56	Very Stiff Grey														
9.00	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test														

RECORD OF BOREHOLE No 12										1 of 1	METRIC			
W.P.	64-00-00	LOCATION	Dougall Parkway, EBL-N Leg Sta. 12+912, o/s 10.1m Lt. of CL						ORIGINATED BY	MR				
DIST	31	HWY	401	BOREHOLE TYPE	Continuous Flight Solid Stem Augers						COMPILED BY	GD		
DATUM	Geodetic		DATE	February 11, 2002						CHECKED BY	MRA			
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.49	Ground Level													
0.00	Sandy silty clay, trace of gravel													
	Stiff Brown (Till)		1	SS	10									
	Hard		2	SS	47									
			3	SS	45									
	Very Stiff		4	SS	27									
			5	SS	17									
	Stiff		6	SS	13									
			7	SS	12									
182.39	End of Borehole													
8.10														
 Water level measured after drilling  Penetrometer Test														

RECORD OF BOREHOLE No 13										1 of 1	METRIC				
W.P. 64-00-00		LOCATION Dougall Parkway, EBL-N Leg Sta. 12+914, o/s 12.6m Rt. of CL								ORIGINATED BY MR					
DIST 31 HWY 401		BOREHOLE TYPE Continuous Flight Solid Stem Augers								COMPILED BY GD					
DATUM Geodetic		DATE February 05, 2002								CHECKED BY MRA					
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20	40	60	80	100			W _P	W
191.11	Ground Level					191									
0.00	Topsoil, silty clay					190									
0.10	Dark Brown Sandy clayey silt, trace of gravel		1	SS	15	189									
189.71	Compact/Very Stiff Rusty Brown (Till)		2	SS	11	188									
1.40	Silty fine sand, some gravel		3	SS	38	187									
1.80	Loose Rusty Brown Saturated		4	SS	29	186									
189.31	Sandy silty clay, trace of gravel		5	SS	18	185									
1.80	Very Stiff to Hard Brown to Grey(at 2.55m) (Till)		6	SS	19*	184									
183.01	End of Borehole		7	SS	19*										
8.10															
		∇	Water level observed during drilling												
													* No recovery		

RECORD OF BOREHOLE No 14										1 of 1	METRIC			
W.P. <u>64-00-00</u>		LOCATION <u>Dougall Parkway, EBL-N Leg Sta. 12+967, o/s 8.5m Lt. of CL</u>								ORIGINATED BY <u>MR</u>				
DIST <u>31</u>		HWY <u>401</u>		BOREHOLE TYPE <u>Continuous Flight Solid Stem Augers</u>						COMPILED BY <u>GD</u>				
DATUM <u>Geodetic</u>		DATE <u>February 12, 2002</u>								CHECKED BY <u>MRA</u>				
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100		
190.56	Ground Level													
0.00	Sandy silty clay, trace of gravel Very Stiff Brown (Till) Hard	1 2 3 4 5 6	SS SS SS SS SS SS	16 41 68 32 21 16										
184.01	Very Stiff													
6.55	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test													



RECORD OF BOREHOLE No 71-1

2 of 4

METRIC

W.P. 64-00-02

LOCATION Dougall Parkway, N-E Ramp
Sta. 12+725, o/s 9.5m Lt. of Lt. EP

ORIGINATED BY MR

DIST 31 HWY 401

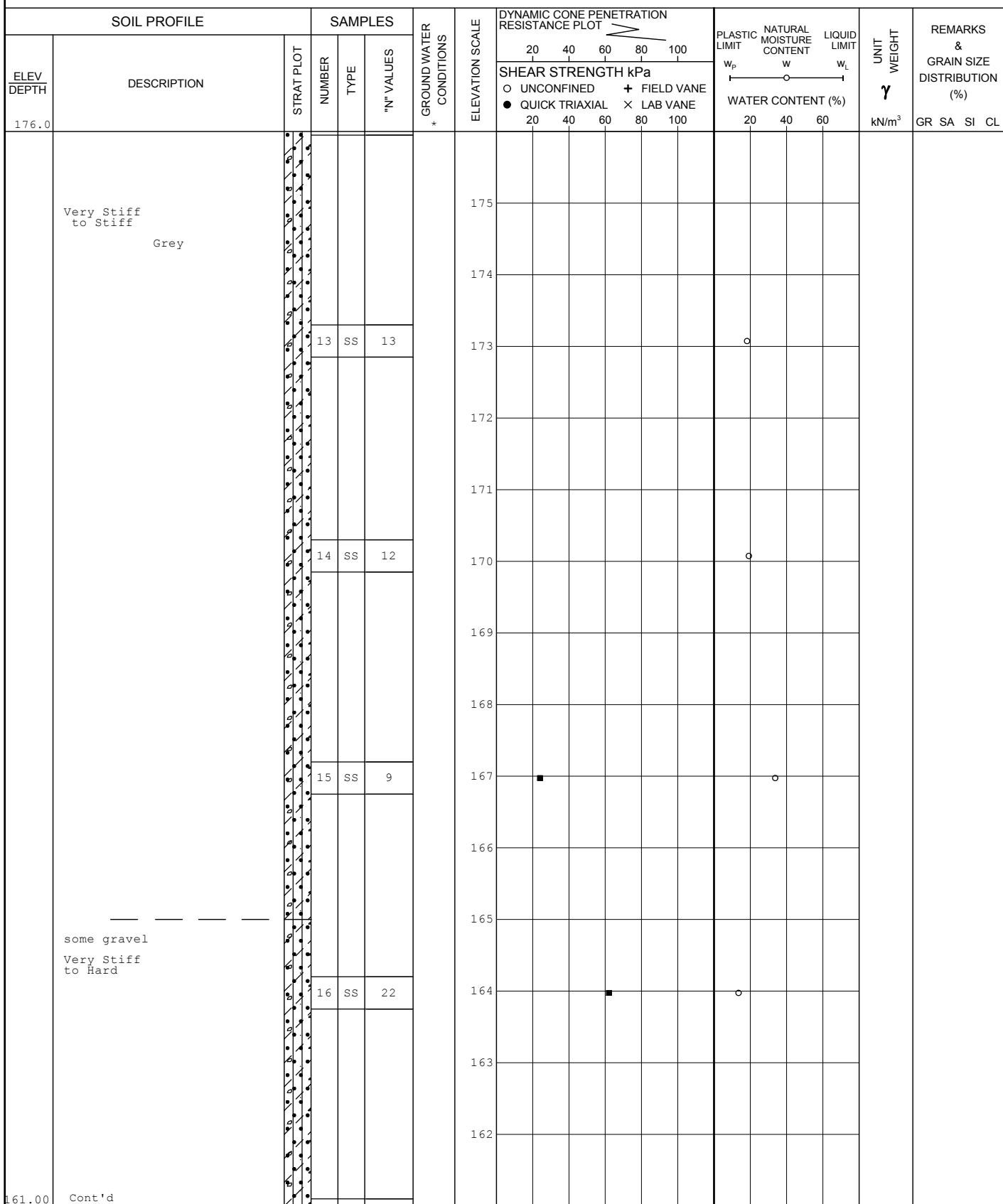
BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring

COMPILED BY MRA

DATUM Geodetic

DATE January 28 to 30, 2002

CHECKED BY DWK



RECORD OF BOREHOLE No 71-1										3 of 4	METRIC		
W.P. 64-00-02		LOCATION Dougall Parkway, N-E Ramp Sta. 12+725, o/s 9.5m Lt. EP								ORIGINATED BY MR			
DIST 31 HWY 401		BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring								COMPILED BY MRA			
DATUM Geodetic		DATE January 28 to 30, 2002								CHECKED BY DWK			
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		GROUND WATER CONDITIONS *	20	40	60	80		
161.00													
30.00	Sandy silty clay, some gravel Very Stiff to Hard Grey (Till)	17 SS 35											
		18 SS 61											
		19 SS 13											
152.90													
38.10	Fine to coarse sand and gravel, some silt Very Dense Grey	20 SS 62											
150.30													
40.70	Bedrock Unweathered, strong limestone Grey to Light Grey	21 RC *										* Lost sample	
		22 RC REC 80%										RQD=70%	
		23 RC REC 90%										RQD=50%	
146.85													
44.15	End of Borehole Cont'd												

RECORD OF BOREHOLE No 71-1

4 of 4

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+725, o/s 9.5m Lt. of Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring COMPILED BY MRA
DATUM Geodetic DATE January 28 to 30, 2002 CHECKED BY DWK

SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	γ	UNIT WEIGHT kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES *		GROUND WATER CONDITIONS	20	40	60	80	100						
	Borehole dry on completion of drilling ■ Penetrometer Test																	

RECORD OF BOREHOLE No 71-2

1 of 1

METRIC

W.P. 64-00-02

LOCATION

Dougall Parkway, N-E Ramp
Sta. 12+688, o/s 9.8m Rt. of Lt. EP

ORIGINATED BY MR

DIST 31 HWY 401

BOREHOLE TYPE

Continuous Flight Solid Stem Augers

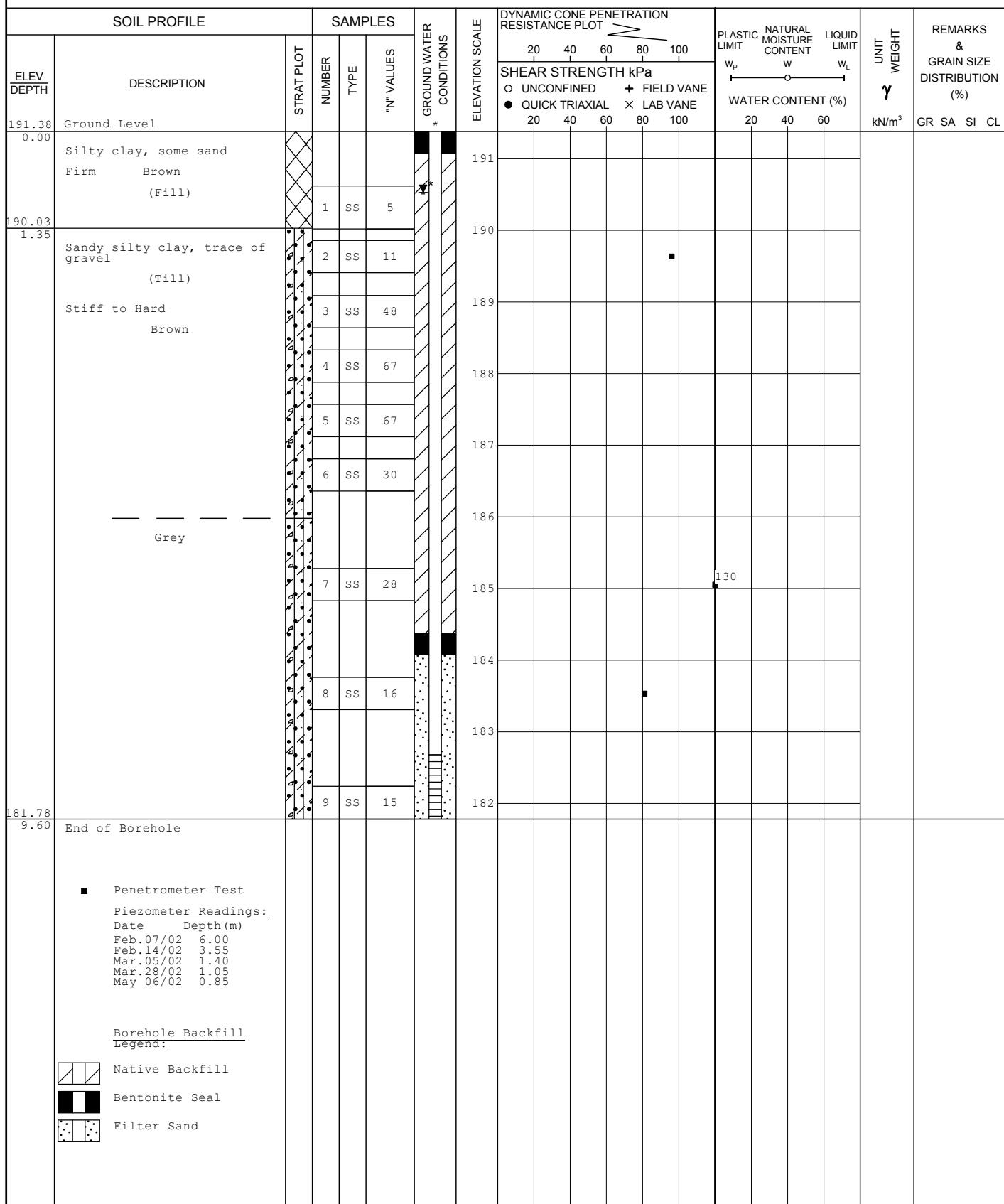
COMPILED BY MRA

DATUM Geodetic

DATE

February 05, 2002

CHECKED BY DWK

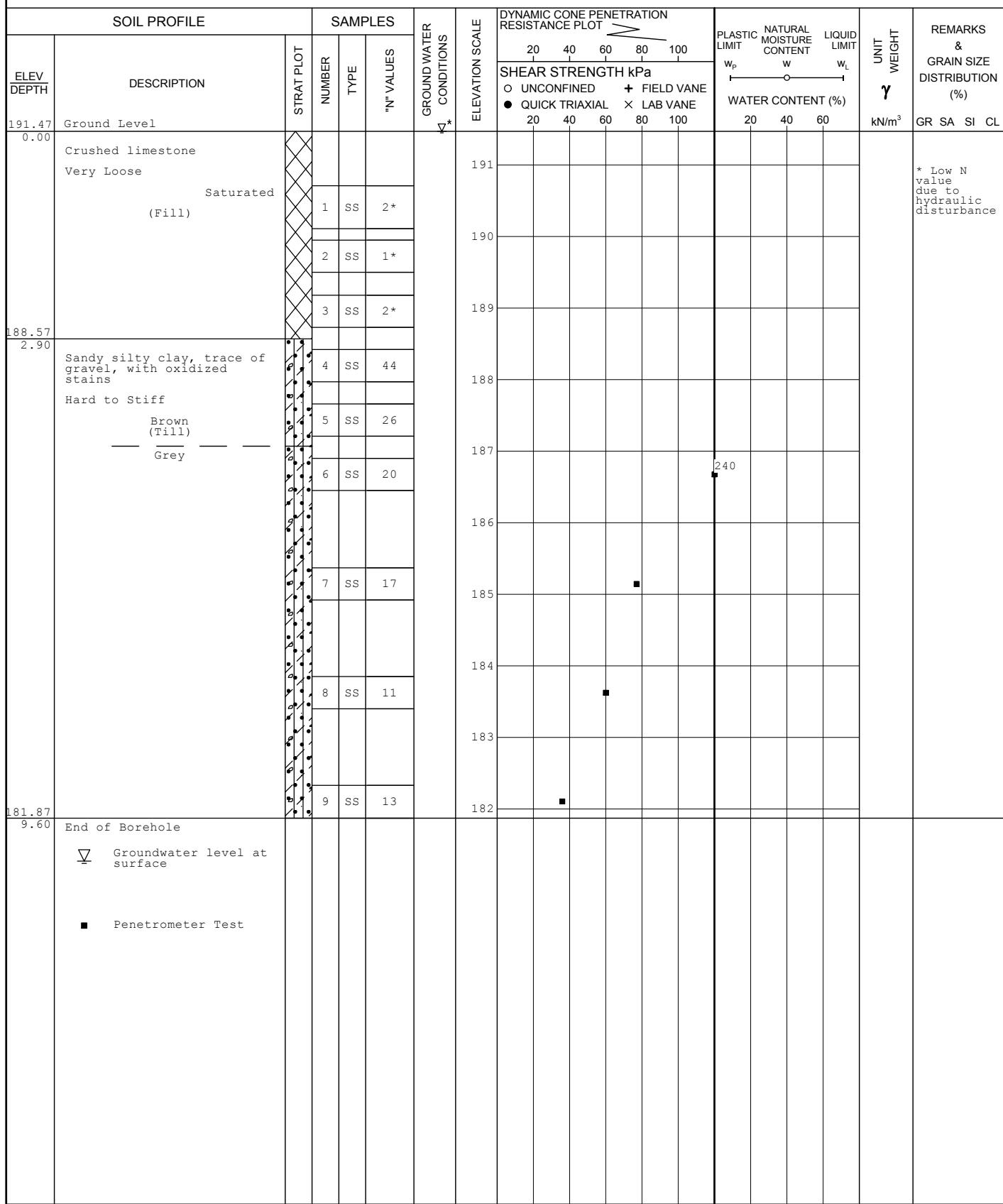


RECORD OF BOREHOLE No 71-3

1 of 1

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+782, o/s 6.0m Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid and Hollow Stem Augers COMPILED BY MRA
DATUM Geodetic DATE February 11, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 71-4											1 of 4	METRIC				
W.P.	64-00-02	LOCATION	Dougall Parkway, N-E Ramp Sta. 12+778, o/s 8.9m Rt. of Lt. EP						ORIGINATED BY	MR						
DIST	31	HWY	401	BOREHOLE TYPE	C.F.H.S.A., Mud Rotary & NQ Rock Coring						COMPILED BY	MRA				
DATUM	Geodetic		DATE	February 05, 2002						CHECKED BY	DWK					
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)			
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		GROUND WATER CONDITIONS *	20	40	60	80			100	W _P	W _L
191.46	Ground Level															
0.00	Silty clay, some sand, trace of gravel, with oxidized stains Stiff Brown (Fill)		1	SS	13											
189.76			2	SS	9											
1.70			3	SS	14											
189.36	Topsoil		4	SS	44											
2.10	Sandy silty clay, trace of gravel Stiff to Hard Brown (Till)		5	SS	25*											
			6	SS	11											
			7	SS	13											
			8	SS	12											
			9	TW	PH**											
			10	TW	PH**											
			11	SS	14											
	Cont'd															
											20	15—○—5	(%) STRAIN AT FAILURE			

* No recovery

2 29 41 28

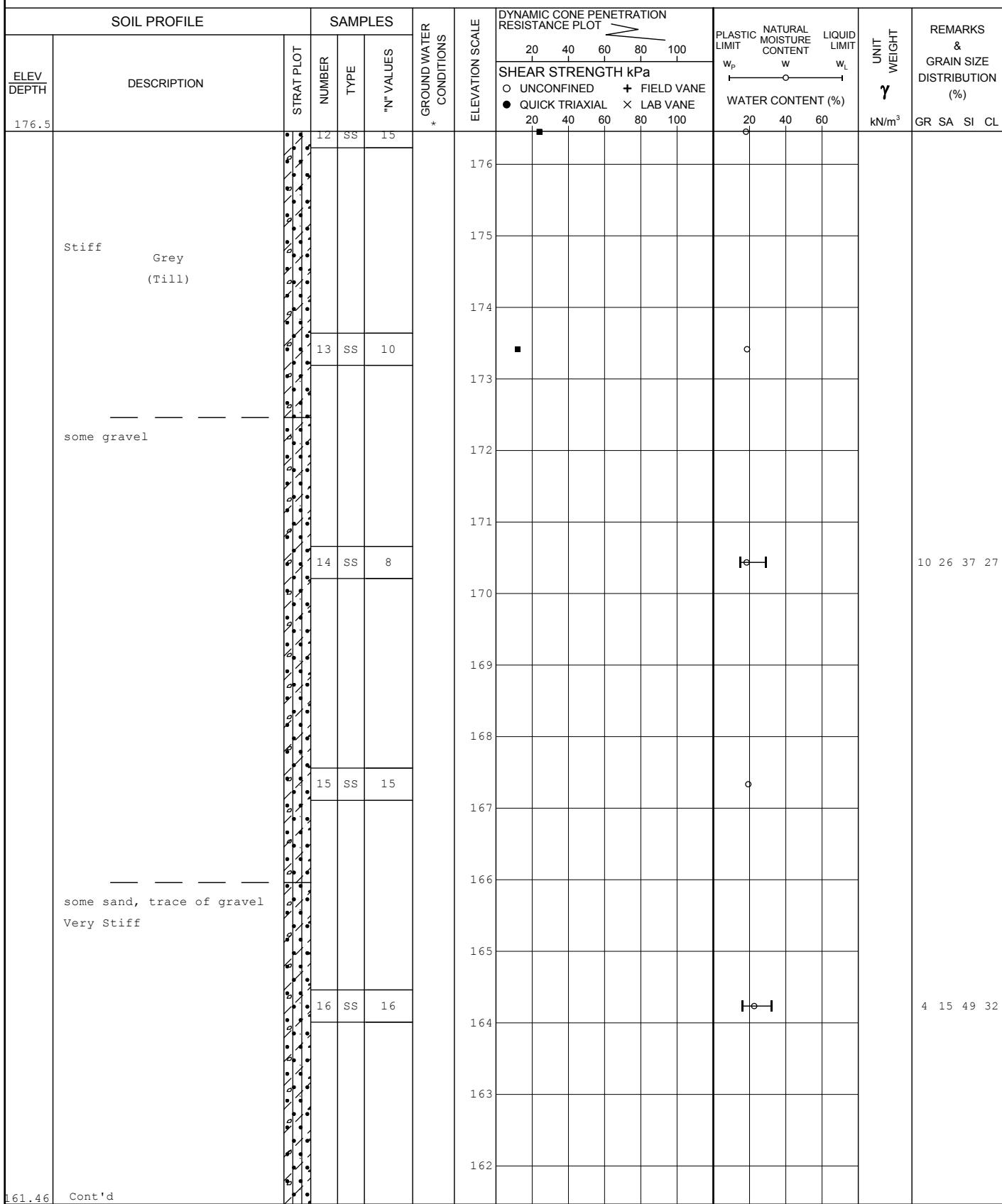
** Lost sample

RECORD OF BOREHOLE No 71-4

2 of 4

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+778, o/s 8.9m Rt. of Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring COMPILED BY MRA
DATUM Geodetic DATE February 05, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 71-4										3 of 4	METRIC						
W.P.	64-00-02	LOCATION	Dougall Parkway, N-E Ramp Sta. 12+778, o/s 8.9m Rt. of Lt. EP								ORIGINATED BY	MR					
DIST	31	HWY	401	BOREHOLE TYPE	C.F.H.S.A., Mud Rotary & NQ Rock Coring								COMPILED BY	MRA			
DATUM	Geodetic		DATE	February 05, 2002								CHECKED BY	DWK				
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		GROUND WATER CONDITIONS *	20	40	60						80	100
161.46	Sandy silty clay, some gravel Hard Grey (Till)	17	SS	30								o					
30.00		18	SS	40													
155.46	Sand, fine to coarse, some silt Compact Grey Saturated	19	SS	25*													
36.00																	* No recovery
151.36	Bedrock Unweathered, strong limestone Light Grey 75mm void at 40.96m 100mm void at 43.86m	20	RC	REC 95%													RQD=50%
40.10		21	RC	REC 95%													
147.16	Unweathered, shaly limestone Grey																RQD=55%
44.30	End of Borehole Cont'd																

RECORD OF BOREHOLE No 71-4

4 of 4

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+778, o/s 8.9m Rt. of Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring COMPILED BY MRA
DATUM Geodetic DATE February 05, 2002 CHECKED BY DWK

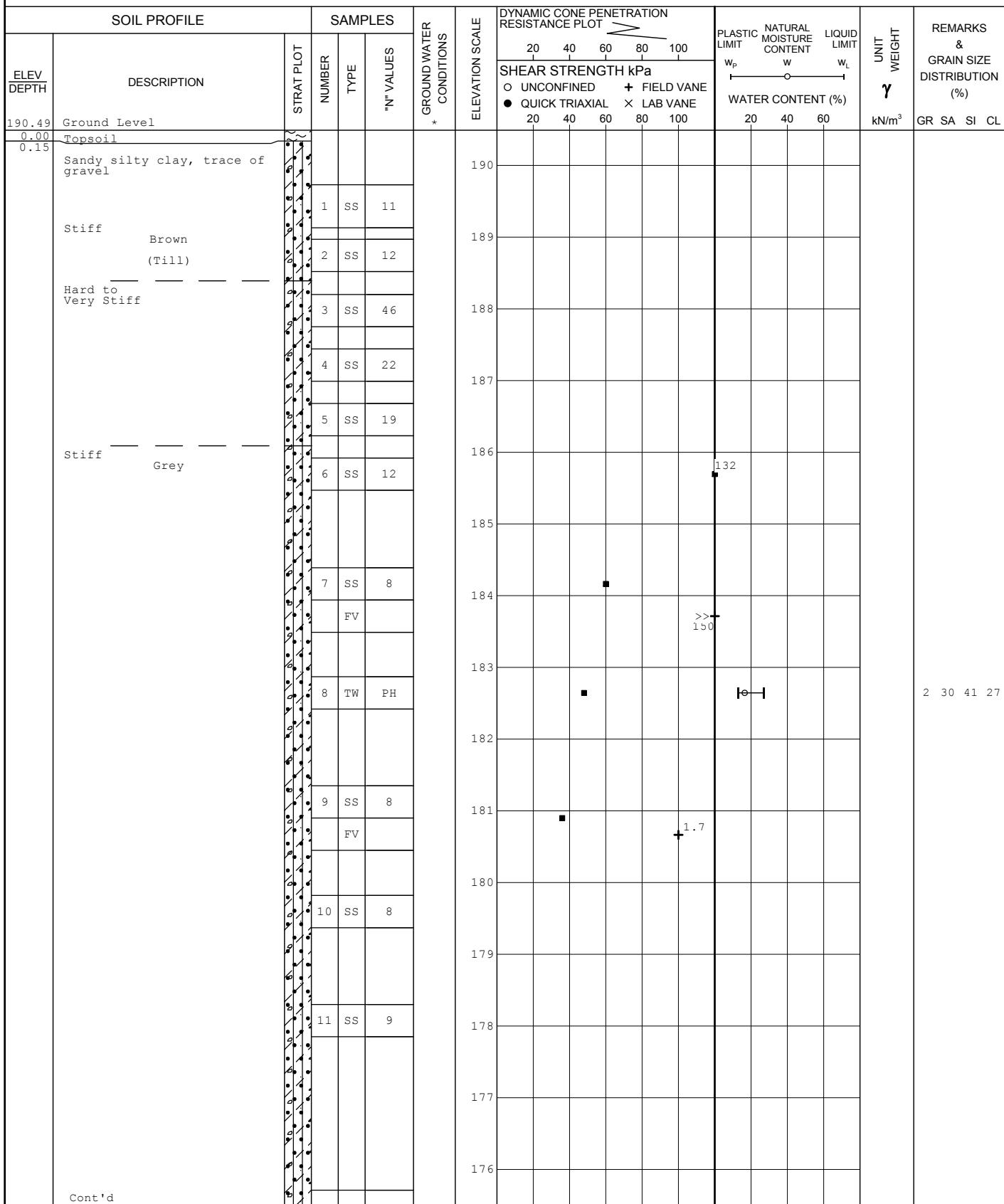
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES *		SHEAR STRENGTH kPa	O UNCONFINED	+ FIELD VANE	● QUICK TRIAXIAL	X LAB VANE						
	Borehole dry on completion of drilling ■ Penetrometer Test						20 40 60 80 100										

RECORD OF BOREHOLE No 71-5

1 of 3

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+850, o/s 6.3m Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring COMPILED BY MRA
DATUM Geodetic DATE February 7, 8 and 11, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 71-5

2 of 3

METRIC

W.P. 64-00-02 LOCATION Dougall Parkway, N-E Ramp
Sta. 12+850, o/s 6.3m Lt. of Lt. EP ORIGINATED BY MR
DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring COMPILED BY MRA
DATUM Geodetic DATE February 7, 8 and 11, 2002 CHECKED BY DWK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
175.5	Stiff to Very Stiff (Till)		12	SS	11												
			13	SS	15												
			14	SS	15*												
			15	SS	19												
			16	SS	24												
	Hard	— — — —															
	(Till)																
	Cont'd																

RECORD OF BOREHOLE No 71-5										3 of 3	METRIC				
W.P. 64-00-02		LOCATION Dougall Parkway, N-E Ramp Sta. 12+850, o/s 6.3m Lt. EP								ORIGINATED BY MR					
DIST 31 HWY 401		BOREHOLE TYPE C.F.H.S.A., Mud Rotary & NQ Rock Coring								COMPILED BY MRA					
DATUM Geodetic		DATE February 7, 8 and 11, 2002								CHECKED BY DWK					
SOIL PROFILE			SAMPLES			ELEVATION SCALE *	DYNAMIC CONE PENETRATION RESISTANCE PLOT					UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20	40	60	80	100			PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w
160.49	(Till)		17	SS	30										
	Very Stiff		18	SS	50										
			19	SS	20										
150.59	Bedrock														
39.90	Unweathered, strong limestone														
	Light Grey														
147.59															
42.90	End of Borehole														
	Borehole dry on completion of drilling														
	■ Penetrometer Test														

RECORD OF BOREHOLE No 71-6										1 of 1	METRIC					
W.P.	64-00-02	LOCATION	Dougall Parkway, N-E Ramp Sta. 12+845, o/s 8.8m Rt. of Lt. EP						ORIGINATED BY	MR						
DIST	31	HWY	401	BOREHOLE TYPE	Continuous Flight Solid Stem Augers						COMPILED BY	MRA				
DATUM	Geodetic		DATE	February 05, 2002						CHECKED BY	DWK					
SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT				PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE					
191.65	Ground Level															
0.00	Silty clay, trace of sand Brown (Fill)	XX														
190.90																
190.76	Topsoil	~~	1	SS	18											
0.95	Sandy silty clay, trace of gravel Very Stiff to Hard Brown (Till)		2	SS	15											
			3	SS	28											
			4	SS	104											
	layers of sandy silt, trace of clay and gravel Very Dense Brown		5	SS	25											
			6	SS	18											
			7	SS	14											
			8	SS	13											
182.05	End of Borehole															
9.60	■ Penetrometer Test															
	<u>Piezometer Readings:</u>															
	Date Depth (m)															
	Feb.07/02 8.55															
	Feb.14/02 7.25															
	Mar.05/02 2.50															
	Mar.28/02 2.30															
	May 06/02 2.15															
	<u>Borehole Backfill</u>															
	<u>Legend:</u>															
	Native Backfill															
	Bentonite Seal															
	Filter Gravel															

METRIC

DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES

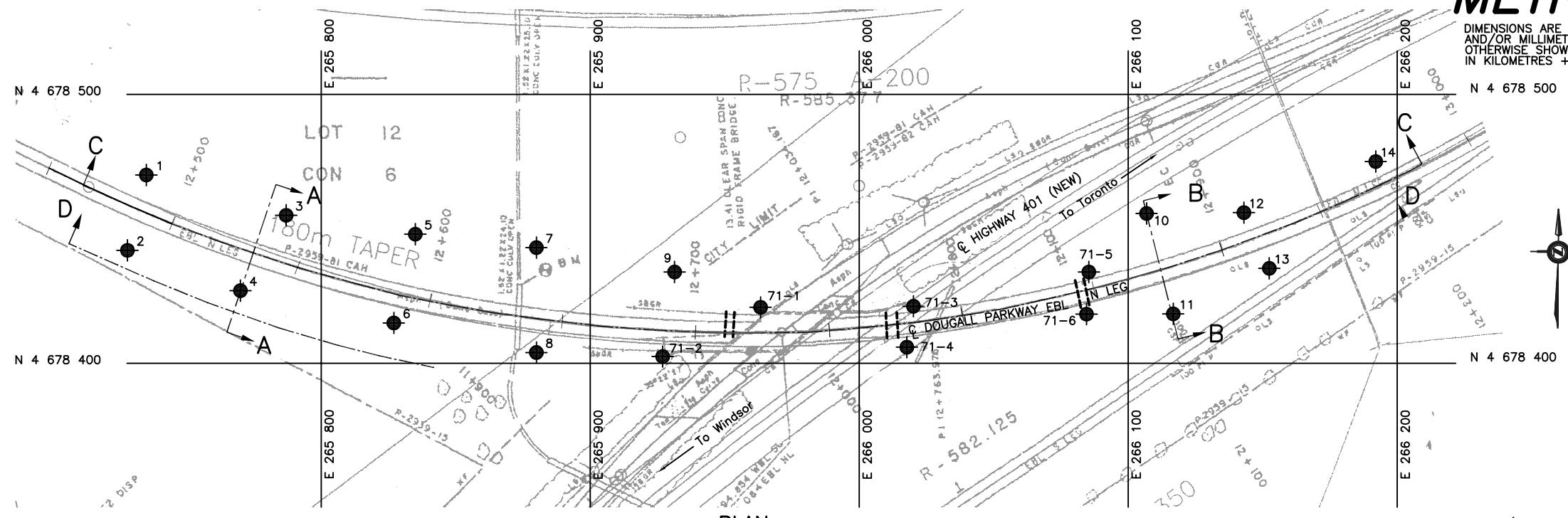
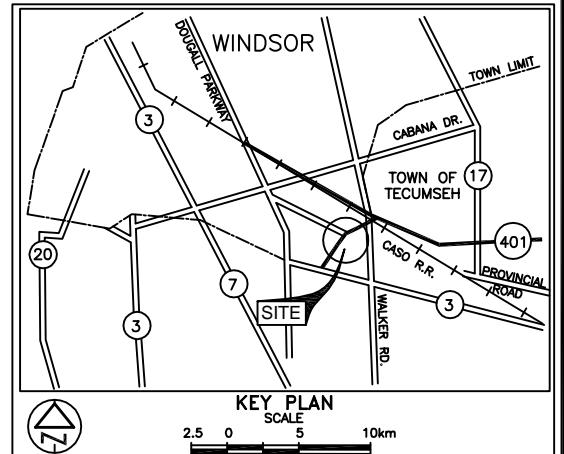
CONT No 2007-3043
WP No 64-00-00



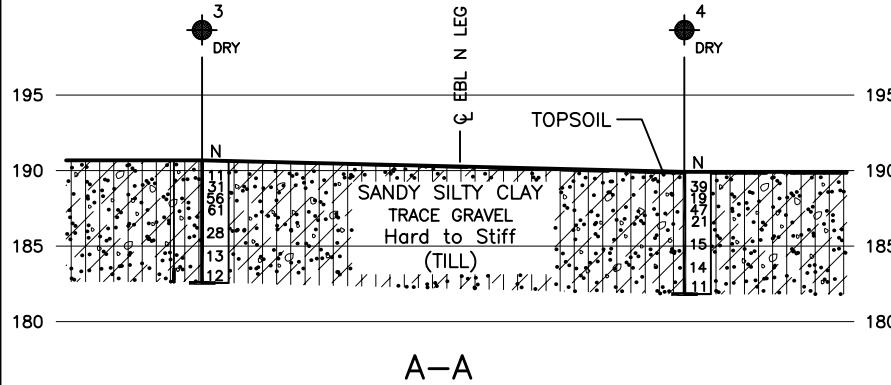
HIGHWAY 401
EMBANKMENT ON EBL
NORTH LEG AT DOUGALL PARKWAY
BOREHOLE LOCATIONS & SOIL STRATA

SHEET
244

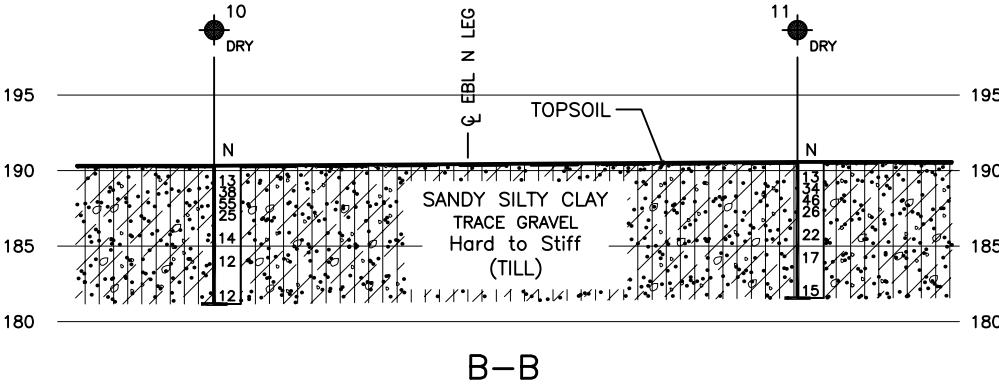
PML Peto MacCallum Ltd.
CONSULTING ENGINEERS



PLAN
SCALE
20 10 0 20 40m



A-A



B-B

(Legend Continued)

BH No	ELEVATION	EBL-N LEG STA.	o/s CL
10	190.32	12+877	21.6m Lt.
11	190.56	12+876	17.0m Rt.
12	190.49	12+912	10.1m Lt.
13	191.11	12+914	12.6m Rt.
14	190.56	12+967	8.5m Lt.
71-1	191.00	12+725	9.5m Lt.
71-2	191.38	12+688	9.8m Rt.
71-3	191.47	12+782	6.0m Lt.
71-4	191.46	12+778	8.9m Rt.
71-5	190.49	12+850	6.3m Lt.
71-6	191.65	12+845	8.8m Rt.

LEGEND

- Borehole
- Dynamic Cone Penetration Test (Cone)
- Borehole & Cone
- N Blows/0.3m (Std. Pen Test, 475 J / blow)
- CONE Blows/0.3m (60° Cone, 475 J / blow)
- ▼ W L at time of investigation
- Head
- ARTESIAN WATER
- Encountered
- PIEZOMETER

BH No	ELEVATION	EBL-N LEG STA.	o/s CL
1	190.70	12+484	13.0m Lt.
2	189.74	12+489	15.5m Rt.
3	190.67	12+539	17.1m Lt.
4	189.92	12+532	14.8m Rt.
5	190.44	12+589	23.5m Lt.
6	190.05	12+589	10.5m Rt.
7	190.28	12+637	26.1m Lt.
8	190.73	12+642	12.5m Rt.
9	190.48	12+691	21.9m Lt.

(Legend Continues)
— NOTE —
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

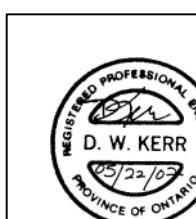
REVISIONS	CHANGED FOR CONTRACT DOCUMENTS AS PER EMAIL
MAY 22/07 CN	DATED APRIL 27, 2007, FROM DILLON CONSULTING LIMITED
DATE BY	DESCRIPTION
Geocodes No. 40J2-43	
HWY No 401	DIST 31
SUBM'D GD	CHECKED MRA
DRAWN MM	APPROVED DWK
DWG 1-1	

NOTES:

- REFER TO DRAWING 1-2 FOR SECTION C-C CONTINUED AND DRAWING 1-3 FOR SECTION D-D.
- SECTIONS ARE PROVIDED SOLELY FOR ILLUSTRATIVE PURPOSES. REFER TO RECORD OF BOREHOLES FOR DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS, IN-SITU TEST DATA AND LABORATORY TEST RESULTS.

SECTIONS
SCALE
5 2.5 0 5 10m

REF No Survey Plan 2001 Site # 6-71, entitled Proposed Bridge Site at Tunnel Diversion and Highway 401 and Plan, undated, untitled, Provided by Planning and Design Section, MTO.



METRIC

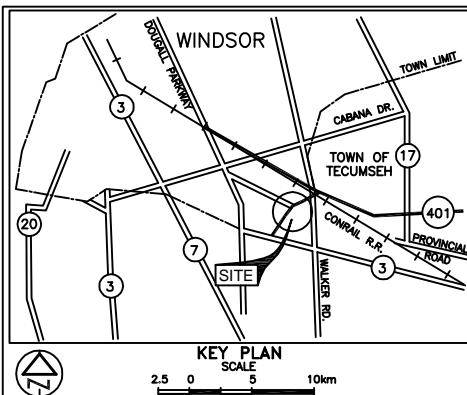
CONT No 2007-3043

WP No 64-00-00

HIGHWAY 401
EMBANKMENT ON EBL
NORTH LEG AT DOUGALL PARKWAY
BOREHOLE LOCATIONS & SOIL STRATA

SHEET 245

PML Peto MacCallum Ltd.
CONSULTING ENGINEER

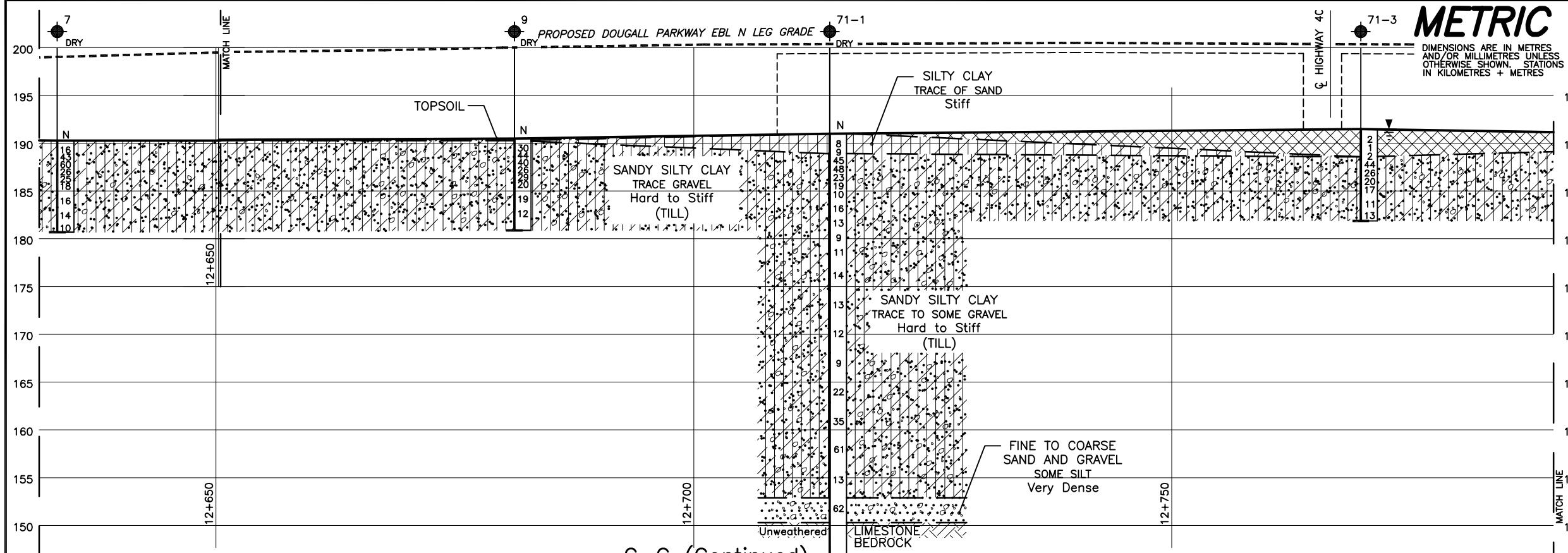


LEGEND

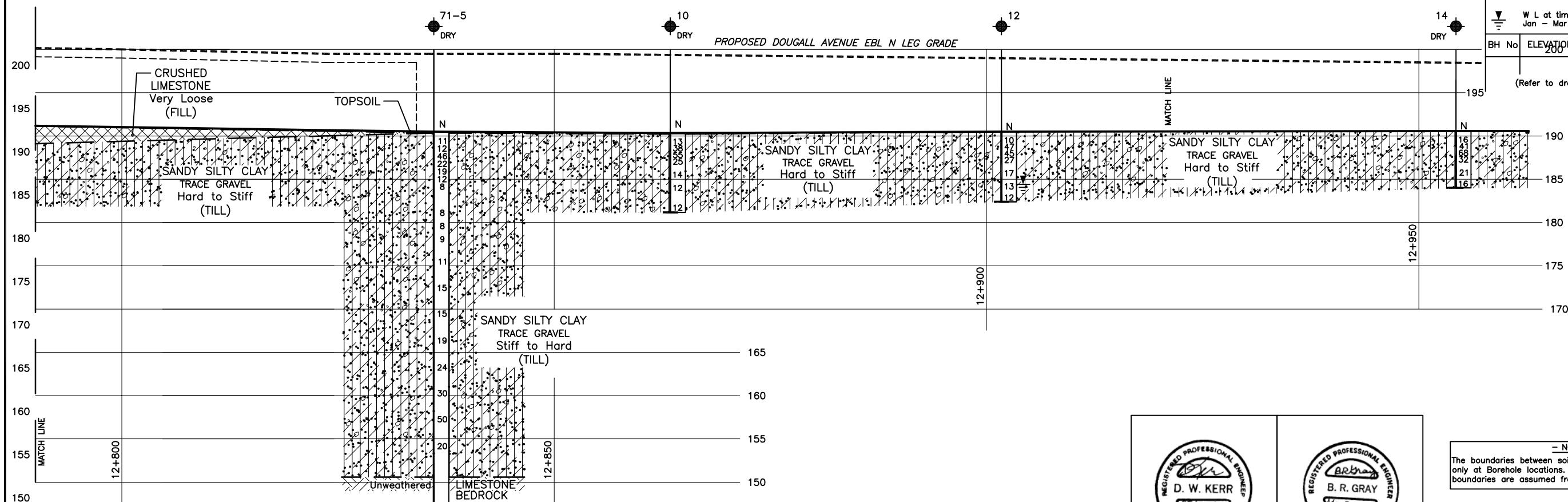
- Borehole
- Dynamic Cone Penetration Test (Cone)
- Borehole & Cone
- N Blows/0.3m (Std. Pen Test, 475 J / blow)
- CONE Blows/0.3m (60° Cone, 475 J / blow)
- ▼ W.L at time of investigation
Jan - Mar 2002

BH No	ELEVATION	CO-ORDINATES
	200	NORTH EAST

(Refer to drawing 1-1 for co-ordinates)



C-C (Continued)



C-C (Continued)

NOTES:

- REFER TO DRAWING 1-1 FOR PLAN AND SECTIONS A-A, B-B, C-C, DRAWING 1-3 FOR SECTION D-D.
- SECTIONS ARE PROVIDED SOLELY FOR ILLUSTRATIVE PURPOSES. REFER TO RECORD OF BOREHOLES FOR DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS, IN-SITU TEST DATA AND LABORATORY TEST RESULTS.

SECTION

SCALE

5 2.5 0 5 10m

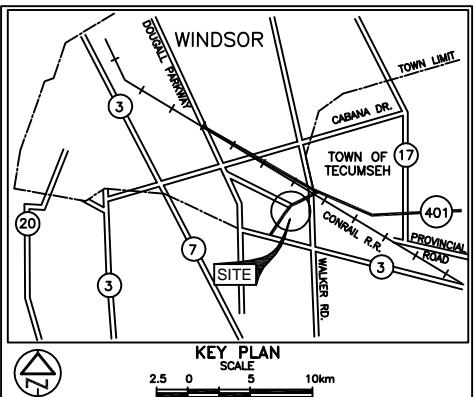


REF No Survey Plan 2001 Site # 6-71, entitled
Proposed Bridge Site at Tunnel Diversion and
Highway 401 and Plan, undated, untitled,
Provided by Planning and Design Section, MTO.

REVISIONS
MAY 22/07 CN DATED APRIL 27, 2007 FROM DILLON CONSULTING LIMITED
DATE BY DESCRIPTION

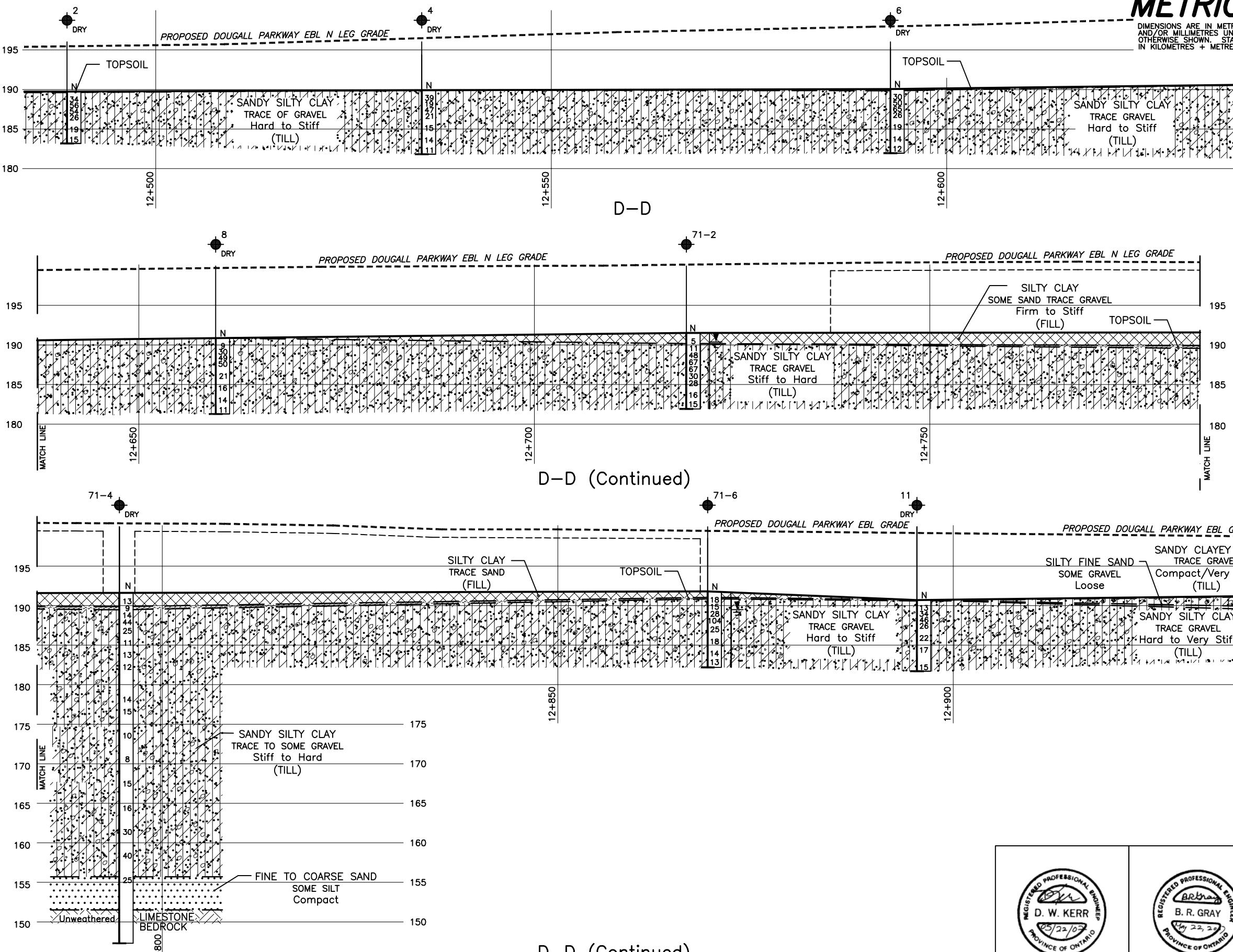
Geocres No. 40J2-43
HWY No 401 DIST 31
SUB'D GD CHECKED MRA DATE NOV 29, 2002 SITE
DRAWN MM CHECKED BRG APPROVED DWK DWG 1-2

- NOTE -
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

METRICCONT No 2007-3043
WP No 64-00-00HIGHWAY 401
EMBANKMENT ON EBL
NORTH LEG AT DOUGALL PARKWAY
BOREHOLE LOCATIONS & SOIL STRATASHEET
246**PML Peto MacCallum Ltd.**
CONSULTING ENGINEERS

LEGEND	
Borehole	
Dynamic Cone Penetration Test (Cone)	
Borehole & Cone	
N Blows/0.3m (Std. Pen Test, 475 J / blow)	
CONE Blows/0.3m (60° Cone, 475 J / blow)	
BH No ELEVATION CO-ORDINATES	
NORTH EAST	

(Refer to drawing 1-1 for co-ordinates)



NOTES:

- REFER TO DRAWING 1-1 FOR PLAN AND SECTIONS A-A, B-B, C-C
DRAWING 1-2 FOR SECTION C-C.
- SECTIONS ARE PROVIDED SOLELY FOR ILLUSTRATIVE PURPOSES. REFER TO RECORD OF BOREHOLES FOR DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS, IN-SITU TEST DATA AND LABORATORY TEST RESULTS.

SECTION
SCALE

5 2.5 0 5 10m



REF No Survey Plan 2001 Site # 6-71, entitled
Proposed Bridge Site at Tunnel Diversion and
Highway 401 and Plan, undated, untitled,
Provided by Planning and Design Section, MTO.

REVISIONS		CHANGED FOR CONTRACT DOCUMENTS AS PER EMAIL		
MAY 22/07	CN	DATED APRIL 27, 2007 FROM DILLON CONSULTING LIMITED		
DATE BY DESCRIPTION				

Geocodes No. 40J2-43

Hwy No	401	DIST	31
SUB'D	GD	CHECKED MRA	DATE NOV 29, 2002 SITE
DRAWN	MM	CHECKED BRG	APPROVED DWK DWG 1-3

- NOTE -
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

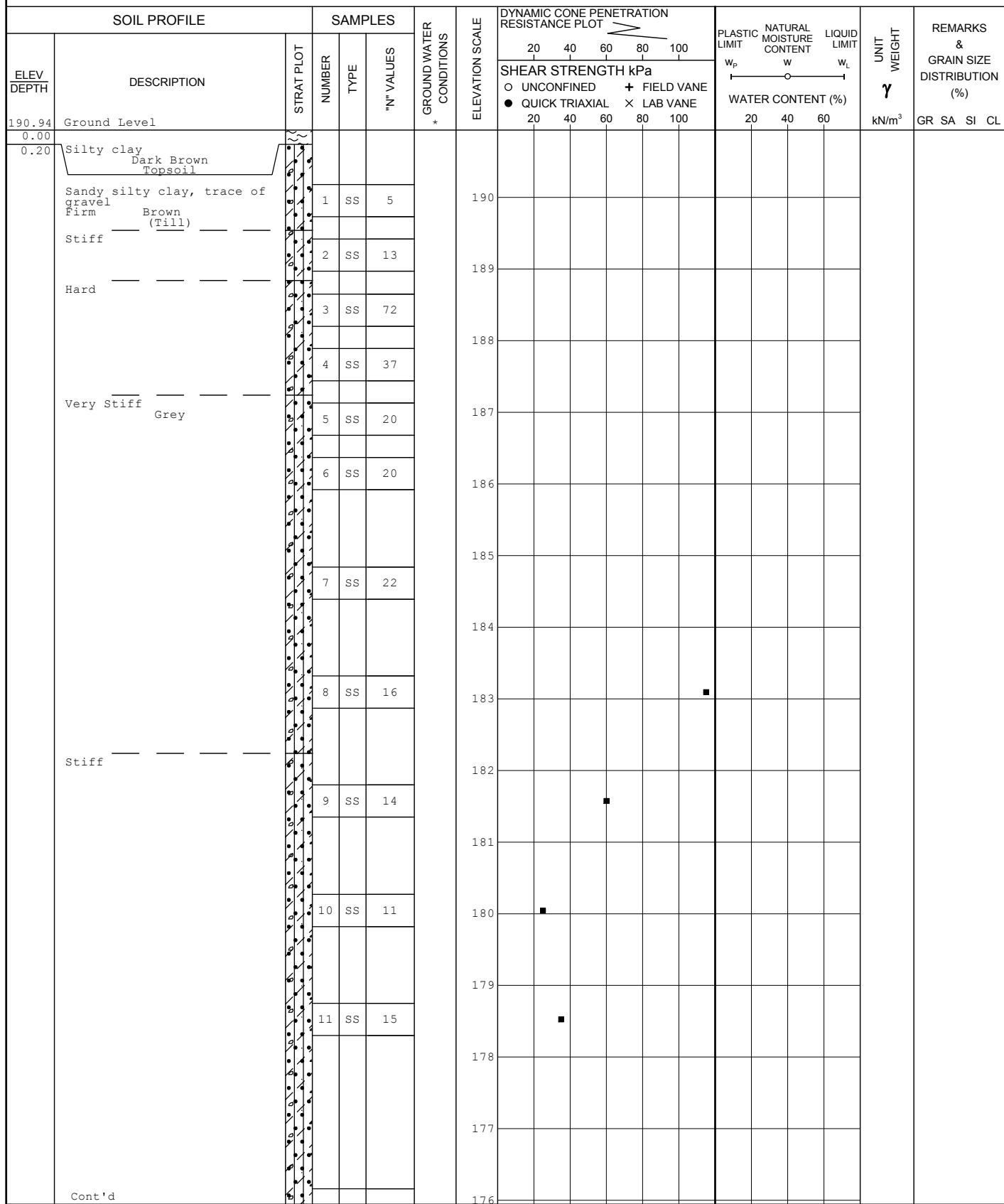
EMBANKMENT 2

RECORD OF BOREHOLE No 72-1

1 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+642, o/s 19.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 25 and 26, 2002 CHECKED BY DWK

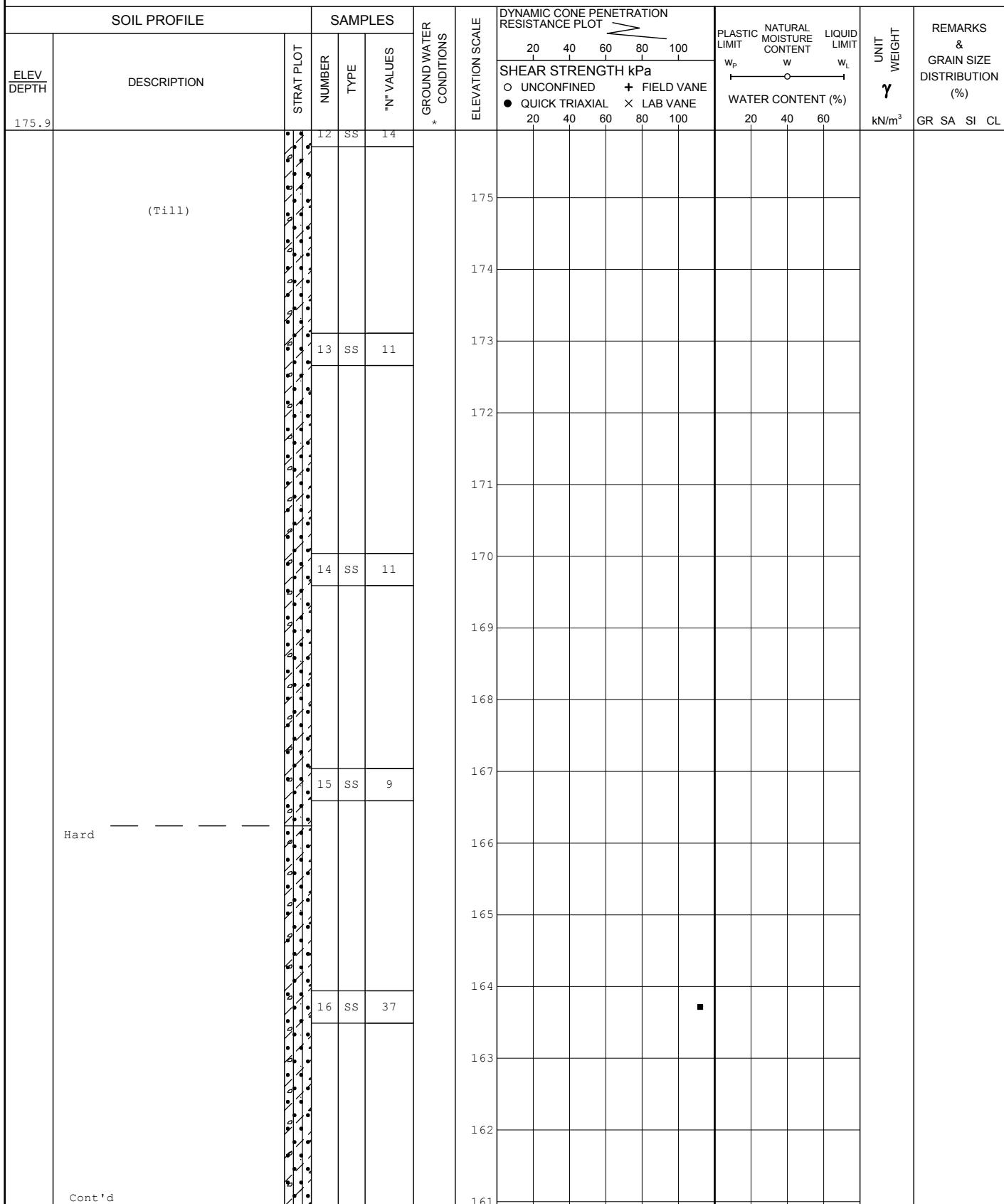


RECORD OF BOREHOLE No 72-1

2 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+642, o/s 19.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 25 and 26, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 72-1

3 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+642, o/s 19.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 25 and 26, 2002 CHECKED BY DWK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _P	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES	20 40 60 80 100	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100	20 40 60	kN/m ³	GR SA SI CL					
160.94	(Till)		17	SS	37												
			18	SS	30												
154.74	Bedrock Unweathered, strong limestone Grey to Light Grey		19	RC	REC 84%												RQD = 76%
36.20	100% drill water return to 37.2m, 0% after; 0.35m void at 37.0m																
151.24	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																
39.70																	

RECORD OF BOREHOLE No 72-2

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+626, o/s 25.5m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY MRA
 DATUM Geodetic DATE February 28, 2002 CHECKED BY DWK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _P	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		SHEAR STRENGTH kPa									
191.20	Ground Level					191	20 40 60 80 100	O UNCONFINED	+ FIELD VANE	20 40 60 80 100	20 40 60	191				
0.00	Granular 'A' crushed limestone (Fill)	X				190						190				
190.45	Sandy silty clay, some gravel	X	1	SS	7	189						189				
0.75	Firm Brown (Fill)	X	2	SS	40	188						188				
189.80	Sandy silty clay, some gravel	X	3	SS	56	187						187				
1.40	Hard Brown (Till)	X	4	SS	33	186						186				
	trace of gravel Grey	X	5	SS	30	185						185				
		X	6	SS	34	184						184				
		X	7	SS	26	183						183				
		X	8	SS	21	182						182				
181.60	Very Stiff	X	9	SS	29											
9.60	End of Borehole Borehole dry on completion of drilling															

RECORD OF BOREHOLE No 72-3

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+665, o/s 16.2m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Hollow Stem Augers COMPILED BY MRA
 DATUM Geodetic DATE February 28, 2002 CHECKED BY DWK

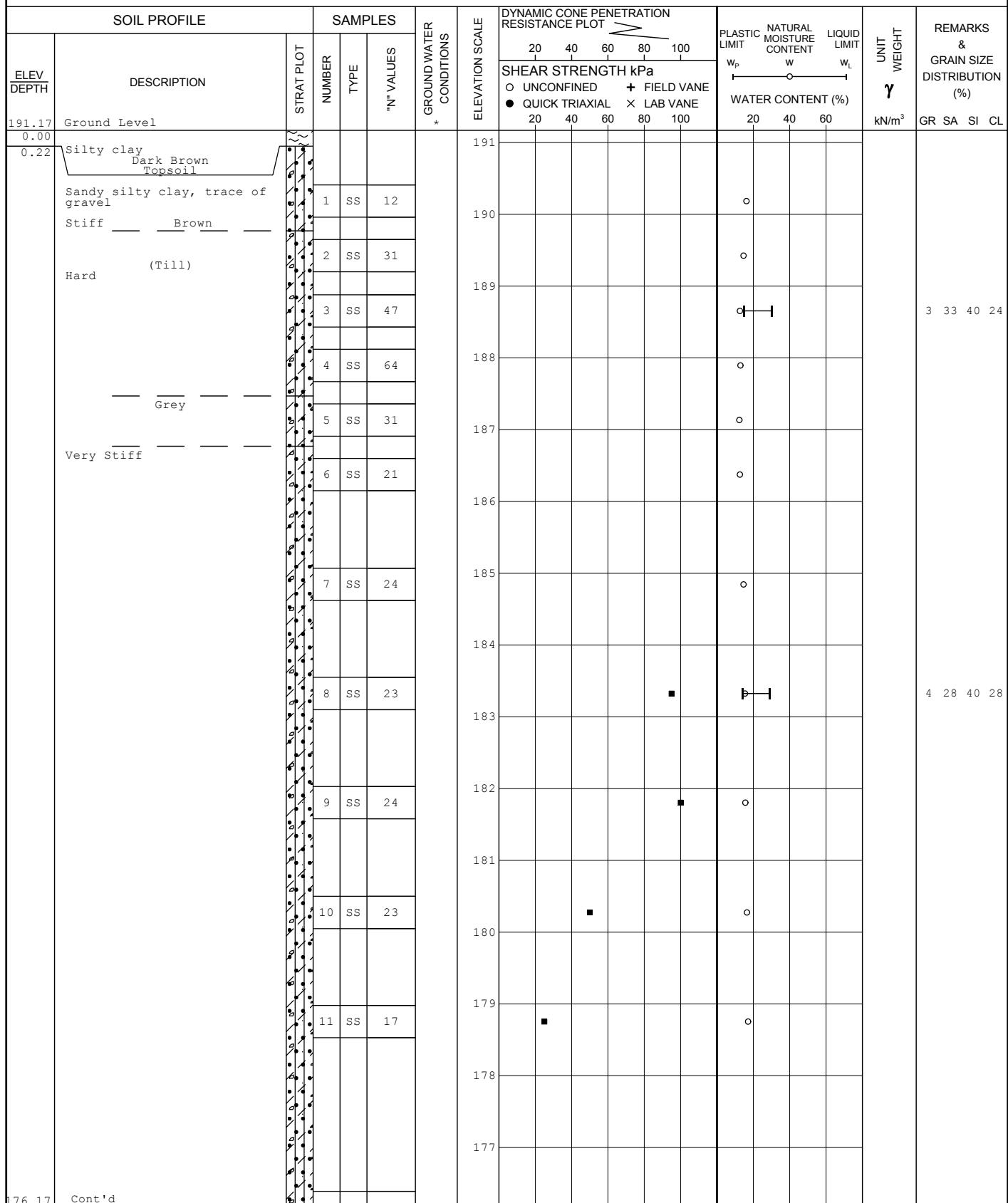
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					SHEAR STRENGTH kPa					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa					O UNCONFINED	+ FIELD VANE	● QUICK TRIAXIAL	X LAB VANE						
191.07	Ground Level																					
0.00	Granular 'A' crushed limestone (Fill)																					
189.87	Silty clay, trace of gravel Firm Brown (Fill)		1	SS	4																	
188.97	Sandy silty clay, trace of gravel Hard Brown (Till)		2	SS	49												o					
2.10			3	SS	82												o					
	Very Stiff Grey		4	SS	25												o					
			5	SS	23												o					
			6	SS	29												o					
			7	SS	20												o					
			8	SS	18												o					
181.47	End of Borehole																					
9.60	Water level observed during drilling																					

RECORD OF BOREHOLE No 72-4

1 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+649, o/s 30.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 26 and 27, 2002 CHECKED BY DWK

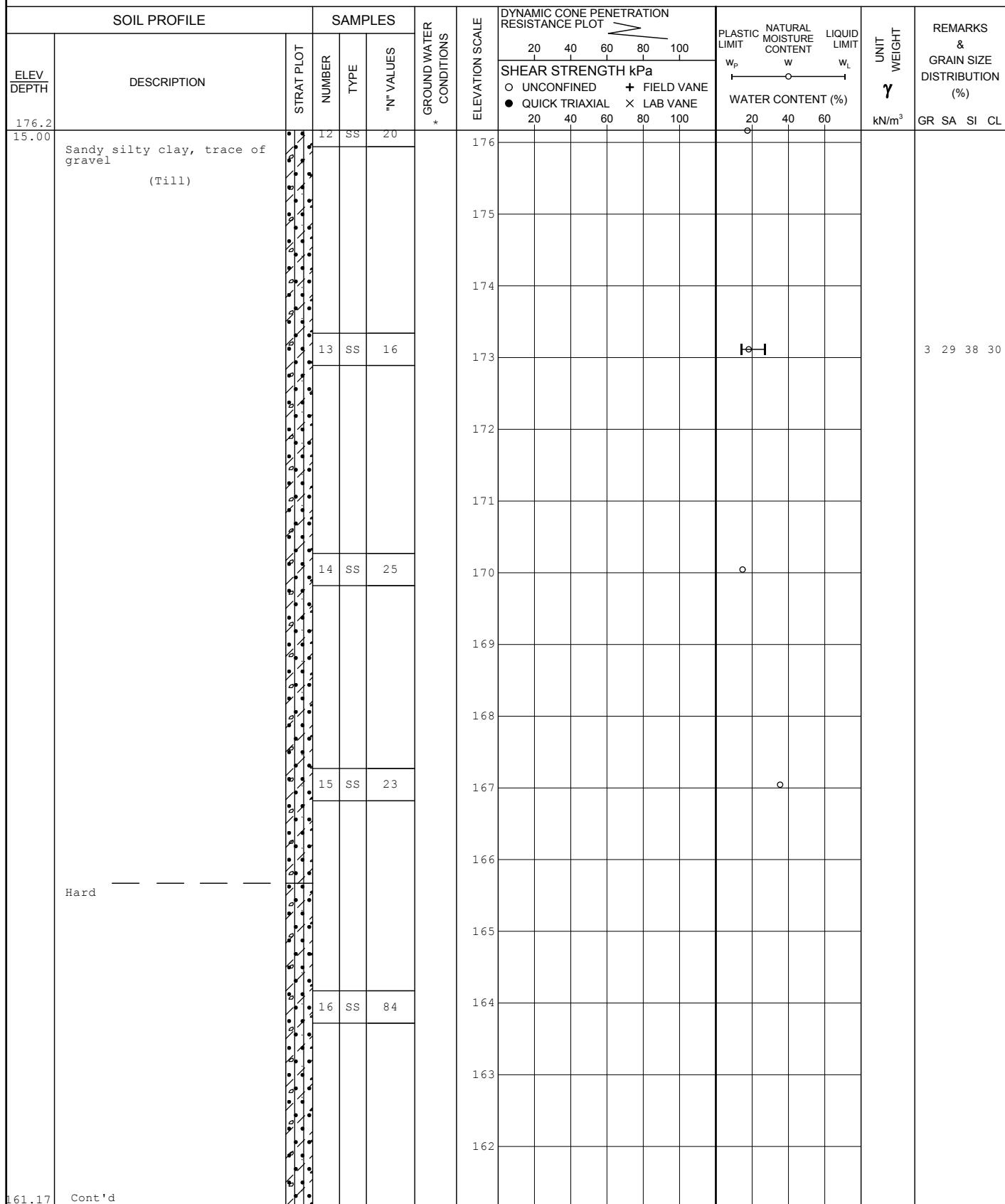


RECORD OF BOREHOLE No 72-4

2 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+649, o/s 30.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 26 and 27, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 72-4

3 of 3

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+649, o/s 30.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 26 and 27, 2002 CHECKED BY DWK

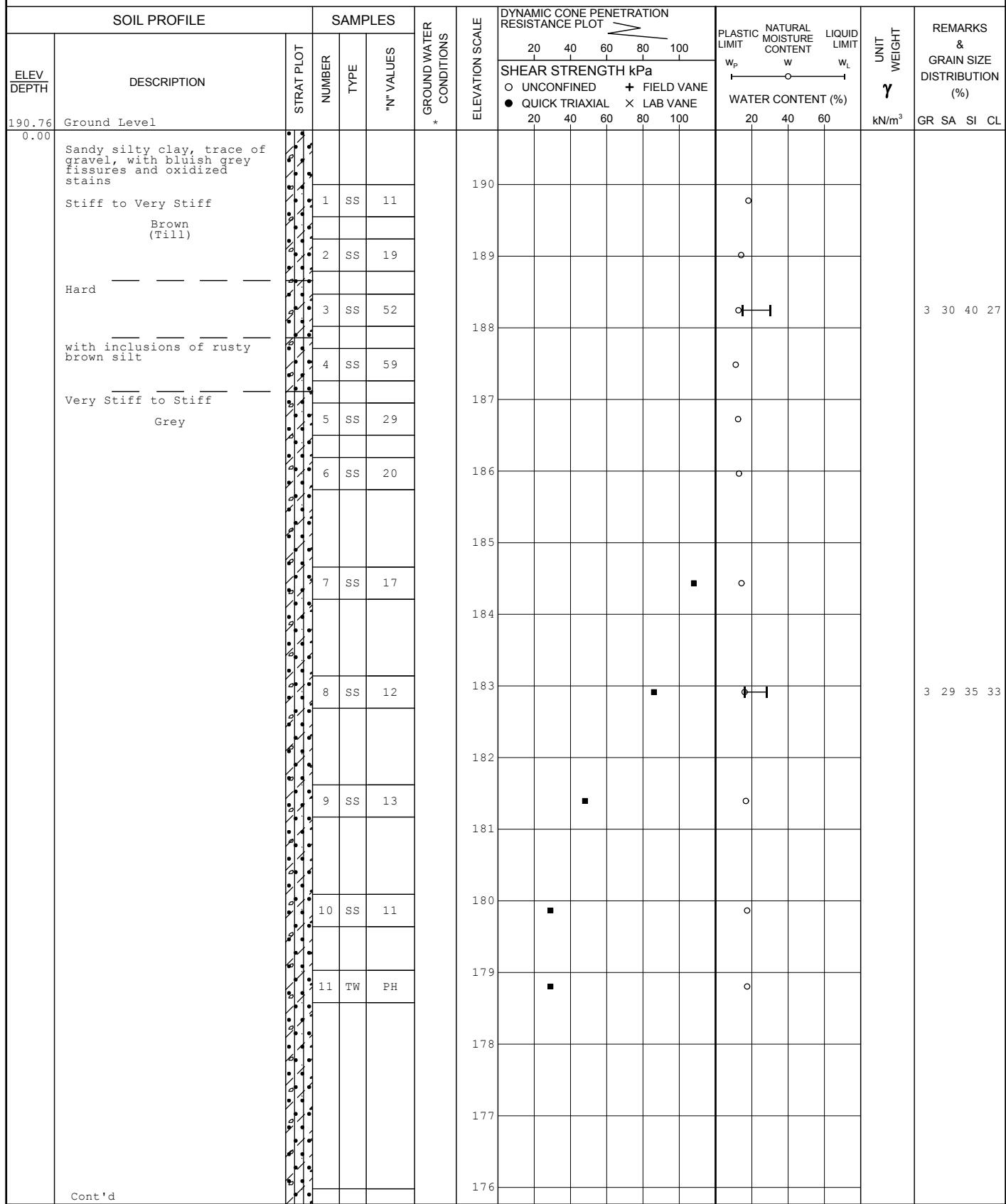
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT W _P	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	WATER CONTENT (%)	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
161.17																	
30.00	Sandy silty clay, trace of gravel (Till)	17	SS	43													
	lenses of fine sand	18	SS104/28cm														
154.37																	
36.80	Bedrock Unweathered, strong limestone Grey 100% drill water return	19	RC	REC 100%													RQD = 81%
152.17																	
39.00	End of Borehole																
	Borehole dry on completion of drilling																
	■ Penetrometer Test																

RECORD OF BOREHOLE No 73-1

1 of 3

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+937, o/s 15.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 20, 2002 CHECKED BY DWK

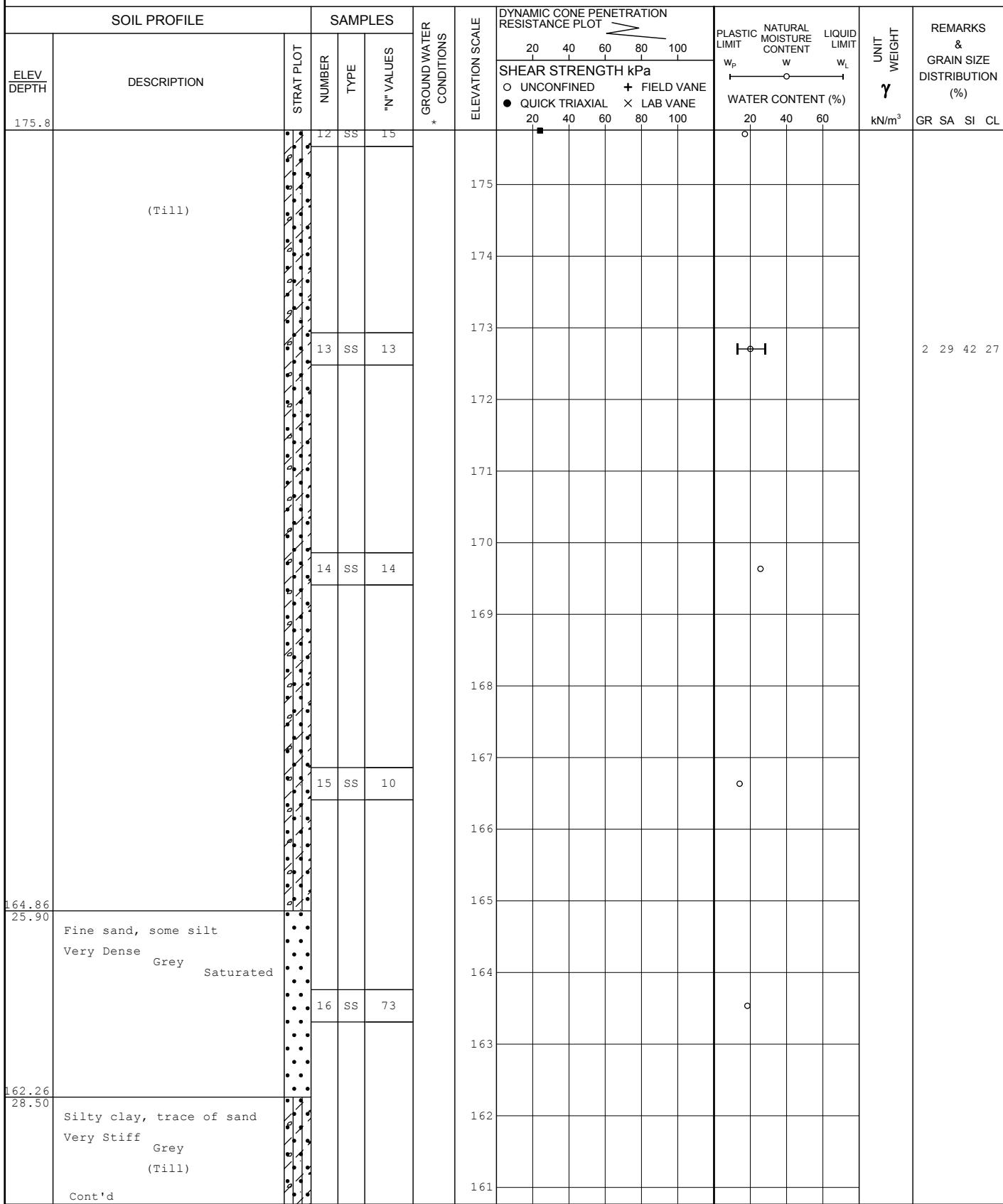


RECORD OF BOREHOLE No 73-1

2 of 3

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+937, o/s 15.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 20, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 73-1

3 of 3

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+937, o/s 15.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY MRA
 DATUM Geodetic DATE February 20, 2002 CHECKED BY DWK

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
160.76			17	SS	25												
158.01	Bedrock Unweathered, strong limestone Light Grey 100% drill water return		18	RC	REC 95%												RQD = 85%
154.96																	
35.80	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 73-2

1 of 1

METRIC

W.P. <u>64-00-04</u>	LOCATION <u>Hwy 401 Sta. 12+959, o/s 18.0m Rt. of CL median</u>	ORIGINATED BY <u>MR</u>	
DIST <u>31</u>	Hwy <u>401</u>	BOREHOLE TYPE <u>Continuous Flight Solid Stem Augers</u>	COMPILED BY <u>MRA</u>
DATUM <u>Geodetic</u>	DATE <u>February 22, 2002</u>	CHECKED BY <u>DWK</u>	

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		SHEAR STRENGTH kPa											
190.96	Ground Level						20 40 60 80 100	O UNCONFINED	+ FIELD VANE	● QUICK TRIAXIAL	X LAB VANE	20 40 60 80 100	20 40 60	20 40 60	kN/m ³	GR SA SI CL		
0.08	Topsoil	Sandy silty clay, trace of gravel, with bluish grey fissures and oxidized stains	1	SS	15													
	Stiff Brown (Till)		2	SS	11													
	Hard		3	SS	23													
	Very Stiff Grey		4	SS	48													
			5	SS	30													
			6	SS	26													
			7	SS	19													
			8	SS	22													
			9	SS	17													
181.36	End of Borehole	Borehole dry on completion of drilling																
9.60		■ Penetrometer Test																

RECORD OF BOREHOLE No 73-3

1 of 1

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+950, o/s 8.4m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Hollow Stem Augers COMPILED BY MRA
 DATUM Geodetic DATE February 19, 2002 CHECKED BY DWK

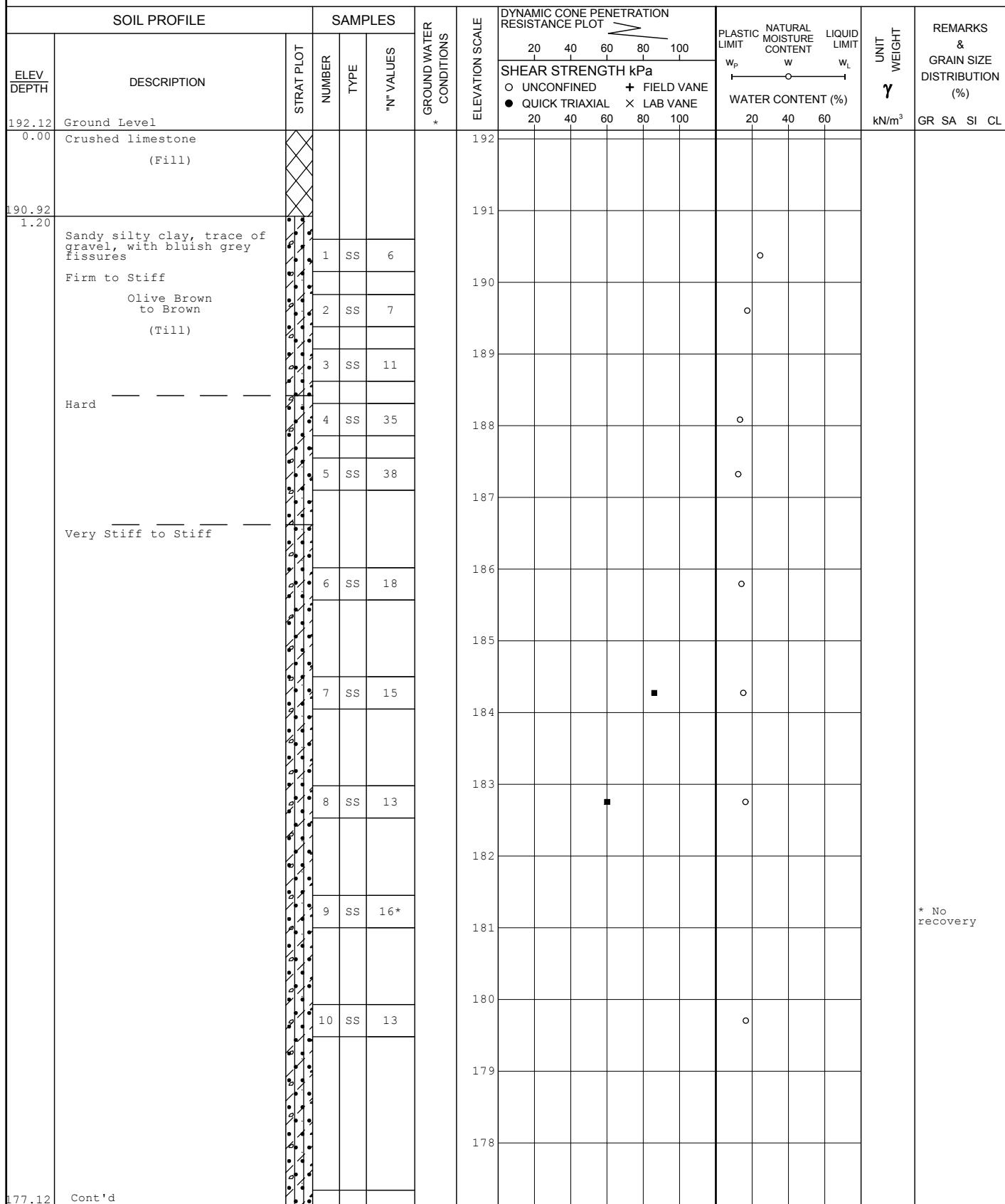
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
192.06	Ground Level					192											
0.00	Limestone, railway ballast (Fill)	X				191											
190.86		X				190											
1.20	Silty clay, some sand Firm Black to Brown	X	1	SS	7*	189											* No recovery
189.96		X	2	SS	11	188											
2.10	Sandy silty clay, trace of gravel, with bluish grey fissures and oxidized stains Stiff to Very Stiff Brown (Till)	X	3	SS	25	187											
	Hard	X	4	SS	52	186											
		X	5	SS	45	185											
	Very Stiff to Stiff Grey	X	6	SS	22	184											
		X	7	SS	15	183											
182.46		X	8	SS	13												
9.60	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 73-4

1 of 3

METRIC

W.P. <u>64-00-04</u>	LOCATION <u>Hwy 401 Sta. 12+978, o/s 19.0m Rt. of CL median</u>	ORIGINATED BY <u>MR</u>	
DIST <u>31</u>	HWY <u>401</u>	BOREHOLE TYPE <u>C.F.H.S.A. and Mud Rotary</u>	COMPILED BY <u>MRA</u>
DATUM <u>Geodetic</u>	DATE <u>February 19, 2002</u>	CHECKED BY <u>DWK</u>	

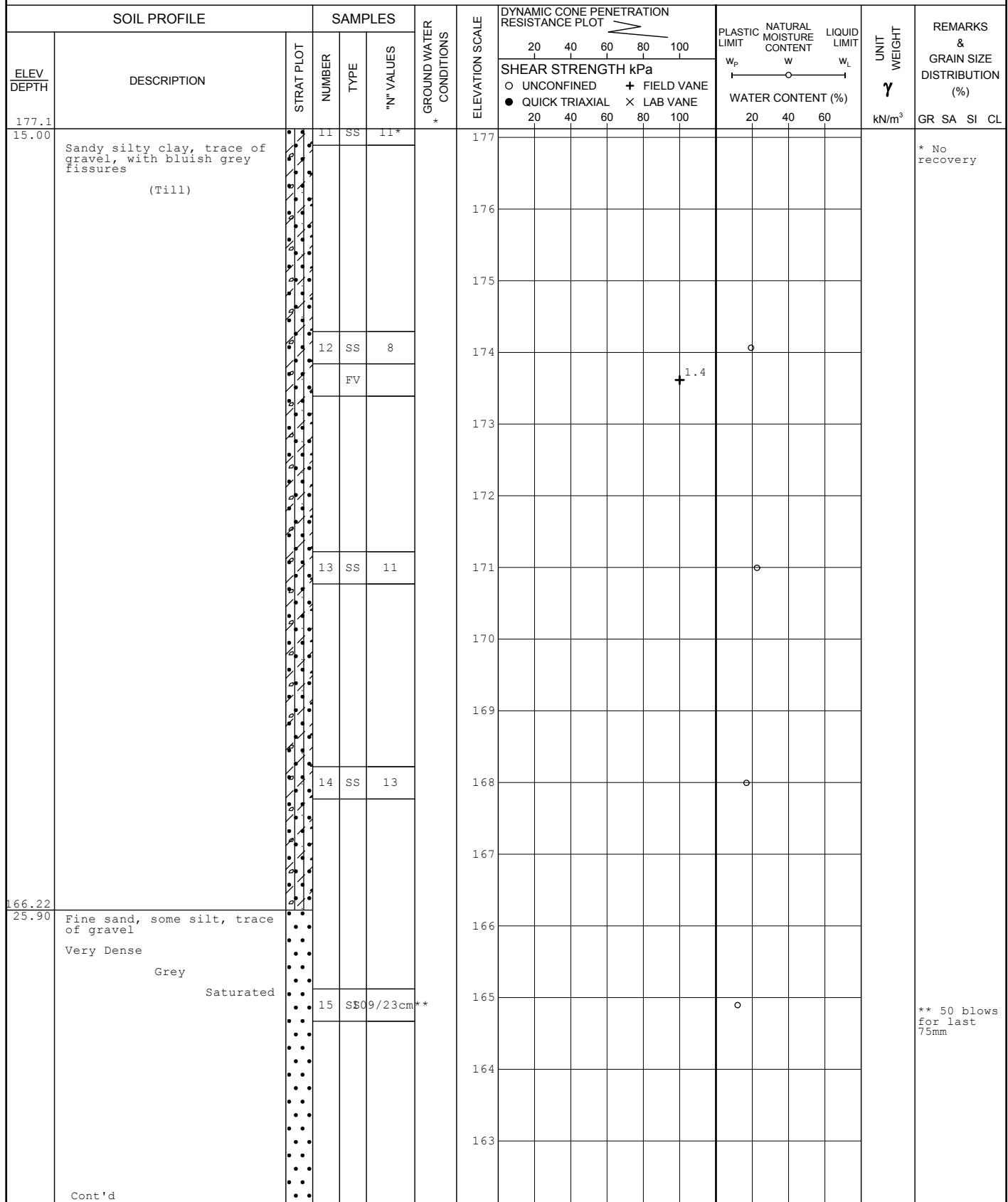


RECORD OF BOREHOLE No 73-4

2 of 3

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+978, o/s 19.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A. and Mud Rotary COMPILED BY MRA
 DATUM Geodetic DATE February 19, 2002 CHECKED BY DWK



RECORD OF BOREHOLE No 73-4

3 of 3

METRIC

W.P. 64-00-04 LOCATION Hwy 401 Sta. 12+978, o/s 19.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A. and Mud Rotary COMPILED BY MRA
 DATUM Geodetic DATE February 19, 2002 CHECKED BY DWK

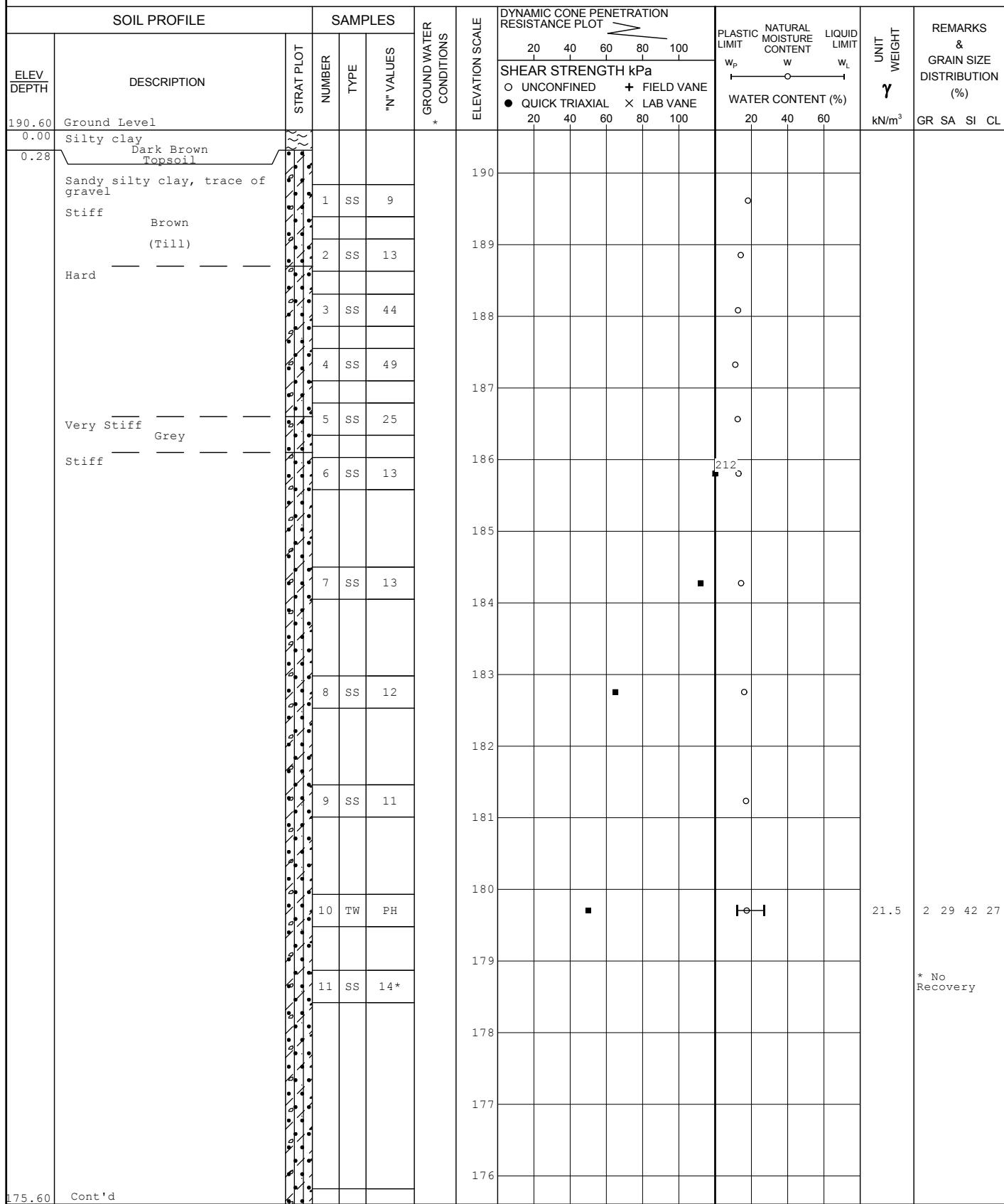
SOIL PROFILE			SAMPLES			ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES *		GROUND WATER CONDITIONS	20	40	60	80	100	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20	40	60	kN/m ³
162.12																		
161.77		• •	16	SS	88			162							c			
30.35	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																	

RECORD OF BOREHOLE No 74-1

1 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+034, o/s 17.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 13 and 14, 2002 CHECKED BY MRA

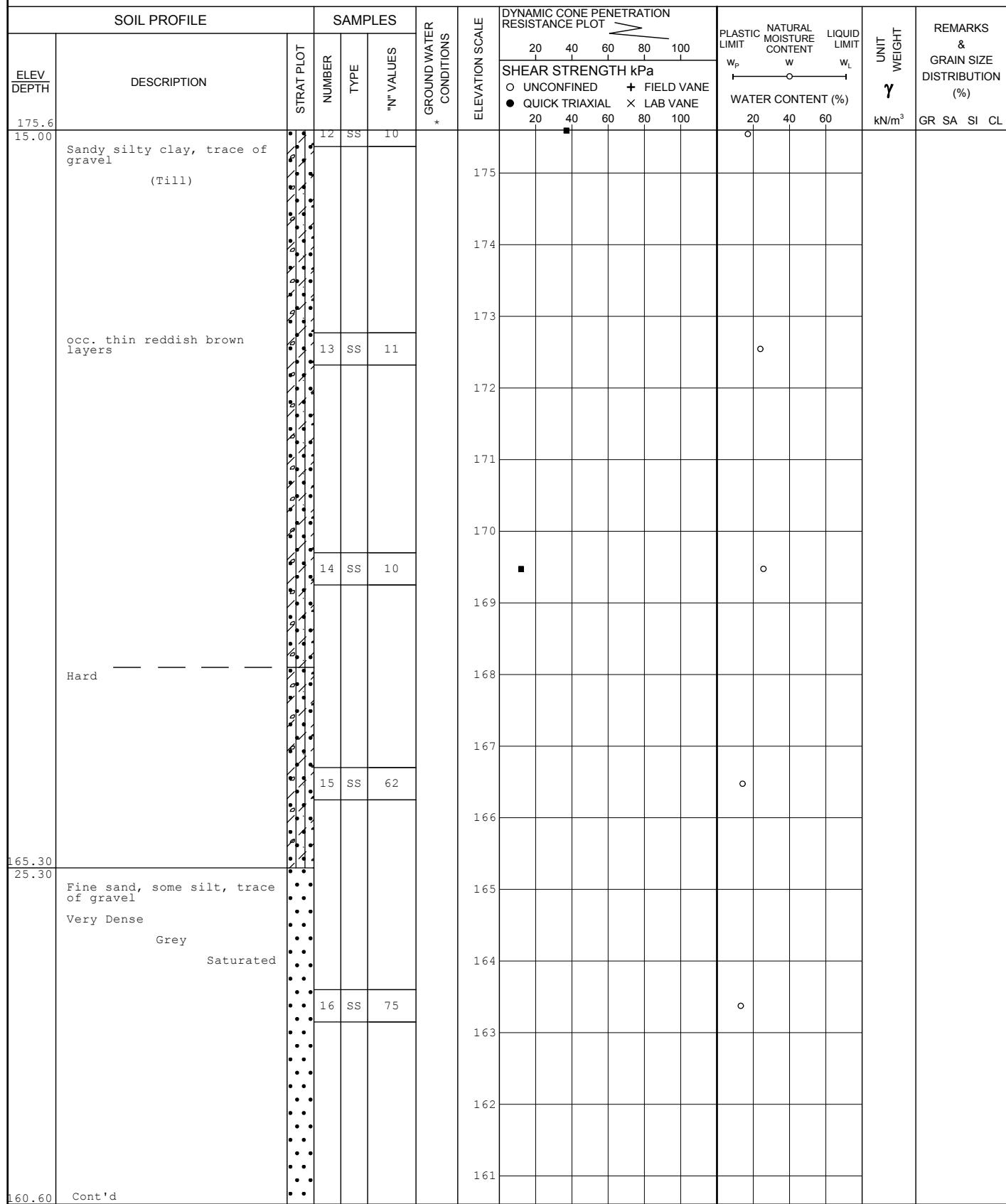


RECORD OF BOREHOLE No 74-1

2 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+034, o/s 17.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 13 and 14, 2002 CHECKED BY MRA



RECORD OF BOREHOLE No 74-1

3 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+034, o/s 17.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 13 and 14, 2002 CHECKED BY MRA

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40	60	80	100						
160.60																		
30.00	Fine sand, some silt, trace of gravel Dense Grey Saturated occ. thin distorted layers of grey silty clay	•••••	17	SS	47									○				
158.70																		
31.90	Bedrock Unweathered, strong limestone Light Grey	██████████	18	RC	REC 88%													RQD = 80%
155.65	loss drill water at 34.6m	██████████																
34.95	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																	

RECORD OF BOREHOLE No 74-3

1 of 1

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+071, o/s 18.1m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 13, 2002 CHECKED BY MRA

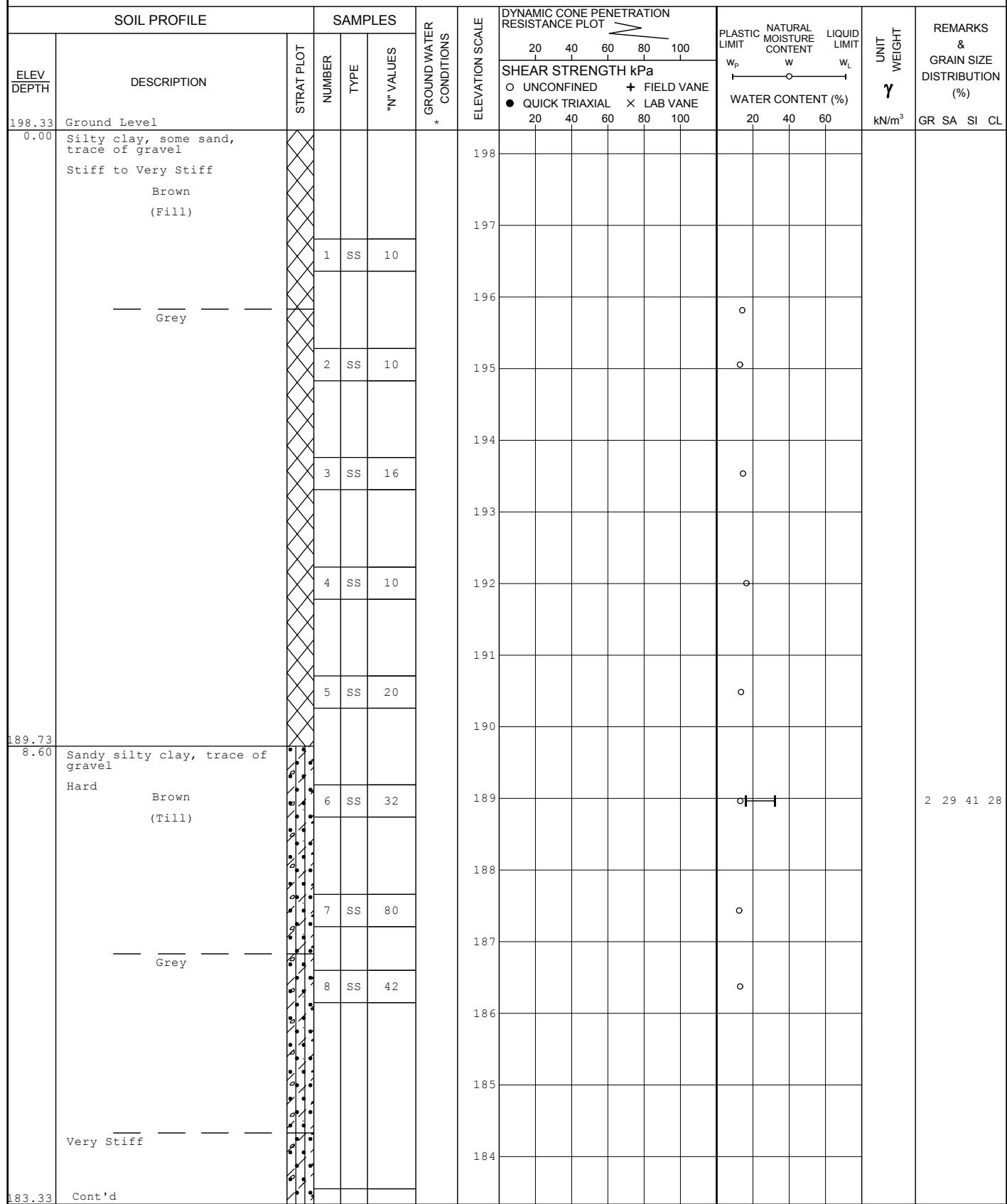
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
190.48	Ground Level																
0.00	Silty clay Dark Brown Topsoil																
0.28	Sandy Silty clay, trace of gravel Stiff Brown (Till) Hard		1	SS	8												
			2	SS	36												
			3	SS	58												
			4	SS	54												
			5	SS	18												
			6	SS	27												
			7	SS	18												
			8	SS	15												
			9	SS	14												
180.88	Very Stiff Grey																
9.60	Stiff																
	End of Borehole Borehole dry on completion of drilling																
	■ Penetrometer Test																

RECORD OF BOREHOLE No 74-4

1 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+096, o/s 22.1m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 28, 2002 CHECKED BY MRA

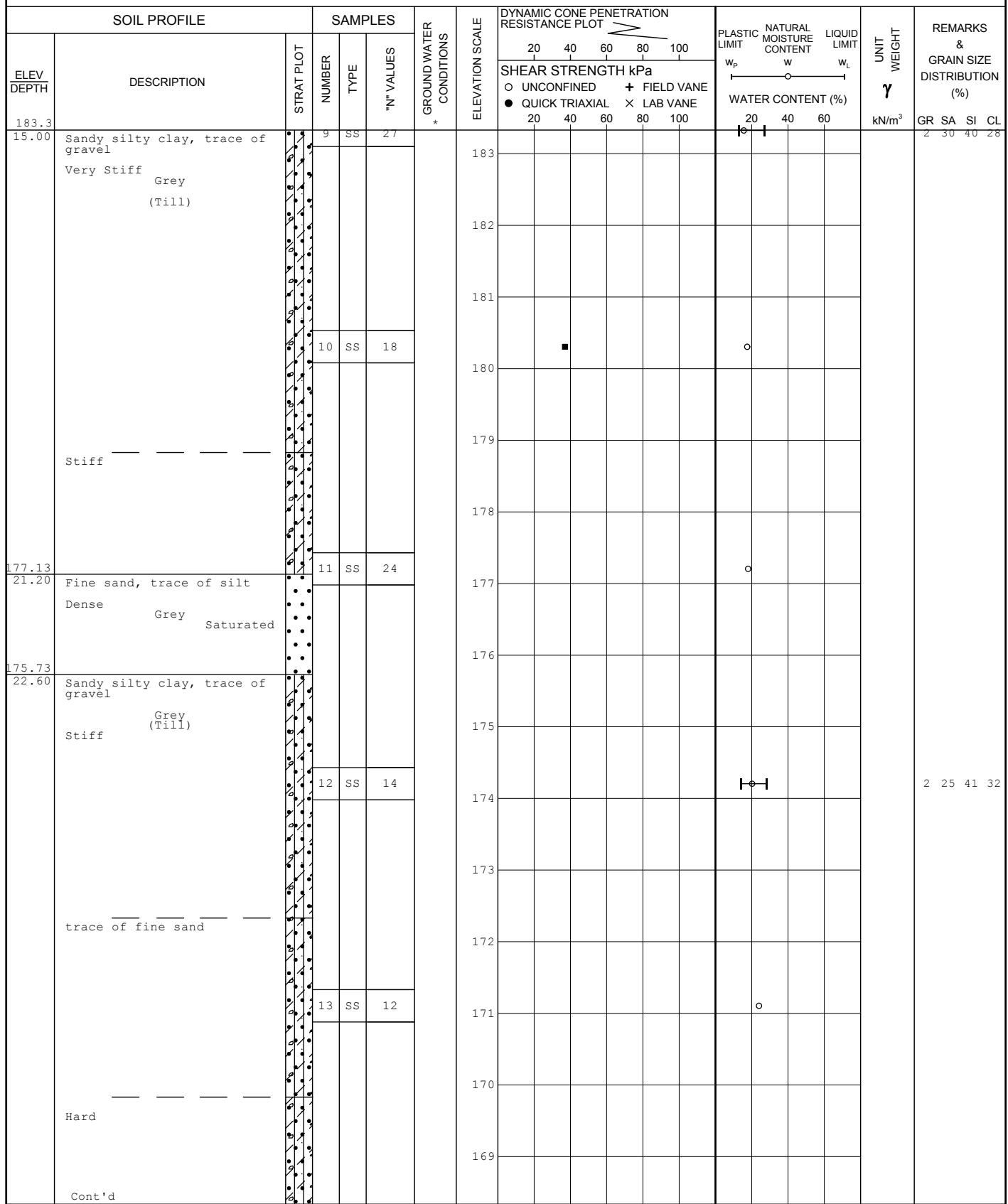


RECORD OF BOREHOLE No 74-4

2 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+096, o/s 22.1m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 28, 2002 CHECKED BY MRA



RECORD OF BOREHOLE No 74-4

3 of 3

METRIC

W.P. 64-00-05 LOCATION Hwy 401 Sta. 13+096, o/s 22.1m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE C.F.H.S.A., Mud Rotary and NQ Rock Coring COMPILED BY GD
 DATUM Geodetic DATE February 28, 2002 CHECKED BY MRA

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100	20 40 60						
168.33			14	SS	53												
166.53			15	SS102/25cm													
31.80	Fine sandy silt Very Dense Grey Saturated		16	SS	80												
161.03			17	RC	REC 100%												
37.30	Bedrock Unweathered, strong limestone Light Grey																RQD = 84%
157.93																	
40.40	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 101

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+567, o/s 35.9m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 26, 2002 CHECKED BY MRA

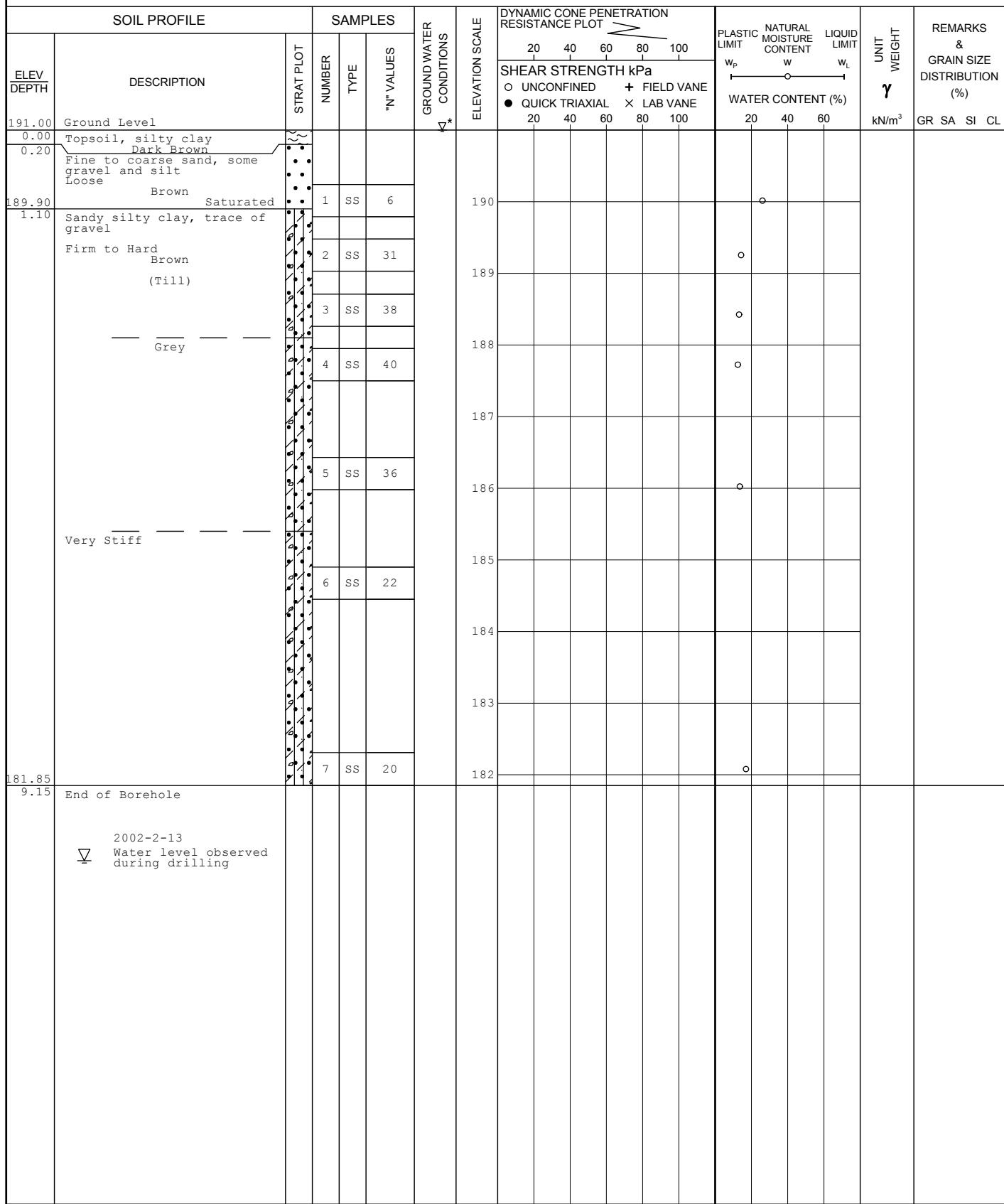
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
190.91	Ground Level																
0.00	Topsoil, silty clay Dark Brown																
0.20	Sandy silty clay, trace of gravel																
	Very Stiff Brown (Till)		1	SS	19												
	Hard		2	SS	46												
			3	SS	52												
			4	SS	28												
			5	SS	22												
			6	SS	20												
181.76	End of Borehole																
9.15	Borehole dry on completion of drilling																
	■ Penetrometer Test																

RECORD OF BOREHOLE No 102

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+623, o/s 24.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 13, 2002 CHECKED BY MRA

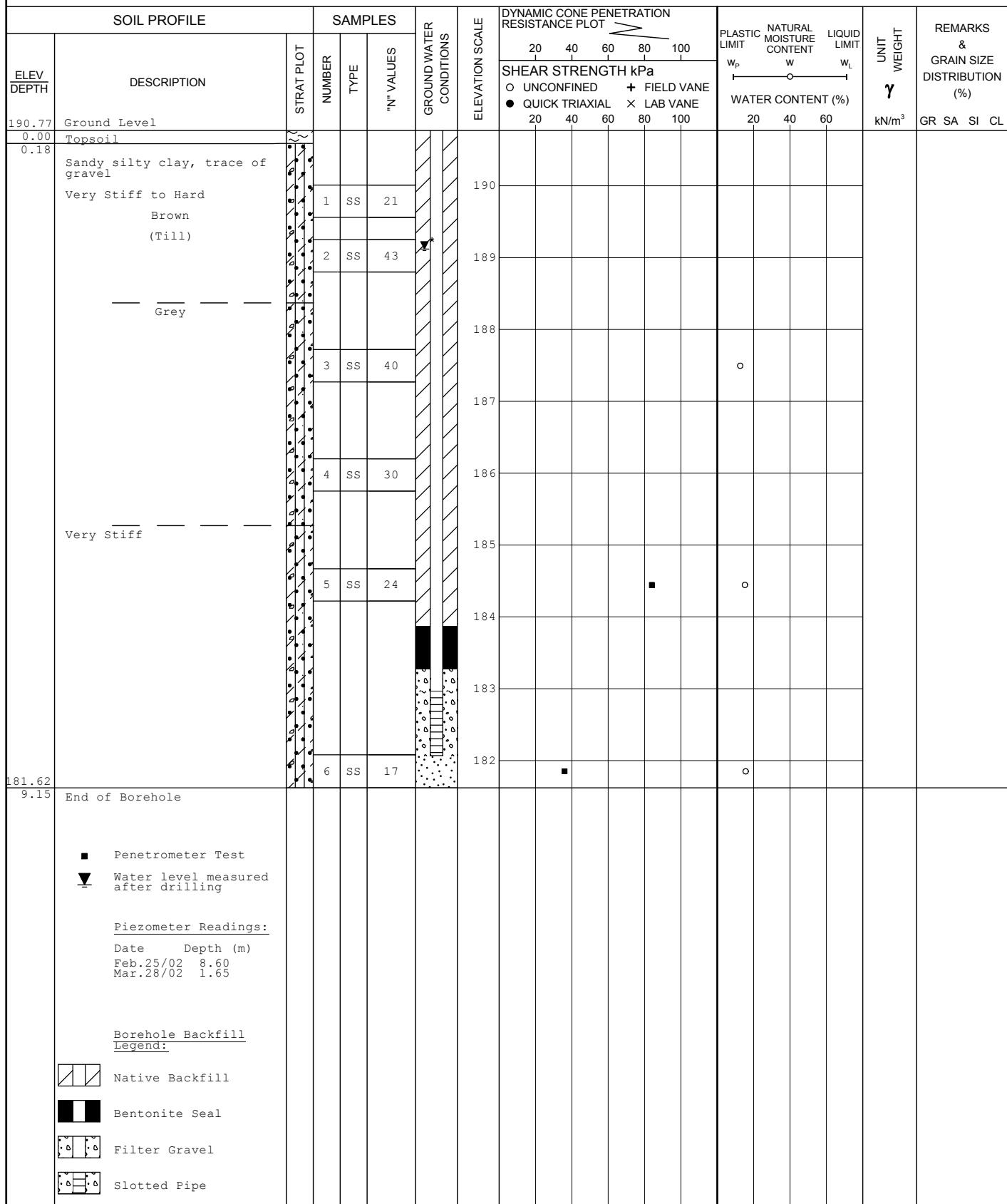


RECORD OF BOREHOLE No 103

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+692, o/s 19.5m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 23, 2002 CHECKED BY MRA

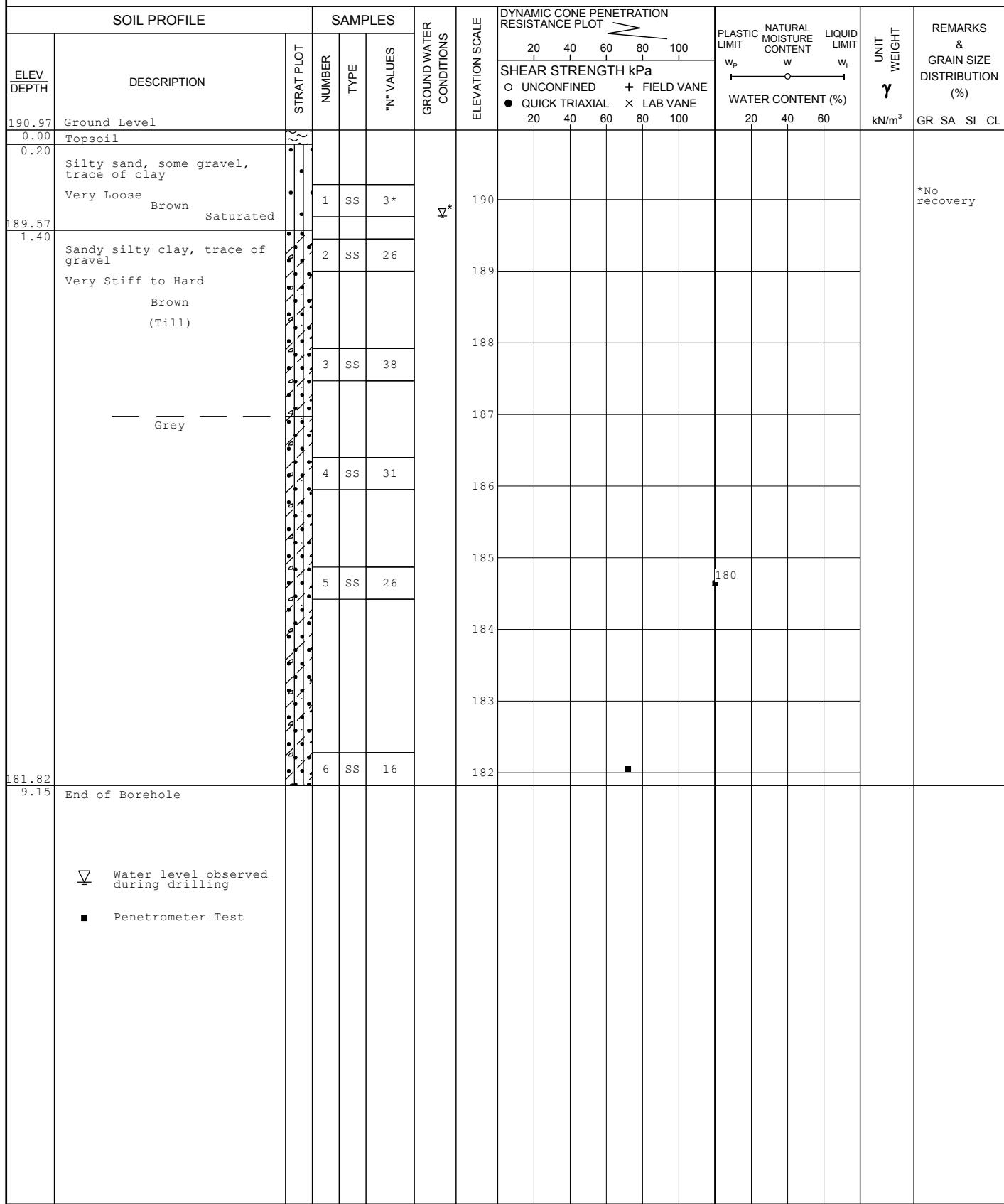


RECORD OF BOREHOLE No 104

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+734, o/s 40.1m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 23, 2002 CHECKED BY MRA



RECORD OF BOREHOLE No 105

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+784, o/s 27.2m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 23, 2002 CHECKED BY MRA

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
190.81	Ground Level																
0.00	Topsoil																
0.15	Sandy silty clay, trace of gravel Stiff to Hard Brown (Till)		1	SS	11												
			2	SS	38												
			3	SS	48												
			4	SS	29												
			5	SS	23												
	Very Stiff Grey		6	SS	16												
181.66																	
9.15	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 106

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+837, o/s 44.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 23, 2002 CHECKED BY MRA

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)	
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES	20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100	20 40 60							
190.96	Ground Level																	
0.00	Peat, fine to coarse, fibrous																	
0.30	Dark Brown																	
	Sandy silty clay, trace of gravel		1	SS	11													
	Stiff to Hard		2	SS	31													
	Brown (Till)		3	SS	62													
	Very Stiff Grey		4	SS	28													
			5	SS	23													
			6	SS	16													
181.81	End of Borehole																	
9.15	2002-2-23 Groundwater level at surface																	

RECORD OF BOREHOLE No 107

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+876, o/s 28.6m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 22, 2002 CHECKED BY MRA

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
191.01	Ground Level																
0.00	Topsoil																
	Sandy silty clay, trace of gravel Stiff to Hard Brown (Till)		1	SS	14												
			2	SS	15												
			3	SS	83												
			4	SS	34												
			5	SS	23												
			6	SS	16												
181.86																	
9.15	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 108

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+914, o/s 21.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 22, 2002 CHECKED BY MRA

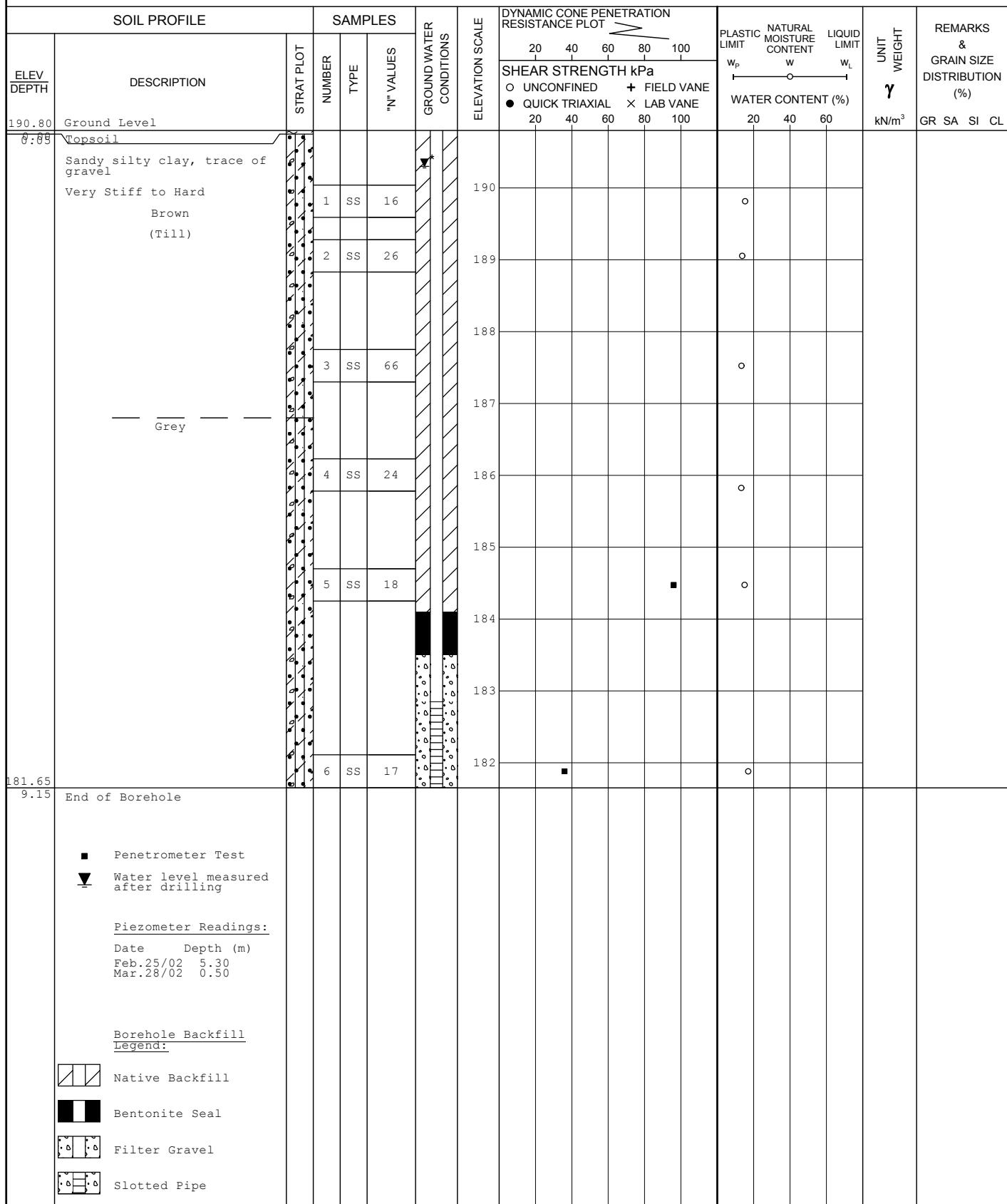
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20	40	60	80	100						
190.76	Ground Level																
0.08	Topsoil																
	Sandy silty clay, trace of gravel																
	Stiff to Hard																
	Brown																
	(Till)																
			1	SS	12												
			2	SS	26												
			3	SS	46												
			4	SS	28												
			5	SS	21												
			6	SS	15												
181.61																	
9.15	End of Borehole																
	Borehole dry on completion of drilling																
	■ Penetrometer Test																

RECORD OF BOREHOLE No 109

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+924, o/s 26.0m Rt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 22, 2002 CHECKED BY MRA



RECORD OF BOREHOLE No 110

1 of 1

METRIC

W.P. 64-00-00 LOCATION Hwy 401 Sta. 12+972, o/s 31.6m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 15, 2002 CHECKED BY MRA

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	SHEAR STRENGTH kPa	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100						
190.71	Ground Level																
0.00	Silty clay, trace of sand Dark Brown																
0.28	Sandy silty clay, trace of gravel Stiff Brown (Till) Very Stiff		1	SS	12												
			2	SS	23												
	Hard		3	SS	53												
			4	SS	17												
	Very Stiff Grey		5	SS	15												
			6	SS	13												
181.56	End of Borehole																
9.15	Borehole dry on completion of drilling ■ Penetrometer Test																

RECORD OF BOREHOLE No 111

1 of 1

METRIC

W.P. 64-00-00 LOCATION Sta. 13+009, o/s 34.0m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 13, 2002 CHECKED BY MRA

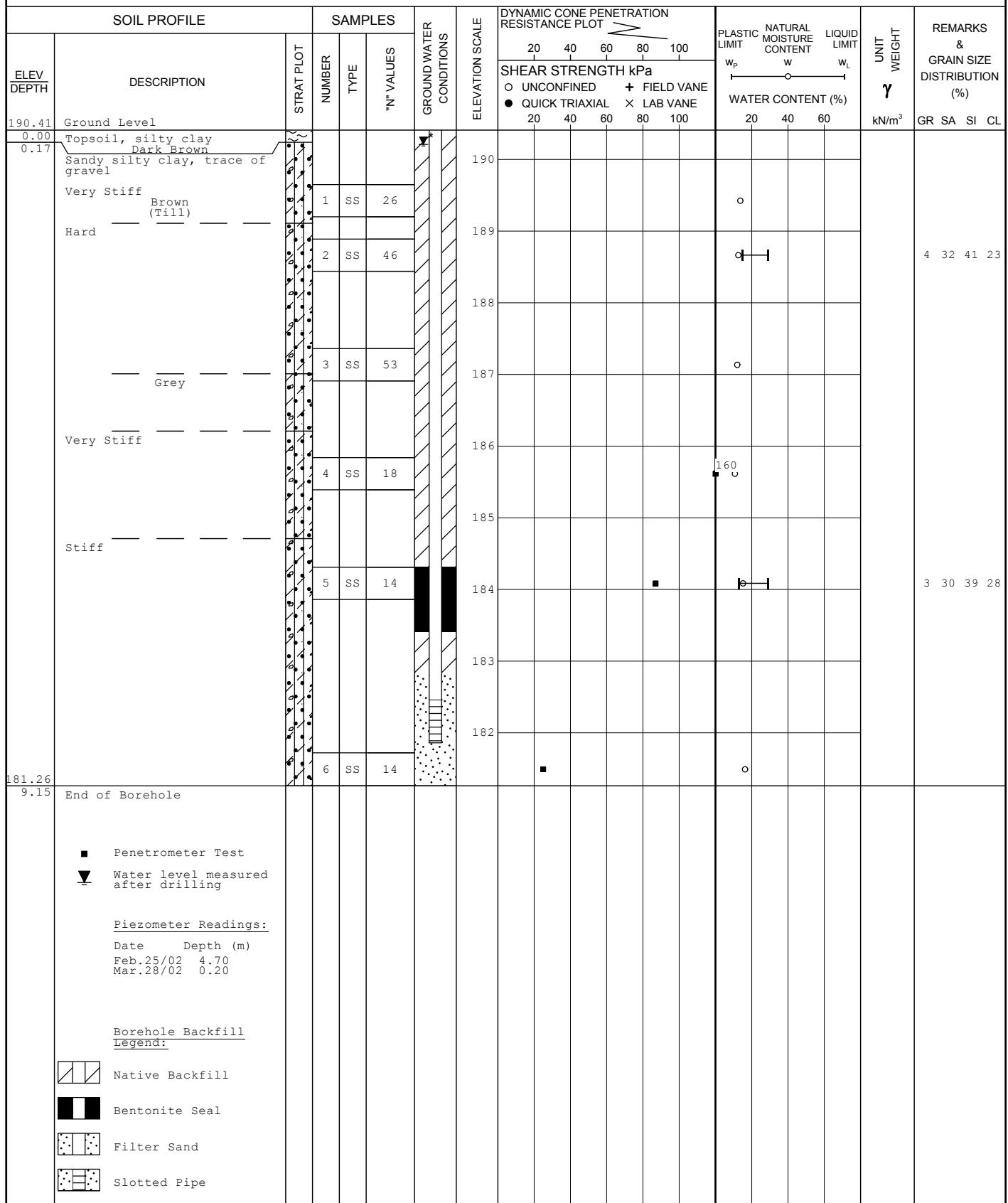
SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS *	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT					PLASTIC LIMIT w_p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w_L	WATER CONTENT (%)	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES		20 40 60 80 100	O UNCONFINED + FIELD VANE	● QUICK TRIAXIAL X LAB VANE	20 40 60 80 100	20 40 60						
190.51	Ground Level																
0.00	Topsoil																
0.22	Sandy silty clay, trace of gravel Stiff Brown (Till) Hard		1	SS	12							190					
			2	SS	39							189					
			3	SS	48							188					
			4	SS	21							187					
			5	SS	15							186					
			6	SS	12							185					
												184	■				
												183					
												182	■				
181.36	End of Borehole Borehole dry on completion of drilling ■ Penetrometer Test																
9.15																	

RECORD OF BOREHOLE No 112

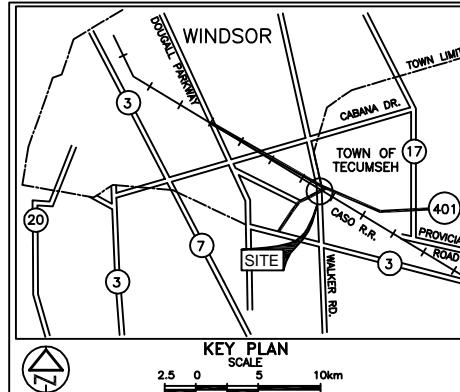
1 of 1

METRIC

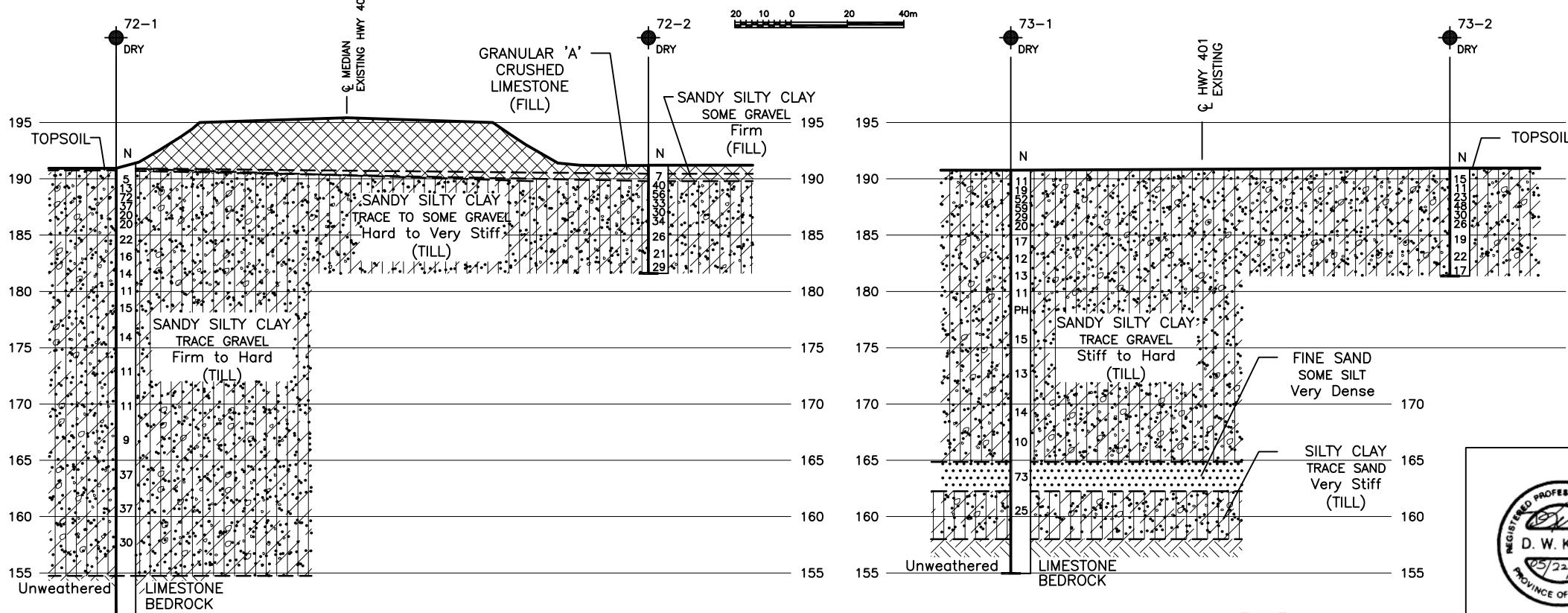
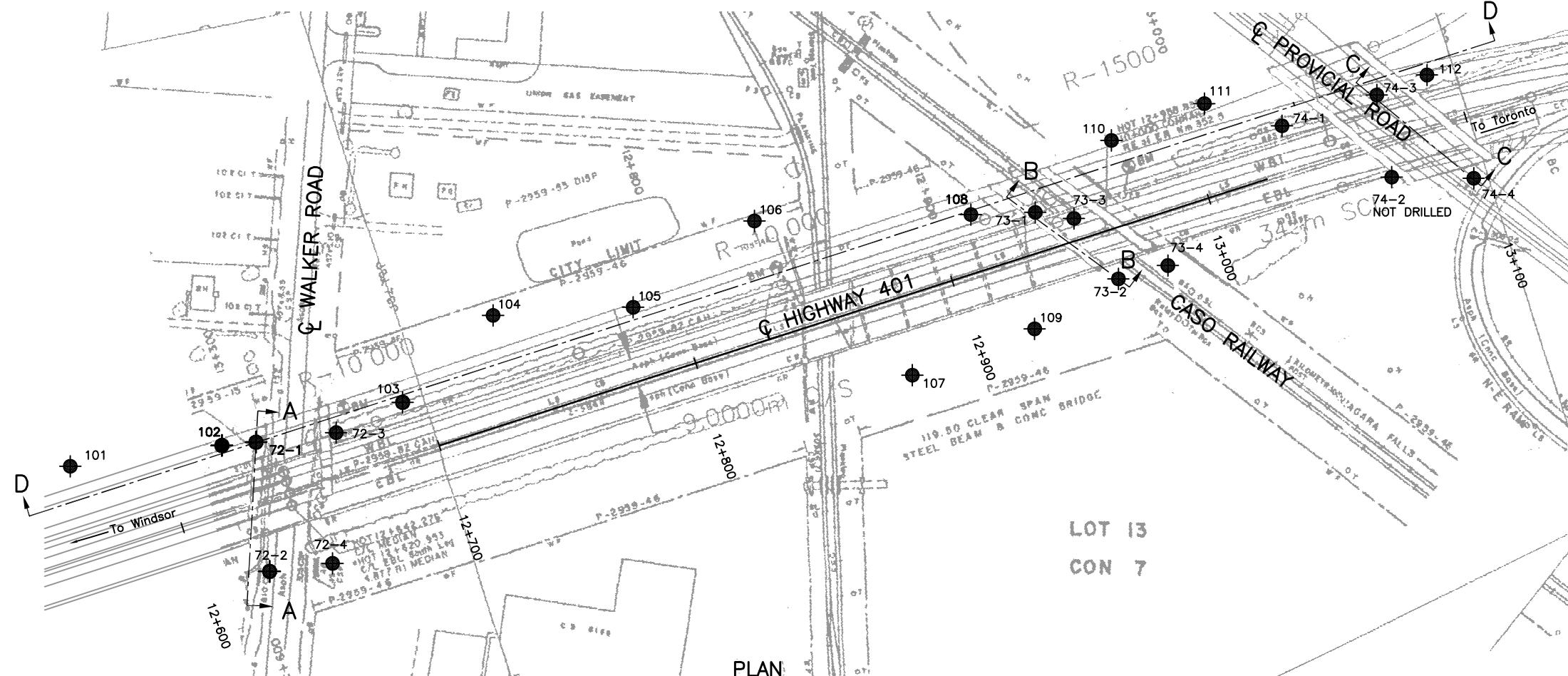
W.P. 64-00-00 LOCATION Hwy 401 Sta. 13+091, o/s 19.7m Lt. of CL median ORIGINATED BY MR
 DIST 31 HWY 401 BOREHOLE TYPE Continuous Flight Solid Stem Augers COMPILED BY GD
 DATUM Geodetic DATE February 13, 2002 CHECKED BY MRA



METRIC

DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRESCONT No 2007-3043
WP No 64-00-00HIGHWAY 401
EMBANKMENT BETWEEN WALKER ROAD
AND PROVINCIAL ROAD
BOREHOLE LOCATIONS & SOIL STRATASHEET
247PML Peto MacCallum Ltd.
CONSULTING ENGINEERS

LEGEND	
●	Borehole
○	Dynamic Cone Penetration Test (Cone)
●○	Borehole & Cone
N	Blows/0.3m (Std. Pen Test, 475 J / blow)
CONE	Blows/0.3m (60° Cone, 475 J / blow)
▼	W L at time of investigation Feb 2002
—	Head
▽	ARTESIAN WATER
—	Encountered
—	PIEZOMETER



REF No Survey Plan 2001 Site # 6-72, entitled Proposed Bridge Site at Walker Road and Highway 401, Site # 6-73, entitled Proposed Bridge Site at Conrail Railway and Highway 401, Site # 6-74, entitled Proposed Bridge Site at County Road 46 and Highway 401 and Plan, untitled, undated, Provided by Planning and Design Section, MTO.

REVISIONS		CHANGED FOR CONTRACT DOCUMENTS AS PER EMAIL DATED APRIL 27, 2007, FROM DILLON CONSULTING LIMITED		
MAY22/07	CN	DATE ADDED		
DATE	BY	DESCRIPTION		
		Geocon No. 40J2-43		
Hwy No	401	DIST	31	
SUB'D	GD	CHECKED	MRA	DATE DEC 2, 2002 SITE
DRAWN	MM	CHECKED	BRG	APPROVED DWK DWG 2-1

— NOTE —
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

METRIC

DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES

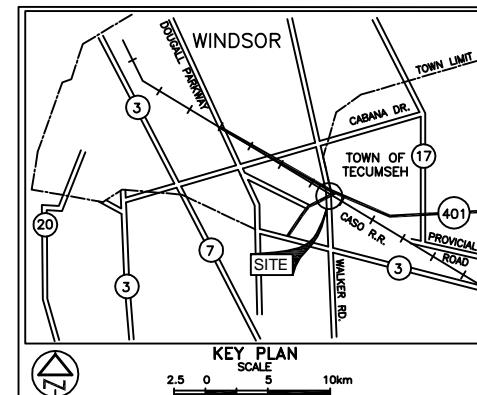
CONT No 2007-3043

WP No 64-00-00

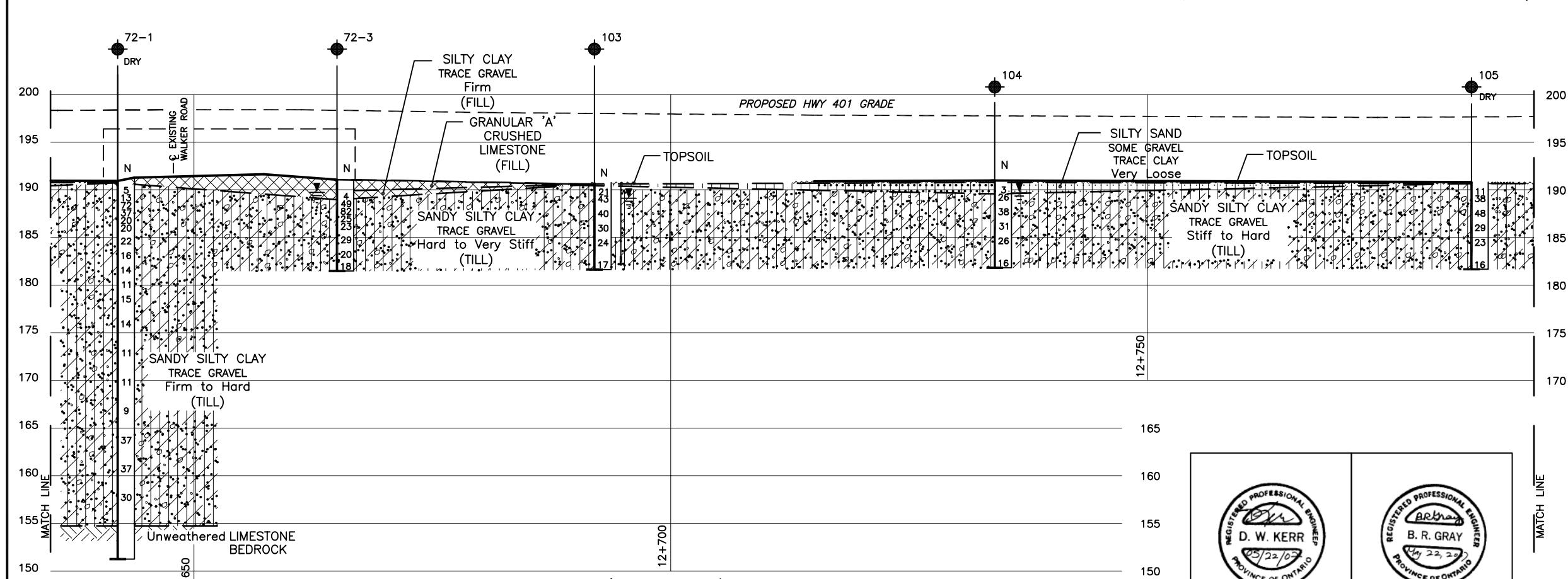
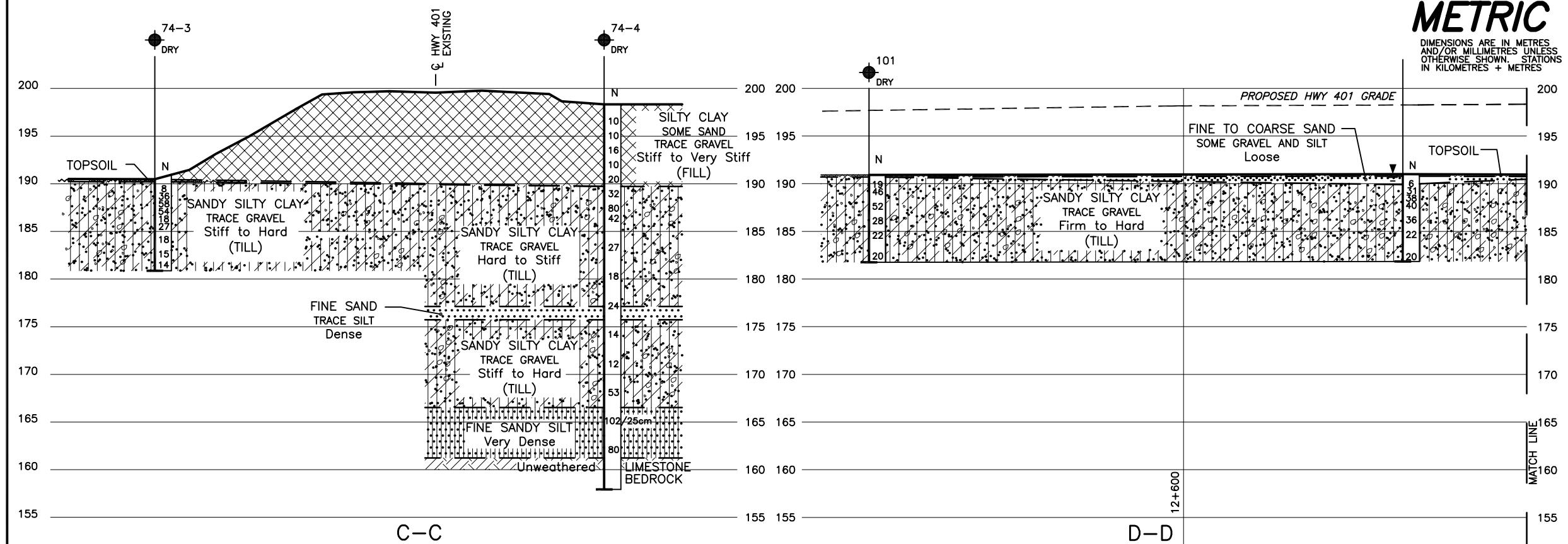
HIGHWAY 401
EMBANKMENT BETWEEN WALKER ROAD
AND PROVINCIAL ROAD
BOREHOLE LOCATIONS & SOIL STRATA

SHEET
248

PML Peto MacCallum Ltd.
CONSULTING ENGINEERS



LEGEND					
●	Borehole				
○	Dynamic Cone Penetration Test (Cone)				
●○	Borehole & Cone				
N	Blows/0.3m (Std. Pen Test, 475 J / blow)				
CONE	Blows/0.3m (60° Cone, 475 J / blow)				
▼	W L at time of investigation Feb 2002				
▽	Head				
▽	ARTESIAN WATER				
—	Encountered				
—	PIEZOMETER				
BH No	ELEVATION	HWY 401 STA.	o/s CL MED		
(Refer to drawing 2-1 for Sta and Offset)					



- NOTES:
- REFER TO DRAWING 2-1 FOR PLAN AND SECTIONS A-A AND B-B, DRAWING 2-3 FOR SECTION D-D CONTINUED.
 - SECTIONS ARE PROVIDED SOLELY FOR ILLUSTRATIVE PURPOSES. REFER TO RECORD OF BOREHOLES FOR DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS, IN-SITU TEST DATA AND LABORATORY TEST RESULTS.

SECTIONS

SCALE

5 2.5 0 5 10m



REF No Survey Plan 2001 Site # 6-72, entitled Proposed Bridge Site at Walker Road and Highway 401, Site # 6-73, entitled Proposed Bridge Site at Conrail Railway and Highway 401, Site # 6-74, entitled Proposed Bridge Site at County Road 46 and Highway 401 and Plan, unitled, undated, Provided by Planning and Design Section, MTO.

R E V I S I O N S
MAY22/07 ON CHANGED FOR CONTRACT DOCUMENTS AS PER EMAIL
DATED APRIL 27, 2007, FROM DILLON CONSULTING LIMITED
DATE BY DESCRIPTION

Geoces No. 40J2-43

HWY No	401	DIST	31
SUBM'D	GD	CHECKED	MRA
DATE	DEC 2, 2002	SITE	
DRAWN	MM	CHECKED	BRG APPROVED DWK
			2-2

METRIC

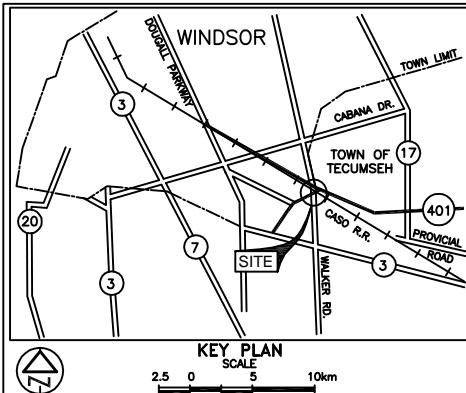
DIMENSIONS ARE IN METRES
AND/OR MILLIMETRES UNLESS
OTHERWISE SHOWN. STATIONS
IN KILOMETRES + METRES

CONT No 2007-3043
GWP No 64-00-00

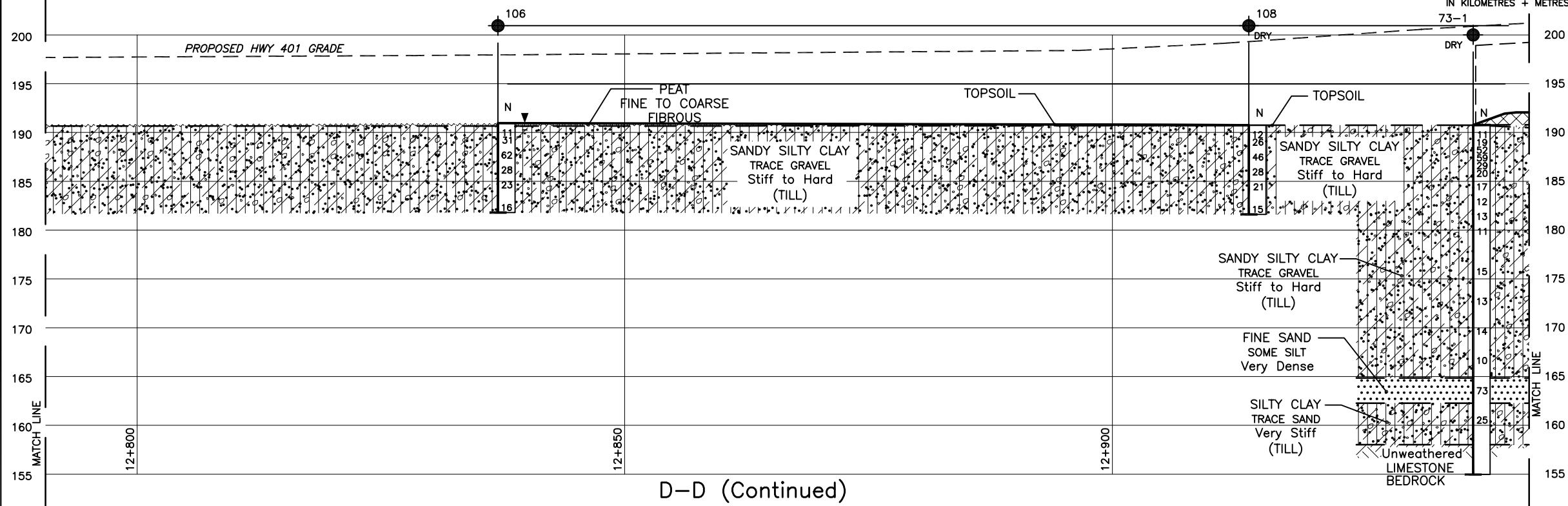
HIGHWAY 401
EMBANKMENT BETWEEN WALKER ROAD
AND PROVINCIAL ROAD
BOREHOLE LOCATIONS & SOIL STRATA

SHEET
249

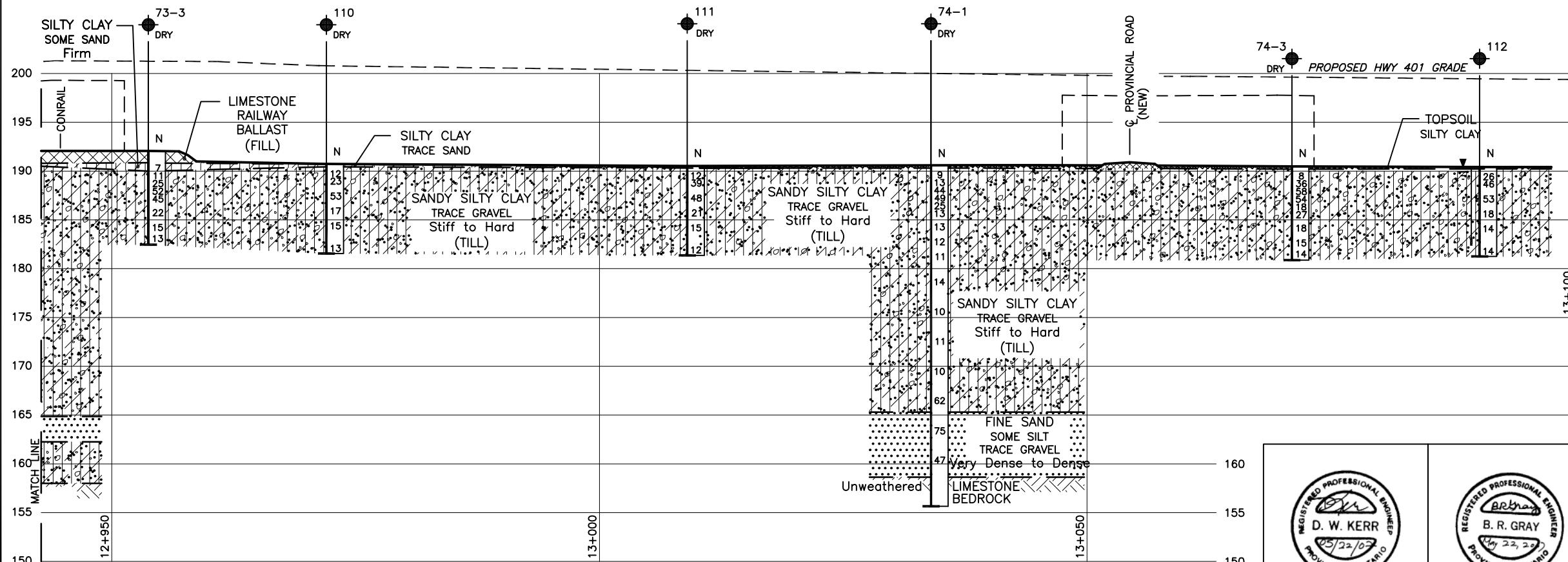
PML Peto MacCallum Ltd.
CONSULTING ENGINEERS



LEGEND			
●	Borehole		
○	Dynamic Cone Penetration Test (Cone)	150	
●○	Borehole & Cone		
N	Blows/0.3m (Std. Pen Test, 475 J / blow)		
CONE	Blows/0.3m (60° Cone, 475 J / blow)		
▼	W L at time of investigation Feb 2002		
—	Head		
▽	ARTESIAN WATER		
—	Encountered		
—	PIEZOMETER		
BH No	ELEVATION	HWY 401 STA.	o/s CL MED
(Refer to drawing 2-1 for Sta and Offset)			



D-D (Continued)



D-D (Continued)

SECTIONS

SCALE
5 2.5 0 5 10m

- NOTES:
- REFER TO DRAWING 2-1 FOR PLAN AND SECTIONS A-A AND B-B, DRAWING 2-2 FOR SECTIONS C-C AND D-D.
 - SECTIONS ARE PROVIDED SOLELY FOR ILLUSTRATIVE PURPOSES. REFER TO RECORD OF BOREHOLES FOR DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS, IN-SITU TEST DATA AND LABORATORY TEST RESULTS.

5 2.5 0 5 10m

REF No Survey Plan 2001 Site # 6-72, entitled Proposed Bridge Site at Walker Road and Highway 401, Site # 6-73, entitled Proposed Bridge Site at Conrail Railway and Highway 401, Site # 6-74, entitled Proposed Bridge Site at County Road 46 and Highway 401 and Plan, unitled, undated, Provided by Planning and Design Section, MTO.



- NOTE -
The boundaries between soil strata have been established only at Borehole locations. Between Boreholes the boundaries are assumed from geological evidence.

REVISIONS			
MAY22/07	CN	CHANGED FOR CONTRACT DOCUMENTS AS PER EMAIL DATED APRIL 27, 2007, FROM DILLON CONSULTING LIMITED	
DATE BY			
DESCRIPTION			
Geocodes No. 40J2-43			
Hwy No 401	CD	Survey Plan 2001 Site # 6-72, entitled Proposed Bridge Site at Walker Road and Highway 401, Site # 6-73, entitled Proposed Bridge Site at Conrail Railway and Highway 401, Site # 6-74, entitled Proposed Bridge Site at County Road 46 and Highway 401 and Plan, unitled, undated, Provided by Planning and Design Section, MTO.	DIST 31
Subn'd MM	GD	CHECKED MRA DATE DEC 2, 2002 SITE	
Drawn MM	CHECKED DWK APPROVED BRG	APPROVED DWG	2-3

EMBANKMENT 3

**RECORDS OF BOREHOLE SHEETS AND FOUNDATION DRAWINGS
RELATED TO EMBANKMENT 3 ARE REMOVED FOR THIS PROJECT**